

# **Geotechnical Investigation**

## **Proposed Residential Development**

Half Moon Bay South – Phase 7 and 8  
3718 Greenbank Road - Ottawa

Prepared for Mattamy Homes

Report PG5690-1 Revision 5 dated April 23, 2025

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## **Appendices**

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## 1.0 Introduction

Paterson Group (Paterson) was commissioned by Mattamy Homes to conduct a geotechnical investigation for the proposed development located at 3718 Greenbank Road, in the City of Ottawa (refer to Figure 1 - Key Plan presented in Appendix 2).

The objectives of the geotechnical investigation were to:

- ☐ Determine the subsoil and groundwater conditions at this site by means of borehole and test pit program.
- ☐ Provide geotechnical recommendations for the design of the proposed development based on the results of the boreholes and other soil information available.

The following report has been prepared specifically and solely for the aforementioned project which is described herein. It contains our findings and includes geotechnical recommendations pertaining to the design and construction of the subject development as they are understood at the time of writing this report.

## 2.0 Proposed Development

The proposed residential development will consist of stacked residential townhouse blocks with or without basements. Associated driveways, parking areas, local roadways and landscaped areas are also anticipated as part of the proposed development.

It is further understood that the proposed development will be serviced by future municipal water, sanitary and storm services.



## **3.0 Method of Investigation**

### **3.1 Field Investigation**

#### **Field Program**

The initial field program for the current geotechnical investigation was carried out between February 17 and 23, 2021 and consisted of advancing a total of 12 boreholes to a maximum depth of 9.8 m below the existing grade.

A supplemental field program for the current geotechnical investigation was carried out between July 11 and 12, 2021 and consisted of advancing a total of 7 boreholes to a maximum depth of 8.2 m below the existing grade. The scope of the supplemental field program was to further delineate the fill material placed throughout the south and southwest portions of the site.

An additional test pitting program was recently conducted on April 25, 2023 and consisted of advancing 6 test pits to a maximum depth of 5.0 m below the existing grade. The scope of the additional field program was to determine the hydraulic conductivity and infiltration rates of the native soils below the inverts of the proposed Low Impact Design (LID) system.

Previous investigations were completed within the general area and surroundings of the subject site and consisted of a series of boreholes and test pits advanced to a maximum depth of 9.1 m below ground surface. The borehole locations were distributed in a manner to provide general coverage of the subject site and taking into consideration current site conditions. The test holes locations and fill locations are shown on Drawings PG5690-1 - Test Hole Location Plan and PG5690-2 - Fill Delineation Plan, respectively, included in Appendix 2.

The test holes were completed using a track mounted drill operated by a two-person crew. All fieldwork was conducted under the full-time supervision of Paterson personnel under the direction of a senior engineer. The drilling procedure consisted of drilling to the required depths at the selected locations, and sampling and testing the overburden.

#### **Sampling and In Situ Testing**

Soil samples were collected from the boreholes by sampling directly from the auger flights (AU) or collected using a 50 mm diameter split- spoon (SS) sampler. Grab samples (G) from the test pits were recovered from the side walls of the open excavation. The depths at which the auger, and split-spoon samples were

recovered from the test holes are shown as AU, SS, and G, respectively, on the Soil Profile and Test Data sheets presented in Appendix 1.

All soil samples were visually inspected and initially classified on site. The auger, split-spoon and grab samples were placed in sealed plastic bags and transported to the our laboratory for examination and classification.

A Standard Penetration Test (SPT) was conducted in conjunction with the recovery of the split-spoon samples. The SPT results are recorded as “N” values on the Soil Profile and Test Data sheets. The “N” value is the number of blows required to drive the split-spoon sampler 300 mm into the soil after a 150 mm initial penetration using a 63.5 kg hammer falling from a height of 760 mm.

The thickness of the silty sand deposit was evaluated by a dynamic cone penetration testing (DCPT) completed at BH 7-21. The DCPT consists of driving a steel drill rod, equipped with a 50 mm diameter cone at the tip, using a 63.5 kg hammer falling from a height of 760 mm. The number of blows required to drive the cone into the soil is recorded for each 300 mm increment.

The subsurface conditions observed at the test hole locations were recorded in detail in the field. Our findings are presented in the Soil Profile and Test Data sheets in Appendix 1.

### **Groundwater Monitoring**

Boreholes BH 1-21 to BH 12-21 were fitted with flexible piezometers to allow groundwater level monitoring. The groundwater observations are discussed in Subsection 4.3 and presented in the Soil Profile and Test Data sheets in Appendix 1.

### **Sample Storage**

All samples from the supplemental field program will be stored in the laboratory for a period of one month after issuance of this report. They will then be discarded unless we are otherwise directed.

## **3.2 Field Survey**

The test hole locations were determined by Paterson personnel and surveyed in the field by Paterson using a handheld, high precision GPS. The ground surface elevation at each test hole location is referenced to a geodetic datum. The locations of the boreholes are presented on Drawing PG5690-1 - Test Hole Location Plan in Appendix 2.

### **3.3 Laboratory Testing**

Soil samples were collected from the subject site during the investigations and were visually examined in our laboratory to review the results of the field logging. A total of five (5) grain size distribution analyses were completed on selected soil samples as part of the initial and additional field programs. The results of our testing are presented in Subsection 4.2 and on Grain Size Distribution Analysis sheets presented in Appendix 1.

### **3.4 Analytical Testing**

One (1) soil sample was submitted for analytical testing to assess the corrosion potential for exposed ferrous metals and the potential of sulphate attacks against subsurface concrete structures. The sample was submitted to determine the concentration of sulphate and chloride, the resistivity and the pH of the sample. The results are presented in Appendix 1 and are discussed further in Subsection 6.7

### **3.5 Permeameter Testing**

In-situ permeameter testing was conducted using a Pask (Constant Head Well) Permeameter to confirm infiltration rates of the surficial soils at the subject site. At each location, two (2) 83 mm holes, located approximately 1.5 m away each other, were excavated using a Riverside/Bucket auger to approximate depths ranging from 2.5 to 2.7 and 3.0 to 3.2 m below the existing ground surface. All soils from the auger flights were visually inspected and initially classified on-site. The permeameter reservoir was filled with water and inverted into the hole, ensuring that it was relatively vertical and rested on the bottom of the hole. As the water infiltrated into the soil, the water level of the reservoir was monitored at various time intervals until the rate of fall reached equilibrium, known as “*quasi steady state*” flow rate. Quasi steady state flow can be considered to have been obtained after measuring 3 to 5 consecutive rate of fall readings with identical values. The values for the steady state rate of fall were recorded for each location.

The results of testing are further discussed in Subsection 4.4.

## **4.0 Observations**

### **4.1 Surface Conditions**

The subject site is former agricultural land. The majority of the proposed development has been recently cleared of topsoil, peat and fill which has been stockpiled in several piles across the site.

Generally, the ground surface across the western portion of the site is currently flat and gradually slopes down in a northern direction from an approximate geodetic elevation of 105 to 103 m and is about 1.5 m lower than the adjacent areas. The eastern portion of the site was observed to contain piles of fill material to an approximate elevation of 106 to 109 m.

It should be noted that parts of the subject site had undergone excavation and in-filling activities as part of a previous sand extraction operation. Historical aerial photographs of the site indicating fill movement activities since 1976 are presented in Appendix 2.

The site is bordered to the north and south by vacant land, to the west by existing residential development, and the east by the future Greenbank Road.

### **4.2 Subsurface Profile**

Generally, the subsurface profile across the subject site consisted of a fill layer and/or a deep deposit of brown silty sand.

Fill, consisting of brown silty sand with varying amounts of gravel, crushed stone, cobble, clay and topsoil, were generally observed in test holes across the subject site with an approximate thickness ranging between 0.2 and 2.9 m. The fill layers extended deep to maximum depths of 4.6, 8.2, 8.2 and 6.7 m in boreholes BH 9-21, BH 10-21, BH 11-21, and BH 19-21, respectively. A significant amount of fill material was present above the existing ground surface within the southern portion of the site with a thickness of 4.6 to 8.2 m and an approximate minimum geodetic elevation of 97.8 m.

The deep deposit of compact to very dense, brown silty sand was observed underlying the fill layer, or at ground surface. Gravel and cobbles were occasionally encountered within the silty sand layer. The silty sand was observed to be underlain by a glacial till deposit composed of dense, brown sandy silt to silty sand with gravel, cobbles and boulders within borehole BH 3-21.

Practical refusal to augering was encountered at a range between 4.6 and 9.0 m below ground surface. Practical refusal to DCPT was encountered at 9.8 m below existing ground surface at borehole BH 7-21.

Reference should be made to the Soil Profile and Test Data sheets in Appendix 1 for specific details of the soil profiles encountered at each test hole location.

## **Bedrock**

Based on available geological mapping, the bedrock in the subject area consists of Paleozoic interbedded Sandstone and Dolomite from the March formation, with an overburden drift thickness of 10 to 15 m depth.

## **Grain Size Distribution and Hydrometer Testing**

Grain size distribution (sieve and hydrometer analysis) testing was completed on three selected soil samples. The results of the grain size analysis are summarized in Table 1 and presented on the Grain-Size Distribution and Hydrometer Testing Results sheets in Appendix 1.

<b>Table 1- Grain Size Distribution</b>						
<b>Test Hole</b>	<b>Sample</b>	<b>Depth (m)</b>	<b>Gravel (%)</b>	<b>Sand (%)</b>	<b>Silt (%)</b>	<b>Clay (%)</b>
BH2-21	SS3 & SS4	1.5-2.9	1.8	89.4	8.8	
BH4-21	SS4 & SS5	2.3-3.7	0.0	88.9	11.1	
BH8-21	SS4 & SS5	2.3-3.7	46.9	43.1	10.0	
TP2-23	G4	2.4-2.7	0.0	94.6	5.4	
TP5-23	G4	2.2-2.5	31.3	67.2	1.5	

## **4.3 Groundwater**

Groundwater levels were measured in piezometers on March 4, 2021. The piezometers in BH 7-21, BH 11-21 and BH 12-21 were damaged or buried and could not be recorded. The remaining boreholes were dry upon completion. Also, no groundwater was observed during the 2023 test pit program.

Long-term groundwater levels can also be estimated based on the observed moisture levels, colour and consistency of the recovered soil samples. Based on these observations, the long-term groundwater table can be expected well below 8 m below existing ground surface. It should be noted that groundwater levels

are subject to seasonal fluctuations. Therefore, the groundwater level could vary at the time of construction.

## 4.4 Low Impact Development Review

Based on the latest Site Servicing Plan prepared by Stantec Consulting Ltd. dated January 14, 2022, it is our understanding that Low Impact Development (LID) measures are being considered for the proposed development. It is further understood that the proposed LID will incorporate a treatment train approach that includes an Etobicoke Exfiltration System (EES) along select roadways within the proposed development.

Upon reviewing the subsurface profile across the subject site and the site servicing plan details, it is anticipated that the subsoil below the proposed exfiltration system will generally consist of either a deep silty sand deposit with varying amounts of gravel, or fill material comprised of silty sand with varying amounts of silty clay, gravel and cobbles. The silty sand deposit has been identified within the north and central portion of the development, while the fill material has been generally observed within the south portion of the development.

### Hydraulic Conductivity and Infiltration Values (Permeameter Tests)

Permeameter tests were conducted at 6 locations (2 tests at each location) to provide general coverage of the subject site on April 25, 2023. Preparation and testing of this investigation are in accordance with the Canadian Standards Association (CSA) B65-12-Annex E. Field saturated hydraulic conductivity ( $K_{fs}$ ) values and estimated infiltration values are presented in Table 2 below.

Field saturated hydraulic conductivity values were determined using the Engineering Technologies Canada (ETC) Ltd. Reference tables provided in the most recent ETC Past Permeameter User Guide dated July 2018. Infiltration rates have been determined based on approximate relationships provided by the Ontario Ministry of Municipal Affairs and Housing – Supplementary Guidelines to the Ontario Building Code, 1997 – SG-6 – Percolation Time and Soil Descriptions.

<b>Table 2 – Summary of field saturated hydraulic conductivity values and infiltration rates</b>					
<b>Test Hole ID</b>	<b>Ground Surface Elevation (m)</b>	<b>Depth of Permeameter Testing (m)</b>	<b><math>K_{fs}</math> (m/sec)</b>	<b>Infiltration Rate (mm/hr)</b>	<b>Soil Type</b>
TP 1-23	103.01	2.7	Too fast to test		Silty to medium sand
		3.2	$3.2 \times 10^{-4}$	216	

Table 2 – Summary of field saturated hydraulic conductivity values and infiltration rates					
Test Hole ID	Ground Surface Elevation (m)	Depth of Permeameter Testing (m)	K <sub>fs</sub> (m/sec)	Infiltration Rate (mm/hr)	Soil Type
TP 2-23	103.87	2.6	9.6x10 <sup>-5</sup>	156	Silty sand
		3.2	Too fast to test		
TP 3-23	104.37	2.5	4.3x10 <sup>-5</sup>	126	Silty sand
		3.0	9.6x10 <sup>-5</sup>	156	
TP 4-23	104.50	2.5	9.6x10 <sup>-5</sup>	156	Silty sand
		3.0	9.6x10 <sup>-5</sup>	156	
TP 5-23	104.70	2.5	3.2x10 <sup>-4</sup>	216	Silty sand with gravel, cobbles and occasional boulders
		3.3	Too fast to test		
TP 6-23	104.94	2.5	1.9x10 <sup>-4</sup>	188	Silty to medium sand
		3.2	2.2x10 <sup>-4</sup>	195	

### Suitability of LID's

Given the measured field saturated hydraulic conductivity and infiltration rates noted in Table 2, both the native silty sand deposit and fill material anticipated below the proposed exfiltration system are considered suitable for the use of LIDs.

Across the majority of the site, infiltration rates ranged from 126 to 216 mm/hr. Therefore, the proposed EES is considered suitable from a geotechnical perspective. However, it is important to note that the infiltration rates derived from the K<sub>fs</sub> values in the table above are unfactored. Prior to use for design purposes, a minimum safety correction factor of 2.5 will need to be applied to the above infiltration rates to account for a number of factors including variations in soil composition and anticipated accumulation of fine-grained material over time. It should also be noted that for most LID measures, the bottom of the facility should be separated at least 1 m from the highest groundwater table.

### Groundwater

Based on the groundwater levels and physical soil parameters that were measured during the field investigations, the long-term groundwater table is expected at a depth greater than 8 to 9 m below existing ground surface. As such, sufficient separation between the proposed exfiltration system and the groundwater table is anticipated at the subject site.



## 5.0 Discussion

### 5.1 Geotechnical Assessment

From a geotechnical perspective, the subject site is suitable for the proposed residential development. It is anticipated that the proposed buildings will be founded over conventional footings placed over an undisturbed compact to dense silty sand or dense glacial till bearing surface or an engineered fill pad over an approved fill subgrade bearing medium.

To adequately distribute the foundation loads in areas where the existing fill is encountered below the building footprint, a woven geotextile liner, such as Terratrack 200 or equivalent, should be placed 500 mm below design underside of footing level and extend at least 1 m horizontally beyond the footing face. A biaxial geogrid, such as Terrafix TBX2500 or equivalent, should be placed over the woven geotextile liner. A minimum 500 mm thick pad, consisting of a Granular B Type II, compacted to 98% of its SPMDD should be placed up to design underside of footing level. Prior to placement of the above noted engineered fill pad, it is recommended that a proof-rolling program be completed by a vibratory roller making several passes and approved by Paterson personnel over the sub-excavated area below the proposed footings.

For areas where a fill layer is encountered below the granular layer for the floor slab, it is recommended to sub-excavate 500 mm below the underside of floor slab granulars and place a woven geotextile liner, such as Terratrack 200W or equivalent, and a biaxial geogrid, such as Terrafix TBX2500 or equivalent. It is recommended that a proof-rolling program be completed by a vibratory roller making several passes and approved by Paterson personnel prior to placement of the geotextile liner and biaxial geogrid. Any poor performing areas should be removed and reinstated with a select subgrade fill compacted to 98% of its SPMDD under dry and above freezing temperatures.

The proof-rolling program should also be completed across paved areas to ensure that any poor performing soils are removed prior to pavement structure placement.

Due to the absence of a silty clay deposit, the aforementioned site will not be subjected to permissible grade raise restrictions. Also, no tree planting setback restrictions are required for the subject phase of the proposed development due to the absence of a silty clay deposit.

The above and other considerations are further discussed in the following sections.



## 5.2 Site Grading and Preparation

### Stripping Depth

Topsoil and deleterious fill, such as those containing organic materials, should be stripped from under any buildings, paved areas, pipe bedding and other settlement sensitive structures.

### Fill Placement

Fill used for grading beneath the proposed building areas should consist, unless otherwise specified, of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A or Granular B Type II. The fill should be placed in lifts of 300 mm thick or less and compacted using suitable compaction equipment for the lift thickness. Fill placed beneath the building areas should be compacted to at least 99% of the Standard Proctor Maximum Dry Density (SPMDD).

Non-specified existing fill along with site-excavated soil can be used as general landscaping fill and beneath parking areas where settlement of the ground surface is of minor concern. In landscaped areas, these materials should be spread in thin lifts and at least compacted by the tracks of the spreading equipment to minimize voids. If these materials are to be used to build up the subgrade level for areas to be paved, they should be compacted in thin lifts to a minimum density of 95% of the SPMDD.

Non-specified existing fill and site-excavated soils are not suitable for use as backfill against foundation walls unless a composite drainage blanket connected to a perimeter drainage system is provided.

### Proof Rolling

Proof rolling of the subgrade is required in areas where the existing fill, free of significant amounts of organics and deleterious materials, is encountered. It is recommended that the subgrade surface be proof-rolled **under dry conditions and in above freezing temperatures** by an adequately sized roller making several passes to achieve optimum compaction levels. The compaction program should be reviewed and approved by the geotechnical consultant at the time of construction.

## 5.3 Foundation Design

### Conventional Spread Footing

Footings placed directly on an undisturbed, compact silty sand or glacial till bearing surface can be designed using a bearing resistance value at serviceability limit states (SLS) of **150 kPa** and a factored bearing resistance value at ultimate limit states (ULS) of **225 kPa**. A geotechnical resistance factor of 0.5 was applied to the above noted bearing resistance value at ULS.

Footings placed over a minimum 500 mm thick geogrid reinforced engineered pad, consisting of a Granular A or Granular B Type II or approved granular fill alternative placed in maximum 300 mm loose lifts and compacted to 98% of its SPMDD, placed over a subgrade soil approved by the Paterson personnel at the time of construction, can be designed using a bearing resistance value at SLS of **150 kPa** and a factored bearing resistance value at ULS of **250 kPa**.

An undisturbed soil bearing surface consists of a surface from which all topsoil and deleterious materials, such as loose, frozen or disturbed soil, whether in situ or not, have been removed, in the dry, prior to the placement of concrete for footings.

Footings placed on a soil bearing surface and designed using the bearing resistance values at SLS given above will be subjected to potential post construction total and differential settlements of 25 and 20 mm, respectively.

Where the silty sand subgrade is found to be in a loose state, the contractor should compact the subgrade under dry conditions and above freezing temperatures, using suitable compaction equipment, making several passes and approved by Paterson.

### Lateral Support

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Adequate lateral support is provided to the in-situ bearing medium soils above the groundwater table when a plane extending down and out from the bottom edge of the footing at a minimum of 1.5H:1V passes only through in situ soil of the same or higher capacity as the bearing medium soil.

## 5.4 Design for Earthquakes

The site class for seismic site response can be taken as **Class D**. Based on the current information, including the level of groundwater table and compactness of the underlying sand layer, the soil underlying the subject site is not susceptible to liquefaction. Reference should be made to the latest revision of the 2012 Ontario Building Code for a full discussion of the earthquake design requirements.

## 5.5 Basement Slab

With the removal of all topsoil and fill, containing significant amounts of deleterious or organic materials, the native soil and/or approved fill pad (placed as per Subsection 5.0) will be considered to be an acceptable subgrade surface on which to commence backfilling for the floor slab. Any poor performing areas should be removed and reinstated with an engineered fill, such as Granular B Type II.

For slab-on-grade areas, it is recommended that the upper 200 mm of sub-slab fill consist OPSS Granular A crushed stone. For basement slabs, it is recommended that the upper 200 mm of sub-floor fill consist of 19 mm clear crushed stone

## 5.6 Basement Wall

There are several combinations of backfill materials and retained soils that could be applicable for the basement walls of the subject structure. However, the conditions can be well-represented by assuming the retained soil consists of a material with an angle of internal friction of 30 degrees and a bulk (drained) unit weight of 18 kN/m<sup>3</sup>.

Where undrained conditions are anticipated (i.e. below the groundwater level), the applicable effective (undrained) unit weight of the retained soil can be taken as 13 kN/m<sup>3</sup>, where applicable. A hydrostatic pressure should be added to the total static earth pressure when using the effective unit weight.

### Lateral Earth Pressure

The static horizontal earth pressure ( $p_o$ ) can be calculated using a triangular earth pressure distribution equal to  $K_o \cdot \gamma \cdot H$  where:

$K_o$  = at-rest earth pressure coefficient of the applicable retained soil (0.5)

$\gamma$  = unit weight of fill of the applicable retained soil (kN/m<sup>3</sup>)

$H$  = height of the wall (m)

An additional pressure having a magnitude equal to  $K_o \cdot q$  and acting on the entire height of the wall should be added to the above diagram for any surcharge loading,  $q$  (kPa), that may be placed at ground surface adjacent to the wall. The surcharge pressure will only be applicable for static analyses and should not be used in conjunction with the seismic loading case.

Actual earth pressures could be higher than the “at-rest” case if care is not exercised during the compaction of the backfill materials to maintain a minimum separation of 0.3 m from the walls with the compaction equipment.

### **Seismic Earth Pressures**

The total seismic force ( $P_{AE}$ ) includes both the earth force component ( $P_o$ ) and the seismic component ( $\Delta P_{AE}$ ).

The seismic earth force ( $\Delta P_{AE}$ ) can be calculated using  $0.375a_c \cdot \gamma \cdot H^2/g$  where:

$$a_c = (1.45 - a_{max}/g)a_{max}$$

$$\gamma = \text{unit weight of fill of the applicable retained soil (kN/m}^3\text{)}$$

$$H = \text{height of the wall (m)}$$

$$g = \text{gravity, 9.81 m/s}^2$$

The peak ground acceleration, ( $a_{max}$ ), for the Ottawa area is 0.32g according to OBC 2012. Note that the vertical seismic coefficient is assumed to be zero.

The earth force component ( $P_o$ ) under seismic conditions can be calculated using  $P_o = 0.5 K_o \gamma H^2$ , where  $K_o = 0.5$  for the soil conditions noted above.

The total earth force ( $P_{AE}$ ) is considered to act at a height,  $h$  (m), from the base of the wall, where:

$$h = \{P_o \cdot (H/3) + \Delta P_{AE} \cdot (0.6)\}$$

## 5.7 Pavement Structure

Driveways, local residential roadways, heavy truck parking/loading areas and roadways with bus traffic are anticipated at this site. The proposed pavement structures are shown in Tables 3, 4 and 5 below.

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project. If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with OPSS Granular B Type II material.

The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 100% of the material's SPMDD using suitable vibratory equipment. Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

<b>Table 3 - Recommended Pavement Structure - Driveways and at-grade car parking areas</b>	
<b>Thickness (mm)</b>	<b>Material Description</b>
50	<b>Wear Course</b> - HL 3 or Superpave 12.5 Asphaltic Concrete
150	<b>BASE</b> - OPSS Granular A Crushed Stone
300	<b>SUBBASE</b> - OPSS Granular B Type II
<b>SUBGRADE</b> - Either fill, in situ soil or OPSS Granular B Type I or II material placed over in situ soil or fill	

<b>Table 4 - Recommended Pavement Structure - Local Residential Roadways and Heavy Truck Parking / Loading Areas</b>	
<b>Thickness (mm)</b>	<b>Material Description</b>
40	<b>Wear Course</b> - Superpave 12.5 Asphaltic Concrete
50	<b>Binder Course</b> - Superpave 19.0 Asphaltic Concrete
150	<b>BASE</b> - OPSS Granular A Crushed Stone
400	<b>SUBBASE</b> - OPSS Granular B Type II
<b>SUBGRADE</b> - Either fill, in situ soil or OPSS Granular B Type I or II material placed over in situ soil	

<b>Table 5 - Recommended Pavement Structure - Roadways with Bus Traffic</b>	
<b>Thickness mm</b>	<b>Material Description</b>
40	<b>Wear Course</b> - Superpave 12.5 Asphaltic Concrete
50	<b>Upper Binder Course</b> - Superpave 19.0 Asphaltic Concrete
50	<b>Lower Binder Course</b> - Superpave 19.0 Asphaltic Concrete
150	<b>BASE</b> - OPSS Granular A Crushed Stone
600	<b>SUBBASE</b> - OPSS Granular B Type II
<b>SUBGRADE</b> - Either in situ soil or OPSS Granular B Type II material placed over in situ soil	

## **6.0 Design and Construction Precautions**

### **6.1 Foundation Drainage and Backfill**

#### **Foundation Drainage**

A perimeter foundation drainage system is recommended for proposed structures. The system should consist of a 100 to 150 mm diameter, geotextile-wrapped, perforated, corrugated, plastic pipe, surrounded on all sides by 150 mm of 10 mm clear crushed stone, placed at the footing level around the exterior perimeter of the structure. The pipe should have a positive outlet, such as a gravity connection to the storm sewer.

#### **Foundation Backfill**

Backfill against the exterior sides of the foundation walls should consist of free-draining, non frost susceptible granular materials. The site materials will be frost susceptible and, as such, are not recommended for re-use as backfill unless a composite drainage system (such as system Delta Drain 6000 or Miradrain G100N) connected to a perimeter drainage system is provided.

### **6.2 Protection Against Frost Action**

Perimeter footings of heated structures are required to be insulated against the deleterious effect of frost action. A minimum 1.5 m thick soil cover should be provided for adequate frost protection of heated structured, or an equivalent combination of soil cover and foundation insulation.

Exterior unheated footings, such as those for isolated exterior piers and loading docks, are more prone to deleterious movement associated with frost action than the exterior walls of the heated structure and require additional protection, such as soil cover of 2.1 m or an equivalent combination of soil cover and foundation insulation.

### **6.3 Excavation Side Slopes**

The side slopes of the excavations in the soil and fill overburden materials should either be cut back at acceptable slopes or should be retained by shoring systems from the start of the excavation until the structure is backfilled. It is expected that sufficient room will be available for the greater part of the excavation to be undertaken by open- cut methods (i.e. unsupported excavations).

## **Unsupported Excavations**

The excavation side slopes above the groundwater level extending to a maximum depth of 3 m should be cut back at 1H:1V or shallower. The shallower slope is required for excavation below groundwater level. The subsoil at this site is considered to be mainly Type 2 and 3 soil according to the Occupational Health and Safety Act and Regulations for Construction Projects.

Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should be kept away from the excavation sides.

Slopes in excess of 3 m in height should be periodically inspected by the geotechnical consultant in order to detect if the slopes are exhibiting signs of distress.

It is recommended that a trench box be used at all times to protect personnel working in trenches with steep or vertical sides. It is expected that services will be installed by “cut and cover” methods and excavations will not be left open for extended periods of time.

## **6.4 Pipe Bedding and Backfill**

Bedding and backfill materials should be in accordance with the most recent Material Specifications and Standard Detail Drawings from the Department of Public Works and Services, Infrastructure Services Branch of the City of Ottawa.

At least 150 mm of OPSS Granular A should be used for pipe bedding for sewer and water pipes. The bedding should extend to the spring line of the pipe. Cover material, from the spring line to at least 300 mm above the obvert of the pipe, should consist of OPSS Granular A or Granular B Type II with a maximum size of 25 mm. The bedding and cover materials should be placed in maximum 225 mm thick lifts compacted to 98% of the material's SPMDD.

It should generally be possible to re-use the site excavated materials above the cover material if the operations are carried out in dry weather conditions.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone, (about 1.5 m below finished grade) and above the cover material should match the soils exposed at the trench walls to minimize differential frost heaving. The trench backfill should be placed in maximum 225 mm thick lifts and compacted to 95% of the materials SPMDD.



## **6.5 Groundwater Control**

It is anticipated that groundwater infiltration into the excavations should be low to moderate and controllable using open sumps. The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium.

### **Permit to Take Water**

A temporary Ministry of the Environment, Conservation and Parks (MECP) permit to take water (PTTW) may be required for this project if more than 400,000 L/day of ground and/or surface water is to be pumped during the construction phase. A minimum of 4 to 5 months should be allowed for completion of the PTTW application package and issuance of the permit by the MECP.

For typical ground or surface water volumes being pumped during the construction phase, between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Person as stipulated under O.Reg. 63/16. If a project qualifies for a PTTW based upon anticipated conditions, an EASR will not be allowed as a temporary dewatering measure while awaiting the MECP review of the PTTW application.

## **6.6 Winter Construction**

Precautions must be taken if winter construction is considered for this project, where excavations are completed in proximity of existing structures which may be adversely affected due to the freezing conditions.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the installation of straw, propane heaters and tarpaulins or other suitable means. The base of the excavations should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

Trench excavations and pavement construction are difficult activities to complete during freezing conditions without introducing frost in the subgrade or in the excavation walls and bottoms. Precautions should be considered if such activities are to be completed during freezing conditions. Additional information could be provided, if required.

## **6.7 Corrosion Potential and Sulphate**

The results on analytical testing show that the sulphate content is less than 0.1%. The results are indicative that Type 10 Portland Cement (normal cement) would be appropriate for this site. The chloride content and the pH of the sample indicate that they are not significant factors in creating a corrosive environment for exposed ferrous metals at this site, whereas the resistivity is indicative of a very low to slightly aggressive corrosive environment.

## 7.0 Recommendations

It is recommended that the following be completed once the master plan and site development are determined.

- ☐ Review detailed grading plan(s) from a geotechnical perspective.
- ☐ Observation of all bearing surfaces prior to the placement of concrete.
- ☐ Sampling and testing of the concrete and fill materials.
- ☐ Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable.
- ☐ Observation of all subgrades prior to backfilling.
- ☐ Field density tests to determine the level of compaction achieved.
- ☐ Sampling and testing of the bituminous concrete including mix design reviews.

A report confirming that these works have been conducted in general accordance with Paterson's recommendations could be issued upon the completion of a satisfactory material testing and observation program by the geotechnical consultant.

## 8.0 Statement of Limitations

The recommendations made in this report are in accordance with Paterson's present understanding of the project. Paterson requests permission to review the grading plan once available. Paterson's recommendations should be reviewed when the drawings and specifications are complete.

The client should be aware that any information pertaining to soils and the test hole log are furnished as a matter of general information only. Test hole descriptions or logs are not to be interpreted as descriptive of conditions at locations other than those of the test holes.

A soils investigation is a limited sampling of a site. Should any conditions at the site be encountered which differ from those at the test locations, Paterson requests to be notified immediately in order to permit reassessment of the recommendations.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Mattamy Homes, or their agents, is not authorized without review by Paterson for the applicability of our recommendations to the alternative use of the report.

### Paterson Group Inc.



Kevin A. Pickard, P.Eng.



Faisal I. Abou-Seido, P.Eng.



Michael Laflamme, P.Geo.

### Report Distribution:

- ☐ Mattamy Homes (email copy)
- ☐ Paterson Group (1 copy)

# APPENDIX 1

SOIL PROFILE AND TEST DATA SHEETS

SYMBOLS AND TERMS

GRAIN SIZE DISTRIBUTION ANALYSIS

ANALYTICAL TESTING RESULTS

DATUM	Geodetic
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REMARKS

**BORINGS BY** Backhoe

**DATE** April 25, 2023

FILE NO.  
**PG5690**

HOLE NO.  
TP 1-23

[illegible]

DATUM	Geodetic
-------	----------

REMARKS

**BORINGS BY** Backhoe

**DATE** April 25, 2023

FILE NO.  
**PG5690**

HOLE NO.  
TP 2-23

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone					Piezometer Construction
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
Ground Surface						0	103.87						
FILL: Brown silty sand, some gravel crushed stone, wood and trash	0.20	G	1					○					
Compact, brown SILTY SAND		G	2			1	102.87	○					
		G	3			2	101.87	○					
		G	4					○					
						3	100.87						
End of Test Pit	3.00												

20406080100

Shear Strength (kPa)

▲ Undisturbed △ Remoulded

## SOIL PROFILE AND TEST DATA

Geotechnical Investigation  
Half Moon Bay South-Phase 8 - 3718 Greenbank Road  
Ottawa, Ontario

DATUM Geodetic



REMARKS

BORINGS BY Backhoe

DATE April 25, 2023

FILE NO.  
**PG5690**

HOLE NO.  
**TP 3-23**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface						0	104.37					
<b>FILL:</b> Brown silty sand, some topsoil, gravel, crushed stone, cobbles, boulders, trace clay and organics		G	1									
1.00						1	103.37					
Compact, brown <b>SILTY SAND</b>		G	2									
		G	3									
		G	4									
</												



## SOIL PROFILE AND TEST DATA

Geotechnical Investigation

Half Moon Bay South-Phase 8 - 3718 Greenbank Road  
Ottawa, Ontario

DATUM Geodetic

REMARKS

BORINGS BY Backhoe

DATE April 25, 2023

FILE NO.  
**PG5690**

HOLE NO.  
**TP 4-23**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone					Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
Ground Surface						0	104.50						
FILL: Brown silty sand, some gravel crushed stone, trace organics	0.20	G	1										
		G	2			1	103.50						
		G	3			2	102.50						
Compact, brown SILTY SAND		G	4			3	101.50						
		G	5			4	100.50						
End of Test Pit	5.00					5	99.50						
								20	40	60	80	100	
								Shear Strength (kPa)					
								▲ Undisturbed    △ Remoulded					

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
Ground Surface						0	104.70	○					
FILL: Brown silty sand, some gravel and crushed stone, occasional cobbles and boulders		G	1					○					
Compact, brown SILTY SAND, trace gravel		G	2			1	103.70						
		G	3					○					
						2	102.70						
		G	4					○					
Dense, brown SILTY SAND with gravel, cobbles and boulders						3	101.70						
						4	100.70	○					
		G	5										
End of Test Pit													

20406080100

Shear Strength (kPa)

▲ Undisturbed△ Remoulded

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
Ground Surface						0	104.94						
FILL: Brown silty sand, trace gravel, crushed stone and organics	0.20	G	1										
Compact, brown SILTY SAND		G	2			1	103.94						
		G	3			2	102.94						
		G	4			3	101.94						
		G	5			4	100.94						
End of Test Pit	4.50												
								20	40	60	80	100	
								Shear Strength (kPa)					
								▲ Undisturbed    △ Remoulded					

## SOIL PROFILE AND TEST DATA

Geotechnical Investigation  
Proposed Mixed Use Development  
3718 Greenbank Road - Ottawa, Ontraio

DATUM Geodetic

REMARKS

BORINGS BY CME 55 Power Auger

DATE 2021 February 17

FILE NO.

PG5690

HOLE NO.

BH 1-21

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE						0	103.45						
Compact to dense, brown <b>SILTY SAND</b>  - Trace gravel by 3.0 m depth		AU	1										
		SS	2	75	17	1	102.45						
		SS	3	75	14	2	101.45						
		SS	4	83	17	3	100.45						
		SS	5	83	13	4	99.45						
		SS	6	67	25	5	98.45						
		SS	7	75	11	6	97.45						
		SS	8	75	20	7	96.45						
		SS	9	83	27	8	95.45						
		SS	10	92	35								
		SS	11	83	24								
		SS	12	83	32								
End of Borehole	8.99												
(Piezometer dry - March 4, 2021)													
								20	40	60	80	100	
								Shear Strength (kPa)					
								▲ Undisturbed    △ Remoulded					

## SOIL PROFILE AND TEST DATA

DATUM	Geodetic
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REMARKS

**BORINGS BY CME 55 Power Auger**

DATE 2021 February 17

FILE NO. PG5690

HOLE NO. **BH 2-21**[illegible]

[illegible]

[illegible]

DATUM Geodetic

REMARKS

BORINGS BY CME 55 Power Auger

DATE 2021 February 18

FILE NO.  
**PG5690**

HOLE NO.  
**BH 5-21**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
GROUND SURFACE												
FILL: Brown silty sand with clay, gravel, trace topsoil	0.81	AU	1			0	105.57					
Compact to dense, reddish brown SILTY SAND		SS	2	58	25	1	104.57					
		SS	3	58	7	2	103.57					
		SS	4	83	14	3	102.57					
		SS	5	83	9	4	101.57					
		SS	6	58	18	5	100.57					
		SS	7	83	32	6	99.57					
		SS	8	100	16	7	98.57					
		SS	9	83	11	8	97.57					
		SS	10	75	19							
		SS	11	75	23							
		SS	12	75	24							
End of Borehole	8.99											
(Piezometer dry - March 4, 2021)												
				</								



## SOIL PROFILE AND TEST DATA

DATUM	Geodetic
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REMARKS

**BORINGS BY CME 55 Power Auger**

DATE 2021 February 19

FILE NO.

PG5690

HOLE NO.

**BH 6-21**

[illegible]

## SOIL PROFILE AND TEST DATA

Geotechnical Investigation  
Proposed Mixed Use Development  
3718 Greenbank Road - Ottawa, Ontraio

DATUM Geodetic

REMARKS

BORINGS BY CME 55 Power Auger

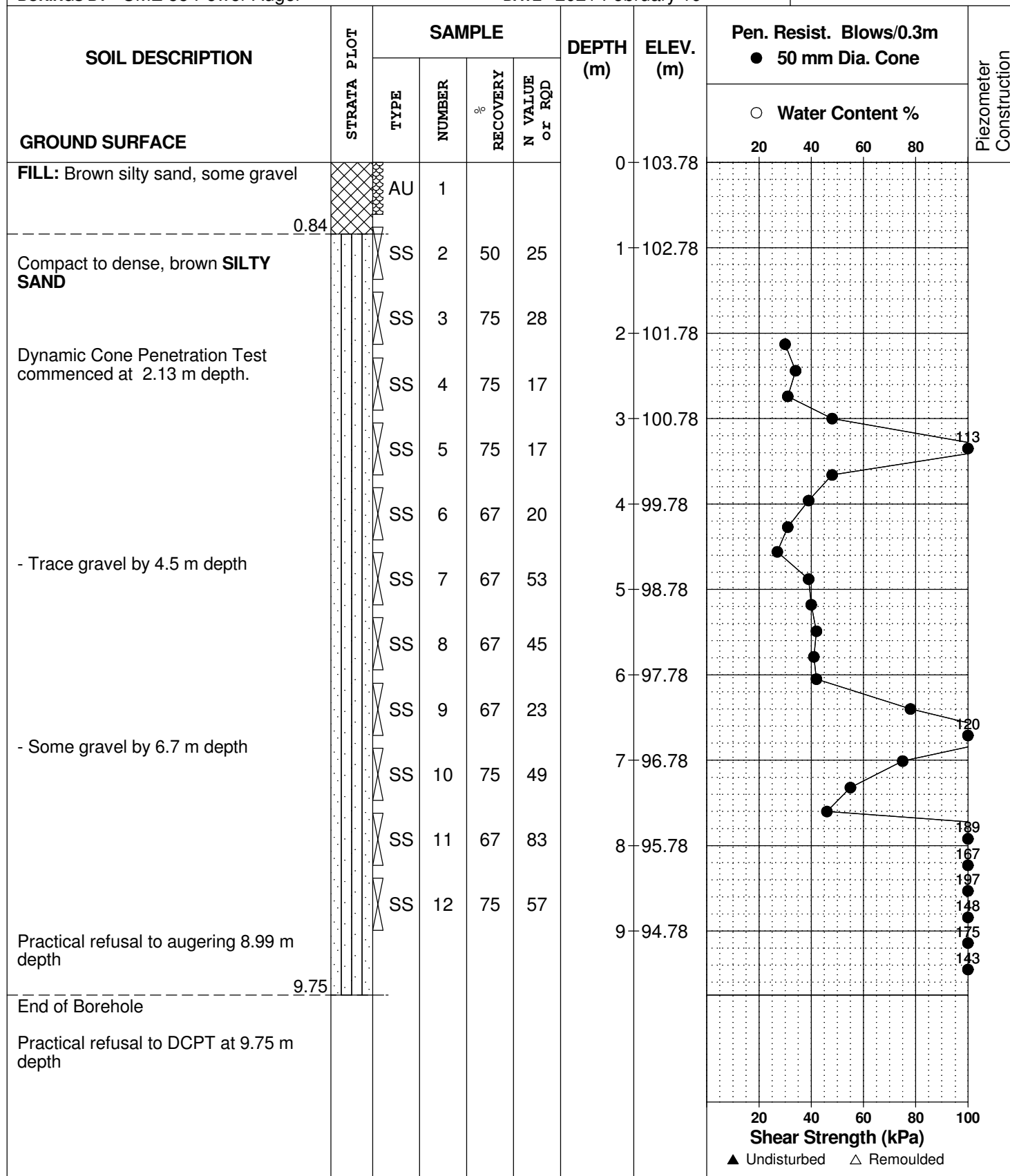
DATE 2021 February 19

FILE NO.

PG5690

HOLE NO.

BH 7-21



## SOIL PROFILE AND TEST DATA

DATUM	Geodetic
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REMARKS

**BORINGS BY CME 55 Power Auger**

**DATE** 2021 February 22

FILE NO. PG5690

HOLE NO. **BH 8-21**

[illegible]

**DATUM**      Geodetic

FILE NO.

**PG5690**

REMARKS

HOLE NO.

**BH 9-21**

**BORINGS BY CME 55 Power Auger**

**DATE** 2021 February 22

[illegible]

[illegible]

## SOIL PROFILE AND TEST DATA

DATUM	Geodetic
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REMARKS

**BORINGS BY CME 55 Power Auger**

**DATE** 2021 February 23

FILE NO.

PG5690

HOLE NO.

**BH11-21**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
GROUND SURFACE												
TOPSOIL	0.05					0	105.87					
FILL: Brown silty clay some sand, gravel, trace topsoil		AU	1									
- Wood fragments present at 0.9 m depth		SS	2	50	4	1	104.87					
		SS	3	33	5	2	103.87					
		SS	4	50	6	3	102.87					
	3.51	SS	5	42	23	4	101.87					
FILL: Brown silty sand with gravel, trace clay		SS	6	8	28	5	100.87					
	5.03	SS	7	33	21	6	99.87					
FILL: Brown silty clay with sand, gravel, cobbles, trace organics		SS	8	25	11	7	98.87					
- Increasing sand with depth		SS	9	33	5	8	97.87					
	7.54	SS	10	17	+50	9	96.87					
FILL: Brown silty sand with gravel, trace topsoil		SS	11	42	28							
Compact brown SILTY SAND with gravel, trace cobbles	8.23	SS	12	42	67							
	9.14											
End of Borehole		SS	13	0	+50							
(Piezometer destroyed - March 4, 2021)												

20 40 60 80 100

**Shear Strength (kPa)**

▲ Undisturbed    △ Remoulded

## SOIL PROFILE AND TEST DATA

**Geotechnical Investigation  
Proposed Mixed Use Development  
3718 Greenbank Road - Ottawa, Ontario**

DATUM	Geodetic
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FILE NO. PG5690

REMARKS

HOLE NO. BH12-21

**BORINGS BY CME 55 Power Auger**

**DATE** 2021 February 23

[illegible]

[illegible]



DATUM	Geodetic
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FILE NO.

**PG5690**

REMARKS

HOLE NO.

**BH 14-21**

**BORINGS BY** CME-55 Low Clearance Drill

**DATE** July 12, 2021

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
GROUND SURFACE												
<div>FILL: Dark brown silty sand, trace crushed stone, gravel and topsoil</div> <div>1.45</div> <div>-----</div> <div>Loose to dense, light brown SILTY SAND, trace gravel</div> <div>5.18</div> <div>-----</div> <div>End of Borehole</div> <div>(BH dry upon completion)</div>	<div><div></div><div></div></div>	AU	1			0	102.34					
	<div><div></div><div></div></div>	SS	2	75	5	1	101.34					
	<div><div></div><div></div></div>	SS	3	83	4	2	100.34					
	<div><div></div><div></div></div>	SS	4	67	2	3	99.34					
	<div><div></div><div></div></div>	SS	5	67	1	4	98.34					
	<div><div></div><div></div></div>	SS	6	83	16	5	97.34					
	<div><div></div><div></div></div>	SS	7		42							
<div><div>20406080100</div><div>Shear Strength (kPa)</div><div>▲ Undisturbed    △ Remoulded</div></div>												

## SOIL PROFILE AND TEST DATA

Geotechnical Investigation  
Proposed Mixed Use Development  
3718 Greenbank Road - Ottawa, Ontraio

DATUM Geodetic

REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE July 12, 2021

FILE NO.

PG5690

HOLE NO.

BH 15-21

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
GROUND SURFACE												
FILL: Brown silty sand, some crushed stone, gravel, trace topsoil	0.30	SS	1	83	11	0	102.82					
Compact, light brown SILTY SAND		SS	2	83	19	1	101.82					
		SS	3	79	31	2	100.82					
		SS	4	75	28	3	99.82					
		SS	5	75	17	4	98.82					
		SS	6	75	28	5	97.82					
		SS	7	67	27							
End of Borehole	5.18											
(BH dry upon completion)												
								Shear Strength (kPa)				
								20	40	60	80	100
								▲ Undisturbed    △ Remoulded				

## SOIL PROFILE AND TEST DATA

Geotechnical Investigation  
Proposed Mixed Use Development  
3718 Greenbank Road - Ottawa, Ontario

DATUM Geodetic

REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE July 12, 2021








FILE NO.

PG5690

HOLE NO.

BH 16-21

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
GROUND SURFACE												
FILL: Brown silty sand, some gravel, trace topsoil	0.15	SS	1	92	10	0	103.04					
Compact, light brown SILTY SAND		SS	2	83	18	1	102.04					
		SS	3	75	29	2	101.04					
		SS	4	92	31	3	100.04					
		SS	5	83	24	4	99.04					
		SS	6	83	22	5	98.04					
		SS	7	75	24	5	98.04					
End of Borehole	5.18											
(BH dry upon completion)												
								Shear Strength (kPa)				
								20	40	60	80	100
								▲ Undisturbed    △ Remoulded				

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE													
FILL: Brown silty sand with gravel, occasional cobbles	0.46		SS	1	83	25	0	103.95					
Compact to dense, light brown SILTY SAND			SS	2	75	14	1	102.95					
			SS	3	75	15	2	101.95					
			SS	4	67	18	3	100.95					
			SS	5	67	50+							
			SS	6		37	4	99.95					
			SS	7		50+	5	98.95					
End of Borehole	5.18												
(BH dry upon completion)													
<div>Shear Strength (kPa) ▲ Undisturbed    △ Remoulded</div>													

DATUM	Geodetic
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FILE NO. PG5690

REMARKS

HOLE NO. **BH 18-21**

**BORINGS BY** CME-55 Low Clearance Drill

**DATE** July 12, 2021

[illegible]

[illegible]

DATUM Geodetic elevations interpolated from City of Ottawa basemap.

FILE NO. **PG3607**

REMARKS

HOLE NO. **BH 5-15**

BORINGS BY CME 75 Power Auger

DATE December 10, 2015

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Monitoring Well Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE						0	108.75						
FILL: Grey silty sand with clay, gravel and wood		SS	1	42	13	1	107.75						
		SS	2	25	21	2	106.75						
		SS	3	33	5	3	105.75						
		SS	4	42	4	4	104.75						
		SS	5	42	3	5	103.75						
		SS	6	42	3	6	102.75						
		SS	7	17	5	7	101.75						
		SS	8	25	37	8	100.75						
		SS	9	0	50+	9	101.75						
		SS	10	58	9	10	100.75						
		SS	11	0	1	11	99.75						
End of Borehole	9.14					9	99.75						
(BH dry to 9.14m depth - July 28, 2016)													
								20	40	60	80	100	
								Shear Strength (kPa)					
								▲ Undisturbed    △ Remoulded					

## SOIL PROFILE AND TEST DATA

**Geotechnical Investigation  
Barrhaven South Urban Expansion  
Ottawa, Ontario**

FILE NO. PG3607

HOLE NO. **TP 1-15**

DATE December 2, 2015

[illegible]



**DATUM** Geodetic elevations interpolated from City of Ottawa basemap.

FILE NO. PG3607

REMARKS

HOLE NO. **TP 2-15**

**BORINGS BY** Backhoe

DATE December 2, 2015

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE						0	106.80						
TOPSOIL	0.10												
Compact, brown <b>SILTY SAND</b>													
	End of Test Pit	3.00					3	103.80					
(TP dry upon completion)													

20406080100

Shear Strength (kPa)

▲ Undisturbed    △ Remoulded

## SOIL PROFILE AND TEST DATA

**Geotechnical Investigation  
Barrhaven South Urban Expansion  
Ottawa, Ontario**

FILE NO. PG3607

HOLE NO. **TP 8-15**

DATE December 1, 2015

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE						0	109.30						
Dense, brown <b>SILTY SAND</b>		G	1			1	108.30						
End of Test Pit (TP dry upon completion)	3.00	G	2			3	106.30						

20406080100

Shear Strength (kPa)

▲ Undisturbed    △ Remoulded

## SOIL PROFILE AND TEST DATA

**Geotechnical Investigation  
Barrhaven South Urban Expansion  
Ottawa, Ontario**

**DATUM** Geodetic elevations interpolated from City of Ottawa basemap.

FILE NO. PG3607

REMARKS

HOLE NO. **TP 9-15**

**BORINGS BY** Backhoe

DATE December 2, 2015

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE						0	108.40						
TOPSOIL	0.20												
Brown <b>SILTY SAND</b> , trace cobbles													

# SYMBOLS AND TERMS

## SOIL DESCRIPTION

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the relative strength of cohesionless soils is the compactness condition, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm. An SPT N value of "P" denotes that the split-spoon sampler was pushed 300 mm into the soil without the use of a falling hammer.

Compactness Condition	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory shear vane tests, unconfined compression tests, or occasionally by the Standard Penetration Test (SPT). Note that the typical correlations of undrained shear strength to SPT N value (tabulated below) tend to underestimate the consistency for sensitive silty clays, so Paterson reviews the applicable split spoon samples in the laboratory to provide a more representative consistency value based on tactile examination.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

## SYMBOLS AND TERMS (continued)

### SOIL DESCRIPTION (continued)

Cohesive soils can also be classified according to their “sensitivity”. The sensitivity,  $S_t$ , is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil. The classes of sensitivity may be defined as follows:

Low Sensitivity:	$S_t < 2$
Medium Sensitivity:	$2 < S_t < 4$
Sensitive:	$4 < S_t < 8$
Extra Sensitive:	$8 < S_t < 16$
Quick Clay:	$S_t > 16$

### ROCK DESCRIPTION

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NQ or larger size core. However, it can be used on smaller core sizes, such as BQ, if the bulk of the fractures caused by drilling stresses (called “mechanical breaks”) are easily distinguishable from the normal in situ fractures.

RQD %	ROCK QUALITY
90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

### SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard Penetration Test (SPT))
TW	-	Thin wall tube or Shelby tube, generally recovered using a piston sampler
G	-	"Grab" sample from test pit or surface materials
AU	-	Auger sample or bulk sample
WS	-	Wash sample
RC	-	Rock core sample (Core bit size BQ, NQ, HQ, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.

## SYMBOLS AND TERMS (continued)

### PLASTICITY LIMITS AND GRAIN SIZE DISTRIBUTION

WC%	-	Natural water content or water content of sample, %
LL	-	Liquid Limit, % (water content above which soil behaves as a liquid)
PL	-	Plastic Limit, % (water content above which soil behaves plastically)
PI	-	Plasticity Index, % (difference between LL and PL)
D <sub>xx</sub>	-	Grain size at which xx% of the soil, by weight, is of finer grain sizes These grain size descriptions are not used below 0.075 mm grain size
D <sub>10</sub>	-	Grain size at which 10% of the soil is finer (effective grain size)
D <sub>60</sub>	-	Grain size at which 60% of the soil is finer
C <sub>c</sub>	-	Concavity coefficient = $(D_{30})^2 / (D_{10} \times D_{60})$
C <sub>u</sub>	-	Uniformity coefficient = $D_{60} / D_{10}$

C<sub>c</sub> and C<sub>u</sub> are used to assess the grading of sands and gravels:

Well-graded gravels have:  $1 < C_c < 3$  and  $C_u > 4$

Well-graded sands have:  $1 < C_c < 3$  and  $C_u > 6$

Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded.

C<sub>c</sub> and C<sub>u</sub> are not applicable for the description of soils with more than 10% silt and clay  
(more than 10% finer than 0.075 mm or the #200 sieve)

### CONSOLIDATION TEST

p' <sub>o</sub>	-	Present effective overburden pressure at sample depth
p' <sub>c</sub>	-	Preconsolidation pressure of (maximum past pressure on) sample
C <sub>cr</sub>	-	Recompression index (in effect at pressures below p' <sub>c</sub> )
C <sub>c</sub>	-	Compression index (in effect at pressures above p' <sub>c</sub> )
OC Ratio		Overconsolidation ratio = $p'_c / p'_o$
Void Ratio		Initial sample void ratio = volume of voids / volume of solids
W <sub>o</sub>	-	Initial water content (at start of consolidation test)

### PERMEABILITY TEST

k	-	Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.
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## SYMBOLS AND TERMS (continued)

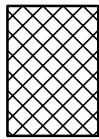
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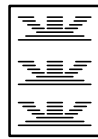
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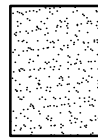
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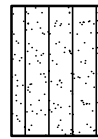
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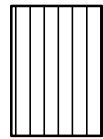
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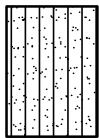
Sand



Silty Sand



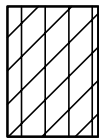
Silt



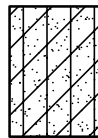
Sandy Silt



Clay



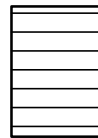
Silty Clay



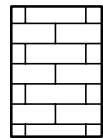
Clayey Silty Sand



Glacial Till



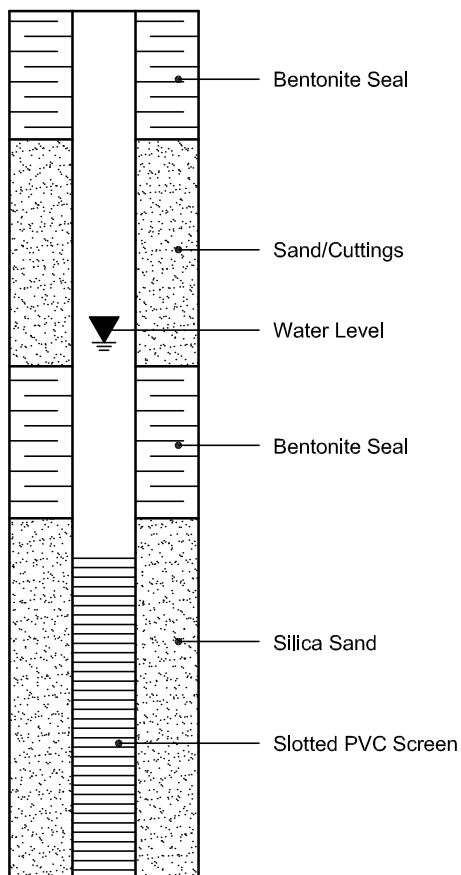
Shale



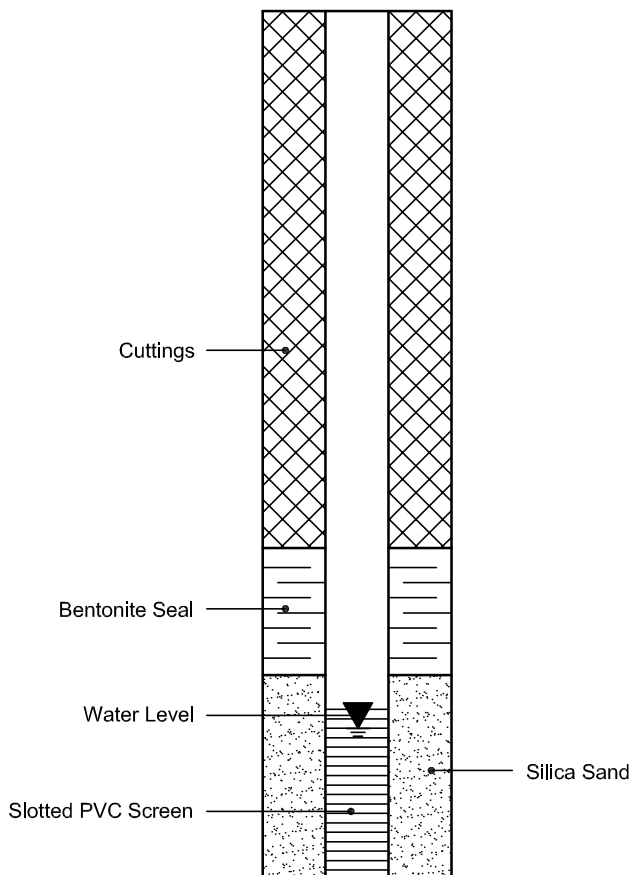
Bedrock

### MONITORING WELL AND PIEZOMETER CONSTRUCTION

#### MONITORING WELL CONSTRUCTION



#### PIEZOMETER CONSTRUCTION





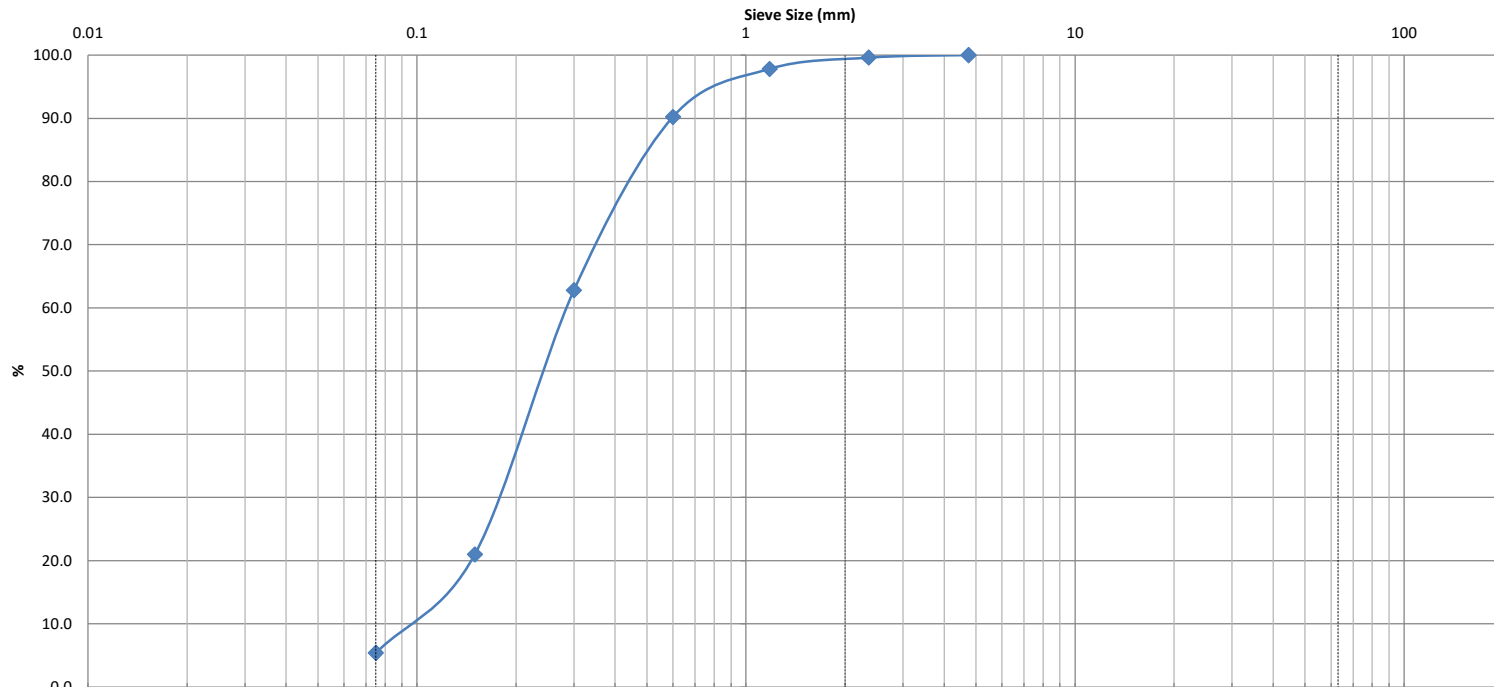
PATERSON  
GROUP

SIEVE ANALYSIS

C136

ASTM

CLIENT:	Mattamy Homes	DESCRIPTION:	Fine Aggregate	FILE NO:	PG5690
CONTRACT NO.:	-	SPECIFICATION:	Sand	LAB NO:	42436
PROJECT:	3718 Greenbank Rd.	INTENDED USE:	-	DATE RECEIVED:	3-May-23
		PIT OR QUARRY:	Pit	DATE TESTED:	4-May-23
DATE SAMPLED:	25-Apr-23	SOURCE LOCATION:	-	DATE REPORTED:	9-May-23
SAMPLED BY:	J.P	SAMPLE LOCATION:	TP2-23 G4 / 2.4-2.7m	TESTED BY:	C.P/A.L/M.O



Identification	Soil Classification					MC(%)	LL	PL	PI	Cc	Cu
	D100	D60	D30	D10	Gravel (%)	Sand (%)		Silt (%)		Clay (%)	
	4.75	0.295	0.185	0.098	0.0	94.6		5.4			

Comments:	
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REVIEWED BY:	Curtis Beadon	Joe Fosyth, P. Eng.



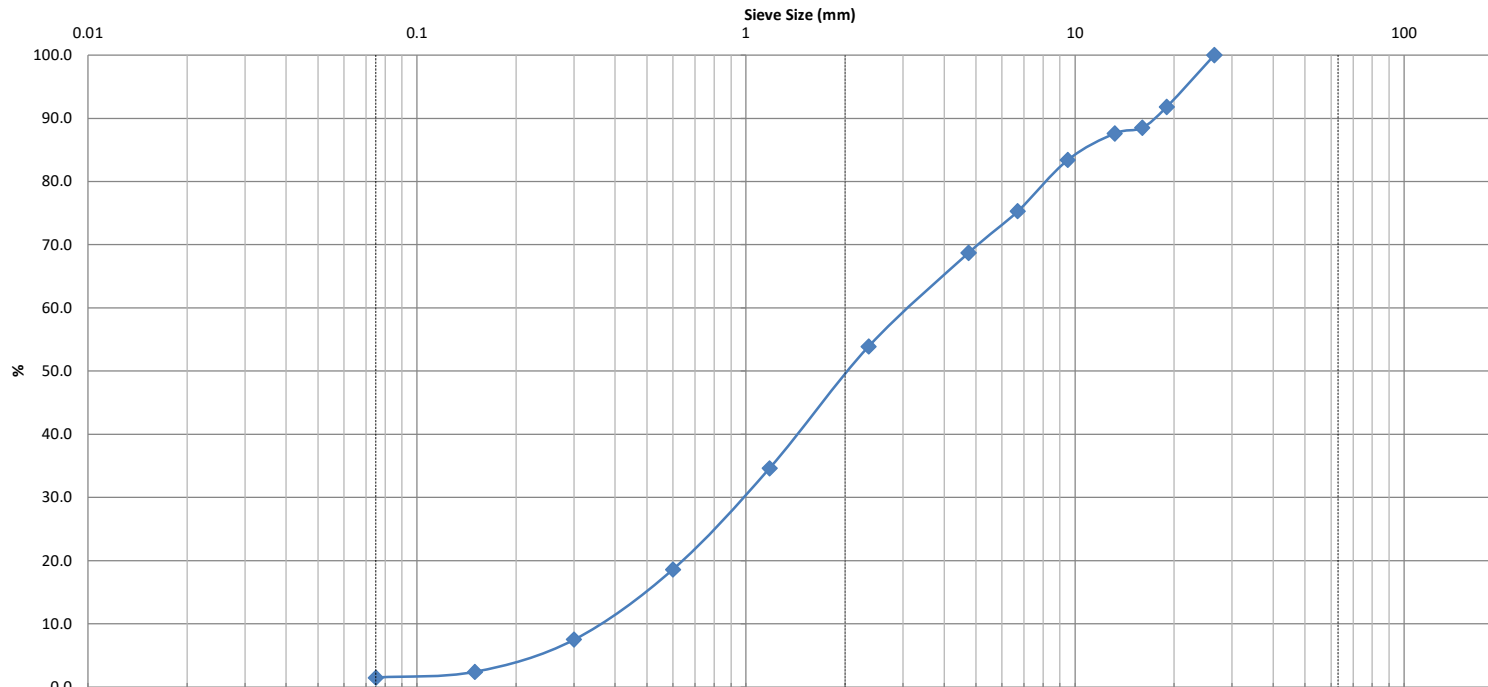


## SIEVE ANALYSIS

C136

ASTM

CLIENT:	Mattamy Homes	DESCRIPTION:	Fine Aggregate	FILE NO:	PG5690
CONTRACT NO.:	-	SPECIFICATION:	Sand/Gravel w Cobbles	LAB NO:	42437
PROJECT:	3718 Greenbank Rd.	INTENDED USE:	-	DATE RECEIVED:	3-May-23
		PIT OR QUARRY:	Pit	DATE TESTED:	4-May-23
DATE SAMPLED:	25-Apr-23	SOURCE LOCATION:	-	DATE REPORTED:	9-May-23
SAMPLED BY:	J.P	SAMPLE LOCATION:	TP5-23 G4 / 2.2-2.5m	TESTED BY:	C.P/A.L/M.O



Identification	Soil Classification					MC(%)	LL	PL	PI	Cc	Cu
	D100	D60	D30	D10	Gravel (%)	Sand (%)	Silt (%)	Clay (%)			
	26.5	3.12	0.98	0.37	31.3	67.2	1.5			0.83	8.4

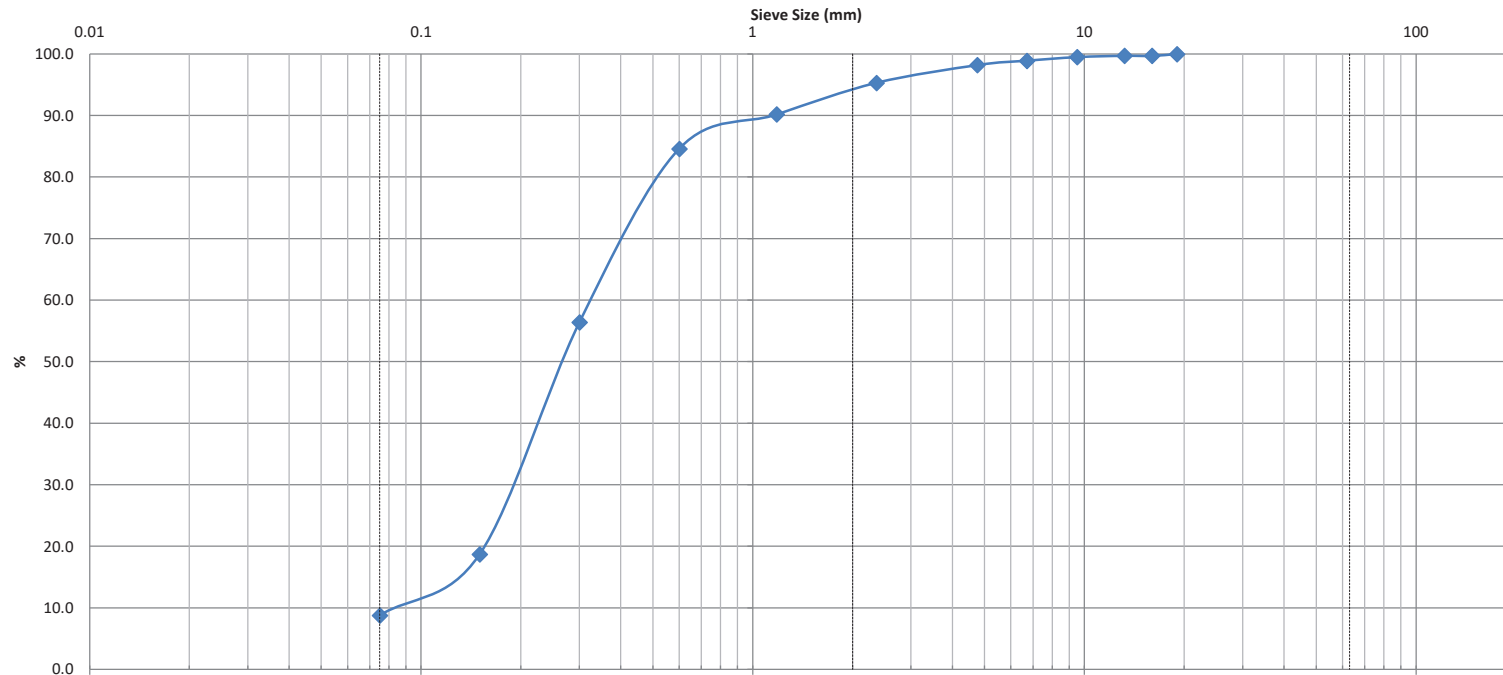
Comments:

REVIEWED BY:

Curtis Beadow

Joe Fosyth, P. Eng.

CLIENT:	Mattamy Homes	DESCRIPTION:	Soil	FILE NO:	PG5690
CONTRACT NO.:	-	SPECIFICATION:	<b>Silty Sand</b>	LAB NO:	23721
PROJECT:	3718 Greenbank Road	INTENDED USE:	-	DATE RECEIVED:	25-Mar-21
		PIT OR QUARRY:	in-Situ	DATE TESTED:	26-Mar-21
DATE SAMPLED:	17-Feb-21	SOURCE LOCATION:	BH2-21 SS3 & SS4	DATE REPORTED:	29-Mar-21
SAMPLED BY:	G. Paterson	SAMPLE LOCATION:	1.5 - 2.9 m	TESTED BY:	DK



Silt and Clay	Sand			Gravel		Cobble
	Fine	Medium	Coarse	Fine	Coarse	

Identification	Soil Classification					MC(%)	LL	PL	PI	Cc	Cu
	D100	D60	D30	D10	Gravel (%)	Sand (%)	Silt (%)	Clay (%)		1.38	3.9
	19.0	0.32	0.19	0.082	1.8	89.4	8.8				

Comments:

REVIEWED BY:

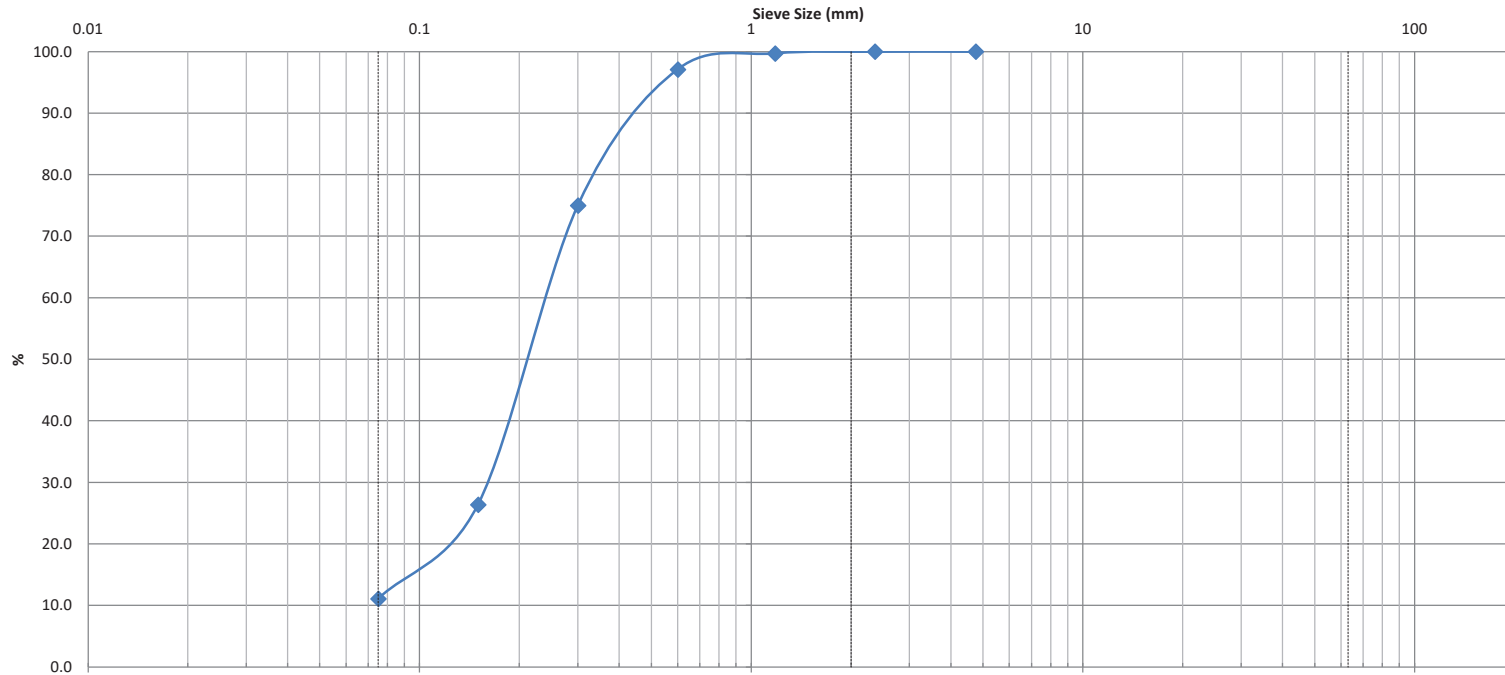
Curtis Beadow

Joe Fosyth, P. Eng.

*[Signature]*

*[Signature]*

CLIENT:	Mattamy Homes	DESCRIPTION:	Soil	FILE NO:	PG5690
CONTRACT NO.:	-	SPECIFICATION:	<b>Silty Sand</b>	LAB NO:	23722
PROJECT:	3718 Greenbank Road	INTENDED USE:	-	DATE RECEIVED:	25-Mar-21
		PIT OR QUARRY:	in-Situ	DATE TESTED:	26-Mar-21
DATE SAMPLED:	17-Feb-21	SOURCE LOCATION:	BH4-21 SS4 & SS5	DATE REPORTED:	29-Mar-21
SAMPLED BY:	G. Paterson	SAMPLE LOCATION:	2.29 - 3.66 m	TESTED BY:	DK



Silt and Clay	Sand			Gravel		Cobble
	Fine	Medium	Coarse	Fine	Coarse	

Identification	Soil Classification					MC(%)	LL	PL	PI	Cc	Cu
	D100	D60	D30	D10	Gravel (%)	Sand (%)	Silt (%)	Clay (%)		1.80	3.3
	4.8	0.23	0.17	0.07	0.0	88.9	11.1				

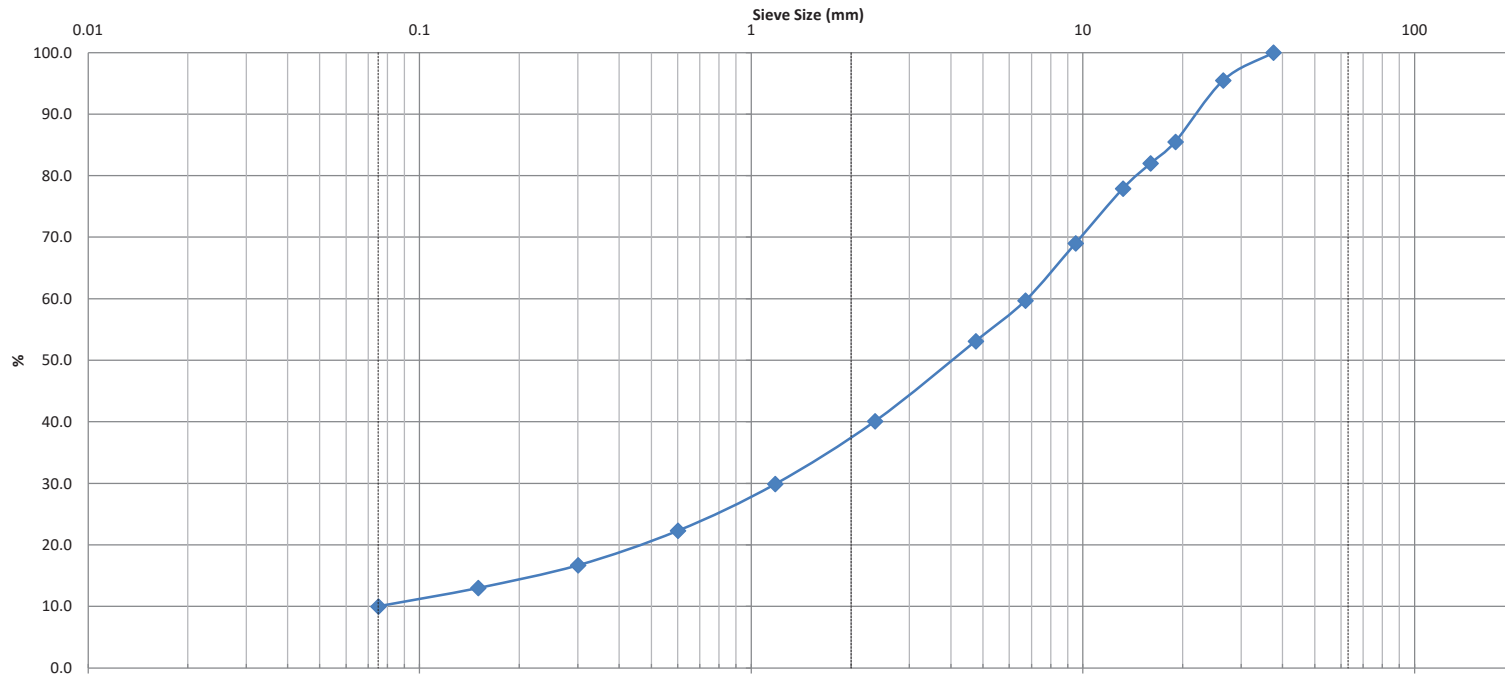
Comments:

REVIEWED BY:

Curtis Beadow

Joe Fosyth, P. Eng.

CLIENT:	Mattamy Homes	DESCRIPTION:	Soil	FILE NO:	PG5690
CONTRACT NO.:	-	SPECIFICATION:	<b>Silty Sand</b>	LAB NO:	23723
PROJECT:	3718 Greenbank Road	INTENDED USE:	-	DATE RECEIVED:	25-Mar-21
		PIT OR QUARRY:	in-Situ	DATE TESTED:	26-Mar-21
DATE SAMPLED:	17-Feb-21	SOURCE LOCATION:	BH8-21 SS4 & SS5	DATE REPORTED:	29-Mar-21
SAMPLED BY:	G. Paterson	SAMPLE LOCATION:	2.29 - 3.66 m	TESTED BY:	DK



Silt and Clay	Sand			Gravel		Cobble
	Fine	Medium	Coarse	Fine	Coarse	

Identification	Soil Classification					MC(%)	LL	PL	PI	Cc	Cu
	D100	D60	D30	D10	Gravel (%)	Sand (%)	Silt (%)	Clay (%)		3.54	104.6
	37.5	6.8	1.25	0.065	46.9	43.1		10.0			

Comments:

REVIEWED BY:

Curtis Beadow

Joe Fosyth, P. Eng.

*[Signature]*

*[Signature]*

# APPENDIX 2

FIGURE 1 - KEY PLAN

FIGURE 2 TO 5 – AERIAL PHOTOGRAPHS

DRAWING PG5690-1 – TEST HOLE LOCATION PLAN



**FIGURE 1**

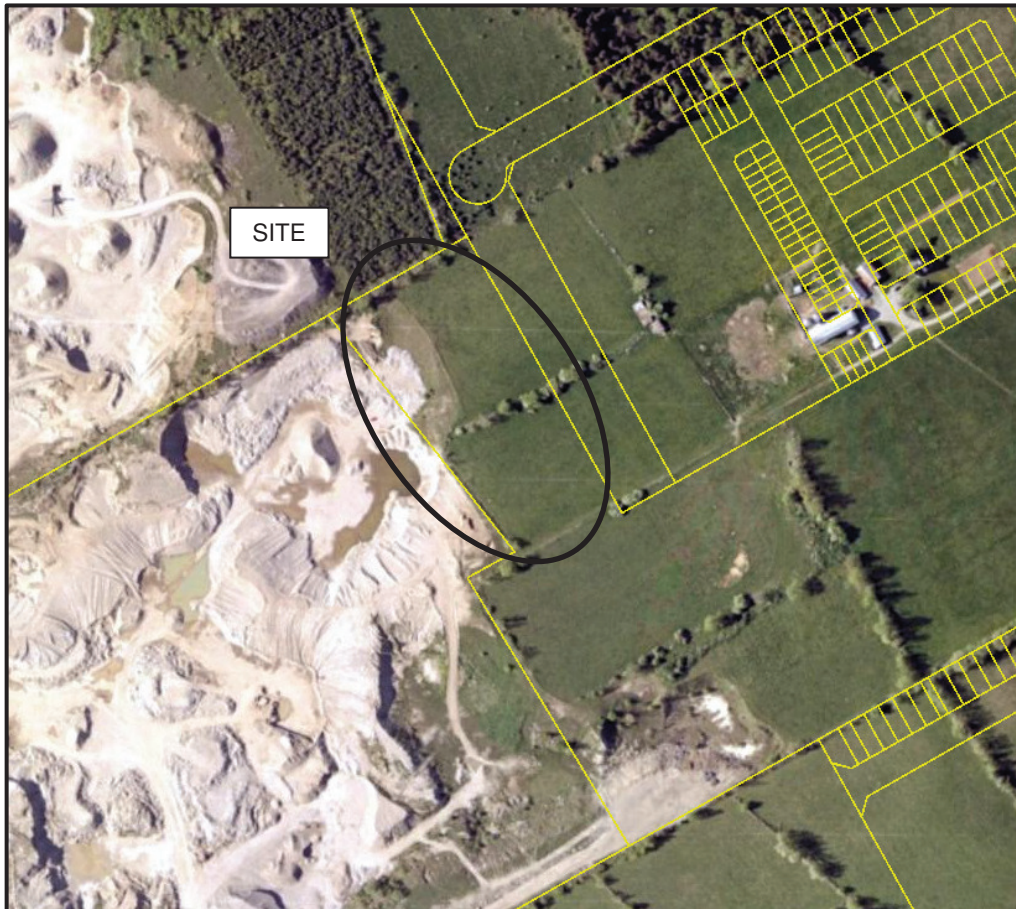
**KEY PLAN**



## FIGURE 2

Aerial Photograph - 1976





## FIGURE 3

Aerial Photograph - 2002





## FIGURE 4

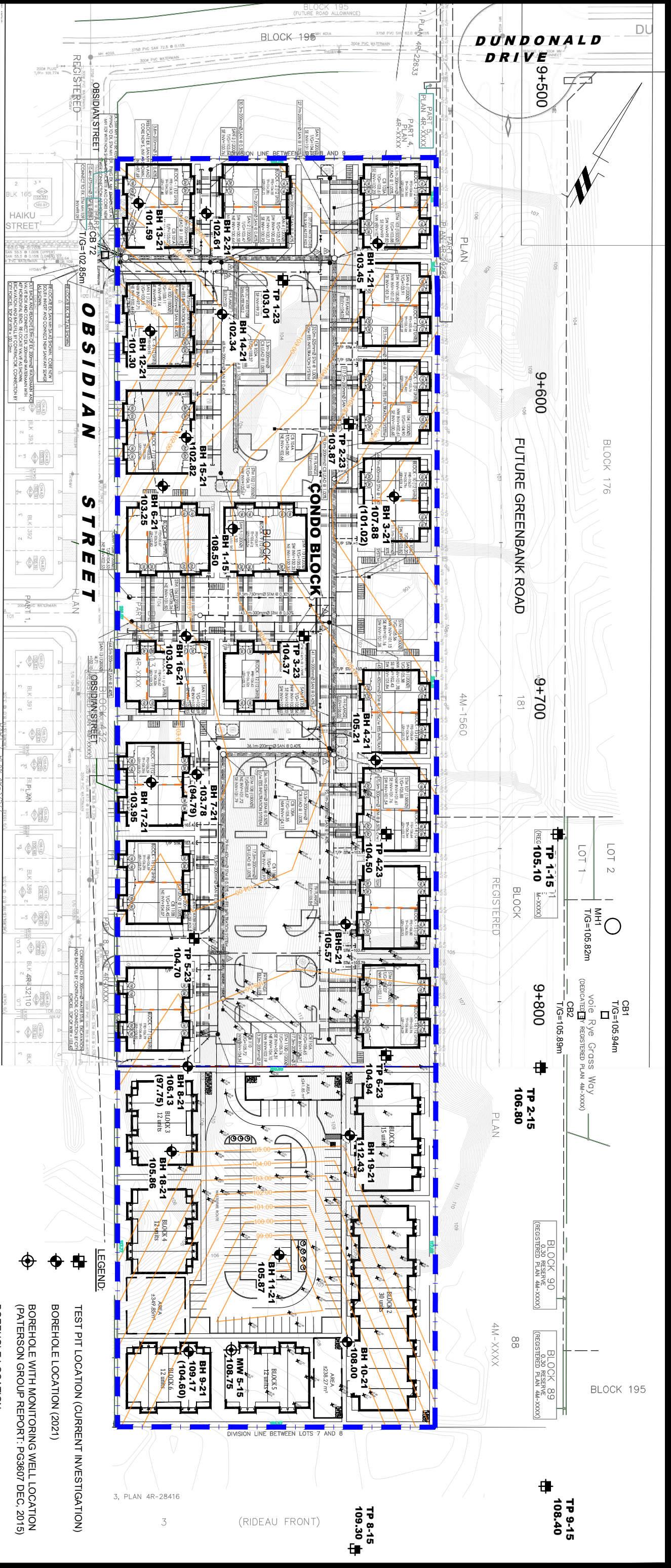
Aerial Photograph - 2008




## FIGURE 5

Aerial Photograph - 2019







PATERSON GROUP

9 AURIGA DRIVE  
OTTAWA, ON  
K2E 7T9  
TEL: (613) 226-7381

NO.	REVISIONS	DATE	INITIAL
1	REVISED TO INCLUDE NEW 2021 TEST PITS	14/07/2021	OC
2	REVISED TO INCLUDE NEW CONCEPTUAL PLAN	05/05/2023	SK
3	REVISED TO INCLUDE NEW CONCEPTUAL PLAN	02/04/2025	UR

MATTAMY HOMES

GEOTECHNICAL INVESTIGATION

HALF MOON BAY - SOUTH - PHASE 7 & 8

ONTARIO

Title:

OTTAWA,

TEST HOLE LOCATION PLAN

Scale:

1:1250

Date:

03/2021

Drawn by:

YA

Report No.:

PG5690-1

Checked by:

KP

Dwg No.:

PG5690-1

Approved by:

DUG

Revision No.:

3

p:\autocad drawings\geotechnical\pg56xx\pg5690\pg5690-1-test hole location plan (rev.03).dwg