

**Burnett Lands - 3370 Greenbank Road**

**Site Serviceability and Stormwater  
Management Report**

**BURNETT LANDS**  
**3370 GREENBANK ROAD**  
**SITE SERVICEABILITY AND STORMWATER**  
**MANAGEMENT REPORT**

Prepared for:

**Claridge Homes**

Prepared By:

**NOVATECH**  
Suite 200, 240 Michael Cowpland Drive  
Ottawa, Ontario  
K2M 1P6

**December 9, 2016**

Novatech File: 111117

**Ref: R-2016-170**

December 9, 2016

City of Ottawa  
Planning, Infrastructure and Economic Development Department  
Planning Services Branch  
110 Laurier Ave. West, 4<sup>th</sup> Floor  
Ottawa, Ontario  
K1P 1J1

**Attention: Mr. Don Herweyer, Manager of Development Review South**

**Reference: Burnett Lands - 3370 Greenbank Road  
Site Serviceability and Stormwater Management Report  
Novatech File No.: 111117**

---

Enclosed herein are six (6) copies of the "Site Serviceability and Stormwater Management Report" for the proposed development of the Burnett Lands located at 3370 Greenbank Road, Ottawa. The report is submitted in support of applications for Official Plan Amendment, Zoning By-Law Amendment and Draft Plan of Subdivision. It will address how the subject development will be serviced with sanitary sewer, storm sewers, watermain and stormwater management.

Should you have any questions or comments, please do not hesitate to contact us.

Sincerely,

**NOVATECH**

Greg MacDonald, P.Eng.  
Director, Land Development and Public Sector Projects

Encl.



## TABLE OF CONTENTS

<b>1.0</b>	<b>INTRODUCTION</b> .....	<b>1</b>
1.1	Background .....	1
1.2	Additional Reports .....	2
<b>2.0</b>	<b>EXISTING CONDITIONS</b> .....	<b>3</b>
2.1	Topography & Drainage.....	3
2.2	Subsurface Conditions.....	3
<b>3.0</b>	<b>SANITARY SERVICING</b> .....	<b>4</b>
<b>4.0</b>	<b>WATERMAIN</b> .....	<b>5</b>
<b>5.0</b>	<b>STORMWATER MANAGEMENT CRITERIA</b> .....	<b>7</b>
<b>6.0</b>	<b>BURNETT MUNICIPAL DRAIN</b> .....	<b>8</b>
<b>7.0</b>	<b>STORMWATER MANAGEMENT DESIGN</b> .....	<b>9</b>
7.1	Storm Sewer Design (Minor System).....	9
7.2	Inlet Control Devices.....	10
7.3	Overland Flow Path (Major System) .....	10
7.4	Street 'B'/ Caivan Lands .....	10
7.5	Water Quality.....	10
<b>8.0</b>	<b>HYDROLOGIC &amp; HYDRAULIC MODELING</b> .....	<b>11</b>
8.1	Design Storms .....	11
8.2	Model Development.....	11
8.2.1	<i>Storm Drainage Areas</i> .....	11
8.2.2	<i>Subcatchment Model Parameters</i> .....	11
8.2.3	<i>Minor System</i> .....	14
8.2.4	<i>Major System</i> .....	15
8.2.5	<i>Modeling Files / Schematic</i> .....	16
8.3	PCSWMM Model Results .....	16
8.3.1	<i>Minor System</i> .....	16
8.3.2	<i>Major System</i> .....	16
8.3.3	<i>Hydraulic Grade Line</i> .....	18
<b>9.0</b>	<b>UTILITIES</b> .....	<b>20</b>
<b>10.0</b>	<b>EROSION AND SEDIMENT CONTROL</b> .....	<b>20</b>
<b>11.0</b>	<b>CONCLUSIONS AND RECOMMENDATIONS</b> .....	<b>21</b>

**LIST OF TABLES**

Table 4.1: Water Demand Summary (Pre Watermain Reconfiguration)

Table 4.2: Water Demand Summary (Post Watermain Reconfiguration)

Table 7.1: Storm Sewer Design Parameters

Table 8.1: Hydrologic Modeling Parameters

Table 8.2: ICD Parameters

Table 8.3: Minor System Peak Flows at Outlets

Table 8.4: Ponding Depths at Catchbasins (100yr Event)

Table 8.5: Storm Sewer Hydraulic Grade Line: 5yr-4hr Chicago Distribution, 100-yr WL in Jock River

**List of Figures**

- Figure 1 Site Location Plan
- Figure 2 Concept Plan
- Figure 3 Realigned Greenbank Road Watermain
- Figure 4 Watermain Layout
- Figure 5 Burnett Municipal Drain

**Appendices**

- Appendix A Design Sheets
- Appendix B Sanitary Report Excerpts
- Appendix C Watermain Boundary Conditions, FUS Calculations, and Modelling Results
- Appendix D SWM Calculations & PCSWMM Model
- Appendix E Burnett Municipal Drain Analysis
- Appendix F Engineering Drawings

**Drawings**

- 111117 – GP General Plan of Services (revision 1)
- 111117 – GR Grading Plan (revision 1)
- 111117 – SAN Sanitary Drainage Area Plan (revision 1)
- 111117 – STM Storm Drainage Area Plan (revision 1)

**Enclosed CD**

- PCSWMM Model Files
- PCSWMM Model Output

## 1.0 INTRODUCTION

Novatech has been retained by Claridge Homes to prepare a Site Serviceability & Stormwater Management Report for the lands located at 3370 Greenbank Road (herein referred to as the “Burnett Lands”). This report has been prepared in support of the application for Official Plan Amendment, Zoning By-Law Amendment, and Draft Plan of Subdivision, and outlines the servicing and proposed storm drainage and stormwater management strategy for the site.

### 1.1 Background

The subject site is approximately 15.5 hectares in area and is located immediately north of the Jock River, south of Strandherd Drive and between the Kennedy Burnett Stormwater Management Facility and the existing Greenbank Road as shown on **Figure 1**. The Burnett Municipal Drain is tributary to the Jock River and travels through the subject site. The property currently has farm and accessory structures located near its southern boundary with an existing gravel access on to Greenbank Road. The remainder of the site is currently used for passive agriculture activities. The topography is generally flat with a gentle slope from the northeastern corner to the southwestern corner.



Figure 1: Site Location (Base Map Source: GeoOttawa)

The following describes the existing and planned land uses adjacent to the subject site:

**North:** Lands to the north, owned by Caivan Communities, are currently under the development approval process and have recently obtained OPA and ZBLA approval (Amendment #144) from the City to permit High Rise and Mid Rise Mixed-Use Residential developments, Mid Rise Residential Dwellings, and a Neighbourhood Park as per *Schedule 1- Land Use Plan, South Nepean Secondary Plan (Area 7)*. Further north of the Caivan Communities' development is the planned Barrhaven Town Centre which will include a variety of retail uses to service the surrounding existing and planned residential developments.

**East:** Lands east of the subject site contain a mixture of low density residential dwellings (single detached houses), a secondary school (St. Joseph Catholic High School), and an existing vegetated area. Greenbank Road currently forms the eastern boundary of the site. The realigned Greenbank Road will bisect the site as per the design by the City.

**South:** The Jock River flows west – east along the majority of the southern boundary of the property until it turns south near the southeastern corner of the site. The lands south of Jock River are within the Barrhaven *South Community Design Plan* and are intended for a future district park and residential uses as shown on *Figure 17* of the *Barrhaven South Community Design Plan*.

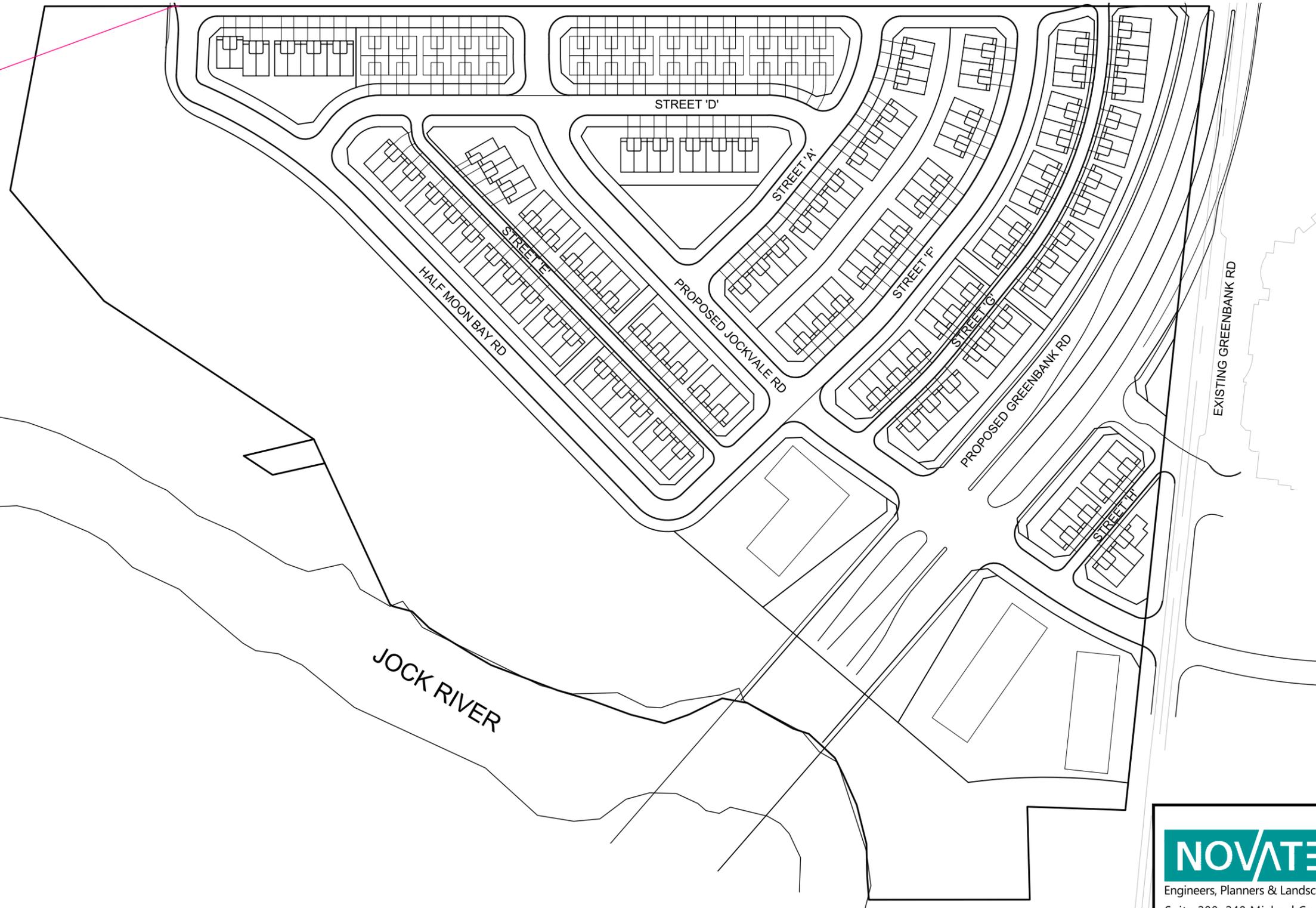
**West:** The Kennedy Burnett stormwater management facility is located north west of the subject site and drains into the Jock River. Lands immediately west are currently vacant and intended for mostly conservation and residential uses as identified in Schedule B of the Official Plan.

Plans are to develop a residential subdivision, as shown on **Figure 2 – Concept Plan**, which will consist of 247 townhome units and 420 condo units for a total of 667 units.

## 1.2 Additional Reports

This report provides information on the considerations and approach by which Novatech has designed and evaluated the proposed servicing for the Burnett Lands. This report should be read in conjunction with the following:

- *South Nepean Collector Sewer Alignment & Finalization Report – Phase 2* prepared by Novatech, dated June 2014
- *Hydrology Report – July 2004: Jock River Flood Risk Mapping (within the City of Ottawa)* prepared by the Rideau Valley Conservation Authority, dated July 2004
- *Hydraulics Report – November 2004: Jock River Flood Risk Mapping (within the City of Ottawa)* prepared by the Rideau Valley Conservation Authority, dated November 2004
- *Greenbank (Burnett Municipal Drain) Headwaters Report*, prepared by Bowfin Environmental Consulting and Muncaster Environmental Planning Inc., dated March 2016.
- *Geotechnical Investigation Proposed Residential Development Burnett Lands, Greenbank Road at the Jock River, Ottawa Ontario*, Prepared by Golder Associates Ltd., dated May 2016 (Report No. 1523044-1000)



I:\NOVATECH\2008\Novatech\2011\1117\CAD\Design\Figures\111117-Fig2-CF.dwg, Fig2-CF, Nov 22, 2016 - 3:55pm, kbanks

**NOVATECH**  
 Engineers, Planners & Landscape Architects  
 Suite 200, 240 Michael Cowpland Drive  
 Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643  
 Facsimile (613) 254-5867  
 Website www.novatech-eng.com

**3370 GREENBANK RD.  
 BURNETT LANDS**

**CONCEPT PLAN**

SCALE 1 : 2000

DATE	JOB	FIGURE
NOV 2016	111117	FIGURE 2

## 2.0 EXISTING CONDITIONS

### 2.1 Topography & Drainage

The proposed site is currently undeveloped and consists of agricultural lands. Access to the site is currently provided at Greenbank Road. The site has a gentle slope from north to south, with most overland flow being directed to the existing Burnett Municipal Drain, which flows into the Jock River at the south end of the site boundary. The Burnett Municipal Drain transects the site in a north to south fashion.

### 2.2 Subsurface Conditions

Golder Associates Ltd. has completed a geotechnical investigation in support of the proposed development. The report is titled "Geotechnical Investigation, Proposed Residential Development, Burnett Lands, Greenbank Road at the Jock River, Ottawa Ontario, dated May 2016 (Report No. 1523044-1000)." The fieldwork for this investigation was carried out between February 18<sup>th</sup> and 23<sup>rd</sup>, 2016. The work consisted of advancing nine (9) boreholes to depths ranging from 1.7m to 8.2 m below ground surface. The principal findings of the Geotechnical Investigation are as follows:

- The site was divided into two distinct areas (A and B), with Area 'A' having a thick deposit of silty clay up to 8.2m deep and Area 'B' having a very stiff to stiff layer of silty clay overlying glacial till, or glacial till near the ground surface.
- Area 'A' generally consists of a 1.3 to 3.1 m layer of a silty clay deposit beneath the topsoil layer, which has been weathered to a grey brown crust. Below this is a grey silty clay, which extends to a depth of 3.1 to 8.2 m.
  - In some of the boreholes within Area 'A', glacial till was encountered beneath the silty clay deposit at a depth of 3.1 to 4.3 m.
- Area 'B' generally consists of a 0.6 to 2.5 m layer of a silty clay deposit beneath the topsoil layer, which has been weathered to a grey brown crust. Below this is a silty sand layer which ranges from 0.3 to 0.9 m thick. In all boreholes, glacial till exists below the silty clay and silty sand at depths ranging from 0.3 to 2.5 m.
- Groundwater inflow was observed in some of the boreholes at depths of between 0.92 m and 2.17 m below ground surface.

The report provides engineering guidelines based on Golder Associates interpretation of the borehole information and project requirements. Refer to the above-noted report for complete details.

### 3.0 SANITARY SERVICING

As per the South Nepean Collector Functional Design Update (FDU) prepared by Dillon Consulting (July 2012), the South Nepean Collector (SNC) is the sanitary outlet for the proposed development and has been sized to accommodate the peak sanitary flows from the proposed Burnett Lands development. Refer to Figure 1, Existing Sanitary Network and Collection Areas, and Table 5.1, Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area, of the FDU, located in **Appendix A**. The noted figure and table confirms the development is located within the Sanitary Drainage Area 8A of the SNC.

The design criteria used to determine the sanitary flows are based on the City of Ottawa's sewer design guidelines and are as follows:

- Residential Average Flow = 350L/capita/day
- Peaking Factor = Harmon Equation (max peaking factor = 4.0)
- Peak Extraneous Flows (Infiltration) = 0.28L/s/ha
- Population Density = 2.7 people/townhouse, 1.8 people/condo
- Minimum Pipe Slope (200mm/250mm) = 0.32% / 0.24%
- Minimum Full Flow Velocity = 0.6m/s
- Maximum Full Flow Velocity = 3.0m/s

In addition to the peak sanitary flows from the proposed Burnett Lands development, the on-site sanitary sewers are sized to accommodate a portion of the peak sanitary flows from the adjacent residential lands located immediately north of the site. The peak sanitary flow from the adjacent lands is based on 100 townhouses per hectare as per the *South Nepean Town Centre Community Design Plan* (July 2006) prepared by the City of Ottawa. For reference a copy of the South Nepean Town Centre Community Design Plan is included in **Appendix B**.

The proposed sanitary sewer system is shown on the General Plan of Services (Drawing **111117-GP**). The Sanitary Drainage Area Plan (Drawing **111117-SAN**) confirms the sanitary drainage areas assumed to outlet in the proposed onsite sanitary sewers. Both drawings are included in **Appendix F**.

The calculated peak sanitary design flow for the development is 39.1L/s: 16.5L/s will outlet into the SNC at the Street D/Jockvale Road intersection and 22.6L/s will outlet into the SNC at the Street F/Jockvale Road intersection. For detailed calculations refer to the Sanitary Sewer Design Sheet located in **Appendix A**.

#### 4.0 WATERMAIN

Ultimately, the Burnett Lands will be serviced with a 250mm and 300 mm looped watermain with connections at both the northeast and northwest limits of the site. At the northeast limits, the watermain will connect to a 400 mm watermain to be located in the realigned Greenbank Road, and at the northwest limits of the site, to a future watermain located within the lands to the north of the site. Refer to the following figures:

- **Figure 3** – Realigned Greenbank Road Watermain
- **Figure 4** – Watermain Layout

As the ultimate watermain connections have not been constructed to date, it is proposed to construct approximately 335 m of the 400 mm realigned Greenbank Road watermain from the Greenbank and Jockvale intersection to south of the northeast limits of the site. It is also proposed to install approximately 255 m of watermain between Jockvale Road and the realigned Greenbank Road to provide a looped watermain for the proposed development prior to the connection to the northwest.

It is noted the proposed watermain works are located in a future Zone 3C pressure zone. The realignment of the pressure zone will be completed by the City of Ottawa and once complete will alter the boundary conditions for the development. The realignment is tentatively scheduled for 2018. The City of Ottawa has provided boundary conditions for the pre and post-realignment conditions. This report considers both conditions.

Fire flow demands have been calculated as per the Fire Underwriter's Survey (FUS) and are included in **Appendix C**. However as per the City of Ottawa's technical bulletin ISDTB-2014-02 (Revisions to Ottawa Design Guidelines – Water), the fire flow is capped at 10,000 L/min (167 L/s). Watermain analysis was completed based on the following criteria:

##### Demands:

- Townhouse Density                      2.7 persons/unit
- Average Daily Demand                      350 L/capita/day
- Max. Daily Demand                      2.5 x Average Daily Demand
- Peak Hour Demand                      2.2 x Maximum Daily Demand
- Fire Flow Demand                      Fire Underwriters Survey

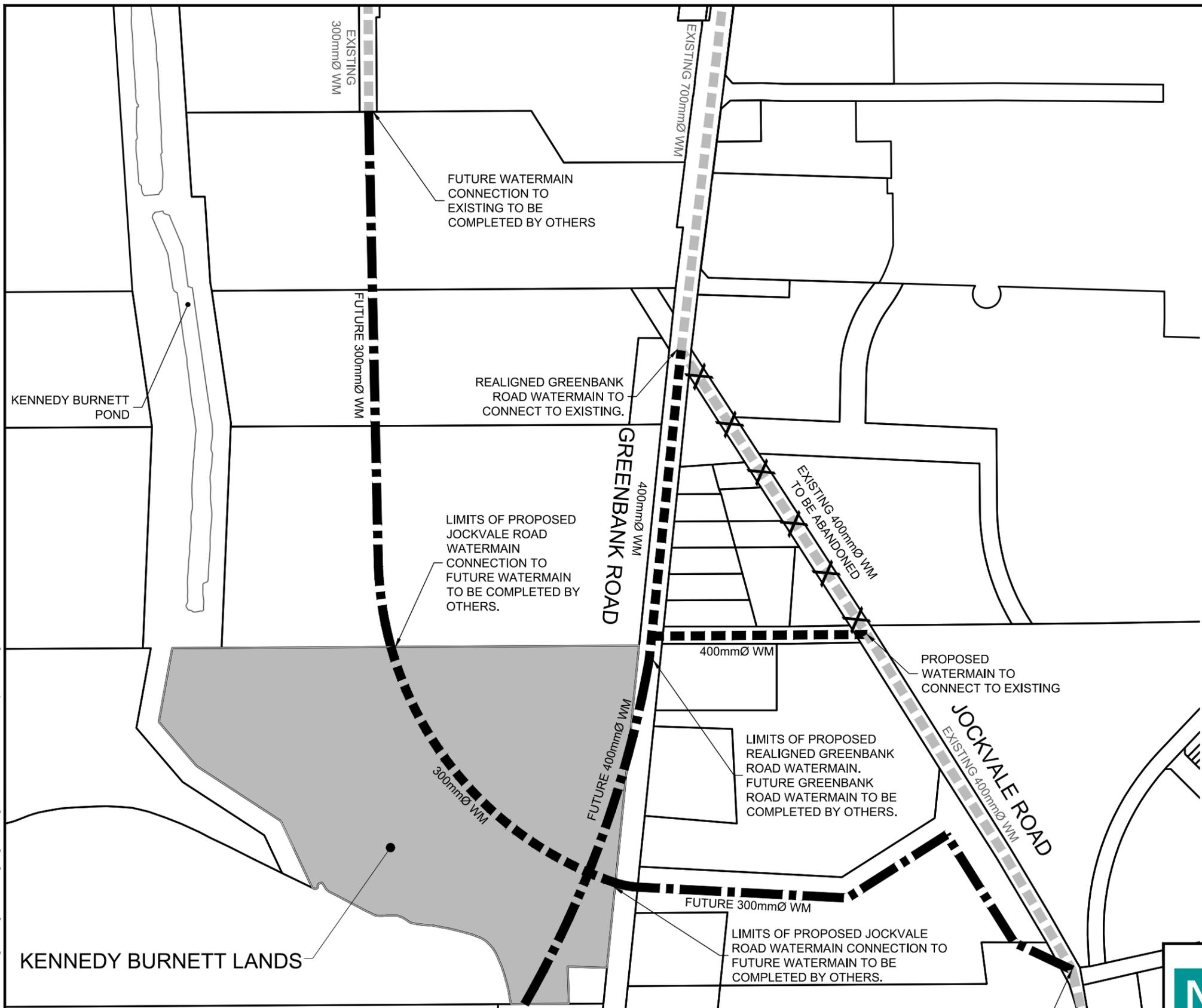
##### System Requirements:

- Max. Pressure (System)                      690 kPa (100 psi)
- Max. Pressure (Service)                      552 kPa (80 psi)
- Min. Pressure                      276 kPa (40 psi) excluding fire flows
- Min. Pressure (Fire)                      138 kPa (20 psi) including fire flows
- Max. Age (Quality)                      192 hours (onsite)

##### Friction Factors:

- Watermain Size                      C-Factor
- 200-250 mm                      110
- 300-400 mm                      120

M:\2011\1117\CAD\Design\Figures\111117-Fig3-RealignedWM.dwg, Nov 17, 2016 - 4:27pm, smdaughlin



**LEGEND:**

-  FUTURE WATERMAIN BY OTHERS
-  PROPOSED WATERMAIN
-  EXISTING WATERMAIN TO BE ABANDONED
-  EXISTING WATERMAIN

KENNEDY BURNETT LANDS



Engineers, Planners & Landscape Architects  
Suite 200, 240 Michael Cowpland Drive  
Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643  
Facsimile (613) 254-5867  
Website www.novatech-eng.com

3370 GREENBANK RD.  
BURNETT LANDS

REALIGNED GREENBANK  
ROAD WATERMAIN

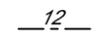
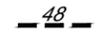
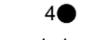
SCALE NOT TO SCALE

DATE NOV 2016 JOB 111117 FIGURE FIGURE 3

M:\2011\1117\CAD\Design\Figures\111117-Fig4-WM.dwg, WM Layout, Nov 22, 2016 - 3:56pm, kbanks



**LEGEND**

-  PROPOSED 250mmØ WATERMAIN PIPE
-  PROPOSED 300mmØ WATERMAIN PIPE
-  PROPOSED 400mmØ WATERMAIN PIPE
-  WATERMAIN NODE
-  RESERVOIR
-  HYDRANT

**NOVATECH**  
 Engineers, Planners & Landscape Architects  
 Suite 200, 240 Michael Cowpland Drive  
 Ottawa, Ontario, Canada K2M 1P6  
 Telephone (613) 254-9643  
 Facsimile (613) 254-5867  
 Website www.novatech-eng.com

**3370 GREENBANK RD.  
BURNETT LANDS**

**WATERMAIN LAYOUT**



DATE NOV 2016 JOB 111117 FIGURE FIGURE 4

The high pressure condition (average daily demand) was analyzed to ensure the system meets the design criteria for maximum pressure and quality. The maximum daily demand plus fire flow and peak hour conditions were analyzed to ensure the system meets the design criteria for maximum flow and minimum pressure.

The hydraulic modelling results for the development prior to the City reconfiguring the watermain are listed in **Table 4.1**.

**Table 4.1: Water Demand Summary (Pre Watermain Reconfiguration)**

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	5.8	N/A	690/100 (Max)	669.5/97.1	116
Maximum Daily Demand	14.5	167	138/20 (Min)	142.0/20.6	N/A
Peak Hour	31.8	N/A	276/40 (Min)	424.0/61.5	N/A

The analysis confirms the proposed watermain can service the Burnett Lands prior to the site being included into the realigned Zone 3C pressure zone. It is noted that pressure in the main is greater than 552 kPa/80psi therefore the use of pressure reducing values will be considered during detailed design.

The hydraulic modelling results for the development after the watermain realignment of the watermain are listed in **Table 4.2**.

**Table 4.2: Water Demand Summary (Post Watermain Reconfiguration)**

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	5.8	N/A	690/100 (Max)	533.0/77.3	116
Maximum Daily Demand	14.5	167	138/20 (Min)	142.0/20.6	N/A
Peak Hour	31.8	N/A	276/40 (Min)	424.0/61.5	N/A

The analysis confirms the proposed watermain can service the Burnett Lands after the watermain realignment. It is noted that pressure in the main is greater than 552 kPa/80psi therefore the use of pressure reducing values will be considered during detailed design.

A copy of the boundary conditions provided by the City of Ottawa, fire flow calculations, detailed hydraulic analysis results, and watermain layout figure are included in **Appendix C**.

## 5.0 STORMWATER MANAGEMENT CRITERIA

The Burnett Lands are tributary to the Jock River, which falls under the jurisdiction of the Rideau Valley Conservation Authority (RVCA). The following stormwater management criteria have been developed based on the requirements of the RVCA and the City of Ottawa Sewer Design Guidelines (October 2012) and Technical Bulletin PIEDTB-2016-01 (September 2016).

### Minor System (Storm Sewers)

- Storm sewers are to be designed using the Rational Method for a 1:5 year return period;
- Inlet control devices (ICDs) will be installed in road and rearyard catchbasins to control inflows to the storm sewers;
- The 100-year hydraulic grade line in the storm sewer shall be at least 0.3 m below the underside of footing (USF) elevations for the proposed development.

### Major System (Overland Flow)

- Overland flows are to be confined within the right-of-ways and/ or defined drainage easements for all storms up to and including the 1:100-year event;
- Maximum depth of flow (static + dynamic) on local and collector streets shall not exceed 0.35 m and shall be confined to the road right-of-way, as well as not touch any part of the building envelope and must remain below the lowest building opening during the stress test event;
- Storm runoff that exceeds the capacity of the minor system is to be stored within road sags;
  - Runoff that exceeds the capacity of the road sags is to be conveyed overland along defined major system flow routes towards the proposed major system outlet to the Jock River.
- ICD flow rates are to be calculated for each drainage area to ensure that the following stormwater management (SWM) objectives are satisfied:
  - Surface water accumulation at street low points, during a 5-year event, shall not be present by the end of the rainfall event;
  - Major system storage in backyards is not to be included/ accounted for in design computations;
  - Maximum flow depths and elevations on streets shall not exceed 0.30 m and shall be confined to the road right-of-way as well as not be within 0.30 m (vertical) to the nearest building opening;
  - The product of the 100-year flow depth (m) on street and flow velocity (m/s) shall not exceed 0.60;
  - The 100-year hydraulic gradeline within the storm sewers shall not be within 0.30 m to adjacent building underside of footing elevations.

### Water Quality & Quantity Control

- An *Enhanced* (80% TSS removal) level of quality control is required for storm outfalls to the Jock River;
- Lot level and conveyance Best Management Practices should be to promote infiltration and treatment of storm runoff.
- Quantity control of post-development runoff to pre-development levels is not required for lands outletting directly to the Jock River, provided that there are no adverse impacts on downstream watercourses, structures, or property resulting from the proposed development.

### Erosion and Sediment Control

- Erosion and sediment control measures are to be implemented during construction in accordance with the “Guidelines on Erosion and Sediment Control for Urban Construction Sites” (Government of Ontario, May 1987).
- All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.

## **6.0 BURNETT MUNICIPAL DRAIN**

Bisecting the site north-to-south is the existing Burnett Municipal Drain – Refer to **Figure 5**. The drain is a tributary to the Jock River, with a total length of approximately 1.3 km. A significant portion of the original Burnett Drain has been replaced by storm sewers. The drain has been fully enclosed and/or abandoned north of the Barrhaven Town Centre commercial plaza. The Burnett Municipal Drain is primarily an open channel between the Barrhaven Town Centre and the confluence with the Jock River, but a portion of the drain is piped across an existing driving range to the north of the Caivan Lands for a distance of approximately 170 m.

Novatech has prepared a memo (South Nepean Collector Culvert Crossings, June 10, 2016 – included in **Appendix E**) that outlines the sizing of culvert crossings to be installed as a part of the Phase 2 South Nepean Collector project. This memo provides an assessment of the design flows and capacity of the existing Burnett Municipal Drain. During the 100-year storm, the peak flow through the drain is approximately 2.7 m<sup>3</sup>/s. The existing drain has a cross-section consisting of a 3.0 m bottom width, 0.6 m depth, and side slopes ranging from 2:1 to 4:1. This cross-section gives a bankfull capacity of approximately 3.9 m<sup>3</sup>/s, based on Manning’s equation.

It is recommended that the drain be re-directed to the west, around the boundary of the site, connecting with the existing outlet for the KB Pond – as shown on **Figure 5**. To maintain the capacity of the drain, the realigned channel should maintain a cross-section consistent with the existing drain. Ultimately, it is anticipated that the drain will be abandoned as part of planned future development upstream.

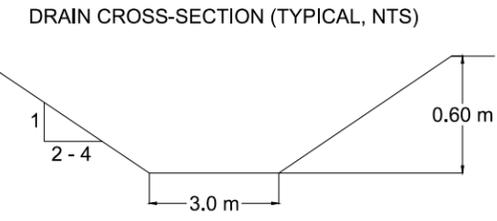
Once construction of the subdivision and surrounding developments has been completed, it is anticipated that the municipal drain will be formally abandoned, as the flows originally directed to the drain will be captured by the proposed storm sewer systems.

M:\2011\1117\CAD\Design\Figures\1117-Figs.dwg, Fig5-Ex.Cond, Dec 07, 2016 - 9:24am, kbanks



**LEGEND**

- PROPERTY BOUNDARY
- WATERCOURSE
- EXISTING BURNETT MUNICIPAL DRAIN
- - - RE-ALIGNED BURNETT MUNICIPAL DRAIN
- KB POND OUTLET (BURIED PIPE)
- ▶ DIRECTION OF FLOW



**NOVATECH**  
 Engineers, Planners & Landscape Architects  
 Suite 200, 240 Michael Cowpland Drive  
 Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643  
 Facsimile (613) 254-5867  
 Website www.novatech-eng.com

**3370 GREENBANK RD.  
 KENNEDY BURNETT LANDS**

**BURNETT  
 MUNICIPAL DRAIN**

SCALE 1 : 2500

DATE DEC 2016 JOB 111117 FIGURE FIGURE 5

As a part of the Municipal Drain Headwaters report, the Burnett Drain was classified using the four step process of the Headwater Guidelines:

- Hydrology Classification
  - Municipal drain provides valued hydrologic function (due to flow in spring);
  - Tributary drains are constructed agricultural drains with limited hydrologic function.
- Riparian Classification
  - Municipal Drain and Tributary 1 have limited riparian functions;
  - Tributaries 2 & 3 have limited to contributing riparian functions.
- Fish and Fish Habitat Classification
  - Municipal Dain is considered to have a contributing fish habitat;
  - Tributaries have no value as fish habitat.
- Terrestrial Classification
  - Municipal Drain and Tributaries have limited terrestrial functions.

As a result of these classifications, it is recommended that there is no management required for the tributaries of the Burnett Municipal Drain and that they can be abandoned. It is recommended that the Burnett Municipal Drain be managed through mitigation. Additional information on the Burnett Drain is available in Headwater Report referenced in **Section 1.2** of this report.

## 7.0 STORMWATER MANAGEMENT DESIGN

Storm servicing for the subject development will be provided using a dual drainage system: Runoff from frequent events will be conveyed by storm sewers (minor system), while flows from large storm events which exceed the capacity of the minor system will be conveyed overland along defined overland flow routes (major system).

The stormwater management design for the Burnett Lands conforms to the recommended stormwater management strategy for the planned Environmental Assessment update for the Kennedy-Burnett SWM Facility – refer to **Figure 113221 FIG-6** in **Appendix D**.

### 7.1 Storm Sewer Design (Minor System)

The proposed storm sewers have been designed using the Rational Method to convey peak flows associated with a 5-year return period. The storm sewer design sheet is provided in **Appendix A**. The corresponding Storm Drainage Area Plan (Drawing **111117-STM**) is provided in **Appendix F**. The design criteria used to size the storm sewers are summarized in **Table 7.1**.

**Table 7.1: Storm Sewer Design Parameters**

Parameter	Design Criteria
Local and Collector Roads	5 Year Return Period
Storm Sewer Design	Rational Method/Modeling
IDF Rainfall Data	Ottawa Sewer Design Guidelines (Oct. 2012)
Initial Time of Concentration ( $T_c$ )	15 minutes (rearyards) / 10 minutes (roads)
Minimum Velocity	0.8 m/s
Maximum Velocity	3.0 m/s
Minimum Diameter	250 mm

## 7.2 Inlet Control Devices

Inlet control devices (ICDs) will be installed in all catchbasins to limit inflows to the minor system during large (>1:5 year) storm events.

All catchbasins will have a single connection to the storm sewers. ICDs will be round orifice plugs with diameters of either 83mm, 94mm, 102mm, 127mm, 152mm, or 178mm.

## 7.3 Overland Flow Path (Major System)

The site has been graded to provide an engineered overland flow route (major system) for large, infrequent storms or in the event that the storm sewer system becomes obstructed. Flows will be directed to the Jock River at the low point in the system. The design of the major system conforms to the design standards outlined in Section 5.5 of the Sewer Design Guidelines.

## 7.4 Street 'B' / Caivan Lands

Street 'B' is a shared road between the Burnett Lands and the Caivan Lands to the north. Approximately 1.69 ha of the Burnett Lands development and approximately 2.61 ha of the Caivan Lands will drain to Street 'B'. Since Street 'B' will be constructed as a part of the Burnett Lands Development, the street and tributary areas have been included in the design.

Storm runoff from the Caivan Lands and Street 'B' will be directed to the outlet channel for Kennedy-Burnett SWM facility by a shared storm sewer. Water quality treatment upstream of this outfall will be provided using a hydrodynamic separator at the western end of Street 'B' (refer to the General Plan of services, **111117-GP** in **Appendix F**).

## 7.5 Water Quality

An *Enhanced* (80% TSS removal) level of water quality control will be provided by using hydrodynamic separators (HDS) upstream of the storm outfalls.

- Storm runoff from the majority of the site will be treated by an HDS unit upstream of the outfall to the Jock River near the southeast corner of the site.
- Storm runoff from Street B will be treated by an HDS unit located upstream of the outfall to the Kennedy-Burnett SWM Facility outlet channel near the northwest corner of the site. This storm outlet and HDS unit will also serve as the storm outlet for the adjacent Caivan development to the north.

In addition to the HDS units, lot level and conveyance Best Management Practices should be considered at the detailed design stage to promote infiltration and treatment of storm runoff;

- Perforated pipes for rear-yard catchbasin leads;
- Direct roof leaders to rear-yard areas;
- Infiltration trenches underlying swales in rear-yard areas;
- The use of fine sandy loam topsoil on residential lawns;

## 8.0 HYDROLOGIC & HYDRAULIC MODELING

The *City of Ottawa Sewer Design Guidelines* (October 2012) requires hydrologic modeling for all dual drainage systems. The performance of the proposed storm drainage system for the Burnett Lands was evaluated using the PCSWMM hydrologic/hydraulic model.

### 8.1 Design Storms

The hydrologic analysis was completed using the following synthetic design. The IDF parameters used to generate the design storms were taken from the *City of Ottawa Sewer Design Guidelines* (October 2012).

#### 4 Hour Chicago Storms:

- 25mm 4hr Chicago storm
- 2-year 4hr Chicago storm
- 5-year 4hr Chicago storm
- 100-year 4hr Chicago storm
- 100-year 4hr +20% Chicago storm

The 4-hour Chicago distribution generates the highest peak flows for both the minor and major systems and was determined to be the critical storm distribution for the design of the storm drainage system.

The proposed drainage system has also been stress tested using a 4-hour Chicago design storm that has a 20% higher intensity and total volume compared to the 100-year event.

Modeling files are provided on the enclosed CD.

### 8.2 Model Development

The PCSWMM model has been developed to account for both minor and major system flows from the development, and ensure no adverse impacts on the downstream drainage system. The results of the analysis were used to:

- Determine the total major and minor system runoff from the site.
- Calculate the storm sewer hydraulic grade line for the 100-year storm event;
- Evaluate overland flow depths and ponding volumes during the 100-year event; and
- Ensure no ponding in the right-of-ways remains at the end of all storm events;

#### 8.2.1 Storm Drainage Areas

For modeling purposes, the site has been divided into subcatchments based on the drainage areas tributary to each inlet of the proposed storm sewer system. The catchment areas are shown on the Storm Drainage Area Plan (111117-STM) in **Appendix F**.

#### 8.2.2 Subcatchment Model Parameters

The hydrologic parameters for each subcatchment were developed based on the Concept Plan (**Figure 2**) and the Storm Drainage Area Plan (111117-STM). An overview of the modeling parameters is provided in

Table 8.1.

Table 8.1: Hydrologic Modeling Parameters

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Imp. (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
<b>Burnett Lands - Claridge</b>						
A-01	0.235	0.65	64%	50%	43	0.75%
A-02	0.249	0.65	64%	50%	36	0.75%
A-03	0.127	0.65	64%	75%	36	0.75%
A-04	0.302	0.65	64%	50%	34	0.75%
A-05	0.275	0.65	64%	75%	37	0.75%
A-06	0.274	0.65	64%	60%	37	0.75%
A-07	0.289	0.65	64%	50%	39	0.75%
A-08	0.246	0.65	64%	75%	41	0.75%
A-09	0.081	0.65	64%	50%	23	0.75%
A-10	0.187	0.65	64%	50%	37	0.75%
A-11	0.225	0.65	64%	50%	25	0.75%
A-12	0.036	0.65	64%	100%	24	0.75%
A-13	0.200	0.65	64%	50%	24	0.75%
A-14	0.195	0.65	64%	50%	23	0.75%
A-15	0.105	0.65	64%	100%	30	0.75%
A-16	0.128	0.65	64%	100%	32	0.75%
A-17	0.246	0.65	64%	50%	49	0.75%
A-18	0.272	0.65	64%	50%	32	0.75%
A-19	0.122	0.65	64%	100%	31	0.75%
A-20	0.130	0.65	64%	100%	29	0.75%
A-21	0.217	0.65	64%	50%	27	0.75%
A-22	0.060	0.65	64%	100%	24	0.75%
A-23	0.457	0.65	64%	75%	46	0.75%
A-24	0.209	0.65	64%	75%	28	0.75%
A-25	0.214	0.65	64%	75%	29	0.75%
A-26	0.448	0.65	64%	70%	45	0.75%
A-27	0.190	0.65	64%	75%	32	0.75%
A-28	0.119	0.65	64%	0%	18	0.75%
A-29	0.231	0.65	64%	60%	46	0.75%
A-30	0.498	0.65	64%	50%	71	0.75%
A-31	0.061	0.65	64%	0%	20	0.75%
A-32	0.450	0.65	64%	75%	56	0.75%
A-33	0.816	0.65	64%	50%	117	0.75%

Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Imp. (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
A-34	0.773	0.65	64%	0%	55	0.75%
A-35	0.983	0.65	64%	0%	70	0.75%
A-36	0.450	0.65	64%	0%	60	0.75%
A-37	0.372	0.65	64%	0%	53	0.75%
<b>Street B - Caivan Lands</b>						
B-01	0.092	0.65	64%	50%	28	0.75%
B-02	0.149	0.65	64%	0%	23	0.75%
B-03	0.097	0.65	64%	10%	32	0.75%
B-04	0.825	0.65	64%	50%	110	0.75%
B-05	0.064	0.65	64%	100%	27	0.75%
B-06	0.076	0.65	64%	0%	19	0.75%
B-07	0.127	0.65	64%	40%	32	0.75%
B-08	0.225	0.65	64%	50%	32	0.75%
B-09	0.245	0.65	64%	50%	34	0.75%
B-10	1.115	0.30	14%	0%	149	0.75%
B-11	0.049	0.65	64%	0%	13	0.75%
B-12	0.088	0.65	64%	50%	20	0.75%
B-13	0.097	0.65	64%	50%	22	0.75%
B-14	0.320	0.65	64%	50%	73	0.75%
B-15	0.120	0.65	64%	50%	27	0.75%
B-16	0.136	0.65	64%	50%	31	0.75%
B-17	0.247	0.65	64%	50%	33	0.75%
B-18	0.121	0.65	64%	50%	28	0.75%
B-19	0.107	0.65	64%	50%	22	0.75%
CaivanLands	8.101	0.65	64%	50%	200	0.50%

### Infiltration

Infiltration losses for all catchment areas were modeled using Horton's infiltration equation, which defines the infiltration capacity of the soil over the duration of a precipitation event using a decay function that ranges from an initial maximum infiltration rate to a minimum rate as the storm progresses. The default values for the City of Ottawa were used for all catchments.

Horton's Equation:  
 $f(t) = f_c + (f_o - f_c)e^{-k(t)}$

Initial infiltration rate:  $f_o = 76.2$  mm/hr  
 Final infiltration rate:  $f_c = 13.2$  mm/hr  
 Decay Coefficient:  $k = 4.14$ /hr

### Depression Storage

The default values for depression storage in the City of Ottawa were used for all catchments.

- Depression Storage (pervious areas): 4.67 mm
- Depression Storage (impervious areas): 1.57 mm

Residential rooftops are assumed to provide no depression storage and all rainfall is converted to runoff.

### Equivalent Width

'Equivalent Width' refers to the width of the sub-catchment flow path. This parameter (Table 5.1) is calculated as described in the *Sewer Design Guidelines, October 2012, Section 5.4.5.6*.

### Impervious Values

Impervious (%IMP) values for each subcatchment area were calculated based on the concept plan (**Figure 2**). The impervious values correspond to the Runoff Coefficients (C) used in the Rational Method calculations using the equation:

$$\%IMP = \frac{C - 0.2}{0.7}$$

For the Storm Sewer Design spreadsheet, typical lots were analyzed with respect to the concept plan and runoff coefficients were determined based on the proposed land use. For development consisting of primarily medium-density residential (multi-unit attached dwellings) a runoff coefficient (C) of 0.65 was selected.

### 8.2.3 Minor System

Inflows to the storm sewer were modeled based on the ICD specified for the inlet and the maximum depth of ponding. Storage volumes within the right-of-way are based on the grading design. ICD parameters are outlined as follows in **Table 8.2**.

**Table 8.2: ICD Parameters**

Structure	ICD Size & Inlet Rate			5-year Peak Flow* (L/s)
	Diameter (mm)	Max Head (m)	5-year Capture Rate* (L/s)	
<b>Burnett Lands - Claridge</b>				
CB01-02	127	1.14	37.74	50.15
CB03-04	127	1.23	38.84	48.26
CB05-06	152	1.12	52.87	58.43
CB07-08	127	1.14	37.29	50.69
CB09-10	127	1.04	4.66	4.80
CB11-12	108	1.19	27.44	54.07
CB13-14	102	1.15	23.96	27.65
CB15-16	108	1.15	28.06	56.86
CB17-18	102	1.15	24.77	49.60
CB19	83	1.16	15.85	17.43
CB20-21	156	1.12	57.10	80.41
CB22	108	1.15	27.33	37.97
CB23	108	1.15	26.97	38.91
CB24-25	127	1.14	36.88	44.09

Structure	ICD Size & Inlet Rate			5-year Peak Flow* (L/s)
	Diameter (mm)	Max Head (m)	5-year Capture Rate* (L/s)	
CB26-27	178	1.11	72.35	86.47
CB28	127	1.15	37.09	47.94
CB29	94	1.15	20.45	29.12
CB30-31	127	1.14	36.75	38.46
CB32-33	127	1.14	37.03	44.92
CB34-35	127	1.14	37.01	44.34
CB40-41	178	1.11	72.83	87.24
CB42-43	152	1.12	51.29	52.25
CB44-45	127	1.23	39.15	50.39
CB59	83	1.16	6.67	6.67
CB60	102	1.15	23.68	24.03
CB61	83	1.16	11.03	11.11
CB62	108	1.15	22.43	22.56
CB63	108	1.15	23.50	23.67
CB64	102	1.15	19.31	19.43
CB66	83	1.16	12.35	13.15
CB67-68-69	178	1.17	74.36	85.77
<b>Street B - Caivan Lands</b>				
CB36	83	1.46	15.93	18.08
CB37	102	1.45	24.08	29.14
CB39	83	1.46	16.11	19.63
CB46-47	83	1.46	14.13	15.06
CB48-49	83	1.44	16.16	24.54
CB50-51	127	1.44	37.25	42.30
CB52-53	178	1.41	73.84	92.98
CB54	108	1.45	9.23	9.67
CB55-56	178	1.41	53.29	91.54
CB57-58	178	1.31	49.98	50.85
CB65	83	1.46	11.78	11.90

#### 8.2.4 Major System

The proposed road network was input into the PCSWMM model to calculate the total inflow into the storm sewers (minor system), and to calculate the overland flows and flow depths within the right-of-way (major system).

The roads are represented in the model as open channels. Model input includes:

- Right-of-way cross-sections;

- Length and slope of the road between each high and low point;
- The location of all storm inlets and whether the inlets are in a sag or on-grade.

The elevations used to define the road network in the PCSWMM model are based on the gutter elevations, as opposed to the centerline of road elevations shown on the Grading Plans (111117-GR1-3 as provided in **Appendix F**). Right-of-way cross sections used in the PCSWMM model are provided in **Appendix D**.

8.2.5 Modeling Files / Schematic

The PCSWMM model schematics and 100-year model output data are provided in **Appendix D**. Digital copies of the modeling files and model output for all storm events are provided on the enclosed CD.

8.3 PCSWMM Model Results

8.3.1 Minor System

The performance of the Burnett Lands storm sewer system has been evaluated using various storm distributions and historical storms. The 4-hour Chicago distribution was found to generate the highest peak flows in the storm sewers and was selected as the critical storm distribution. The results are summarized in **Table 8.3**.

The 100-year 4-hour Chicago storm was also increased by 20% (intensity + total precipitation) to evaluate the impact of an extreme event on the performance of the minor system. The results of this analysis indicate there is no significant impact on the minor system peak flows, due to the fact that inflows to the storm sewer system will be controlled using ICDs.

**Table 8.3: Minor System Peak Flows at Outlets**

Storm Distribution->	4hr Chicago				
Return Period->	25mm	2yr	5yr	100yr	100yr +20%
<b>Burnett Lands - Flows to Jock River</b>					
Minor System - HW-01	591	843	1028	1328	1346
Major System - OverlandOutlet	0	0	0	4	32
<b>Burnett &amp; Caivan Lands - Flows to Fraser-Clarke Drain</b>					
Outlet from Street B - HW-02	720	1063	1531	1596	1622
<b>Offsite Flows</b>					
Greenbank Road - GreenbankOut	237	331	465	867	1083

8.3.2 Major System

The major system network was evaluated using the PCSWMM model to ensure that the ponding depths conform to City standards. A summary of ponding depths and volumes for the 100-year event is provided in **Table 8.4**. Ponding volumes and depths for all storm events (including the 100-yr+20% event and 5-year event) are provided in **Appendix D**.

**Table 8.4: Ponding Depths at Catchbasins (100yr Event)**

Structure	T/G (m)	Max. Static Ponding (Spill Depth)		100-yr Event (4hr)			
		Elev. (m)	Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Spill Flow (L/s)
<b>Burnett Lands - Claridge</b>							
CB01-02	93.25	93.44	0.19	93.41	0.16	N	0
CB03-04	93.18	93.44	0.26	93.42	0.24	N	0
CB05-06	93.05	93.34	0.29	93.19	0.14	N	0
CB07-08	92.92	93.34	0.42	93.08	0.16	N	0
CB09-10	93.01	93.34	0.33	93.26	0.25	N	0
CB15-16	93.20	93.37	0.17	93.43	0.23	Y	56
CB17-18	93.51	93.76	0.25	93.71	0.20	N	0
CB19	93.44	93.64	0.20	93.56	0.12	N	0
CB20-21	93.34	93.60	0.26	93.56	0.22	N	0
CB22	93.49	93.64	0.15	93.64	0.15	N	0
CB23	93.28	93.51	0.23	93.37	0.09	N	0
CB24-25	93.25	93.48	0.23	93.35	0.10	N	0
CB26-27	93.08	93.36	0.28	93.24	0.16	N	0
CB28	92.99	93.34	0.35	93.08	0.09	N	0
CB29	92.98	93.34	0.36	93.08	0.10	N	0
CB30-31	93.31	93.58	0.27	93.60	0.29	Y	77
CB40-41	93.35	93.65	0.30	93.52	0.17	N	0
CB42-43	93.47	93.72	0.25	93.63	0.16	N	0
CB44-45	93.62	93.87	0.25	93.77	0.15	N	0
CB59	93.70	93.91	0.21	93.14	0.00	N	0
CB60	93.50	93.80	0.30	93.81	0.31	Y	1
CB61	93.50	93.78	0.28	93.64	0.14	N	0
CB62	93.49	93.78	0.29	93.77	0.28	N	0
CB63	93.55	93.84	0.29	93.85	0.30	Y	2
CB64	93.66	93.95	0.29	93.91	0.25	N	0
CB66	93.90	94.01	0.11	93.97	0.07	N	0
CB67-68-69	93.66	94.15	0.49	93.83	0.17	N	0
<b>Street B - Caivan Lands</b>							
CB39	93.78	93.86	0.08	93.92	0.14	Y	52
CB46-47	93.67	93.77	0.10	93.84	0.17	Y	52
CB48-49	93.72	93.86	0.14	93.87	0.15	Y	9
CB50-51	93.59	93.84	0.25	93.74	0.15	N	0

Structure	T/G (m)	Max. Static Ponding (Spill Depth)		100-yr Event (4hr)			
		Elev. (m)	Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Spill Flow (L/s)
CB52-53	93.52	93.76	0.24	93.77	0.25	Y	4
CB55-56	93.38	93.67	0.29	93.59	0.21	N	0
CB57-58	93.30	93.56	0.26	93.47	0.17	N	0

### 8.3.3 Hydraulic Grade Line

The results of the analysis were used to ensure that a minimum freeboard of 0.30m is provided between the 100-year HGL and the designed underside of footing elevations. The 100-year HGL is indicated on the Plan and Profile Drawings (111117-P1-9). The HGL analysis confirms that all dwellings within the Burnett Lands will have at least 0.30m of freeboard between the modeled hydraulic grade line and the underside of footing elevation.

Storm runoff from the Burnett Lands is to be conveyed to the Jock River, after flowing through a Vortechs unit for water quality treatment.

Peak flows from the subdivision, tributary to the Jock River will not coincide with peak flows in the river. Consequently, a combined frequency analysis was used to assess the maximum HGL in the storm sewers. The hydraulic analysis was initially modeled for two scenarios:

- 1) 5-year flows in the storm sewers, 100-year flood elevation at the outlet.
- 2) 100-year flows in the storm sewers, 5-year flood elevation at the outlet.

The 5-year peak flow combined with the 100-year flood elevation of 91.28 at the Jock River and 91.58 at the Fraser-Clarke Drain produced the highest HGL values, and this represents the worst-case scenario. This scenario was used for all subsequent analysis.

**Table 8.5** provides a summary of the 5-year peak flow combined with the 100-year flood elevation HGL elevation at each storm manhole within the proposed development. HGL elevations for the 100-year flows in the storm sewers, 5-year flood elevation at the outlet as well as a summary of the HGL elevations for a 20% increase (rainfall intensity and total precipitation) in the 100-year design event. The results of this stress testing indicates that, even under this scenario, the hydraulic grade line in the sewers will only slightly increase (max increase of 0.03m), ensuring that the HGL will remain below the undersides of footing of the proposed units.

**Table 8.5: Storm Sewer Hydraulic Grade Line:  
5yr-4hr Chicago Distribution, 100-yr WL in Jock River**

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elevation (m)	Design USF (m)	Clearance from USF (m)
<b>Burnett Lands - Claridge</b>					
102 (STM)	90.52	93.78	92.04	92.62	0.58
104 (STM)	90.37	93.93	92.03	92.60	0.57

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elevation (m)	Design USF (m)	Clearance from USF (m)
106 (STM)	90.27	93.87	92.04	92.60	0.56
108 (STM)	90.10	93.75	92.01	92.51	0.50
110 (STM)	89.94	93.46	92.00	92.43	0.43
304 (STM)	90.56	93.67	91.72	92.25	0.53
306 (STM)	90.35	93.58	91.72	92.22	0.50
308 (STM)	90.11	93.41	91.70	92.17	0.47
310 (STM)	89.74	93.46	91.66	92.06	0.40
312 (STM)	89.26	93.36	91.65	92.06	0.41
314 (STM)	89.19	92.61	91.58	-	-
316 (STM)	89.00	93.09	91.49	-	-
324 (STM)	88.96	93.34	91.41	-	-
326 (STM)	89.95	93.38	91.35	-	-
328 (STM)	88.91	92.16	91.30	-	-
330 (STM)	88.94	92.53	91.38	-	-
402 (STM)	90.62	93.82	92.02	92.40	0.38
404 (STM)	90.37	93.80	92.01	92.45	0.44
406 (STM)	90.46	93.71	92.04	92.55	0.51
408 (STM)	90.36	93.80	92.04	92.55	0.51
608 (STM)	90.25	93.71	91.99	92.55	0.56
610 (STM)	89.58	93.30	91.79	92.38	0.59
612 (STM)	89.82	93.93	91.64	-	-
614 (STM)	89.92	93.76	91.71	-	-
616 (STM)	90.23	94.27	91.72	92.65	0.93
618 (STM)	90.03	93.65	91.80	92.25	0.45
902 (STM)	90.34	93.98	91.82	92.55	0.73
904 (STM)	90.29	93.76	91.83	92.35	0.52
906 (STM)	90.12	93.53	91.82	92.35	0.53
908 (STM)	89.97	93.57	91.81	92.25	0.44
<b>Street B - Caivan Lands</b>					
200 (STM)	91.55	94.54	92.38	-	-
202 (STM)	91.40	94.14	92.37	-	-
204 (STM)	91.08	93.94	92.26	92.55	0.29
206 (STM)	90.84	93.62	92.14	92.45	0.31
208 (STM)	90.69	93.82	92.06	92.45	0.39
210 (STM)	90.53	93.52	91.89	92.45	0.56
212 (STM)	90.40	93.73	91.80	92.40	0.60
214 (STM)	90.30	93.47	91.73	92.25	0.52
216 (STM)	90.01	93.37	91.65	92.25	0.60

## 9.0 UTILITIES

The development will be serviced by hydro, phone, cable, and gas from the existing services on Greenbank Road. During detailed design the works will be coordinated with local utilities companies.

## 10.0 EROSION AND SEDIMENT CONTROL

Erosion and sediment control measures will be implemented during construction in accordance with the "Guidelines on Erosion and Sediment Control for Urban Construction Sites" (Government of Ontario, May 1987). Detailed plans will be provided at the detailed design stage.

Typical erosion and sediment control measures recommended include, but are not limited to, the use of silt fences around perimeter of site (OPSD 219.110), filter fabric or inserts under catch basin/maintenance hole lids, heavy duty silt fence barrier (OPSD 219.130), straw bale check dams (OPSD 219.180), rock check dams (219.210 or OPSD 219.211), turbidity curtain (OPSD 219.260), dewatering trap (OPSD 219.240), temporary water passage system (OPSD 221.030), riprap (OPSS 511), mud mats, silt bags for dewatering operations, topsoil and sod to disturbed areas and natural grassed waterways. Dewatering and sediment control techniques will be developed for the individual situations based on the above guidelines and utilizing typical measures to ensure erosion and sediment control is controlled in an acceptable manner and there is no negative impact to adjacent lands, water bodies or water treatment/conveyance facilities.

It will be the responsibility of the Contractor to submit a detailed construction schedule and appropriate staging, dewatering and erosion and sediment control plans to the Contract Administrator for review and approval prior to the commencement of work.

All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.

- A qualified inspector should conduct daily visits during construction to ensure that the contractor is working in accord with the design drawings and that mitigation measures are being implemented as specified.
  - A light duty silt fence barrier is to be installed in the locations shown on the Erosion and Sediment Control Plan.
  - Straw bale barriers are to be installed in drainage ditches
  - Inserts are to be placed under the grates of all proposed and existing catchbasins and structures.
  - After complete build-out, all sewers are to be inspected and cleaned and all sediment and construction fencing is to be removed.
- The contractor shall ensure that proper dust control is provided with the application of water (and if required, calcium chloride) during dry periods.
- The contractor shall immediately report to the engineer or inspector any accidental discharges of sediment material into any ditch or sewer system. Appropriate response measures shall be carried out by the contractor without delay.
- The contractor acknowledges that failure to implement erosion and sediment control measures may result in penalties imposed by any applicable regulatory agency.

## 11.0 CONCLUSIONS AND RECOMMENDATIONS

### Sanitary Servicing

The analysis of the proposed sanitary servicing conforms the following:

- The site is located within Sanitary Drainage Area 8A of Phase 2 of the SNC.
- The on-site sanitary sewers have been sized to accommodate a portion of the peak sanitary flows from the adjacent lands to the north, to be developed by others.
- The proposed sanitary sewers have adequate capacity to accommodate the peak sanitary flow.

### Watermain

The analysis of the proposed watermain network conforms the following:

- Approximately 560m of the 400mm Greenbank Road watermain is to be installed and connected to the existing 400mm Jockvale Road watermain to provide a looped watermain feed for the proposed site.
- Ultimately the site will connect to future watermain, to be completed by others, to the northwest.
- The site is located in a future Zone 3C pressure zone that once complete will increase the pressure in the system onsite.
- This report confirms the proposed watermain can service the site pre and post zone reconfiguration.
- During detailed design the use of pressure reducing valves will be explored.

### Stormwater Management

The stormwater management design for the Burnett Lands development conforms to the criteria established as part of this report.

The conclusions based on the results of the stormwater management analysis are as follows:

#### *Storm Drainage / Conveyance*

- Storm sewers (minor system) have been designed to convey the uncontrolled 5-year peak flow using the Rational Method.
- Inflows to the minor system will be controlled using inlet control devices (ICDs). Proposed ICDs will consist of round orifice plates with various standard diameters (83mm, 94mm, 102mm, 108mm, 127mm, 152mm, and 178mm).
- The site has been graded to provide surface storage at low points along roadways, and a major system outlet to the Jock River for flows which exceed the capacity of the road sags.
- Ponding depths will not exceed 0.30m for all storms up to and including the 100-year event.
- The post-development peak flows from the site will have no adverse impact on the Jock River downstream.
- A minimum clearance of 0.30m will be provided between the 100-year hydraulic grade line (HGL) and the designed underside of footing elevations.

### *Stormwater Management*

- An *Enhanced* level of water quality control (80% TSS removal) will be provided using hydrodynamic separators upstream of the storm outfalls.
- Quantity control is not required for the Burnett Lands, as they are located adjacent to the Jock River.

### *Erosion and Sediment control*

- Erosion and sediment control measures (i.e. filter fabric, silt fences, etc.) will be implemented prior to construction and are to remain in place until vegetation is established.

The preceding report is respectfully submitted for review and approval. Please contact the undersigned should you have questions or require additional information.

**NOVATECH**

Stormwater Management and Modeling

Prepared by:



Kallie Auld, P.Eng.  
Project Coordinator, Water Resources

Reviewed by:

A handwritten signature in blue ink, appearing to read "Michael Petepiece".

Michael Petepiece, P.Eng.  
Senior Project Manager, Water Resources

Sanitary and Water Modeling

Prepared by:

A handwritten signature in black ink, appearing to read "Mark Bowen".

Mark Bowen, B.Eng  
Project Manager, Land Development  
Engineering

Approved by:



Greg MacDonald, P.Eng.  
Director, Land Development and  
Public Sector Projects

**Appendix A**  
Design Sheets



# SANITARY SEWER DESIGN SHEET

CLARIDGE SUBDIVISION  
DEVELOPER: CLARIDGE HOMES



PROJECT : 111117  
DESIGNED BY: LSC  
CHECKED BY: GJM  
DATE PREPARED: Dec. 2015  
DATE REVISED: Nov. 25/16

LOCATION					CUMULATIVE				PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	PROPOSED SEWER									
STREET	FROM MH	TO MH	Condos	TOWNS	POPULATION (in 1000's)	AREA (ha.)	POPULATION (in 1000's)	AREA (ha.)					LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	Roughness Coef.	GRADE (%)	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap	
Greenbank	705	707		6	0.0162	0.447	0.054	1.279	4.00	0.88	0.36	1.23	60.5	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.06	
Greenbank	707	613		11	0.0297	0.690	0.084	1.969	4.00	1.36	0.55	1.91	93.1	200	201.2	DR 35	0.013	0.35	19.7	0.62	0.10	
Jockvale	613	611	140	0	0.2520	0.539	0.872	3.837	3.84	13.56	1.07	14.63	71.1	250	251.5	DR 35	0.013	0.25	30.2	0.61	0.48	
Jockvale/Steet F	611	SNC		0	0.0000	0.000	1.320	9.520	3.72	19.89	2.67	22.56	2.8	250	251.5	DR 35	0.013	1.00	60.4	1.22	0.37	
					14.20																	

- Notes:
- Q(d) = Design Flow (L/sec)  
Q(p) = Population Flow (L/sec)  
Q(i) = Extraneous Flow (L/sec)

Q(p) = (PxqxM/86,400), where P = Population (2.7 persons per town/semi, 1.8 persons per condo)  
q = Average per capita flow = 350 L/cap/day  
M = Harmon Formula (maximum of 4.0)
  - Future population based on zoning density

Project: 111117  
 Location: 3370 Greenbank Rd.  
 Client: Claridge Homes

DATE: November 2016



Storm Sewer Design Sheet

STREET	FROM	TO	AREA ID	FLOW							PROPOSED SEWER							Q/Qfull	
				R= 0.30	R= 0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)		EXCESS CAPACITY (l/s)
Half Moon Bay	MH 304	MH 306	A1		0.235	0.42	0.42	10.00	104.19	44.25	600	609.6	0.30	65.5	351.20	1.20	0.91	306.96	0.13
Half Moon Bay	MH 306	MH 308	A2		0.249	0.45	0.87	10.91	99.63	87.13	600	609.6	0.25	89.4	320.60	1.10	1.36	233.47	0.27
Street E	CB 14	MH 308	A3		0.127	0.23	0.23	10.00	104.19	23.91	250	254.0	1.00	39.4	62.10	1.22	0.54	38.19	0.39
Half Moon Bay	MH 308	MH 310	A4		0.302	0.55	1.65	12.27	93.58	154.38	600	609.6	0.25	73.3	320.60	1.10	1.11	166.22	0.48
Street E	CB 12	MH 310	A5		0.275	0.50	0.50	10.00	104.19	51.78	250	254.0	1.00	39.4	62.10	1.22	0.54	10.33	0.83
Half Moon Bay	MH 310	MH 312	A6		0.274	0.50	2.64	13.38	89.19	235.62	750	762.0	0.15	41.4	450.27	0.99	0.70	214.65	0.52
Street D	MH 402	MH 404	A7-A9		0.616	1.11	1.11	10.00	104.19	115.98	600	609.6	0.25	78.0	320.60	1.10	1.18	204.62	0.36
Jockvale	MH 404	MH 608	A10, A11		0.412	0.74	1.86	11.18	98.33	182.65	600	609.6	0.25	34.2	320.60	1.10	0.52	137.95	0.57
Jockvale	MH 608	MH 110	A12, A13		0.236	0.43	2.28	11.70	95.98	219.22	600	609.6	0.20	79.6	286.76	0.98	1.35	67.54	0.76
Street D	MH 406	MH 408	A14		0.195	0.35	0.35	10.00	104.19	36.71	600	609.6	0.20	105.5	286.76	0.98	1.79	250.04	0.13
Street D	MH 408	MH 106				0.00	0.35	11.79	95.60	33.68	600	609.6	0.20	20.5	286.76	0.98	0.35	253.07	0.12
Street A	MH 102	MH 104				0.00	0.00	10.00	104.19	0.00	600	609.6	0.50	24.3	453.40	1.55	0.26	453.40	0.00
Street A	MH 104	MH 106	A15		0.105	0.19	0.19	10.26	102.83	19.51	600	609.6	0.30	28.2	351.20	1.20	0.39	331.69	0.06
Street A	MH 106	MH 108	A16-A18		0.646	1.17	1.71	12.14	94.10	160.86	600	609.6	0.20	47.2	286.76	0.98	0.80	125.89	0.56
Street A	MH 108	MH 110	A19-A21		0.469	0.85	2.56	12.94	90.86	232.32	675	685.8	0.15	55.4	339.98	0.92	1.00	107.66	0.68
Jockvale	MH 110	MH 610	A22		0.060	0.11	4.95	13.06	90.41	447.50	750	762.0	0.21	76.5	532.76	1.17	1.09	85.27	0.84
Street F	MH 902	MH 904				0.00	0.00	10.00	104.19	0.00	600	609.6	0.31	37.7	357.01	1.22	0.51	357.01	0.00
Street F	MH 904	MH 906	A23		0.457	0.83	0.83	10.51	101.55	83.86	600	609.6	0.20	40.1	286.76	0.98	0.68	202.89	0.29
Street F	MH 906	MH 908	A24		0.209	0.38	1.20	11.20	98.28	118.27	600	609.6	0.20	70.5	286.76	0.98	1.20	168.48	0.41
Street F	MH 908	MH 610	A25, A26		0.662	1.20	2.40	12.39	93.05	223.30	675	685.8	0.15	67.3	339.98	0.92	1.22	116.68	0.66
Jockvale	MH 618	MH 610	A27		0.190	0.34	0.34	10.00	104.19	35.77	600	609.6	0.30	41.1	351.20	1.20	0.57	315.43	0.10
Half Moon Bay	MH 610	MH 312	A28, A29		0.350	0.63	8.32	14.15	86.41	719.38	975	990.6	0.14	79.4	875.66	1.14	1.17	156.28	0.82
Pathway Blk	MH 312	MH 314	A30		0.498	0.90	11.87	15.31	82.56	979.66	1200	1219.2	0.10	58.1	1287.49	1.10	0.88	307.84	0.76
Pathway Blk	MH 314	MH 316				0.00	11.87	16.19	79.89	948.03	1200	1219.2	0.12	120.0	1410.38	1.21	1.66	462.35	0.67
Street H	MH 616	MH 614	A31		0.061	0.11	0.11	10.00	104.19	11.48	450	457.2	0.20	73.5	133.15	0.81	1.51	121.67	0.09
Jockvale	MH 614	MH 612	A32		0.450	0.81	0.92	11.51	96.83	89.41	600	609.6	0.21	24.1	293.84	1.01	0.40	204.43	0.30
Apartment Blk	MH 612	MH 316	A33		0.816	1.47	2.40	11.91	95.08	227.98	600	609.6	0.28	81.4	339.29	1.16	1.17	111.31	0.67
Outlet	MH 316	MH 324				0.00	14.26	17.85	75.35	1074.86	1200	1219.2	0.10	15.3	1287.49	1.10	0.23	212.63	0.83
Outlet	MH 324	MH 326				0.00	14.26	18.08	74.76	1066.46	1200	1219.2	0.28	3.6	2154.39	1.84	0.03	1087.92	0.50
Outlet	MH 326	HW1				0.00	14.26	18.11	74.68	1065.29	1200	1219.2	0.10	47.0	1287.49	1.10	0.71	222.20	0.83

STREET	FROM	TO	AREA ID	FLOW							PROPOSED SEWER							Q/Qfull	
				R= 0.30	R= 0.65	INDIV 2.78 AR	ACCUM 2.78 AR	TIME OF CONC.	RAINFALL INTENSITY I	PEAK FLOW Q (l/s)	NOMINAL SIZE (mm)	PIPE SIZE (mm)	PIPE SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min.)		EXCESS CAPACITY (l/s)
Street B	MH200	MH202	B01, B02		0.241	0.44	0.44	10.00	104.19	45.37	675	685.8	0.20	37.4	392.57	1.06	0.59	347.20	0.12
Street B	MH202	MH204	B03, B04		0.922	1.67	2.10	10.59	101.19	212.66	675	685.8	0.20	82.6	392.57	1.06	1.30	179.92	0.54
Street B	MH204	MH206	B05, B06, B07		0.267	0.48	2.58	11.88	95.19	245.98	750	762.0	0.20	120.0	519.92	1.14	1.76	273.94	0.47
Street B	MH206	MH208	B08, B09, B10	1.115	0.470	1.78	4.36	13.64	88.23	384.95	750	762.0	0.20	38.5	519.92	1.14	0.56	134.98	0.74
Caivan Lands	CAP	MH208	B12		0.088	0.16	0.16	10.00	104.19	16.57	450	457.2	0.40	12.0	188.30	1.15	0.17	171.73	0.09
Street B	MH208	MH210	B11		0.049	0.09	4.45	14.20	86.22	383.84	750	762.0	0.15	29.3	450.27	0.99	0.50	66.42	0.85
Caivan Lands	CAP	MH210	B13		0.097	0.18	0.18	10.00	104.19	18.26	300	304.8	0.40	12.0	63.87	0.87	0.23	45.60	0.29
Street B	MH210	MH212	B14		0.320	0.58	5.03	14.70	84.54	425.26	750	762.0	0.15	28.5	450.27	0.99	0.48	25.01	0.94
Caivan Lands	CAP	MH212	B15		0.120	0.22	0.22	10.00	104.19	22.59	300	304.8	0.50	12.0	71.41	0.98	0.20	48.81	0.32
Street B	MH212	MH214				0.00	5.03	15.18	82.98	417.39	825	838.2	0.15	35.5	580.56	1.05	0.56	163.17	0.72
Caivan Lands	CAP	MH214	B16		0.136	0.25	0.25	10.00	104.19	25.61	300	304.8	0.50	12.0	71.41	0.98	0.20	45.80	0.36
Street B	MH214	MH216				0.00	5.03	15.74	81.23	408.60	900	914.4	0.10	35.5	597.83	0.91	0.65	189.23	0.68
Caivan Lands	CAP	MH216	B18		0.121	0.22	0.22	10.00	104.19	22.78	300	304.8	0.50	12.0	71.41	0.98	0.20	48.62	0.32
Street B	MH216	MH218	B17		0.274	0.50	5.53	16.39	79.31	438.20	975	990.6	0.10	36.1	740.07	0.96	0.63	301.87	0.59
Caivan Lands	CAP	MH218	B19, Caivan Lands		7.607	13.75	13.75	10.00	104.19	1432.22	975	990.6	0.50	12.0	1654.85	2.15	0.09	222.63	0.87
Outlet	MH218	H2				0.00	5.53	17.02	77.55	428.48	1500	1524.0	0.14	79.8	2762.07	1.51	0.88	2333.60	0.16

Definitions  
Q = 2.78 AIR  
Q = Peak Flow, in Litres per second (L/s)  
A = Area in hectares (ha)  
I = Rainfall Intensity (mm/h)

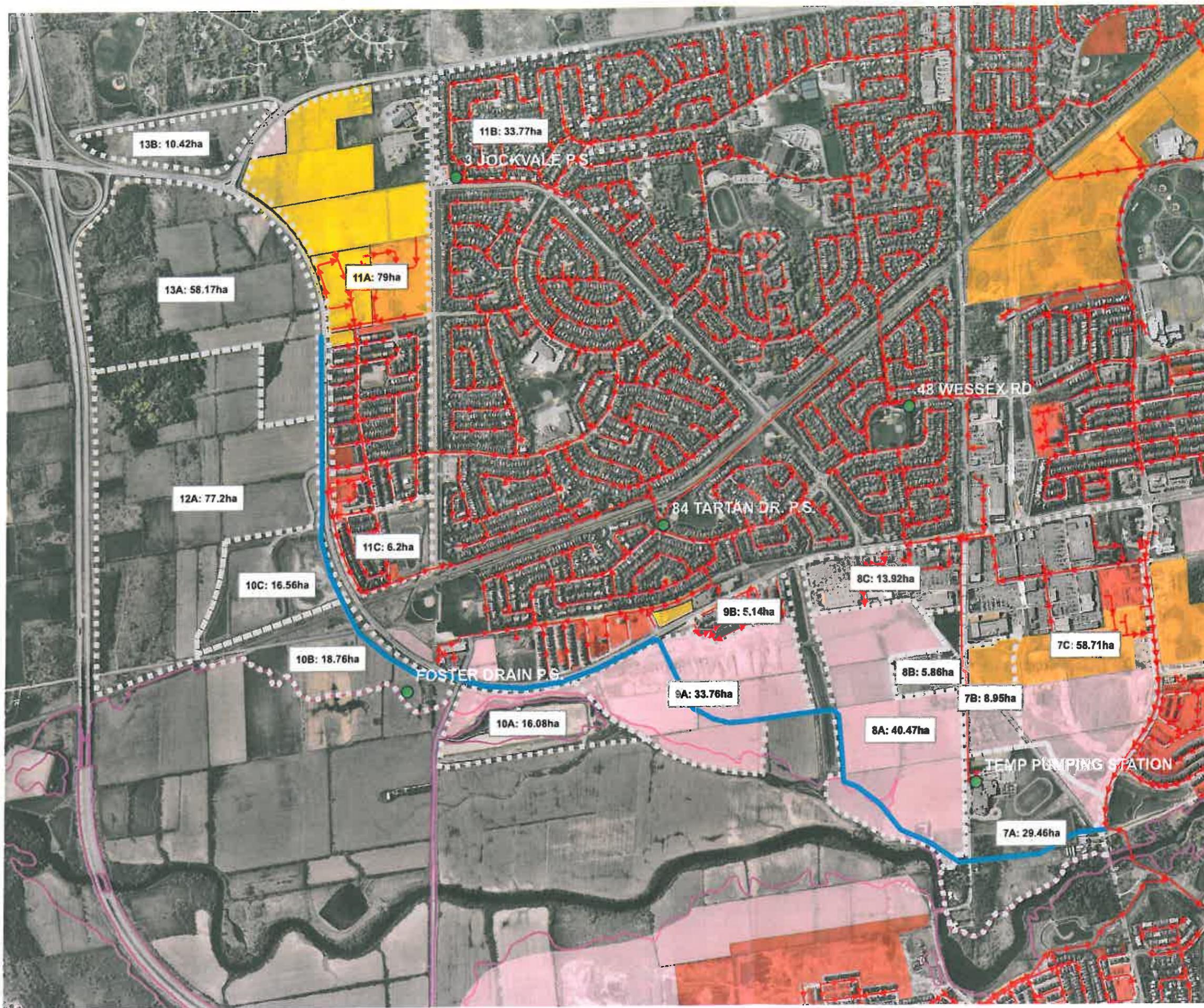
Notes:  
1) Ottawa Rainfall-Intensity Curve  
2) Min Velocity = 0.80 m/sec.

Includes the remaining 7.5ha portion of the Caivan Lands not included in Area IDs B01-B19 (4.33ha). The total area of the Caivain Lands is 11.83ha.

**Appendix B**  
Sanitary Report Excerpts

**City of Ottawa**  
South Nepean Collector

**Figure 01**  
Existing Sanitary Network and Collection Areas



- Pump Station
  - Existing Sanitary Main (With Flow Direction)
  - Proposed Alignment for South Nepean Collector
  - Collection Area
- DEVELOPMENT STATUS**
- Registered
  - Draft Approved
  - Pending
  - No Plan
  - Floodplain

NOT TO SCALE

MAP DRAWING INFORMATION:  
DATA PROVIDED BY THE CITY OF OTTAWA

MAP CREATED BY: BC  
MAP CHECKED BY: MBM  
MAP PROJECTION: NO PROJECTION

File: S:\115681\115681\_01\_South Nepean Collector.dwg Date: 2012/07/12 11:51:11  
115681\_01\_South Nepean Collector.dwg Date: 2012/07/12 11:51:11

Table 5.1: Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area

Collection Area	Discharging Node	Estimated from GIS			City of Ottawa VURL Data			Other Space <sup>1</sup> (ha)	Population (PE)	Residential Density (PE/net ha)	Comments	Additional Source(s)
		Gross Institutional/Commercial (ha)	Gross Residential (ha)	Gross Area (ha)	Net Residential (ha)	Units (#)	Unit Density (#/ha)					
7A	70	13.5	7.4	29.5	4.0	605	0.3	9.1	1637	4.25	Flow calculations include St Joseph H.S. Pump Station firm capacity of 7.0 L/s Additional 600 units (TAC)	3.4ppu (TAC)
7B		0.0	9.24	9.24	6.23	1474	136.7	3.0	3321	638.8	Population from split VURL allocated by area. VURL parcel id 323 - inconsistency between net and gross reported area.	2.7ppu (TAC)
8A		0.0	40.0	40.0	24.1	4462	185.1	15.9	12047.4	499.9		2.7ppu (TAC)
8B		5.9	0.0	5.9	0.0	0	0	0.0			Future Commercial area	
8C		13.9	0.0	13.9	0.0	0	0	0.0			Commercial area includes Home Depot	
9A	80	0.0	33.8	33.8	18.6	635	34.1	15.2	2210	116.2		3.4ppu (TAC)
10A	90	0.0	16.1	16.1	9.7	451	28.0	6.4	1533.4	158.0	Assume net population = 60% gross.	3.4ppu (TAC)
10B	100	18.8	0.0	35.3	0.0	0	0	16.5			Allocated as potential future I/C use as directed by TAC	
10C	110	16.6	0.0	35.3	0.0	0	0	18.7			Area includes current Municipal Snow Dump. Flow allowance is made for potential future I/C use	
11C		0.0	6.2	6.2				2.5	306	82.7	This area is south of '11 block' in the existing development	From IBI Apr 2010 Report Figure 1
11A	120	12.5	66.5	79.0				26.6	3923	98.3	Institutional includes 4.38ha church site and 6.89 ha institution at northeast corner, as well and Claridge Commercial (0.56ha) and DCR/Phoenix Commercial (0.64ha)	From IBI Apr2010 Report Figure 1
11B		0.0	37.0	37.0				14.8	1550	69.8	Presently serviced by Jockvale pump station; to be redirected to SNC.	Estimated from 2011 Census Block data
12A		77.2	0.0	77.2				0.0			Allow sanitary peak flow 79.0 L/s	Novatech, Employment Lands Report, Revised Jan 2012
13A	130	58.5	0.0	58.5				0.0			Allow sanitary peak flow 62.8 L/s plus Collection Area 13B, total 82.2 L/s	
13B		12.5	0.0	12.5				0.0			Allow sanitary peak flow 19.4 L/s; gravity discharge to Collection Area 13A	IBI/Novatech

Notes:

1. Other space includes other residential space accounting for the difference between gross area (measured with GIS) and net area (provided in VURL data), such as sidewalks, roads, greenspace, etc.
2. Collection Area 11A and 11B population and land use as identified under Additional Source(s). Other space reported as 60% of gross residential area, consistent with VURL average.

# *South Nepean Town Centre Community Design Plan*



**Planning and Growth Management Department  
Community Planning and Design Division  
July 2006  
Publication #03-14**

OTTAWA CITY COUNCIL  
28 AND 29 JUNE 2006  
ANDREW S. HAYDON HALL  
9:00 a.m.

MINUTES 61

PLANNING AND ENVIRONMENT COMMITTEE REPORT 50

13. SOUTH NEPEAN TOWN CENTRE COMMUNITY  
DESIGN PLAN  
AND OFFICIAL PLAN AMENDMENT

COMMITTEE RECOMMENDATIONS

**That Council:**

1. Approve the South Nepean Town Centre Community Design Plan in Document 8, which has been submitted under separate cover.
2. Adopt Official Plan Amendment No. XX to the City of Ottawa Official Plan (2003), as detailed in Document 5, to implement the Community Design Plan.

CARRIED

**BY-LAW NO. 2006 - 260**

A by-law of the City of Ottawa to amend the Official Plan for the City of Ottawa to change the designation of the lands which are the subject of the South Nepean Town Centre Community Design Plan.

**WHEREAS** Planning and Environment Committee convened a public meeting to consider the adoption of the aforementioned official plan amendment;

**AND WHEREAS** Planning and Environment Committee recommends the adoption of the aforementioned official plan amendment;

**AND WHEREAS** Council on June 28, 2006 carried the recommendation of Planning and Environment Committee;

**THEREFORE** the Council of the City of Ottawa enacts as follows:

1. Attachment A, being Official Plan Amendment No. 44 to the Official Plan for the City of Ottawa is hereby adopted.
2. This by-law shall come into force in accordance with the provisions of the Planning Act, R.S.O. 1990, c.P.13, as amended.

**ENACTED AND PASSED** this 29th day of June, 2006.

CITY CLERK

MAYOR

## 4.4 Policy Area – High Rise Residential

The High Rise Residential policy area identifies sites within the Town Centre that will accommodate the highest density residential uses, located in proximity to the transit “hub”. Apartment buildings will be the only residential type permitted.

### Policies

For the High Rise Residential policy area:

- (1) Apartments are the only permitted use. As part of an apartment building, retail, office and commercial uses at grade are also permitted.
- (2) The minimum building height is 6 storeys and the maximum building height is 12 storeys.
- (3) The net density target for residential uses is 300 units per hectare.
- (4) At least 90% of required parking for each development must be provided in parking structures, either above-grade or below-grade.

## 4.5 Policy Area – Mid Rise Residential

The Mid Rise Residential policy area is intended to accommodate the majority of the Town Centre’s ground-oriented multiple unit dwellings. This policy area will provide an appropriate transition between the low density neighbourhoods surrounding the Town Centre to the higher intensity uses within it.

### Policies

For the Mid Rise Residential policy area:

- (1) Apartments, street, block and stacked townhouses, public and institutional uses, schools, places of worship and community facilities are permitted. Ground floor retail uses within a mixed-use building with residential uses above are permitted along Greenbank Road.

- (2) The minimum building height is 2 storeys and the maximum building height is 4 storeys.

- (3) The net density target for residential uses is 100 units per hectare.

## 4.6 Policy Area – Neighbourhood Park

The Neighbourhood Park policy area will include both public parks and public plazas. The five public parks within the Town Centre will be designed to support the area’s urban nature, as accessible amenities with open frontages and clearly defined entrances that work within the grid pattern of streets and blocks. The two public plazas within the Town Centre will be designed as predominately hard surfaced areas fronted by buildings in order to create a built form edge and generate pedestrian activity.

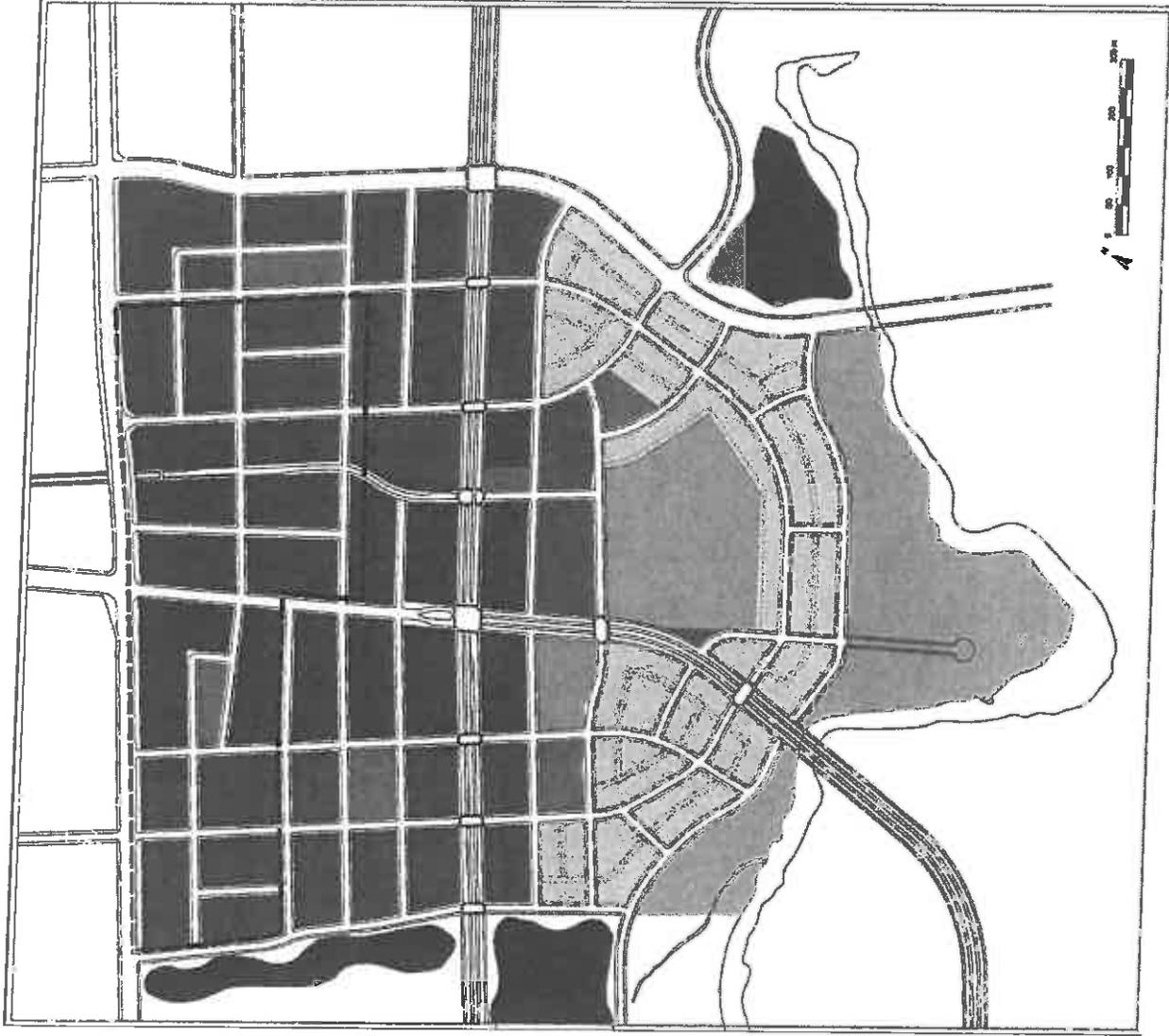
### Policies

For the Neighbourhood Park policy area:

- (1) Public parks, public plazas, community facilities, and conservation uses are permitted.
- (2) The locations of the five public parks and the two public plazas are identified on Schedule 5 of this CDP.
- (3) Acquisition of all neighbourhood parks will be as per Section 8.4 and Schedule 5 of this CDP.
- (4) Public parks must:
  - (a) Generally be between 0.4 and 1.0 hectares of level land;
  - (b) Have at least three sides that are entirely open to the street;
  - (c) Not have rear or side yards adjacent on their fourth side;
  - (d) Be designed with an emphasis on hard surfacing and seating areas as compared to traditional suburban neighbourhood parks; and
  - (e) Not contain sports fields.
- (5) Public plazas must:
  - (a) Generally be less than 0.4 hectares of level land;

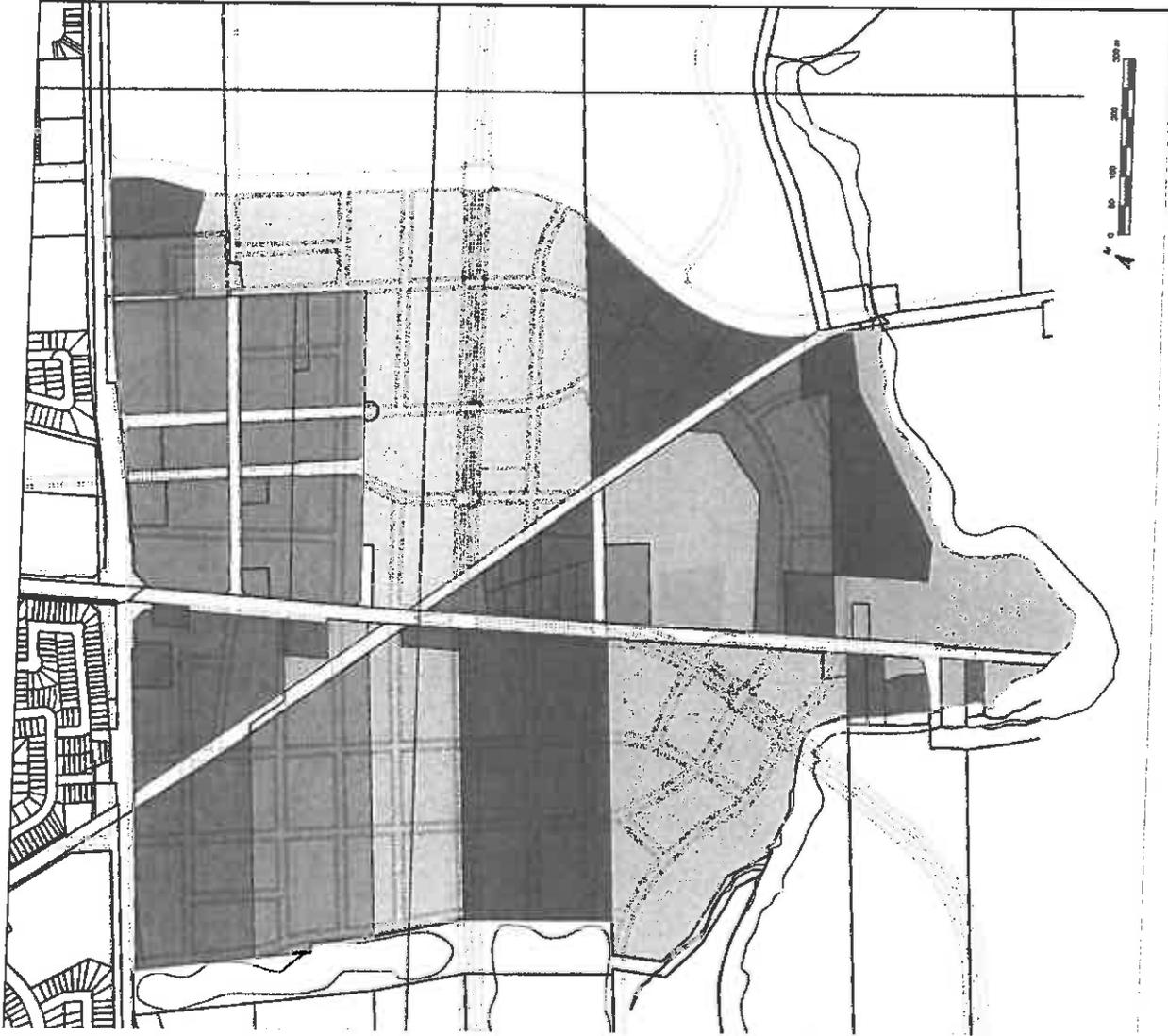
**Schedule 1 - Land Use Plan**

-  High Rise Mixed-Use
-  Mid Rise Mixed-Use
-  High Rise Residential
-  Mid Rise Residential
-  Neighbourhood Park
-  District Park
-  School
-  Civic Complex
-  Strandherd Retail District
-  Stormwater Management Pond



**Note:**

The colour of different parcels illustrates the boundaries of different parcels, or groups of parcels, and does not indicate land ownership.



## **Appendix C**

Watermain Boundary Conditions, FUS Calculations, and Modelling Results

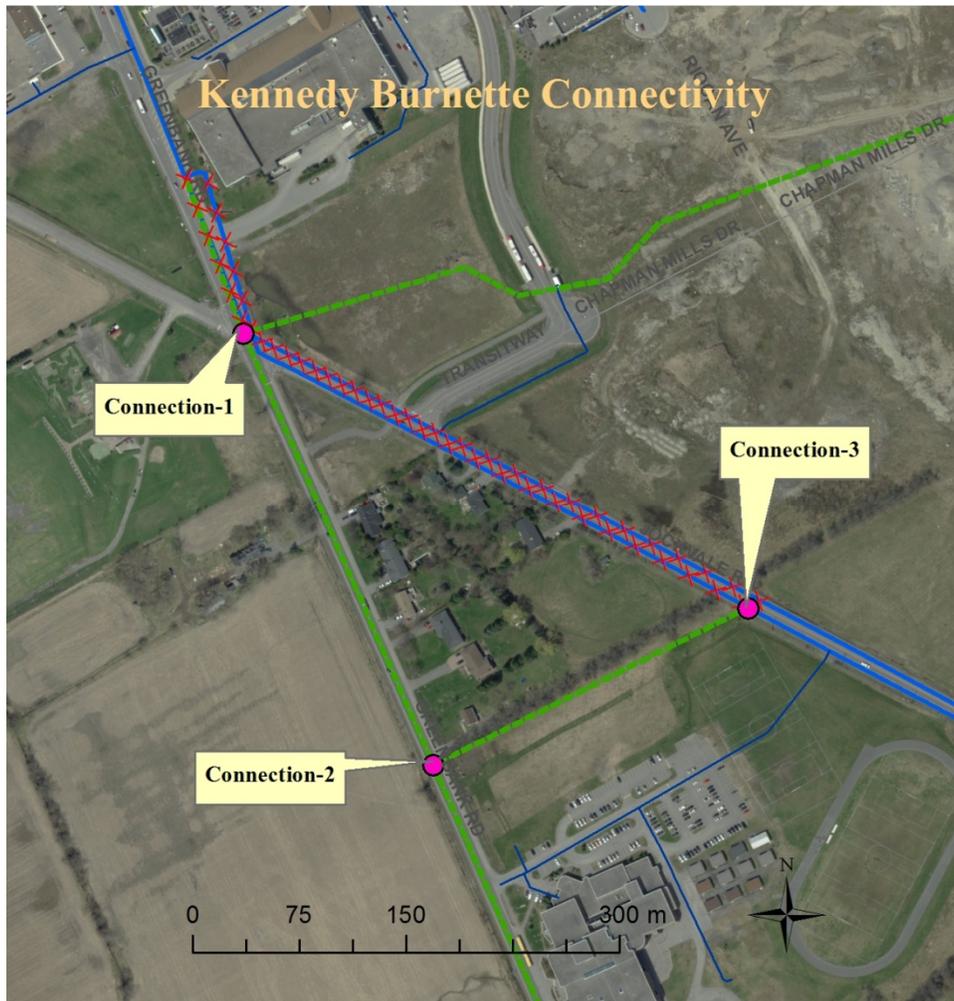
## Boundary Conditions at Kennedy Brunette

### Information Provided:

Date provided: 19 April 2016

For Residential and School	
Criteria	Demand (L/s)
Average Demand	2.7
Maximum Daily Demand	6.7
Peak Hourly Demand	14.8
Fire Flow Demand	250
Maximum Daily + Fire Flow Demand	256.7

### Location:



## Results:

### Connection-1: Pre Zone Configuration

Criteria	Head (m)	Pressure (psi)
Max HGL	162.1	92.9
PKHR	138.5	59.4
MXDY + Fire Flow (250 L/s)	112.3	22.2

### Connection-2:

Criteria	Head (m)	Pressure (psi)
Max HGL	161.7	95.3
PKHR	137.9	61.6
MXDY + Fire Flow (250 L/s)	110.5	22.6

### Connection-3:

Criteria	Head (m)	Pressure (psi)
Max HGL	161.4	88.2
PKHR	137.6	54.4
MXDY + Fire Flow (250 L/s)	110.1	22.3

## Results: Post zone-reconfiguration

### Connection-1:

Criteria	Head (m)	Pressure (psi)
Max HGL	174.7	72.6
PKHR	146.3	70.5
MXDY + Fire Flow (250 L/s)	145.4	69.2

### Connection-2:

Criteria	Head (m)	Pressure (psi)
Max HGL	147.7	75.5
PKHR	145.7	72.7
MXDY + Fire Flow (250 L/s)	144.3	70.6

### Connection-3:

Criteria	Head (m)	Pressure (psi)
Max HGL	147.7	75.8
PKHR	145.6	72.8
MXDY + Fire Flow (250 L/s)	144.1	70.6

## **Considerations:**

1. According to the City of Ottawa Water Design Guidelines as well as the Ontario Building Code, the maximum pressure at any point within a distribution system shall not exceed 80 psi in occupied areas. In scenario-2, measures should be taken to try to reduce the residual pressure below 80 psi without the use of special pressure control equipment. In circumstances where the residual pressure cannot be reduced below 80 psi without the use of pressure control equipment, a pressure reducing valve (**PRV**) should be installed at site.

## **Disclaimer**

*The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.*

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 3

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	440		880	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
<b>F</b>	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			<b>0%</b>			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	> 45.1m		0%	4,675
		East Side	0 - 3 m		25%	
		South Side	30.1- 45 m		5%	
		West Side	0 - 3 m		25%	
<b>Cumulative Total</b>			<b>55%</b>			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>13,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>217</b>	
				or	<b>3,435</b>	
	Required Duration of Fire Flow (hours)			Hours	2.5	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1950		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 5

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	415		830	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
<b>F</b>	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			<b>0%</b>			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	20.1 - 30 m		10%	4,250
		East Side	20.1 - 30 m		10%	
		South Side	30.1 - 45 m		5%	
		West Side	0 - 3 m		25%	
<b>Cumulative Total</b>			<b>50%</b>			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>13,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>L/s</b>	<b>217</b>
				or	<b>USGPM</b>	<b>3,435</b>
	Required Duration of Fire Flow (hours)			Hours	2.5	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1950		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 6

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	506			
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )				1,012		
F	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	3.1 - 10 m		20%	5,525
		East Side	10.1 - 20 m		15%	
		South Side	3.1 - 10 m		20%	
		West Side	20.1 - 30 m		10%	
<b>Cumulative Total</b>			65%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	14,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	233
				or	USGPM	3,699
	Required Duration of Fire Flow (hours)			Hours	3	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	2520		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 7

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	678		1,356	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				12,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	10,200
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	> 45.1m		0%	6,120
		East Side	0 - 3 m		25%	
		South Side	20.1 - 30 m		10%	
		West Side	0 - 3 m		25%	
<b>Cumulative Total</b>			60%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>16,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>267</b>	
				or	<b>4,227</b>	
	Required Duration of Fire Flow (hours)			Hours	3.5	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	3360		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 9

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	252		504	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				7,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	5,950
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	30.1 - 45 m		5%	2,678
		East Side	10.1 - 20 m		15%	
		South Side	3.1 - 10 m		20%	
		West Side	30.1 - 45 m		5%	
<b>Cumulative Total</b>			45%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>9,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>150</b>	
				or	<b>2,378</b>	
	Required Duration of Fire Flow (hours)			Hours	2	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1080		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 10

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	415		830	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			<b>0%</b>			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	30.1 - 45 m		5%	3,825
		East Side	> 45.1m		0%	
		South Side	0 - 3 m		25%	
		West Side	10.1 - 20 m		15%	
<b>Cumulative Total</b>			<b>45%</b>			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>12,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>200</b>	
				or	<b>3,170</b>	
	Required Duration of Fire Flow (hours)			Hours	2.5	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1800		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 13

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	275		550	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				8,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	6,800
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	3.1 - 10 m		20%	4,080
		East Side	> 45.1m		0%	
		South Side	0 - 3 m		25%	
		West Side	10.1 - 20 m		15%	
<b>Cumulative Total</b>			60%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	11,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	183
				or	USGPM	2,906
	Required Duration of Fire Flow (hours)			Hours	2	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1320		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 16

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	575		1,150	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				11,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	9,350
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	3.1 - 10 m		20%	4,675
		East Side	10.1 - 20 m		15%	
		South Side	30.1 - 45 m		5%	
		West Side	20.1 - 30 m		10%	
<b>Cumulative Total</b>			50%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	14,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	233
				or	USGPM	3,699
	Required Duration of Fire Flow (hours)			Hours	3	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	2520		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 20

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	575			
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )				1,150		
F	<b>Base fire flow without reductions</b>				11,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	9,350
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	3.1 - 10 m		20%	4,675
		East Side	20.1 - 30 m		10%	
		South Side	30.1 - 45 m		5%	
		West Side	10.1 - 20 m		15%	
<b>Cumulative Total</b>			50%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	14,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	233
				or	USGPM	3,699
	Required Duration of Fire Flow (hours)			Hours	3	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	2520		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 22

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	450		900	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
<b>F</b>	<b>Base fire flow without reductions</b>				10,000	
	$F = 220 C (A)^{0.5}$					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	0 - 3 m		25%	5,100
		East Side	30.1 - 45 m		5%	
		South Side	3.1 - 10 m		20%	
		West Side	20.1 - 30 m		10%	
<b>Cumulative Total</b>			60%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	14,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	233
				or	USGPM	3,699
	Required Duration of Fire Flow (hours)			Hours	3	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	2520		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117

Project Name: Kennedy Burnette

Date: 07-May-16

Input By: Mark Bowen

Legend

Input by User

No Information or Input Required

Building Description: Node 25 (6 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	500		1,000	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	0 - 3 m		25%	6,375
		East Side	20.1 - 30 m		10%	
		South Side	0 - 3 m		25%	
		West Side	10.1 - 20 m		15%	
<b>Cumulative Total</b>			75%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	15,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	250
				or	USGPM	3,963
	Required Duration of Fire Flow (hours)			Hours	3	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	2700		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 32( 6 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	678			
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )				1,356		
F	<b>Base fire flow without reductions</b>				12,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	10,200
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	> 45.1m		0%	6,120
		East Side	0 - 3 m		25%	
		South Side	20.1 - 30 m		10%	
		West Side	0 - 3 m		25%	
<b>Cumulative Total</b>			60%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	16,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	267
				or	USGPM	4,227
	Required Duration of Fire Flow (hours)			Hours	3.5	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	3360		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 36

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	415		830	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
<b>F</b>	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			<b>0%</b>			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	30.1 - 45 m		5%	4,250
		East Side	10.1 - 20 m		15%	
		South Side	30.1 - 45 m		5%	
		West Side	0 - 3 m		25%	
<b>Cumulative Total</b>			<b>50%</b>			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>13,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>L/s</b>	
				or	<b>USGPM</b>	
					<b>3,435</b>	
Required Duration of Fire Flow (hours)				Hours	2.5	
Required Volume of Fire Flow (m <sup>3</sup> )				m <sup>3</sup>	1950	

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 39

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	442		884	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			<b>0%</b>			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	3.1 - 10 m		20%	4,675
		East Side	20.1 - 30 m		10%	
		South Side	0 - 3 m		25%	
		West Side	> 45.1m		0%	
<b>Cumulative Total</b>			<b>55%</b>			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>13,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>L/s</b>	<b>217</b>
				or	<b>USGPM</b>	<b>3,435</b>
	Required Duration of Fire Flow (hours)			Hours	2.5	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1950		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 41 ( 6 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	443			
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )				886		
F	<b>Base fire flow without reductions</b>				10,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	8,500
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	20.1 - 30 m		10%	5,100
		East Side	0 - 3 m		25%	
		South Side	> 45.1m		0%	
		West Side	0 - 3 m		25%	
<b>Cumulative Total</b>			60%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	14,000	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	233
				or	USGPM	3,699
	Required Duration of Fire Flow (hours)			Hours	3	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	2520		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 43

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	286		572	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
F	<b>Base fire flow without reductions</b>				8,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	6,800
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			0%			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	> 45.1m		0%	1,700
		East Side	0 - 3 m		25%	
		South Side	> 45.1m		0%	
		West Side	> 45.1m		0%	
<b>Cumulative Total</b>			25%			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>9,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>150</b>	
				or	<b>2,378</b>	
	Required Duration of Fire Flow (hours)			Hours	2	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1080		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 46

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)	
<b>Required Fire Flow</b>						
1	<b>Construction Material</b>					
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5	
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Fire resistive construction (< 3 hrs)		0.7		
Fire resistive construction (> 3 hrs)			0.6			
2	<b>Floor Area</b>					
	<b>A</b>	Building Footprint (m <sup>2</sup> )	375		750	
		Number of Floors/Storeys	2			
Area of structure considered (m <sup>2</sup> )						
<b>F</b>	<b>Base fire flow without reductions</b>				9,000	
	<b>F = 220 C (A)<sup>0.5</sup></b>					
<b>Reductions or Surcharges</b>						
3	<b>Occupancy hazard reduction or surcharge</b>					
	<b>(1)</b>	Non-combustible		-25%	-15%	7,650
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	<b>Sprinkler Reduction</b>					
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0	
		Standard Water Supply	No	-10%		
		Fully Supervised System	No	-10%		
<b>Cumulative Total</b>			<b>0%</b>			
5	<b>Exposure surcharge (cumulative %)</b>					
	<b>(3)</b>	North Side	> 45.1m		0%	1,530
		East Side	> 45.1m		0%	
		South Side	30.1 - 45 m		5%	
		West Side	10.1 - 20 m		15%	
<b>Cumulative Total</b>			<b>20%</b>			
<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			<b>L/min</b>	<b>9,000</b>	
	(2,000 L/min < Fire Flow < 45,000 L/min)			or	<b>150</b>	
				or	<b>2,378</b>	
	Required Duration of Fire Flow (hours)			Hours	<b>2</b>	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	<b>1080</b>		

# FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech #: 111117  
 Project Name: Kennedy Burnette  
 Date: 07-May-16  
 Input By: Mark Bowen

Legend	Input by User
	No Information or Input Required

Building Description: Node 48 (4 Towns)

Step		Choose	Multiplier Options	Value Used	Total Fire Flow (L/min)		
<b>Required Fire Flow</b>							
1	<b>Construction Material</b>						
	<b>Coefficient related to type of construction</b> <b>C</b>	Wood frame	Yes	1.5	1.5		
		Ordinary construction		1			
		Non-combustible construction		0.8			
		Fire resistive construction (< 3 hrs)		0.7			
Fire resistive construction (> 3 hrs)			0.6				
2	<b>Floor Area</b>						
	<b>A</b>	Building Footprint (m <sup>2</sup> )	270				
		Number of Floors/Storeys	2				
Area of structure considered (m <sup>2</sup> )				540			
	<b>F</b>	<b>Base fire flow without reductions</b>			8,000		
		$F = 220 C (A)^{0.5}$					
<b>Reductions or Surcharges</b>							
3	<b>Occupancy hazard reduction or surcharge</b>						
	<b>(1)</b>	Non-combustible		-25%	-15%	6,800	
		Limited combustible	Yes	-15%			
		Combustible		0%			
		Free burning		15%			
Rapid burning			25%				
4	<b>Sprinkler Reduction</b>						
	<b>(2)</b>	Adequately Designed System (NFPA 13)	No	-30%	0		
		Standard Water Supply	No	-10%			
		Fully Supervised System	No	-10%			
<b>Cumulative Total</b>			0%				
5	<b>Exposure surcharge (cumulative %)</b>						
	<b>(3)</b>	North Side	> 45.1m		0%	2,720	
		East Side	10.1 - 20 m		15%		
		South Side	0 - 3 m		25%		
		West Side	> 45.1m		0%		
<b>Cumulative Total</b>			40%				
	<b>(1) + (2) + (3)</b>	<b>Total Required fire Flow, rounded to nearest 1000L/min</b>			L/min	9,520	
		(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	159
					or	USGPM	2,515
		Required Duration of Fire Flow (hours)			Hours	2	
Required Volume of Fire Flow (m <sup>3</sup> )			m <sup>3</sup>	1142.4			

<b>Table 1 Watermain Demand Calculations</b>						
<b>Node</b>	<b>Unit</b>		<b>Pop.</b>	<b>Demand (L/s)</b>		
	<b>Town</b>	<b>Condo</b>		<b>High Pres.</b>	<b>Max Daily</b>	<b>Peak Hour</b>
1	0	0	0	0.00	0.00	0.00
2	10	0	27	0.11	0.27	0.60
3	0	0	0	0.00	0.00	0.00
4	10	0	27	0.11	0.27	0.60
5	0	0	0	0.00	0.00	0.00
6	8	0	22	0.09	0.22	0.49
7	10	0	27	0.11	0.27	0.60
8	0	0	0	0.00	0.00	0.00
9	0	0	0	0.00	0.00	0.00
10	0	0	0	0.00	0.00	0.00
11	0	0	0	0.00	0.00	0.00
12	10	0	27	0.11	0.27	0.60
13	16	0	44	0.18	0.45	0.98
14	10	0	27	0.11	0.27	0.60
15	10	0	27	0.11	0.27	0.60
16	0	0	0	0.00	0.00	0.00
17	12	0	33	0.13	0.33	0.74
18	0	140	252	1.02	2.55	5.61
19	0	0	0	0.00	0.00	0.00
20	4	0	11	0.04	0.11	0.25
21	6	0	17	0.07	0.17	0.38
22	6	0	17	0.07	0.17	0.38
23	9	0	25	0.10	0.25	0.56
24	8	0	22	0.09	0.22	0.49
25	10	0	27	0.11	0.27	0.60
26	6	0	17	0.07	0.17	0.38
27	7	0	19	0.08	0.19	0.42
28	3	0	9	0.04	0.09	0.20
29	6	0	17	0.07	0.17	0.38

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

M:\2011\111117\DATA\Calculations\Sewer Calcs\Water\20161110\Unit Count.xls

<b>Table 1 Watermain Demand Calculations</b>						
Node	Unit		Pop.	Demand (L/s)		
	Town	Condo		High Pres.	Max Daily	Peak Hour
30	6	0	17	0.07	0.17	0.38
31	6	0	17	0.07	0.17	0.38
32	11	0	30	0.12	0.30	0.67
33	9	0	25	0.10	0.25	0.56
34	0	0	0	0.00	0.00	0.00
35	0	0	0	0.00	0.00	0.00
36	10	0	27	0.11	0.27	0.60
37	0	0	0	0.00	0.00	0.00
38	10	0	27	0.11	0.27	0.60
39	0	0	0	0.00	0.00	0.00
40	10	0	27	0.11	0.27	0.60
41	10	0	27	0.11	0.27	0.60
42	0	0	0	0.00	0.00	0.00
43	0	0	0	0.00	0.00	0.00
44	0	0	0	0.00	0.00	0.00
45	0	0	0	0.00	0.00	0.00
46	0	0	0	0.00	0.00	0.00
47	4	0	11	0.04	0.11	0.25
48	0	0	0	0.00	0.00	0.00
49	8	0	22	0.09	0.22	0.49
50	0	280	504	2.04	5.10	11.23
				<b>5.78</b>	<b>14.46</b>	<b>31.82</b>

1. Population density: 2.7 people/town
2. High Pressure demand = 350L/s/p/d
3. Maximum Daily demand = 2.5 x High Pressure Demand
4. Peak Hour Demand = 2.2 x Maximum Daily Demand

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

M:\2011\111117\DATA\Calculations\Sewer Calcs\Water\20161110\Unit Count.xls

<b>Table 2</b>			
<b>Pipe Data</b>			
<b>Pipe</b>	<b>Length (m)</b>	<b>Diameter (mm)</b>	<b>Roughness</b>
1	43	250	110
2	31	250	110
3	32	250	110
4	56	250	110
5	43	250	110
6	50	250	110
7	62	250	110
8	17	250	110
9	50	250	110
10	36	250	110
11	30	250	110
12	45	250	110
13	47	250	110
14	43	250	110
15	20	250	110
16	25	250	110
17	34	250	110
18	26	300	120
19	14	300	120
20	54	300	120
21	33	300	120
22	48	300	120
23	21	300	120
24	44	250	110
25	71	250	110
26	61	250	110
27	34	250	110
28	33	250	110
29	34	250	110

<b>Table 2</b>			
<b>Pipe Data</b>			
<b>Pipe</b>	<b>Length</b> (m)	<b>Diameter</b> (mm)	<b>Roughness</b>
30	18	250	110
31	42	250	110
32	31	250	110
33	27	250	110
34	46	250	110
35	29	250	110
36	26	250	110
37	7	300	120
38	59	250	110
39	56	250	110
40	71	250	110
41	46	250	110
42	47	250	110
43	36	250	110
44	75	250	110
45	45	250	110
46	73	250	110
47	27	250	110
48	270	400	120
50	54	300	120
51	61	250	110
52	69	250	110
53	51	250	110
54	42	250	110
55	34	300	120
56	13	300	120
57	50	250	110
58	41	300	120
59	36	300	120

**Table 3**  
**Pre Configuration Condition**  
**High Pressure Check**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
1	93.5	0.00	161.7	68.2	96.7	68
2	93.6	0.11	161.7	68.1	96.6	50
3	93.8	0.00	161.7	67.9	96.3	27
4	93.6	0.11	161.7	68.1	96.6	21
5	93.9	0.00	161.7	67.8	96.1	17
6	93.7	0.09	161.7	68.0	96.4	7
7	93.8	0.11	161.7	67.9	96.3	4
8	94.0	0.00	161.7	67.7	96.0	2
9	93.8	0.00	161.7	67.9	96.3	2
10	94.1	0.00	161.7	67.6	95.9	1
11	94.5	0.00	161.7	67.2	95.3	0
12	94.5	0.11	161.7	67.2	95.3	1
13	94.4	0.18	161.7	67.3	95.4	2
14	94.0	0.11	161.7	67.7	96.0	3
15	93.8	0.11	161.7	67.9	96.3	4
16	93.7	0.00	161.7	68.0	96.4	5
17	93.6	0.13	161.7	68.1	96.6	6
18	93.6	1.02	161.7	68.1	96.6	4
19	93.3	0.00	161.7	68.4	97.0	6
20	93.5	0.04	161.7	68.2	96.7	6
21	93.4	0.07	161.7	68.3	96.8	8
22	93.6	0.07	161.7	68.1	96.6	9
23	93.6	0.10	161.7	68.1	96.6	12
24	93.8	0.09	161.7	67.9	96.3	2
25	93.8	0.11	161.7	67.9	96.3	4
26	93.4	0.07	161.7	68.3	96.8	7
27	94.1	0.08	161.7	67.6	95.9	3
28	93.8	0.04	161.7	67.9	96.3	5
29	93.7	0.07	161.7	68.0	96.4	21

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

M:\2011\111117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\HighPressure.xls

**Table 3**  
**Pre Configuration Condition**  
**High Pressure Check**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
30	93.8	0.07	161.7	67.9	96.3	11
31	93.8	0.07	161.7	67.9	96.3	6
32	93.9	0.12	161.7	67.8	96.1	10
33	93.8	0.10	161.7	67.9	96.3	19
34	93.6	0.00	161.7	68.1	96.6	14
35	93.7	0.00	161.7	68.0	96.4	14
36	93.7	0.11	161.7	68.0	96.4	19
37	93.2	0.00	161.7	68.5	97.1	9
38	93.2	0.11	161.7	68.5	97.1	11
39	93.3	0.00	161.7	68.4	97.0	14
40	93.3	0.11	161.7	68.4	97.0	17
41	93.4	0.11	161.7	68.3	96.8	35
42	93.6	0.00	161.7	68.1	96.6	31
43	93.6	0.00	161.7	68.1	96.6	58
44	94.1	0.00	161.7	67.6	95.9	3
45	94.3	2.04	161.7	67.4	95.6	3
46	94.0	0.00	161.7	67.7	96.0	23
47	94.0	0.04	161.7	67.7	96.0	39
48	94.5	0.00	161.7	67.2	95.3	116
49	94.4	0.09	161.7	67.3	95.4	28
50	93.8	0.00	161.7	67.9	96.3	36
51*	N/A	N/A	161.7	N/A	N/A	N/A

\* Boundary Condition

Maximum Pressure  
 Maximum Time

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

M:\2011\111117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\HighPressure.xls

<b>Table 4a</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 3</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	109.2	15.7	22.3
2	93.6	0.3	108.8	15.2	21.5
3	93.8	167.0	108.5	14.7	20.8
4	93.6	0.3	109.1	15.5	22.0
5	93.9	0.0	110.4	16.5	23.3
6	93.7	0.2	110.5	16.8	23.8
7	93.8	0.3	110.7	16.9	23.9
8	94.0	0.0	110.9	16.9	23.9
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.2
29	93.7	0.2	110.9	17.2	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4a**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 3**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.4	16.7	23.6
37	93.2	0.0	111.0	17.8	25.3
38	93.2	0.3	110.9	17.7	25.0
39	93.3	0.0	110.7	17.4	24.7
40	93.3	0.3	110.6	17.3	24.5
41	93.4	0.3	110.4	17.0	24.0
42	93.6	0.0	110.2	16.6	23.6
43	94.1	0.0	111.8	17.7	25.1
44	93.6	0.0	109.5	15.9	22.5
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.4	16.6	23.5
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

<b>Table 4b</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 5</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.4	16.9	23.9
2	93.6	0.3	110.3	16.7	23.7
3	93.8	0.0	110.2	16.4	23.3
4	93.6	0.3	110.2	16.6	23.5
5	93.9	167.0	110.1	16.2	23.0
6	93.7	0.2	110.3	16.6	23.6
7	93.8	0.3	110.5	16.7	23.7
8	94.0	0.0	110.8	16.8	23.9
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.3
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.8	16.7	23.7
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

<b>Table 4b</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 5</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.6	16.7	23.7
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.5	16.9	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.5	16.8	23.8
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.2
39	93.3	0.0	110.9	17.6	24.9
40	93.3	0.3	110.8	17.5	24.8
41	93.4	0.3	110.6	17.2	24.4
42	93.6	0.0	110.5	16.9	24.0
43	93.6	0.0	110.4	16.8	23.8
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.2	16.4	23.3
51*	N/A	N/A	112.3	N/A	N/A

**\* Boundary Condition**

 Minimum Pressure

<b>Table 4c</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 6</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.5	17.0	24.2
2	93.6	0.3	110.5	16.9	24.0
3	93.8	0.0	110.5	16.7	23.6
4	93.6	0.3	110.4	16.8	23.9
5	93.9	0.0	110.4	16.5	23.4
6	93.7	167.2	109.5	15.8	22.4
7	93.8	0.3	110.1	16.3	23.0
8	94.0	0.0	110.7	16.7	23.7
9	93.8	0.0	110.9	17.1	24.2
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4c**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 6**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.3
39	93.3	0.0	110.9	17.6	25.0
40	93.3	0.3	110.9	17.6	24.9
41	93.4	0.3	110.7	17.3	24.6
42	93.6	0.0	110.6	17.0	24.2
43	93.6	0.0	110.6	17.0	24.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.6
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

<b>Table 4d</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 7</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.6	16.8	23.9
4	93.6	0.3	110.6	17.0	24.1
5	93.9	0.0	110.6	16.7	23.7
6	93.7	0.2	110.1	16.4	23.2
7	93.8	167.3	109.5	15.7	22.2
8	94.0	0.0	110.6	16.6	23.6
9	93.8	0.0	110.8	17.0	24.1
10	94.1	0.0	111.3	17.2	24.3
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.9	17.3	24.5
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

<b>Table 4d</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 7</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.7	17.2	24.4
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.3
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.1	17.9	25.4
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.7	17.1	24.2
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.6	16.8	23.9
51*	N/A	N/A	112.3	N/A	N/A

**\* Boundary Condition**

 Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

<b>Table 4e</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 9</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.0	17.5	24.8
2	93.6	0.3	111.0	17.4	24.7
3	93.8	0.0	111.0	17.2	24.4
4	93.6	0.3	111.0	17.4	24.7
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	110.9	17.2	24.3
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.6	16.6	23.5
9	93.8	167.0	110.3	16.5	23.4
10	94.1	0.0	111.2	17.1	24.2
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.2
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.6
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.2	17.4	24.7
25	93.8	0.3	111.3	17.5	24.7
26	93.4	0.2	111.3	17.9	25.4
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4e**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 9**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	111.0	17.1	24.2
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.7
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	111.0	17.3	24.6
37	93.2	0.0	111.3	18.1	25.6
38	93.2	0.3	111.2	18.0	25.6
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.1	17.8	25.3
41	93.4	0.3	111.1	17.7	25.1
42	93.6	0.0	111.0	17.4	24.7
43	93.6	0.0	111.0	17.4	24.7
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.0	17.2	24.4
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 4f**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 10**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure	
				(m)	(PSI)
1	93.5	0.0	111.3	17.8	25.3
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.7
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.2	17.4	24.7
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.1	17.3	24.5
10	94.1	167.0	110.9	16.8	23.8
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	111.5	17.0	24.1
13	94.4	0.5	111.5	17.1	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.6	17.8	25.2
16	93.7	0.0	111.6	17.9	25.3
17	93.6	0.3	111.6	18.0	25.5
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.4	18.1	25.7
20	93.5	0.1	111.4	17.9	25.4
21	93.4	0.2	111.4	18.0	25.5
22	93.6	0.2	111.4	17.8	25.2
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	111.0	17.2	24.4
25	93.8	0.3	111.2	17.4	24.6
26	93.4	0.2	111.4	18.0	25.5
27	94.1	0.2	111.2	17.1	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	25.0

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\1117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4f**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 10**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.3	17.5	24.9
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.7
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	25.0
36	93.7	0.3	111.3	17.6	25.0
37	93.2	0.0	111.4	18.2	25.8
38	93.2	0.3	111.4	18.2	25.8
39	93.3	0.0	111.4	18.1	25.7
40	93.3	0.3	111.4	18.1	25.6
41	93.4	0.3	111.4	18.0	25.5
42	93.6	0.0	111.3	17.7	25.2
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

<b>Table 4g</b>					
<b>Pre Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node13</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.6	18.1	25.6
2	93.6	0.3	111.6	18.0	25.5
3	93.8	0.0	111.6	17.8	25.2
4	93.6	0.3	111.6	18.0	25.5
5	93.9	0.0	111.6	17.7	25.0
6	93.7	0.2	111.6	17.9	25.3
7	93.8	0.3	111.6	17.8	25.2
8	94.0	0.0	111.6	17.6	24.9
9	93.8	0.0	111.6	17.8	25.2
10	94.1	0.0	111.5	17.4	24.7
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	110.9	16.4	23.2
13	94.4	167.5	109.9	15.5	22.0
14	94.0	0.3	110.4	16.4	23.2
15	93.8	0.3	110.8	17.0	24.1
16	93.7	0.0	111.0	17.3	24.5
17	93.6	0.3	111.2	17.6	25.0
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.6	18.3	25.9
20	93.5	0.1	111.6	18.1	25.6
21	93.4	0.2	111.6	18.2	25.8
22	93.6	0.2	111.6	18.0	25.5
23	93.6	0.3	111.6	18.0	25.5
24	93.8	0.2	111.5	17.7	25.2
25	93.8	0.3	111.6	17.8	25.2
26	93.4	0.2	111.6	18.2	25.8
27	94.1	0.2	111.6	17.5	24.8
28	93.8	0.1	111.6	17.8	25.2
29	93.7	0.2	111.6	17.9	25.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4g**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node13**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.6	17.8	25.2
31	93.8	0.2	111.6	17.8	25.2
32	93.9	0.3	111.6	17.7	25.0
33	93.8	0.3	111.6	17.8	25.2
34	93.6	0.0	111.6	18.0	25.5
35	93.7	0.0	111.6	17.9	25.3
36	93.7	0.3	111.6	17.9	25.3
37	93.2	0.0	111.6	18.4	26.0
38	93.2	0.3	111.6	18.4	26.0
39	93.3	0.0	111.6	18.3	25.9
40	93.3	0.3	111.6	18.3	25.9
41	93.4	0.3	111.6	18.2	25.8
42	93.6	0.0	111.6	18.0	25.5
43	93.6	0.0	111.6	18.0	25.5
44	94.1	0	111.84	17.74	25.2
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.6	17.8	25.2
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 4h**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 16**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.5	18.0	25.6
2	93.6	0.3	111.5	17.9	25.4
3	93.8	0.0	111.5	17.7	25.1
4	93.6	0.3	111.5	17.9	25.4
5	93.9	0.0	111.5	17.6	25.0
6	93.7	0.2	111.5	17.8	25.3
7	93.8	0.3	111.5	17.7	25.1
8	94.0	0.0	111.5	17.5	24.9
9	93.8	0.0	111.5	17.7	25.2
10	94.1	0.0	111.6	17.5	24.8
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.3	16.8	23.9
13	94.4	0.5	111.0	16.6	23.5
14	94.0	0.3	110.6	16.6	23.5
15	93.8	0.3	110.2	16.4	23.3
16	93.7	167.0	110.1	16.4	23.2
17	93.6	0.3	110.7	17.1	24.2
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.5	18.2	25.8
20	93.5	0.1	111.5	18.0	25.6
21	93.4	0.2	111.5	18.1	25.7
22	93.6	0.2	111.5	17.9	25.4
23	93.6	0.3	111.5	17.9	25.4
24	93.8	0.2	111.6	17.8	25.2
25	93.8	0.3	111.5	17.7	25.1
26	93.4	0.2	111.5	18.1	25.7
27	94.1	0.2	111.5	17.4	24.7
28	93.8	0.1	111.5	17.7	25.1
29	93.7	0.2	111.5	17.8	25.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4h**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 16**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.5	17.7	25.1
31	93.8	0.2	111.5	17.7	25.1
32	93.9	0.3	111.5	17.6	25.0
33	93.8	0.3	111.5	17.7	25.1
34	93.6	0.0	111.5	17.9	25.4
35	93.7	0.0	111.5	17.8	25.3
36	93.7	0.3	111.5	17.8	25.3
37	93.2	0.0	111.5	18.3	26.0
38	93.2	0.3	111.5	18.3	26.0
39	93.3	0.0	111.5	18.2	25.8
40	93.3	0.3	111.5	18.2	25.8
41	93.4	0.3	111.5	18.1	25.7
42	93.6	0.0	111.5	17.9	25.4
43	93.6	0.0	111.5	17.9	25.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.5	17.7	25.1
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 4i**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 20**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.1	17.6	25.0
2	93.6	0.3	111.1	17.5	24.9
3	93.8	0.0	111.1	17.3	24.6
4	93.6	0.3	111.1	17.5	24.9
5	93.9	0.0	111.1	17.2	24.4
6	93.7	0.2	111.2	17.5	24.8
7	93.8	0.3	111.2	17.4	24.6
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.4	17.3	24.6
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.4	17.8	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	167.1	111.0	17.5	24.8
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.1	17.5	24.8
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.2	17.1	24.2
28	93.8	0.1	111.1	17.3	24.6
29	93.7	0.2	111.1	17.4	24.7

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\1117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4i**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 20**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.6
32	93.9	0.3	111.1	17.2	24.4
33	93.8	0.3	111.1	17.3	24.6
34	93.6	0.0	111.1	17.5	24.8
35	93.7	0.0	111.1	17.4	24.7
36	93.7	0.3	111.1	17.4	24.7
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.2	18.0	25.5
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.2	17.9	25.3
41	93.4	0.3	111.1	17.7	25.2
42	93.6	0.0	111.1	17.5	24.9
43	93.6	0.0	111.1	17.5	24.9
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.7	17.4	24.7
46	94.0	0.0	111.7	17.7	25.2
47	94.0	0.1	111.7	17.7	25.2
48	94.5	0.0	111.7	17.2	24.4
49	94.4	0.2	111.7	17.3	24.6
50	93.8	0.0	111.1	17.3	24.6
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 4j**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 22**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.8	17.3	24.5
2	93.6	0.3	110.8	17.2	24.3
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.3
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.2
7	93.8	0.3	110.9	17.1	24.2
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.1	17.6	25.0
21	93.4	0.2	110.8	17.4	24.6
22	93.6	167.2	110.5	16.9	23.9
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.2

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4j**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 22**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	110.8	17.0	24.1
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.2
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.8	17.2	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 4k**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 25**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.0	111.3	17.8	25.2
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.6
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.3	17.5	24.8
8	94.0	0.0	111.3	17.3	24.5
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.2	17.1	24.3
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.3	17.9	25.4
22	93.6	0.2	111.3	17.7	25.1
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	110.6	16.8	23.9
25	93.8	167.3	109.7	15.9	22.5
26	93.4	0.2	110.7	17.3	24.5
27	94.1	0.2	111.3	17.2	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	24.9

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4k**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 25**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.3	17.5	24.8
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.6
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	24.9
36	93.7	0.3	111.3	17.6	24.9
37	93.2	0.0	111.3	18.1	25.7
38	93.2	0.3	111.3	18.1	25.7
39	93.3	0.0	111.3	18.0	25.5
40	93.3	0.3	111.3	18.0	25.5
41	93.4	0.3	111.3	17.9	25.4
42	93.6	0.0	111.3	17.7	25.1
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 4I**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 32**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.2
5	93.9	0.0	110.7	16.8	23.8
6	93.7	0.2	110.7	17.0	24.1
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.8	16.8	23.8
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.7
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.6	16.8	23.8
29	93.7	0.2	110.6	16.9	24.0

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 41**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 32**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	110.8	17.0	24.0
31	93.8	0.2	110.2	16.4	23.3
32	93.9	167.3	109.6	15.7	22.3
33	93.8	0.3	110.2	16.4	23.2
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.7	17.1	24.3
43	93.6	0.0	110.7	17.1	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	23.9
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

**Table 4m**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 36**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.2	16.7	23.6
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.3	16.5	23.4
4	93.6	0.3	110.4	16.8	23.8
5	93.9	0.0	110.5	16.6	23.6
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4m**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 36**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.9
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.9
36	93.7	167.3	109.3	15.6	22.1
37	93.2	0.0	111.0	17.8	25.2
38	93.2	0.3	110.8	17.6	24.9
39	93.3	0.0	110.6	17.3	24.5
40	93.3	0.3	110.4	17.1	24.3
41	93.4	0.3	110.1	16.7	23.7
42	93.6	0.0	109.9	16.3	23.2
43	93.6	0.0	110.1	16.5	23.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

**Table 4n**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 39**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	110.6	17.1	24.2
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.8	17.2	24.4
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	111.0	17.3	24.5
7	93.8	0.3	111.1	17.3	24.5
8	94.0	0.0	111.1	17.1	24.3
9	93.8	0.0	111.2	17.4	24.7
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.1	17.0	24.1
28	93.8	0.1	111.1	17.3	24.5
29	93.7	0.2	111.1	17.4	24.7

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4n**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 39**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.5
32	93.9	0.3	111.0	17.1	24.3
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.6
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	109.9	16.7	23.7
38	93.2	0.3	109.1	15.9	22.6
39	93.3	167.0	108.3	15.0	21.3
40	93.3	0.3	108.8	15.5	21.9
41	93.4	0.3	109.8	16.4	23.2
42	93.6	0.0	110.3	16.7	23.7
43	93.6	0.0	110.5	16.9	24.0
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.7
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	111.0	17.2	24.3
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

**Table 4o**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 41**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	110.1	16.6	23.5
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.4	16.6	23.5
4	93.6	0.3	110.5	16.9	24.0
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.3
7	93.8	0.3	110.9	17.1	24.3
8	94.0	0.0	111.0	17.0	24.2
9	93.8	0.0	111.1	17.3	24.6
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.0	17.6	25.0
22	93.6	0.2	110.9	17.3	24.6
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.0	16.9	24.0
28	93.8	0.1	111.0	17.2	24.4
29	93.7	0.2	111.0	17.3	24.5

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4o**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 41**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	110.9	17.0	24.0
33	93.8	0.3	110.8	17.0	24.1
34	93.6	0.0	110.8	17.2	24.4
35	93.7	0.0	110.7	17.0	24.2
36	93.7	0.3	110.2	16.5	23.4
37	93.2	0.0	110.6	17.4	24.6
38	93.2	0.3	110.1	16.9	24.0
39	93.3	0.0	109.7	16.4	23.2
40	93.3	0.3	109.4	16.1	22.8
41	93.4	167.3	108.7	15.3	21.7
42	93.6	0.0	109.7	16.1	22.8
43	93.6	0.0	110.0	16.4	23.2
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

**Table 4p**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 43**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	108.7	15.2	21.5
2	93.6	0.3	109.1	15.5	22.0
3	93.8	0.0	109.5	15.7	22.2
4	93.6	0.3	109.9	16.3	23.1
5	93.9	0.0	110.5	16.6	23.5
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4p**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 43**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	110.9	17.1	24.3
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	23.9
36	93.7	0.3	110.2	16.5	23.3
37	93.2	0.0	110.9	17.7	25.1
38	93.2	0.3	110.7	17.5	24.8
39	93.3	0.0	110.5	17.2	24.4
40	93.3	0.3	110.3	17.0	24.1
41	93.4	0.3	110.0	16.6	23.5
42	93.6	0.0	109.8	16.2	23.0
43	93.6	167.0	108.4	14.8	20.9
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

**Table 4q**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 46**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4q**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 46**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	167.0	110.2	16.2	22.9
47	94.0	0.1	110.3	16.3	23.2
48	94.5	0.0	110.5	16.0	22.7
49	94.4	0.2	110.7	16.3	23.1
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

\* **Boundary Condition**

Minimum Pressure

**Table 4r**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 48**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\MaxDailyFF.xls

**Table 4r**  
**Pre Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 48**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	0.0	110.6	16.6	23.5
47	94.0	0.1	109.9	15.9	22.5
48	94.5	167.0	109.1	14.6	20.6
49	94.4	0.2	110.0	15.6	22.2
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

<b>Table 5</b>					
<b>Pre Configuration Condition</b>					
<b>Peak Hour Check</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.00	137.87	44.4	62.9
2	93.6	0.60	137.87	44.3	62.8
3	93.8	0.00	137.87	44.1	62.5
4	93.6	0.60	137.87	44.3	62.8
5	93.9	0.00	137.87	44.0	62.3
6	93.7	0.49	137.87	44.2	62.6
7	93.8	0.60	137.87	44.1	62.5
8	94.0	0.00	137.87	43.9	62.2
9	93.8	0.00	137.87	44.1	62.5
10	94.1	0.00	137.87	43.8	62.1
11	94.5	0.00	137.88	43.4	61.5
12	94.5	0.60	137.88	43.4	61.5
13	94.4	0.98	137.88	43.5	61.7
14	94.0	0.60	137.87	43.9	62.2
15	93.8	0.60	137.87	44.1	62.5
16	93.7	0.00	137.87	44.2	62.6
17	93.6	0.74	137.87	44.3	62.8
18	93.6	5.61	137.87	44.3	62.8
19	93.3	0.00	137.87	44.6	63.2
20	93.5	0.25	137.87	44.4	62.9
21	93.4	0.38	137.87	44.5	63.1
22	93.6	0.38	137.87	44.3	62.8
23	93.6	0.56	137.87	44.3	62.8
24	93.8	0.49	137.87	44.1	62.5
25	93.8	0.60	137.87	44.1	62.5
26	93.4	0.38	137.87	44.5	63.1
27	94.1	0.42	137.87	43.8	62.1
28	93.8	0.20	137.87	44.1	62.5
29	93.7	0.38	137.87	44.2	62.6

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016 \\11117\DATA\Calculations\Sewer Calcs\Water\20161110\Pre Configuration\PeakHour.xls

<b>Table 5</b>					
<b>Pre Configuration Condition</b>					
<b>Peak Hour Check</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.38	137.87	44.1	62.5
31	93.8	0.38	137.87	44.1	62.5
32	93.9	0.67	137.87	44.0	62.3
33	93.8	0.56	137.87	44.1	62.5
34	93.6	0.00	137.87	44.3	62.8
35	93.7	0.00	137.87	44.2	62.6
36	93.7	0.60	137.87	44.2	62.6
37	93.2	0.00	137.87	44.7	63.3
38	93.2	0.60	137.87	44.7	63.3
39	93.3	0.00	137.87	44.6	63.2
40	93.3	0.60	137.87	44.6	63.2
41	93.4	0.60	137.87	44.5	63.1
42	93.6	0.00	137.87	44.3	62.8
43	93.6	0.00	137.87	44.3	62.8
44	94.1	0.00	137.87	43.8	62.1
45	94.3	11.23	137.87	43.6	61.8
46	94.0	0.00	137.87	43.9	62.2
47	94.0	0.25	137.87	43.9	62.2
48	94.5	0.00	137.87	43.4	61.5
49	94.4	0.49	137.87	43.5	61.6
50	93.8	0.00	137.87	44.1	62.5
51*	N/A	N/A	137.9	N/A	N/A

**\* Boundary Condition**

Minimum Pressure

**Table 6**  
**Post Configuration Condition**  
**High Pressure Check**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
1	93.5	0.00	147.7	54.2	76.9	68
2	93.6	0.11	147.7	54.1	76.7	50
3	93.8	0.00	147.7	53.9	76.4	27
4	93.6	0.11	147.7	54.1	76.7	21
5	93.9	0.00	147.7	53.8	76.3	17
6	93.7	0.09	147.7	54.0	76.6	7
7	93.8	0.11	147.7	53.9	76.4	4
8	94.0	0.00	147.7	53.7	76.1	2
9	93.8	0.00	147.7	53.9	76.4	2
10	94.1	0.00	147.7	53.6	76.0	1
11	94.5	0.00	147.7	53.2	75.4	0
12	94.5	0.11	147.7	53.2	75.4	1
13	94.4	0.18	147.7	53.3	75.6	2
14	94.0	0.11	147.7	53.7	76.1	3
15	93.8	0.11	147.7	53.9	76.4	4
16	93.7	0.00	147.7	54.0	76.6	5
17	93.6	0.13	147.7	54.1	76.7	6
18	93.6	1.02	147.7	54.1	76.7	4
19	93.3	0.00	147.7	54.4	77.1	6
20	93.5	0.04	147.7	54.2	76.9	6
21	93.4	0.07	147.7	54.3	77.0	8
22	93.6	0.07	147.7	54.1	76.7	9
23	93.6	0.10	147.7	54.1	76.7	12
24	93.8	0.09	147.7	53.9	76.4	2
25	93.8	0.11	147.7	53.9	76.4	4
26	93.4	0.07	147.7	54.3	77.0	7
27	94.1	0.08	147.7	53.6	76.0	3
28	93.8	0.04	147.7	53.9	76.4	5
29	93.7	0.07	147.7	54.0	76.6	21

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

M:\2011\111117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\HighPressure.xls

**Table 6**  
**Post Configuration Condition**  
**High Pressure Check**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)	Age (hrs)
30	93.8	0.07	147.7	53.9	76.4	11
31	93.8	0.07	147.7	53.9	76.4	6
32	93.9	0.12	147.7	53.8	76.3	10
33	93.8	0.10	147.7	53.9	76.4	19
34	93.6	0.00	147.7	54.1	76.7	14
35	93.7	0.00	147.7	54.0	76.6	14
36	93.7	0.11	147.7	54.0	76.6	19
37	93.2	0.00	147.7	54.5	77.3	9
38	93.2	0.11	147.7	54.5	77.3	11
39	93.3	0.00	147.7	54.4	77.1	14
40	93.3	0.11	147.7	54.4	77.1	17
41	93.4	0.11	147.7	54.3	77.0	35
42	93.6	0.00	147.7	54.1	76.7	31
43	93.6	0.00	147.7	54.1	76.7	58
44	94.1	0.00	147.7	53.6	76.0	3
45	94.3	2.04	147.7	53.4	75.7	3
46	94.0	0.00	147.7	53.7	76.1	23
47	94.0	0.04	147.7	53.7	76.1	39
48	94.5	0.00	147.7	53.2	75.4	116
49	94.4	0.09	147.7	53.3	75.6	28
50	93.8	0.00	147.7	53.9	76.4	36
51*	N/A	N/A	147.7	N/A	N/A	N/A

\* Boundary Condition

Maximum Pressure  
 Maximum Time

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 14, 2016

M:\2011\111117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\HighPressure.xls

**Table 7a**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 3**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.0	109.2	15.7	22.3
2	93.6	0.3	108.8	15.2	21.5
3	93.8	167.0	108.5	14.7	20.8
4	93.6	0.3	109.1	15.5	22.0
5	93.9	0.0	110.4	16.5	23.3
6	93.7	0.2	110.5	16.8	23.8
7	93.8	0.3	110.7	16.9	23.9
8	94.0	0.0	110.9	16.9	23.9
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.2
29	93.7	0.2	110.9	17.2	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7a**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 3**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.4	16.7	23.6
37	93.2	0.0	111.0	17.8	25.3
38	93.2	0.3	110.9	17.7	25.0
39	93.3	0.0	110.7	17.4	24.7
40	93.3	0.3	110.6	17.3	24.5
41	93.4	0.3	110.4	17.0	24.0
42	93.6	0.0	110.2	16.6	23.6
43	94.1	0.0	111.8	17.7	25.1
44	93.6	0.0	109.5	15.9	22.5
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.4	16.6	23.5
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

<b>Table 7b</b>					
<b>Post Confirnation Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 5</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.4	16.9	23.9
2	93.6	0.3	110.3	16.7	23.7
3	93.8	0.0	110.2	16.4	23.3
4	93.6	0.3	110.2	16.6	23.5
5	93.9	167.0	110.1	16.2	23.0
6	93.7	0.2	110.3	16.6	23.6
7	93.8	0.3	110.5	16.7	23.7
8	94.0	0.0	110.8	16.8	23.9
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.3
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.8	16.7	23.7
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7b**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 5**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.6	16.7	23.7
33	93.8	0.3	110.6	16.8	23.8
34	93.6	0.0	110.5	16.9	24.0
35	93.7	0.0	110.5	16.8	23.8
36	93.7	0.3	110.5	16.8	23.8
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.2
39	93.3	0.0	110.9	17.6	24.9
40	93.3	0.3	110.8	17.5	24.8
41	93.4	0.3	110.6	17.2	24.4
42	93.6	0.0	110.5	16.9	24.0
43	93.6	0.0	110.4	16.8	23.8
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.2	16.4	23.3
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

<b>Table 7c</b>					
<b>Post Confirnation Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 6</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.5	17.0	24.2
2	93.6	0.3	110.5	16.9	24.0
3	93.8	0.0	110.5	16.7	23.6
4	93.6	0.3	110.4	16.8	23.9
5	93.9	0.0	110.4	16.5	23.4
6	93.7	167.2	109.5	15.8	22.4
7	93.8	0.3	110.1	16.3	23.0
8	94.0	0.0	110.7	16.7	23.7
9	93.8	0.0	110.9	17.1	24.2
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

<b>Table 7c</b>					
<b>Post Confirnation Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 6</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.0	17.8	25.3
39	93.3	0.0	110.9	17.6	25.0
40	93.3	0.3	110.9	17.6	24.9
41	93.4	0.3	110.7	17.3	24.6
42	93.6	0.0	110.6	17.0	24.2
43	93.6	0.0	110.6	17.0	24.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.6
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

<b>Table 7d</b>					
<b>Post Confirnation Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 7</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.6	16.8	23.9
4	93.6	0.3	110.6	17.0	24.1
5	93.9	0.0	110.6	16.7	23.7
6	93.7	0.2	110.1	16.4	23.2
7	93.8	167.3	109.5	15.7	22.2
8	94.0	0.0	110.6	16.6	23.6
9	93.8	0.0	110.8	17.0	24.1
10	94.1	0.0	111.3	17.2	24.3
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.9	17.3	24.5
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.7	16.9	24.0
29	93.7	0.2	110.8	17.1	24.2

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

<b>Table 7d</b>					
<b>Post Confiration Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 7</b>					
Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure	
				(m)	(PSI)
1	93.5	0.0	110.7	17.2	24.4
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.7	16.9	24.0
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.3
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.1	17.9	25.4
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.7	17.1	24.2
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.6	16.8	23.9
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

<b>Table 7e</b>					
<b>Post Confirnation Condition</b>					
<b>Max Daily Demand and Fire Flow at Node 9</b>					
<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.0	17.5	24.8
2	93.6	0.3	111.0	17.4	24.7
3	93.8	0.0	111.0	17.2	24.4
4	93.6	0.3	111.0	17.4	24.7
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	110.9	17.2	24.3
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.6	16.6	23.5
9	93.8	167.0	110.3	16.5	23.4
10	94.1	0.0	111.2	17.1	24.2
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.2
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.6
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.2	17.4	24.7
25	93.8	0.3	111.3	17.5	24.7
26	93.4	0.2	111.3	17.9	25.4
27	94.1	0.2	110.7	16.6	23.6
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7e**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 9**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	111.0	17.1	24.2
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.7
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	111.0	17.3	24.6
37	93.2	0.0	111.3	18.1	25.6
38	93.2	0.3	111.2	18.0	25.6
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.1	17.8	25.3
41	93.4	0.3	111.1	17.7	25.1
42	93.6	0.0	111.0	17.4	24.7
43	93.6	0.0	111.0	17.4	24.7
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.0	17.2	24.4
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 7f**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 10**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure	
				(m)	(PSI)
1	93.5	0.0	111.3	17.8	25.3
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.7
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.2	17.4	24.7
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.1	17.3	24.5
10	94.1	167.0	110.9	16.8	23.8
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	111.5	17.0	24.1
13	94.4	0.5	111.5	17.1	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.6	17.8	25.2
16	93.7	0.0	111.6	17.9	25.3
17	93.6	0.3	111.6	18.0	25.5
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.4	18.1	25.7
20	93.5	0.1	111.4	17.9	25.4
21	93.4	0.2	111.4	18.0	25.5
22	93.6	0.2	111.4	17.8	25.2
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	111.0	17.2	24.4
25	93.8	0.3	111.2	17.4	24.6
26	93.4	0.2	111.4	18.0	25.5
27	94.1	0.2	111.2	17.1	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	25.0

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7f**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 10**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.3	17.5	24.9
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.7
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	25.0
36	93.7	0.3	111.3	17.6	25.0
37	93.2	0.0	111.4	18.2	25.8
38	93.2	0.3	111.4	18.2	25.8
39	93.3	0.0	111.4	18.1	25.7
40	93.3	0.3	111.4	18.1	25.6
41	93.4	0.3	111.4	18.0	25.5
42	93.6	0.0	111.3	17.7	25.2
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 7g**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node13**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
1	93.5	0.0	111.6	18.1	25.6
2	93.6	0.3	111.6	18.0	25.5
3	93.8	0.0	111.6	17.8	25.2
4	93.6	0.3	111.6	18.0	25.5
5	93.9	0.0	111.6	17.7	25.0
6	93.7	0.2	111.6	17.9	25.3
7	93.8	0.3	111.6	17.8	25.2
8	94.0	0.0	111.6	17.6	24.9
9	93.8	0.0	111.6	17.8	25.2
10	94.1	0.0	111.5	17.4	24.7
11	94.5	0.0	111.5	17.0	24.1
12	94.5	0.3	110.9	16.4	23.2
13	94.4	167.5	109.9	15.5	22.0
14	94.0	0.3	110.4	16.4	23.2
15	93.8	0.3	110.8	17.0	24.1
16	93.7	0.0	111.0	17.3	24.5
17	93.6	0.3	111.2	17.6	25.0
18	93.6	2.6	111.6	18.0	25.5
19	93.3	0.0	111.6	18.3	25.9
20	93.5	0.1	111.6	18.1	25.6
21	93.4	0.2	111.6	18.2	25.8
22	93.6	0.2	111.6	18.0	25.5
23	93.6	0.3	111.6	18.0	25.5
24	93.8	0.2	111.5	17.7	25.2
25	93.8	0.3	111.6	17.8	25.2
26	93.4	0.2	111.6	18.2	25.8
27	94.1	0.2	111.6	17.5	24.8
28	93.8	0.1	111.6	17.8	25.2
29	93.7	0.2	111.6	17.9	25.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7g**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node13**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.6	17.8	25.2
31	93.8	0.2	111.6	17.8	25.2
32	93.9	0.3	111.6	17.7	25.0
33	93.8	0.3	111.6	17.8	25.2
34	93.6	0.0	111.6	18.0	25.5
35	93.7	0.0	111.6	17.9	25.3
36	93.7	0.3	111.6	17.9	25.3
37	93.2	0.0	111.6	18.4	26.0
38	93.2	0.3	111.6	18.4	26.0
39	93.3	0.0	111.6	18.3	25.9
40	93.3	0.3	111.6	18.3	25.9
41	93.4	0.3	111.6	18.2	25.8
42	93.6	0.0	111.6	18.0	25.5
43	93.6	0.0	111.6	18.0	25.5
44	94.1	0	111.84	17.74	25.2
45	94.3	5.1	111.8	17.5	24.9
46	94.0	0.0	111.8	17.8	25.3
47	94.0	0.1	111.8	17.8	25.3
48	94.5	0.0	111.8	17.3	24.6
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.6	17.8	25.2
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 7h**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 16**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure	
				(m)	(PSI)
1	93.5	0.0	111.5	18.0	25.6
2	93.6	0.3	111.5	17.9	25.4
3	93.8	0.0	111.5	17.7	25.1
4	93.6	0.3	111.5	17.9	25.4
5	93.9	0.0	111.5	17.6	25.0
6	93.7	0.2	111.5	17.8	25.3
7	93.8	0.3	111.5	17.7	25.1
8	94.0	0.0	111.5	17.5	24.9
9	93.8	0.0	111.5	17.7	25.2
10	94.1	0.0	111.6	17.5	24.8
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.3	16.8	23.9
13	94.4	0.5	111.0	16.6	23.5
14	94.0	0.3	110.6	16.6	23.5
15	93.8	0.3	110.2	16.4	23.3
16	93.7	167.0	110.1	16.4	23.2
17	93.6	0.3	110.7	17.1	24.2
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.5	18.2	25.8
20	93.5	0.1	111.5	18.0	25.6
21	93.4	0.2	111.5	18.1	25.7
22	93.6	0.2	111.5	17.9	25.4
23	93.6	0.3	111.5	17.9	25.4
24	93.8	0.2	111.6	17.8	25.2
25	93.8	0.3	111.5	17.7	25.1
26	93.4	0.2	111.5	18.1	25.7
27	94.1	0.2	111.5	17.4	24.7
28	93.8	0.1	111.5	17.7	25.1
29	93.7	0.2	111.5	17.8	25.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7h**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 16**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.5	17.7	25.1
31	93.8	0.2	111.5	17.7	25.1
32	93.9	0.3	111.5	17.6	25.0
33	93.8	0.3	111.5	17.7	25.1
34	93.6	0.0	111.5	17.9	25.4
35	93.7	0.0	111.5	17.8	25.3
36	93.7	0.3	111.5	17.8	25.3
37	93.2	0.0	111.5	18.3	26.0
38	93.2	0.3	111.5	18.3	26.0
39	93.3	0.0	111.5	18.2	25.8
40	93.3	0.3	111.5	18.2	25.8
41	93.4	0.3	111.5	18.1	25.7
42	93.6	0.0	111.5	17.9	25.4
43	93.6	0.0	111.5	17.9	25.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.5	17.7	25.1
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 7i**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 20**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.1	17.6	25.0
2	93.6	0.3	111.1	17.5	24.9
3	93.8	0.0	111.1	17.3	24.6
4	93.6	0.3	111.1	17.5	24.9
5	93.9	0.0	111.1	17.2	24.4
6	93.7	0.2	111.2	17.5	24.8
7	93.8	0.3	111.2	17.4	24.6
8	94.0	0.0	111.2	17.2	24.4
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.4	17.3	24.6
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.4	17.8	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	167.1	111.0	17.5	24.8
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.1	17.5	24.8
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.2	17.1	24.2
28	93.8	0.1	111.1	17.3	24.6
29	93.7	0.2	111.1	17.4	24.7

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17:04:17 \DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7i**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 20**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.6
32	93.9	0.3	111.1	17.2	24.4
33	93.8	0.3	111.1	17.3	24.6
34	93.6	0.0	111.1	17.5	24.8
35	93.7	0.0	111.1	17.4	24.7
36	93.7	0.3	111.1	17.4	24.7
37	93.2	0.0	111.2	18.0	25.5
38	93.2	0.3	111.2	18.0	25.5
39	93.3	0.0	111.2	17.9	25.3
40	93.3	0.3	111.2	17.9	25.3
41	93.4	0.3	111.1	17.7	25.2
42	93.6	0.0	111.1	17.5	24.9
43	93.6	0.0	111.1	17.5	24.9
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.7	17.4	24.7
46	94.0	0.0	111.7	17.7	25.2
47	94.0	0.1	111.7	17.7	25.2
48	94.5	0.0	111.7	17.2	24.4
49	94.4	0.2	111.7	17.3	24.6
50	93.8	0.0	111.1	17.3	24.6
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 7j**  
**Post Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 22**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.8	17.3	24.5
2	93.6	0.3	110.8	17.2	24.3
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.3
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.2
7	93.8	0.3	110.9	17.1	24.2
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.1	17.6	25.0
21	93.4	0.2	110.8	17.4	24.6
22	93.6	167.2	110.5	16.9	23.9
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.8	17.0	24.1
29	93.7	0.2	110.8	17.1	24.2

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17:00:00 \\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7j**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 22**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m) (PSI)	
30	93.8	0.2	110.8	17.0	24.1
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	24.0
34	93.6	0.0	110.7	17.1	24.2
35	93.7	0.0	110.7	17.0	24.1
36	93.7	0.3	110.7	17.0	24.2
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.8	17.2	24.3
43	93.6	0.0	110.8	17.2	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

 Minimum Pressure

**Table 7k**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 25**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	111.3	17.8	25.2
2	93.6	0.3	111.3	17.7	25.1
3	93.8	0.0	111.3	17.5	24.8
4	93.6	0.3	111.3	17.7	25.1
5	93.9	0.0	111.3	17.4	24.6
6	93.7	0.2	111.3	17.6	24.9
7	93.8	0.3	111.3	17.5	24.8
8	94.0	0.0	111.3	17.3	24.5
9	93.8	0.0	111.3	17.5	24.8
10	94.1	0.0	111.2	17.1	24.3
11	94.5	0.0	111.6	17.1	24.2
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.3
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.3	17.8	25.2
21	93.4	0.2	111.3	17.9	25.4
22	93.6	0.2	111.3	17.7	25.1
23	93.6	0.3	111.3	17.7	25.1
24	93.8	0.2	110.6	16.8	23.9
25	93.8	167.3	109.7	15.9	22.5
26	93.4	0.2	110.7	17.3	24.5
27	94.1	0.2	111.3	17.2	24.3
28	93.8	0.1	111.3	17.5	24.8
29	93.7	0.2	111.3	17.6	24.9

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17:04:17 \DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7k**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 25**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	111.3	17.5	24.8
31	93.8	0.2	111.3	17.5	24.8
32	93.9	0.3	111.3	17.4	24.6
33	93.8	0.3	111.3	17.5	24.8
34	93.6	0.0	111.3	17.7	25.1
35	93.7	0.0	111.3	17.6	24.9
36	93.7	0.3	111.3	17.6	24.9
37	93.2	0.0	111.3	18.1	25.7
38	93.2	0.3	111.3	18.1	25.7
39	93.3	0.0	111.3	18.0	25.5
40	93.3	0.3	111.3	18.0	25.5
41	93.4	0.3	111.3	17.9	25.4
42	93.6	0.0	111.3	17.7	25.1
43	93.6	0.0	111.3	17.7	25.1
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.7
50	93.8	0.0	111.3	17.5	24.8
51*	N/A	N/A	112.3	N/A	N/A

\* **Boundary Condition**

 Minimum Pressure

**Table 71**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 32**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	110.7	17.2	24.4
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.7	17.1	24.2
5	93.9	0.0	110.7	16.8	23.8
6	93.7	0.2	110.7	17.0	24.1
7	93.8	0.3	110.7	16.9	24.0
8	94.0	0.0	110.8	16.8	23.8
9	93.8	0.0	110.9	17.1	24.3
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.2
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.4
19	93.3	0.0	111.3	18.0	25.5
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.7
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.7	16.6	23.5
28	93.8	0.1	110.6	16.8	23.8
29	93.7	0.2	110.6	16.9	24.0

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 71**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 32**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	110.8	17.0	24.0
31	93.8	0.2	110.2	16.4	23.3
32	93.9	167.3	109.6	15.7	22.3
33	93.8	0.3	110.2	16.4	23.2
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	24.0
36	93.7	0.3	110.7	17.0	24.1
37	93.2	0.0	111.1	17.9	25.4
38	93.2	0.3	111.1	17.9	25.3
39	93.3	0.0	111.0	17.7	25.1
40	93.3	0.3	110.9	17.6	25.0
41	93.4	0.3	110.8	17.4	24.7
42	93.6	0.0	110.7	17.1	24.3
43	93.6	0.0	110.7	17.1	24.3
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	23.9
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7m**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 36**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.0	110.2	16.7	23.6
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.3	16.5	23.4
4	93.6	0.3	110.4	16.8	23.8
5	93.9	0.0	110.5	16.6	23.6
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.8
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.6	17.0	24.1
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.3

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7m**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 36**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	110.9	17.1	24.2
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.8
33	93.8	0.3	110.6	16.8	23.9
34	93.6	0.0	110.6	17.0	24.0
35	93.7	0.0	110.5	16.8	23.9
36	93.7	167.3	109.3	15.6	22.1
37	93.2	0.0	111.0	17.8	25.2
38	93.2	0.3	110.8	17.6	24.9
39	93.3	0.0	110.6	17.3	24.5
40	93.3	0.3	110.4	17.1	24.3
41	93.4	0.3	110.1	16.7	23.7
42	93.6	0.0	109.9	16.3	23.2
43	93.6	0.0	110.1	16.5	23.4
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

\* **Boundary Condition**

Minimum Pressure

**Table 7n**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 39**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	110.6	17.1	24.2
2	93.6	0.3	110.7	17.1	24.2
3	93.8	0.0	110.7	16.9	24.0
4	93.6	0.3	110.8	17.2	24.4
5	93.9	0.0	111.0	17.1	24.2
6	93.7	0.2	111.0	17.3	24.5
7	93.8	0.3	111.1	17.3	24.5
8	94.0	0.0	111.1	17.1	24.3
9	93.8	0.0	111.2	17.4	24.7
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.3
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.1	17.7	25.1
22	93.6	0.2	111.1	17.5	24.8
23	93.6	0.3	111.0	17.4	24.7
24	93.8	0.2	111.4	17.6	24.9
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.1	17.0	24.1
28	93.8	0.1	111.1	17.3	24.5
29	93.7	0.2	111.1	17.4	24.7

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17:04:17 \DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7n**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 39**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	111.1	17.3	24.5
31	93.8	0.2	111.1	17.3	24.5
32	93.9	0.3	111.0	17.1	24.3
33	93.8	0.3	111.0	17.2	24.4
34	93.6	0.0	111.0	17.4	24.6
35	93.7	0.0	111.0	17.3	24.5
36	93.7	0.3	110.6	16.9	24.0
37	93.2	0.0	109.9	16.7	23.7
38	93.2	0.3	109.1	15.9	22.6
39	93.3	167.0	108.3	15.0	21.3
40	93.3	0.3	108.8	15.5	21.9
41	93.4	0.3	109.8	16.4	23.2
42	93.6	0.0	110.3	16.7	23.7
43	93.6	0.0	110.5	16.9	24.0
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.7
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	111.0	17.2	24.3
51*	N/A	N/A	112.3	N/A	N/A

\* **Boundary Condition**

Minimum Pressure

**Table 7o**  
**Post Confiration Condition**  
**Max Daily Demand and Fire Flow at Node 41**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	110.1	16.6	23.5
2	93.6	0.3	110.3	16.7	23.6
3	93.8	0.0	110.4	16.6	23.5
4	93.6	0.3	110.5	16.9	24.0
5	93.9	0.0	110.7	16.8	23.9
6	93.7	0.2	110.8	17.1	24.3
7	93.8	0.3	110.9	17.1	24.3
8	94.0	0.0	111.0	17.0	24.2
9	93.8	0.0	111.1	17.3	24.6
10	94.1	0.0	111.4	17.3	24.5
11	94.5	0.0	111.7	17.2	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.2
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	111.0	17.6	25.0
22	93.6	0.2	110.9	17.3	24.6
23	93.6	0.3	110.8	17.2	24.4
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.2	17.8	25.3
27	94.1	0.2	111.0	16.9	24.0
28	93.8	0.1	111.0	17.2	24.4
29	93.7	0.2	111.0	17.3	24.5

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7o**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 41**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	111.0	17.2	24.4
31	93.8	0.2	110.9	17.1	24.3
32	93.9	0.3	110.9	17.0	24.0
33	93.8	0.3	110.8	17.0	24.1
34	93.6	0.0	110.8	17.2	24.4
35	93.7	0.0	110.7	17.0	24.2
36	93.7	0.3	110.2	16.5	23.4
37	93.2	0.0	110.6	17.4	24.6
38	93.2	0.3	110.1	16.9	24.0
39	93.3	0.0	109.7	16.4	23.2
40	93.3	0.3	109.4	16.1	22.8
41	93.4	167.3	108.7	15.3	21.7
42	93.6	0.0	109.7	16.1	22.8
43	93.6	0.0	110.0	16.4	23.2
44	94.1	0.0	111.8	17.7	25.0
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.7	16.9	24.0
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

**Table 7p**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 43**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	108.7	15.2	21.5
2	93.6	0.3	109.1	15.5	22.0
3	93.8	0.0	109.5	15.7	22.2
4	93.6	0.3	109.9	16.3	23.1
5	93.9	0.0	110.5	16.6	23.5
6	93.7	0.2	110.6	16.9	24.0
7	93.8	0.3	110.8	17.0	24.0
8	94.0	0.0	110.9	16.9	24.0
9	93.8	0.0	111.0	17.2	24.4
10	94.1	0.0	111.3	17.2	24.4
11	94.5	0.0	111.6	17.1	24.3
12	94.5	0.3	111.6	17.1	24.3
13	94.4	0.5	111.6	17.2	24.4
14	94.0	0.3	111.6	17.6	24.9
15	93.8	0.3	111.5	17.7	25.1
16	93.7	0.0	111.5	17.8	25.3
17	93.6	0.3	111.5	17.9	25.4
18	93.6	2.6	111.5	17.9	25.3
19	93.3	0.0	111.2	17.9	25.4
20	93.5	0.1	111.2	17.7	25.1
21	93.4	0.2	110.9	17.5	24.9
22	93.6	0.2	110.8	17.2	24.4
23	93.6	0.3	110.7	17.1	24.2
24	93.8	0.2	111.3	17.5	24.8
25	93.8	0.3	111.3	17.5	24.8
26	93.4	0.2	111.3	17.9	25.3
27	94.1	0.2	110.9	16.8	23.8
28	93.8	0.1	110.9	17.1	24.2
29	93.7	0.2	110.9	17.2	24.4

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7p**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 43**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	110.9	17.1	24.3
31	93.8	0.2	110.8	17.0	24.1
32	93.9	0.3	110.7	16.8	23.9
33	93.8	0.3	110.7	16.9	23.9
34	93.6	0.0	110.6	17.0	24.1
35	93.7	0.0	110.6	16.9	23.9
36	93.7	0.3	110.2	16.5	23.3
37	93.2	0.0	110.9	17.7	25.1
38	93.2	0.3	110.7	17.5	24.8
39	93.3	0.0	110.5	17.2	24.4
40	93.3	0.3	110.3	17.0	24.1
41	93.4	0.3	110.0	16.6	23.5
42	93.6	0.0	109.8	16.2	23.0
43	93.6	167.0	108.4	14.8	20.9
44	94.1	0.0	111.8	17.7	25.1
45	94.3	5.1	111.8	17.5	24.8
46	94.0	0.0	111.8	17.8	25.2
47	94.0	0.1	111.8	17.8	25.2
48	94.5	0.0	111.8	17.3	24.5
49	94.4	0.2	111.8	17.4	24.6
50	93.8	0.0	110.5	16.7	23.7
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7q**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 46**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7q**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 46**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	167.0	110.2	16.2	22.9
47	94.0	0.1	110.3	16.3	23.2
48	94.5	0.0	110.5	16.0	22.7
49	94.4	0.2	110.7	16.3	23.1
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

\* **Boundary Condition**

Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7r**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 48**

Node	Elevation	Demand (LPS)	Head (m)	Pressure	
	(m)			(m)	(PSI)
1	93.5	0.0	111.7	18.2	25.8
2	93.6	0.3	111.7	18.1	25.6
3	93.8	0.0	111.7	17.9	25.4
4	93.6	0.3	111.7	18.1	25.7
5	93.9	0.0	111.7	17.8	25.2
6	93.7	0.2	111.7	18.0	25.5
7	93.8	0.3	111.7	17.9	25.4
8	94.0	0.0	111.7	17.7	25.1
9	93.8	0.0	111.7	17.9	25.4
10	94.1	0.0	111.8	17.7	25.1
11	94.5	0.0	111.9	17.4	24.7
12	94.5	0.3	111.9	17.4	24.6
13	94.4	0.5	111.8	17.4	24.7
14	94.0	0.3	111.8	17.8	25.2
15	93.8	0.3	111.7	17.9	25.4
16	93.7	0.0	111.7	18.0	25.5
17	93.6	0.3	111.7	18.1	25.6
18	93.6	2.6	111.6	18.0	25.6
19	93.3	0.0	111.7	18.4	26.0
20	93.5	0.1	111.7	18.2	25.8
21	93.4	0.2	111.7	18.3	25.9
22	93.6	0.2	111.7	18.1	25.6
23	93.6	0.3	111.7	18.1	25.6
24	93.8	0.2	111.8	18.0	25.5
25	93.8	0.3	111.7	17.9	25.4
26	93.4	0.2	111.7	18.3	25.9
27	94.1	0.2	111.7	17.6	25.0
28	93.8	0.1	111.7	17.9	25.4
29	93.7	0.2	111.7	18.0	25.5

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 7r**  
**Post Confirnation Condition**  
**Max Daily Demand and Fire Flow at Node 48**

Node	Elevation (m)	Demand (LPS)	Head (m)	Pressure (m)	Pressure (PSI)
30	93.8	0.2	111.7	17.9	25.4
31	93.8	0.2	111.7	17.9	25.4
32	93.9	0.3	111.7	17.8	25.2
33	93.8	0.3	111.7	17.9	25.4
34	93.6	0.0	111.7	18.1	25.6
35	93.7	0.0	111.7	18.0	25.5
36	93.7	0.3	111.7	18.0	25.5
37	93.2	0.0	111.7	18.5	26.2
38	93.2	0.3	111.7	18.5	26.2
39	93.3	0.0	111.7	18.4	26.0
40	93.3	0.3	111.7	18.4	26.1
41	93.4	0.3	111.7	18.3	25.9
42	93.6	0.0	111.7	18.1	25.6
43	93.6	0.0	111.7	18.1	25.6
44	94.1	0.0	111.6	17.5	24.8
45	94.3	5.1	110.8	16.5	23.4
46	94.0	0.0	110.6	16.6	23.5
47	94.0	0.1	109.9	15.9	22.5
48	94.5	167.0	109.1	14.6	20.6
49	94.4	0.2	110.0	15.6	22.2
50	93.8	0.0	111.7	17.9	25.4
51*	N/A	N/A	112.3	N/A	N/A

\* Boundary Condition

Minimum Pressure

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 24, 2016 17\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\MaxDailyFF.xls

**Table 8**  
**Post Configuration Condition**  
**Peak Hour Check**

<b>Node</b>	<b>Elevation (m)</b>	<b>Demand (LPS)</b>	<b>Head (m)</b>	<b>Pressure</b>	
				<b>(m)</b>	<b>(PSI)</b>
1	93.5	0.00	137.87	44.4	62.9
2	93.6	0.60	137.87	44.3	62.8
3	93.8	0.00	137.87	44.1	62.5
4	93.6	0.60	137.87	44.3	62.8
5	93.9	0.00	137.87	44.0	62.3
6	93.7	0.49	137.87	44.2	62.6
7	93.8	0.60	137.87	44.1	62.5
8	94.0	0.00	137.87	43.9	62.2
9	93.8	0.00	137.87	44.1	62.5
10	94.1	0.00	137.87	43.8	62.1
11	94.5	0.00	137.88	43.4	61.5
12	94.5	0.60	137.88	43.4	61.5
13	94.4	0.98	137.88	43.5	61.7
14	94.0	0.60	137.87	43.9	62.2
15	93.8	0.60	137.87	44.1	62.5
16	93.7	0.00	137.87	44.2	62.6
17	93.6	0.74	137.87	44.3	62.8
18	93.6	5.61	137.87	44.3	62.8
19	93.3	0.00	137.87	44.6	63.2
20	93.5	0.25	137.87	44.4	62.9
21	93.4	0.38	137.87	44.5	63.1
22	93.6	0.38	137.87	44.3	62.8
23	93.6	0.56	137.87	44.3	62.8
24	93.8	0.49	137.87	44.1	62.5
25	93.8	0.60	137.87	44.1	62.5
26	93.4	0.38	137.87	44.5	63.1
27	94.1	0.42	137.87	43.8	62.1
28	93.8	0.20	137.87	44.1	62.5
29	93.7	0.38	137.87	44.2	62.6

Prepared By:

NOVATECH ENGINEERING CONSULTANTS LTD.

Date: November 20, 2016 \\117\DATA\Calculations\Sewer Calcs\Water\20161110\Post Configuration\PeakHour.xls

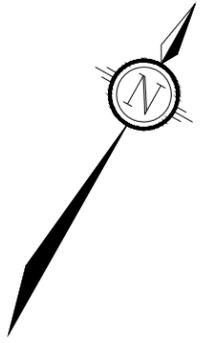
<b>Table 8</b>					
<b>Post Configuration Condition</b>					
<b>Peak Hour Check</b>					
<b>Node</b>	<b>Elevation</b> (m)	<b>Demand</b> (LPS)	<b>Head</b> (m)	<b>Pressure</b>	
				(m)	(PSI)
30	93.8	0.38	137.87	44.1	62.5
31	93.8	0.38	137.87	44.1	62.5
32	93.9	0.67	137.87	44.0	62.3
33	93.8	0.56	137.87	44.1	62.5
34	93.6	0.00	137.87	44.3	62.8
35	93.7	0.00	137.87	44.2	62.6
36	93.7	0.60	137.87	44.2	62.6
37	93.2	0.00	137.87	44.7	63.3
38	93.2	0.60	137.87	44.7	63.3
39	93.3	0.00	137.87	44.6	63.2
40	93.3	0.60	137.87	44.6	63.2
41	93.4	0.60	137.87	44.5	63.1
42	93.6	0.00	137.87	44.3	62.8
43	93.6	0.00	137.87	44.3	62.8
44	94.1	0.00	137.87	43.8	62.1
45	94.3	11.23	137.87	43.6	61.8
46	94.0	0.00	137.87	43.9	62.2
47	94.0	0.25	137.87	43.9	62.2
48	94.5	0.00	137.87	43.4	61.5
49	94.4	0.49	137.87	43.5	61.6
50	93.8	0.00	137.87	44.1	62.5
51*	N/A	N/A	137.9	N/A	N/A

**\* Boundary Condition**

Minimum Pressure

## **Appendix D**

### SWM Calculations & PCSWMM Model



AREA REQUIRING QUANTITY CONTROL TO MEET ALLOWABLE RELEASE RATE TO FRASER-CLARKE DRAIN

FUTURE SCHOOL SITE

KENNEDY-BURNETT SWMF

STRANDHERD DR

SITE

KEYPLAN

REALIGNED FRASER-CLARKE DRAIN

FRASER-CLARKE DRAIN

JOCK RIVER

**LEGEND**

- MAJOR SYSTEM TO KENNEDY BURNETT
- LANDS TRIBUTARY TO KENNEDY BURNETT SWM FACILITY
- LANDS TRIBUTARY TO FRASER CLARKE DRAIN
- LANDS TRIBUTARY TO FRASER CLARKE DRAIN (REQUIRING QUANTITY CONTROL)
- LANDS TRIBUTARY TO JOCK RIVER
- EXISTING WATERCOURSE
- PROPOSED ROAD
- PROPOSED STORM SEWER
- PROPOSED STORM SEWER (SUBMERGED)
- PROPOSED OUTLET
- PROPOSED MAINTENANCE HOLE
- PROPOSED HYDRO DYNAMIC SEPARATOR (HDS)
- MAJOR OVERLAND FLOW DIRECTION



Engineers, Planners & Landscape Architects  
Suite 200, 240 Michael Cowpland Drive  
Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643  
Facsimile (613) 254-5867  
Website www.novatech-eng.com

**KENNEDY-BURNETT SWMF  
SERVICING OPTIONS**

**OPTION 4: HYBRID EXPANDED  
K-B SWMF / HDS UNITS**

SCALE 1 : 7500

DATE DEC 2016 JOB 113221 FIGURE FIG-6

M:\2013\113221\CAD\Design\Figures\SWM\113221-FIGs 5-6.dwg, FIG-6 (2), Dec 12, 2016 - 2:35pm, kbanks

CUT11V17 DWG 270mm X 132mm

**Burnett Lands - 3370 Greenbank Road**  
**Post-Development Model Parameters**



Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Impervious (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
<b>Burnett Lands - Claridge</b>						
A-01	0.235	0.65	64%	50%	43	0.75%
A-02	0.249	0.65	64%	50%	36	0.75%
A-03	0.127	0.65	64%	75%	36	0.75%
A-04	0.302	0.65	64%	50%	34	0.75%
A-05	0.275	0.65	64%	75%	37	0.75%
A-06	0.274	0.65	64%	60%	37	0.75%
A-07	0.289	0.65	64%	50%	39	0.75%
A-08	0.246	0.65	64%	75%	41	0.75%
A-09	0.081	0.65	64%	50%	23	0.75%
A-10	0.187	0.65	64%	50%	37	0.75%
A-11	0.225	0.65	64%	50%	25	0.75%
A-12	0.036	0.65	64%	100%	24	0.75%
A-13	0.200	0.65	64%	50%	24	0.75%
A-14	0.195	0.65	64%	50%	23	0.75%
A-15	0.105	0.65	64%	100%	30	0.75%
A-16	0.128	0.65	64%	100%	32	0.75%
A-17	0.246	0.65	64%	50%	49	0.75%
A-18	0.272	0.65	64%	50%	32	0.75%
A-19	0.122	0.65	64%	100%	31	0.75%
A-20	0.130	0.65	64%	100%	29	0.75%
A-21	0.217	0.65	64%	50%	27	0.75%
A-22	0.060	0.65	64%	100%	24	0.75%
A-23	0.457	0.65	64%	75%	46	0.75%
A-24	0.209	0.65	64%	75%	28	0.75%
A-25	0.214	0.65	64%	75%	29	0.75%
A-26	0.448	0.65	64%	70%	45	0.75%
A-27	0.190	0.65	64%	75%	32	0.75%
A-28	0.119	0.65	64%	0%	18	0.75%
A-29	0.231	0.65	64%	60%	46	0.75%
A-30	0.498	0.65	64%	50%	71	0.75%
A-31	0.061	0.65	64%	0%	20	0.75%
A-32	0.450	0.65	64%	75%	56	0.75%
A-33	0.816	0.65	64%	50%	117	0.75%
A-34	0.773	0.65	64%	0%	55	0.75%
A-35	0.983	0.65	64%	0%	70	0.75%
A-36	0.450	0.65	64%	0%	60	0.75%
A-37	0.372	0.65	64%	0%	53	0.75%

**Burnett Lands - 3370 Greenbank Road**  
**Post-Development Model Parameters**



Area ID	Catchment Area (ha)	Runoff Coefficient	Percent Impervious (%)	No Depression (%)	Equivalent Width (m)	Average Slope (%)
<b>Street B - Caivan Lands</b>						
B-01	0.092	0.65	64%	50%	28	0.75%
B-02	0.149	0.65	64%	0%	23	0.75%
B-03	0.097	0.65	64%	10%	32	0.75%
B-04	0.825	0.65	64%	50%	110	0.75%
B-05	0.064	0.65	64%	100%	27	0.75%
B-06	0.076	0.65	64%	0%	19	0.75%
B-07	0.127	0.65	64%	40%	32	0.75%
B-08	0.225	0.65	64%	50%	32	0.75%
B-09	0.245	0.65	64%	50%	34	0.75%
B-10	1.115	0.30	14%	0%	149	0.75%
B-11	0.049	0.65	64%	0%	13	0.75%
B-12	0.088	0.65	64%	50%	20	0.75%
B-13	0.097	0.65	64%	50%	22	0.75%
B-14	0.320	0.65	64%	50%	73	0.75%
B-15	0.120	0.65	64%	50%	27	0.75%
B-16	0.136	0.65	64%	50%	31	0.75%
B-17	0.247	0.65	64%	50%	33	0.75%
B-18	0.121	0.65	64%	50%	28	0.75%
B-19	0.107	0.65	64%	50%	22	0.75%
CaivanLands	8.101	0.65	64%	50%	200	0.50%

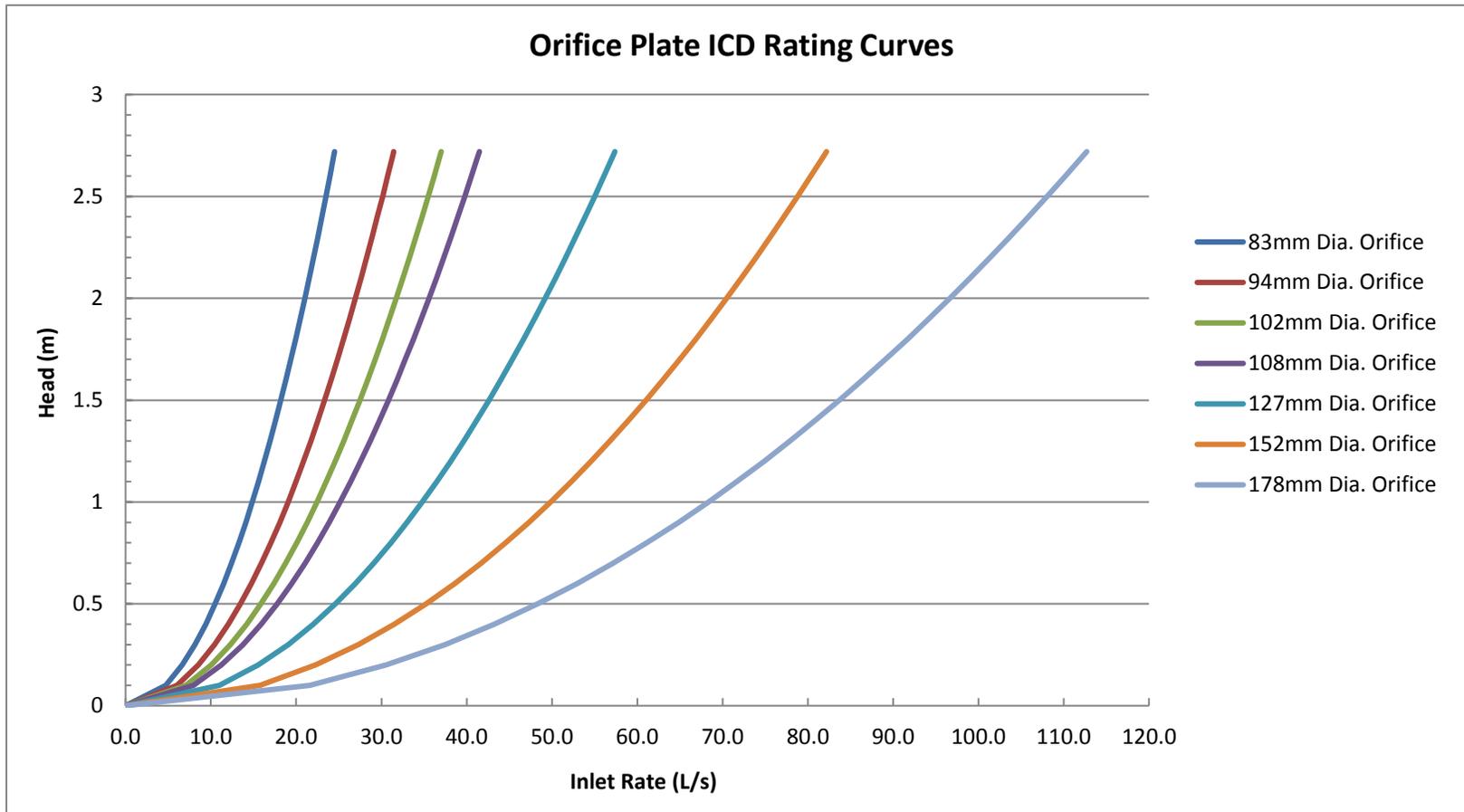
**Burnett Lands - 3370 Greenbank Road**  
**Inlet Control Device Parameters**

Structure	ICD Size & Inlet Rate			5-year Peak Flow* (L/s)
	Diameter (mm)	Max Head (m)	5-year Inlet Capture (L/s)	
<b>Burnett Lands - Claridge</b>				
CB01-02	127	1.14	37.09	50.15
CB03-04	127	1.23	38.53	48.26
CB05-06	152	1.12	52.83	58.43
CB07-08	127	1.14	37.09	50.69
CB09-10	127	1.04	35.42	4.80
CB11-12	108	1.19	27.40	54.07
CB13-14	102	1.15	24.05	27.65
CB15-16	108	1.15	26.93	56.86
CB17-18	102	1.15	24.05	49.60
CB19	83	1.16	15.99	17.43
CB20-21	156	1.12	55.60	80.41
CB22	108	1.15	26.93	37.97
CB23	108	1.15	26.93	38.91
CB24-25	127	1.14	37.09	44.09
CB26-27	178	1.11	72.03	86.47
CB28	127	1.15	37.25	47.94
CB29	94	1.15	20.46	29.12
CB30-31	127	1.14	37.09	38.46
CB32-33	127	1.14	37.09	44.92
CB34-35	127	1.14	37.09	44.34
CB40-41	178	1.11	72.03	87.24
CB42-43	152	1.12	52.83	52.25
CB44-45	127	1.23	38.53	50.39
CB59	83	1.16	15.99	6.67
CB60	102	1.15	24.05	24.03
CB61	83	1.16	15.99	11.11
CB62	108	1.15	26.93	22.56
CB63	108	1.15	26.93	23.67
CB64	102	1.15	24.05	19.43
CB66	83	1.16	15.99	13.15
CB67-68-69	178	1.17	73.95	85.77

**Burnett Lands - 3370 Greenbank Road**  
**Inlet Control Device Parameters**

Structure	ICD Size & Inlet Rate			5-year Peak Flow* (L/s)
	Diameter (mm)	Max Head (m)	5-year Inlet Capture (L/s)	
<b>Street B - Caivan Lands</b>				
CB36	83	1.46	17.94	18.08
CB37	102	1.45	27.01	29.14
CB39	83	1.46	17.94	19.63
CB46-47	83	1.46	17.94	15.06
CB48-49	83	1.44	17.82	24.54
CB50-51	127	1.44	41.70	42.30
CB52-53	178	1.41	81.18	92.98
CB54	108	1.45	30.25	9.67
CB55-56	178	1.41	81.18	91.54
CB57-58	178	1.31	78.25	50.85
CB65	83	1.46	17.94	11.90

*\*From SSA Model, 5-year 4-hour Chicago storm distribution*



**Burnett Lands - 3370 Greenbank Road**  
**HGL Elevations**

Manhole ID	MH Invert Elevation (m)	T/G Elevation (m)	HGL Elev - 5yr 4hr (100-yr Fixed) (m)	HGL Elev - 100yr 4hr (5-yr Fixed) (m)	HGL Elev - 100yr 4hr +20% (5-yr Fixed) (m)	Design USF (m)	Clearance 5yr 4hr (100-yr Fixed) (m)	Clearance 100yr 4hr (5-yr Fixed) (m)	Clearance 100yr 4hr +20% (5-yr Fixed) (m)
<b>Burnett Lands - Claridge</b>									
102 (STM)	90.52	93.78	92.04	91.54	91.59	92.62	0.58	1.08	1.03
104 (STM)	90.37	93.93	92.03	91.54	91.59	92.60	0.57	1.06	1.01
106 (STM)	90.27	93.87	92.04	91.53	91.59	92.60	0.56	1.07	1.01
108 (STM)	90.10	93.75	92.01	91.49	91.54	92.51	0.50	1.02	0.97
110 (STM)	89.94	93.46	92.00	91.42	91.46	92.43	0.43	1.01	0.97
304 (STM)	90.56	93.67	91.72	91.15	91.17	92.25	0.53	1.10	1.08
306 (STM)	90.35	93.58	91.72	91.14	91.17	92.22	0.50	1.08	1.05
308 (STM)	90.11	93.41	91.70	91.12	91.15	92.17	0.47	1.05	1.02
310 (STM)	89.74	93.46	91.66	91.08	91.10	92.06	0.40	0.98	0.96
312 (STM)	89.26	93.36	91.65	91.06	91.09	92.06	0.41	1.00	0.97
314 (STM)	89.19	92.61	91.58	90.98	91.00	-	-	-	-
316 (STM)	89.00	93.09	91.49	90.88	90.89	-	-	-	-
324 (STM)	88.96	93.34	91.41	90.79	90.79	-	-	-	-
326 (STM)	89.95	93.38	91.35	90.71	90.72	-	-	-	-
328 (STM)	88.91	92.16	91.30	90.66	90.66	-	-	-	-
330 (STM)	88.94	92.53	91.38	90.75	90.75	-	-	-	-
402 (STM)	90.62	93.82	92.02	91.49	91.53	92.40	0.38	0.91	0.87
404 (STM)	90.37	93.80	92.01	91.48	91.52	92.45	0.44	0.97	0.93
406 (STM)	90.46	93.71	92.04	91.53	91.67	92.55	0.51	1.02	0.88
408 (STM)	90.36	93.80	92.04	91.53	91.60	92.55	0.51	1.02	0.95
608 (STM)	90.25	93.71	91.99	91.46	91.50	92.55	0.56	1.09	1.05
610 (STM)	89.58	93.30	91.79	91.24	91.27	92.38	0.59	1.14	1.11
612 (STM)	89.82	93.93	91.64	91.06	91.08	-	-	-	-
614 (STM)	89.92	93.76	91.71	91.15	91.16	-	-	-	-
616 (STM)	90.23	94.27	91.72	91.15	91.17	92.65	0.93	1.50	1.48
618 (STM)	90.03	93.65	91.80	91.24	91.27	92.25	0.45	1.01	0.98
902 (STM)	90.34	93.98	91.82	91.28	91.31	92.55	0.73	1.27	1.24
904 (STM)	90.29	93.76	91.83	91.28	91.31	92.35	0.52	1.07	1.04
906 (STM)	90.12	93.53	91.82	91.27	91.31	92.35	0.53	1.08	1.04
908 (STM)	89.97	93.57	91.81	91.25	91.29	92.25	0.44	1.00	0.96
<b>Street B - Caivan Lands</b>									
200 (STM)	91.55	94.54	92.38	92.34	92.37	-	-	-	-
202 (STM)	91.40	94.14	92.37	92.33	92.36	-	-	-	-
204 (STM)	91.08	93.94	92.26	91.92	91.95	92.55	0.29	0.63	0.60
206 (STM)	90.84	93.62	92.14	91.69	91.70	92.45	0.31	0.76	0.75
208 (STM)	90.69	93.82	92.06	91.53	91.54	92.45	0.39	0.92	0.91
210 (STM)	90.53	93.52	91.89	91.33	91.34	92.45	0.56	1.12	1.11
212 (STM)	90.40	93.73	91.80	91.21	91.22	92.40	0.60	1.19	1.18
214 (STM)	90.30	93.47	91.73	91.12	91.12	92.25	0.52	1.13	1.13
216 (STM)	90.01	93.37	91.65	90.94	90.94	92.25	0.60	1.31	1.31

**Burnett Lands - 3370 Greenbank Road**  
**Ponding in Road Calculations**

Structure	T/G (m)	Max. Static Ponding (Spill Depth)		5-yr Event				100-yr Event (4hr)					100-yr Event (+20%) (4hr)			
		Elev. (m)	Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)	Spill Flow (L/s)	Elev. (m)	Total Dynamic Depth (m)	Cascading Flow? (Y/N)	Cascade Depth (m)
<b>Burnett Lands - Claridge</b>																
CB01-02	93.25	93.44	0.19	93.30	0.05	N	0.00	93.41	0.16	N	0.00	0	93.46	0.21	Y	0.02
CB03-04	93.18	93.44	0.26	93.22	0.04	N	0.00	93.42	0.24	N	0.00	0	93.46	0.28	Y	0.02
CB05-06	93.05	93.34	0.29	93.06	0.01	N	0.00	93.19	0.14	N	0.00	0	93.24	0.19	N	0.00
CB07-08	92.92	93.34	0.42	92.96	0.04	N	0.00	93.08	0.16	N	0.00	0	93.09	0.17	N	0.00
CB09-10	93.01	93.34	0.33	93.06	0.05	N	0.00	93.26	0.25	N	0.00	0	93.31	0.30	N	0.00
CB15-16	93.20	93.37	0.17	93.32	0.12	N	0.00	93.43	0.23	Y	0.06	56	93.46	0.26	Y	0.09
CB17-18	93.51	93.76	0.25	93.66	0.15	N	0.00	93.71	0.20	N	0.00	0	93.73	0.22	N	0.00
CB19	93.44	93.64	0.20	93.32	0.00	N	0.00	93.56	0.12	N	0.00	0	93.62	0.18	N	0.00
CB20-21	93.34	93.60	0.26	93.42	0.08	N	0.00	93.56	0.22	N	0.00	0	93.62	0.28	Y	0.02
CB22	93.49	93.64	0.15	93.54	0.05	N	0.00	93.64	0.15	N	0.00	0	93.67	0.18	Y	0.03
CB23	93.28	93.51	0.23	93.30	0.02	N	0.00	93.37	0.09	N	0.00	0	93.42	0.14	N	0.00
CB24-25	93.25	93.48	0.23	93.25	0.00	N	0.00	93.35	0.10	N	0.00	0	93.40	0.15	N	0.00
CB26-27	93.08	93.36	0.28	93.10	0.02	N	0.00	93.24	0.16	N	0.00	0	93.29	0.21	N	0.00
CB28	92.99	93.34	0.35	92.11	0.00	N	0.00	93.08	0.09	N	0.00	0	93.17	0.18	N	0.00
CB29	92.98	93.34	0.36	93.01	0.03	N	0.00	93.08	0.10	N	0.00	0	93.17	0.19	N	0.00
CB30-31	93.31	93.58	0.27	93.49	0.18	N	0.00	93.60	0.29	Y	0.02	77	93.61	0.30	Y	0.03
CB40-41	93.35	93.65	0.30	93.39	0.04	N	0.00	93.52	0.17	N	0.00	0	93.58	0.23	N	0.00
CB42-43	93.47	93.72	0.25	93.37	0.00	N	0.00	93.63	0.16	N	0.00	0	93.70	0.23	N	0.00
CB44-45	93.62	93.87	0.25	93.68	0.06	N	0.00	93.77	0.15	N	0.00	0	93.78	0.16	N	0.00
CB59	93.70	93.91	0.21	92.75	0.00	N	0.00	93.14	0.00	N	0.00	0	93.39	0.00	N	0.00
CB60	93.50	93.80	0.30	93.27	0.00	N	0.00	93.81	0.31	Y	0.01	1	93.90	0.40	Y	0.10
CB61	93.50	93.78	0.28	92.90	0.00	N	0.00	93.64	0.14	N	0.00	0	93.68	0.18	N	0.00
CB62	93.49	93.78	0.29	93.16	0.00	N	0.00	93.77	0.28	N	0.00	0	93.79	0.30	Y	0.01
CB63	93.55	93.84	0.29	93.29	0.00	N	0.00	93.85	0.30	Y	0.01	2	93.85	0.30	Y	0.01
CB64	93.66	93.95	0.29	93.27	0.00	N	0.00	93.91	0.25	N	0.00	0	93.96	0.30	Y	0.01
CB66	93.90	94.01	0.11	93.35	0.00	N	0.00	93.97	0.07	N	0.00	0	94.01	0.11	N	0.00
CB67-68-69	93.66	94.15	0.49	93.70	0.04	N	0.00	93.83	0.17	N	0.00	0	93.88	0.22	N	0.00
<b>Street B - Caivan Lands</b>																
CB39	93.78	93.86	0.08	93.82	0.04	N	0.00	93.92	0.14	Y	0.06	52	93.93	0.15	Y	0.07
CB46-47	93.67	93.77	0.10	93.43	0.00	N	0.00	93.84	0.17	Y	0.07	52	93.90	0.23	Y	0.13
CB48-49	93.72	93.86	0.14	93.79	0.07	N	0.00	93.87	0.15	Y	0.01	9	93.90	0.18	Y	0.04
CB50-51	93.59	93.84	0.25	93.62	0.03	N	0.00	93.74	0.15	N	0.00	0	93.80	0.21	N	0.00
CB52-53	93.52	93.76	0.24	93.60	0.08	N	0.00	93.77	0.25	Y	0.01	4	93.81	0.29	Y	0.05
CB55-56	93.38	93.67	0.29	93.42	0.04	N	0.00	93.59	0.21	N	0.00	0	93.66	0.28	N	0.00
CB57-58	93.30	93.56	0.26	93.30	0.00	N	0.00	93.47	0.17	N	0.00	0	93.53	0.23	N	0.00

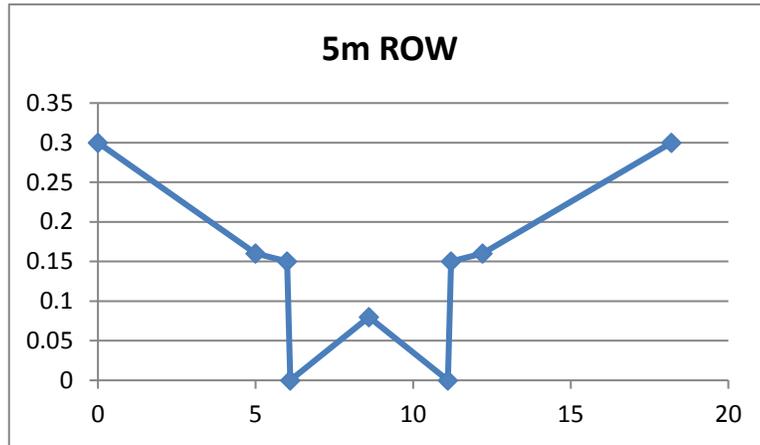
# Burnett Lands - 3370 Greenbank Road

## Roadway Cross-Sections



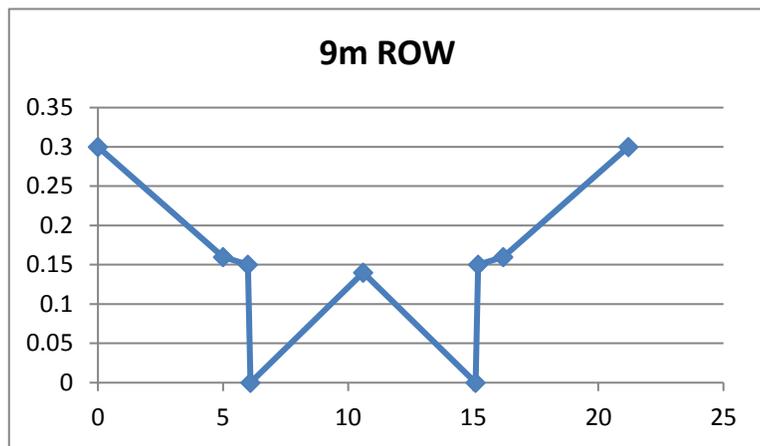
### 5m - ROW

0	0.3
5	0.16
6	0.15
6.1	0
8.6	0.08
11.1	0
11.2	0.15
12.2	0.16
18.2	0.3



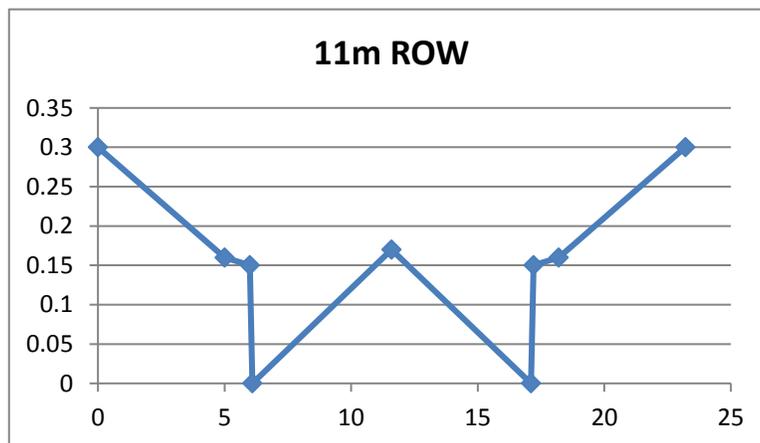
### 9m - ROW

0	0.3
5	0.16
6	0.15
6.1	0
10.6	0.14
15.1	0
15.2	0.15
16.2	0.16
21.2	0.3



### 11m - ROW

0	0.3
5	0.16
6	0.15
6.1	0
11.6	0.17
17.1	0
17.2	0.15
18.2	0.16
23.2	0.3



**Burnett Lands - 3370 Greenbank Road**  
**Design Storm Time Series Data**  
**Chicago Design Storms**

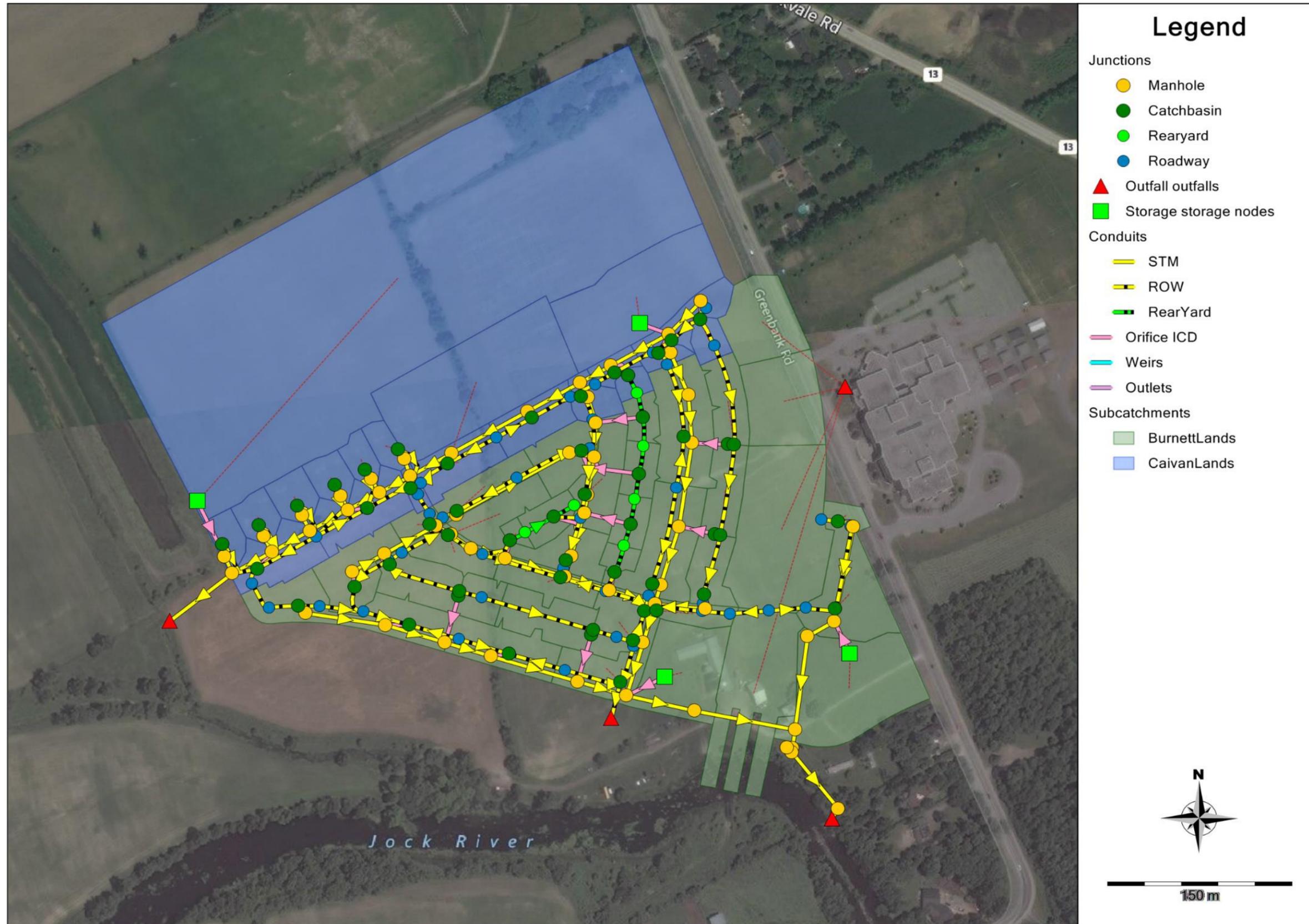


C25mm-4.stm		C2-4.stm		C5-4.stm	
Duration	Intensity	Duration	Intensity	Duration	Intensity
min	mm/hr	min	mm/hr	min	mm/hr
0:00	0	0:00	0	0:00	0
0:10	1.34	0:10	1.98	0:10	2.49
0:20	1.49	0:20	2.23	0:20	2.77
0:30	1.69	0:30	2.58	0:30	3.14
0:40	1.96	0:40	3.06	0:40	3.62
0:50	2.33	0:50	3.81	0:50	4.31
1:00	2.91	1:00	5.1	1:00	5.37
1:10	3.91	1:10	7.91	1:10	7.19
1:20	6.1	1:20	19.04	1:20	11.14
1:30	14.53	1:30	76.81	1:30	26.25
1:40	58.72	1:40	23.64	1:40	104.19
1:50	17.11	1:50	11.91	1:50	30.86
2:00	8.32	2:00	7.98	2:00	15.15
2:10	5.5	2:10	6.03	2:10	10.07
2:20	4.13	2:20	4.87	2:20	7.58
2:30	3.32	2:30	4.1	2:30	6.11
2:40	2.79	2:40	3.55	2:40	5.14
2:50	2.41	2:50	3.14	2:50	4.45
3:00	2.12	3:00	2.82	3:00	3.93
3:10	1.9	3:10	2.57	3:10	3.53
3:20	1.73	3:20	2.35	3:20	3.21
3:30	1.58	3:30	2.18	3:30	2.94
3:40	1.46	3:40	2.03	3:40	2.72
3:50	1.36	3:50	1.9	3:50	2.53
4:00	1.27	4:00	1.79	4:00	2.37

**Burnett Lands - 3370 Greenbank Road**  
**Design Storm Time Series Data**  
**Chicago Design Storms**



C100-4.stm		C100-4+20%.stm	
Duration	Intensity	Duration	Intensity
min	mm/hr	min	mm/hr
0:00	0	0:00	0
0:10	4.07	0:10	4.88
0:20	4.54	0:20	5.45
0:30	5.14	0:40	7.14
0:40	5.95	0:50	8.51
0:50	7.09	1:00	10.62
1:00	8.85	1:10	14.28
1:10	11.9	1:20	22.25
1:20	18.54	1:30	53.03
1:30	44.19	1:40	214.27
1:40	178.56	1:50	62.45
1:50	52.04	2:00	30.37
2:00	25.31	2:10	20.08
2:10	16.73	2:20	15.07
2:20	12.56	2:30	12.11
2:30	10.09	2:40	10.16
2:40	8.47	2:50	8.78
2:50	7.32	3:00	7.75
3:00	6.46	3:10	6.95
3:10	5.79	3:20	6.3
3:20	5.25	3:30	5.78
3:30	4.82	3:40	5.34
3:40	4.45	3:50	4.97
3:50	4.14	4:00	4.66
4:00	3.88		



**Burnett Lands – 3370 Greenbank Road  
5-year Storm, 100-year Fixed Outlet Elevations  
Model Output**



EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)

M:\2011\111117\CAD\Design\111117 - GP.DWG  
M:\2011\111117\CAD\Design\111117 - GP.DWG

\*\*\*\*\*  
NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.  
\*\*\*\*\*

\*\*\*\*\*  
Analysis Options  
\*\*\*\*\*

Flow Units ..... LPS  
Process Models:  
  Rainfall/Runoff ..... YES  
  Snowmelt ..... NO  
  Groundwater ..... NO  
  Flow Routing ..... YES  
  Ponding Allowed ..... YES  
  Water Quality ..... NO  
Infiltration Method ..... HORTON  
Flow Routing Method ..... DYNWAVE  
Starting Date ..... JAN-21-2016 00:00:00  
Ending Date ..... JAN-21-2016 12:00:00  
Antecedent Dry Days ..... 0.0  
Report Time Step ..... 00:01:00  
Wet Time Step ..... 00:01:00  
Dry Time Step ..... 01:00:00  
Routing Time Step ..... 5.00 sec

WARNING 03: negative offset ignored for Link CB10-11ROADLINK  
  
WARNING 04: minimum elevation drop used for Conduit CB12-13ROADLINK  
  
WARNING 04: minimum elevation drop used for Conduit CB27-28ROADLINK  
  
WARNING 04: minimum elevation drop used for Conduit CB29-30ROADLINK  
  
WARNING 02: maximum depth increased for Node CB07-08  
  
WARNING 02: maximum depth increased for Node CB09-10  
  
WARNING 02: maximum depth increased for Node CB13-14  
  
WARNING 02: maximum depth increased for Node CB13-14ROAD  
  
WARNING 02: maximum depth increased for Node CB32-33  
  
WARNING 02: maximum depth increased for Node CB32-33 ROAD  
  
WARNING 02: maximum depth increased for Node CB34-35  
  
WARNING 02: maximum depth increased for Node CB34-35 ROAD  
  
WARNING 02: maximum depth increased for Node CB57-58  
  
WARNING 02: maximum depth increased for Node CB60  
  
WARNING 02: maximum depth increased for Node CB61

\*\*\*\*\*  
Element Count  
\*\*\*\*\*  
Number of rain gages ..... 1

Number of subcatchments ... 57  
Number of nodes ..... 162  
Number of links ..... 220  
Number of pollutants ..... 0  
Number of land uses ..... 0

\*\*\*\*\*  
Raingage Summary  
\*\*\*\*\*

Name	Data Source	Data Type	Recording Interval
RAIN	C5-4	INTENSITY	10 min.

\*\*\*\*\*  
Subcatchment Summary  
\*\*\*\*\*

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
A-01	0.23	43.00	64.00	0.7500	RAIN	CB01-02
A-02	0.25	36.00	64.00	0.7500	RAIN	CB03-04
A-03	0.13	36.00	64.00	0.7500	RAIN	CB13-14ROAD
A-04	0.30	34.00	64.00	0.7500	RAIN	CB05-06
A-05	0.28	37.00	64.00	0.7500	RAIN	CB11-12ROAD
A-06	0.27	37.00	64.00	0.7500	RAIN	CB07-08
A-07	0.29	39.00	64.00	0.7500	RAIN	CB15-16
A-08	0.25	41.00	64.00	0.7500	RAIN	CB17-18
A-09	0.08	23.00	64.00	0.7500	RAIN	CB19
A-10	0.19	37.00	64.00	0.7500	RAIN	CB20-21
A-11	0.23	25.00	64.00	0.7500	RAIN	CB20-21
A-12	0.04	24.00	64.00	0.7500	RAIN	CB59
A-13	0.20	24.00	64.00	0.7500	RAIN	CB23
A-14	0.20	23.00	64.00	0.7500	RAIN	CB22
A-15	0.11	30.00	64.00	0.7500	RAIN	CB64
A-16	0.13	32.00	64.00	0.7500	RAIN	CB63
A-17	0.25	49.00	64.00	0.7500	RAIN	CB44-45
A-18	0.27	32.00	64.00	0.7500	RAIN	CB42-43
A-19	0.12	31.00	64.00	0.7500	RAIN	CB62
A-20	0.13	29.00	64.00	0.7500	RAIN	CB60
A-21	0.22	27.00	64.00	0.7500	RAIN	CB24-25
A-22	0.06	24.00	64.00	0.7500	RAIN	CB61
A-23	0.46	46.00	64.00	0.7500	RAIN	CB40-41
A-24	0.21	28.00	64.00	0.7500	RAIN	CB34-35 ROAD
A-25	0.21	29.00	64.00	0.7500	RAIN	CB32-33 ROAD
A-26	0.45	45.00	64.00	0.7500	RAIN	CB26-27
A-27	0.19	32.00	64.00	0.7500	RAIN	CB30-31
A-28	0.23	18.00	64.00	0.7500	RAIN	CB29
A-29	0.23	46.00	64.00	0.7500	RAIN	CB09-10
A-30	0.50	71.00	64.00	0.7500	RAIN	A30-STOR
A-31	0.06	20.00	64.00	0.7500	RAIN	CB66
A-32	0.45	56.00	64.00	0.7500	RAIN	CB67-68-69
A-33	0.82	117.00	64.00	0.7500	RAIN	A33-STOR
A-34	0.77	55.00	64.00	0.7500	RAIN	4-GreenbankOut
A-35	0.98	70.00	64.00	0.7500	RAIN	4-GreenbankOut
A-36	0.45	60.00	64.00	0.7500	RAIN	4-GreenbankOut
A-37	0.37	53.00	64.00	0.7500	RAIN	4-GreenbankOut
B-01	0.09	28.00	64.00	0.7500	RAIN	CB36
B-02	0.15	23.00	64.00	0.7500	RAIN	CB37
B-03	0.10	32.00	64.00	0.7500	RAIN	CB39
B-04	0.83	110.00	64.00	0.7500	RAIN	B04 Stor
B-05	0.06	27.00	64.00	0.7500	RAIN	CB-65
B-06	0.08	19.00	64.00	0.7500	RAIN	CB46-47
B-07	0.13	32.00	64.00	0.7500	RAIN	CB48-49
B-08	0.23	32.00	64.00	0.7500	RAIN	CB50-51
B-09	0.25	34.00	64.00	0.7500	RAIN	CB52-53
B-10	1.12	149.00	14.00	0.1000	RAIN	CB52-53
B-11	0.05	13.00	64.00	0.7500	RAIN	CB54

**Burnett Lands – 3370 Greenbank Road**  
**5-year Storm, 100-year Fixed Outlet Elevations**  
**Model Output**

B-12	0.09	20.00	64.00	0.7500	RAIN
B-13	0.10	22.00	64.00	0.7500	RAIN
B-14	0.32	73.00	64.00	0.7500	RAIN
B-15	0.12	27.00	64.00	0.7500	RAIN
B-16	0.14	31.00	64.00	0.7500	RAIN
B-17	0.27	33.00	64.00	0.7500	RAIN
B-18	0.12	28.00	64.00	0.7500	RAIN
B-19	0.11	22.00	64.00	0.7500	RAIN
CaivanLands	8.10	200.00	64.00	0.5000	RAIN

CB100
CB102
CB55-56
CB104
CB106
CB57-58
CB108
CB110
Caivan-Stor

206 (STM)	JUNCTION	90.77	2.85	0.0
208 (STM)	JUNCTION	90.69	3.13	0.0
208a	JUNCTION	90.65	3.02	0.0
210 (STM)	JUNCTION	90.53	2.99	0.0
212 (STM)	JUNCTION	90.40	3.33	0.0
214 (STM)	JUNCTION	90.30	3.17	0.0
216 (STM)	JUNCTION	90.01	3.36	0.0
304 (STM)	JUNCTION	90.86	2.51	0.0
306 (STM)	JUNCTION	90.65	2.93	0.0
306a (STM)	JUNCTION	90.52	2.96	0.0
308 (STM)	JUNCTION	90.41	3.00	0.0
310 (STM)	JUNCTION	90.07	3.39	0.0
312 (STM)	JUNCTION	89.56	3.80	0.0
314 (STM)	JUNCTION	89.49	3.15	0.0
316 (STM)	JUNCTION	89.36	3.90	0.0
324 (STM)	JUNCTION	89.35	3.30	0.0
326 (STM)	JUNCTION	89.30	3.24	0.0
328 (STM)	JUNCTION	88.91	3.25	0.0
330 (STM)	JUNCTION	89.33	3.21	0.0
402 (STM)	JUNCTION	90.91	2.91	0.0
402a (STM)	JUNCTION	90.84	2.97	0.0
404 (STM)	JUNCTION	90.67	3.13	0.0
404a	JUNCTION	90.63	3.13	0.0
406 (STM)	JUNCTION	90.45	3.25	0.0
408 (STM)	JUNCTION	90.36	3.44	0.0
608 (STM)	JUNCTION	90.55	3.16	0.0
608a	JUNCTION	90.49	3.13	0.0
610 (STM)	JUNCTION	89.88	3.42	0.0
610a	JUNCTION	89.84	3.48	0.0
612 (STM)	JUNCTION	90.12	4.03	0.0
614 (STM)	JUNCTION	90.22	3.50	0.0
616 (STM)	JUNCTION	90.53	3.74	0.0
618 (STM)	JUNCTION	90.36	3.28	0.0
902 (STM)	JUNCTION	90.70	3.27	0.0
904 (STM)	JUNCTION	90.58	3.18	0.0
906 (STM)	JUNCTION	90.49	3.04	0.0
908 (STM)	JUNCTION	90.27	3.30	0.0
908a	JUNCTION	90.19	3.17	0.0
C100 (STM)	JUNCTION	90.89	2.61	0.0
C102 (STM)	JUNCTION	90.85	2.65	0.0
C104 (STM)	JUNCTION	90.81	2.69	0.0
C106 (STM)	JUNCTION	90.75	2.75	0.0
C108 (STM)	JUNCTION	90.72	2.78	0.0
C110 (STM)	JUNCTION	90.67	2.83	0.0
CB01-02	JUNCTION	92.05	1.50	0.0
CB03-04	JUNCTION	91.89	1.59	0.0
CB05-06	JUNCTION	91.85	1.50	0.0
CB07-08	JUNCTION	91.72	1.65	0.0
CB09-10	JUNCTION	91.91	1.40	0.0
CB100	JUNCTION	92.60	1.75	0.0
CB102	JUNCTION	92.60	1.75	0.0
CB104	JUNCTION	92.60	1.75	0.0
CB106	JUNCTION	92.60	1.75	0.0
CB108	JUNCTION	92.60	1.75	0.0
CB110	JUNCTION	92.60	1.75	0.0
CB11-12	JUNCTION	92.38	1.54	0.0
CB11-12ROAD	JUNCTION	93.62	0.30	0.0
CB13-14	JUNCTION	92.64	1.61	0.0
CB13-14ROAD	JUNCTION	93.84	0.41	0.0
CB15-16	JUNCTION	92.00	1.50	0.0
CB17-18	JUNCTION	92.31	1.50	0.0
CB19	JUNCTION	92.24	1.50	0.0
CB20-21	JUNCTION	92.14	1.50	0.0
CB22	JUNCTION	92.29	1.50	0.0
CB23	JUNCTION	92.08	1.50	0.0
CB24-25	JUNCTION	92.05	1.50	0.0
CB26-27	JUNCTION	91.88	1.50	0.0
CB28	JUNCTION	91.78	1.51	0.0
CB29	JUNCTION	91.78	1.50	0.0

\*\*\*\*\*  
Node Summary  
\*\*\*\*\*

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
01+011	JUNCTION	93.48	0.30	0.0	
01+051	JUNCTION	93.76	0.30	0.0	
01+118	JUNCTION	93.72	0.30	0.0	
01+143	JUNCTION	93.93	0.30	0.0	
02+196	JUNCTION	93.67	0.30	0.0	
02+288	JUNCTION	93.76	0.30	0.0	
02+365	JUNCTION	93.84	0.30	0.0	
02+432	JUNCTION	93.86	0.30	0.0	
02+458	JUNCTION	93.77	0.30	0.0	
02+512	JUNCTION	93.86	0.30	0.0	
02+564	JUNCTION	94.40	0.30	0.0	
03+026	JUNCTION	93.56	0.30	0.0	
03+054	JUNCTION	93.61	0.30	0.0	
03+109	JUNCTION	93.44	0.30	0.0	
03+127	JUNCTION	93.37	0.30	0.0	
03+200	JUNCTION	93.42	0.30	0.0	
03+291	JUNCTION	93.34	0.30	0.0	
03+384	JUNCTION	93.34	0.30	0.0	
04+078	JUNCTION	93.65	0.30	0.0	
04+089	JUNCTION	93.76	0.30	0.0	
04+129	JUNCTION	93.64	0.30	0.0	
04+207	JUNCTION	93.87	0.30	0.0	
05+116	JUNCTION	93.93	0.30	0.0	
05+200	JUNCTION	93.49	0.30	0.0	
06+023	JUNCTION	93.64	0.30	0.0	
06+043	JUNCTION	93.74	0.30	0.0	
06+108	JUNCTION	93.60	0.30	0.0	
06+194	JUNCTION	93.51	0.30	0.0	
06+273	JUNCTION	93.40	0.30	0.0	
06+309	JUNCTION	93.88	0.30	0.0	
06+332	JUNCTION	94.51	0.30	0.0	
06+355	JUNCTION	94.15	0.30	0.0	
09+000	JUNCTION	93.36	0.30	0.0	
09+097	JUNCTION	93.65	0.30	0.0	
09+203	JUNCTION	93.89	0.30	0.0	
10+011	JUNCTION	93.58	0.30	0.0	
10+232	JUNCTION	94.36	0.30	0.0	
102 (STM)	JUNCTION	90.82	2.95	0.0	
104 (STM)	JUNCTION	90.67	3.27	0.0	
106 (STM)	JUNCTION	90.57	3.30	0.0	
106a	JUNCTION	90.50	3.29	0.0	
108 (STM)	JUNCTION	90.40	3.35	0.0	
108a	JUNCTION	90.34	3.21	0.0	
11+000	JUNCTION	94.01	0.30	0.0	
11+086	JUNCTION	94.26	0.30	0.0	
110 (STM)	JUNCTION	90.24	3.22	0.0	
110a	JUNCTION	90.17	3.21	0.0	
200 (STM)	JUNCTION	91.33	3.21	0.0	
202 (STM)	JUNCTION	91.25	2.89	0.0	
202a	JUNCTION	91.14	2.87	0.0	
204 (STM)	JUNCTION	91.01	2.93	0.0	
204a	JUNCTION	90.91	2.90	0.0	







**Burnett Lands – 3370 Greenbank Road  
 5-year Storm, 100-year Fixed Outlet Elevations  
 Model Output**

S10-02	5m-ROW	0.30	2.40	0.13	18.20	1	2729.26
S10-03	5m-ROW	0.30	2.40	0.13	18.20	1	2733.56
S10-04	RECT_OPEN	0.30	3.00	0.28	10.00	1	6642.05
S10-05	RECT_OPEN	0.30	3.00	0.28	10.00	1	5422.66
S11-01	5m-ROW	0.30	2.40	0.13	18.20	1	3310.53
S11-02	5m-ROW	0.30	2.40	0.13	18.20	1	5989.76
S11-03	5m-ROW	0.30	2.40	0.13	18.20	1	3313.94
ST10-00	5m-ROW	0.30	2.40	0.13	18.20	1	2467.78
ST6-01a	11m-ROW	0.30	3.40	0.14	23.20	1	5368.01
ST6-01b	11m-ROW	0.30	3.40	0.14	23.20	1	4305.74
ST6-01c	11m-ROW	0.30	3.40	0.14	23.20	1	8291.98
STB-00	9m-ROW	0.30	3.10	0.14	21.20	1	5778.40
STB-01	9m-ROW	0.30	3.10	0.14	21.20	1	5810.07
STB-02	9m-ROW	0.30	3.10	0.14	21.20	1	4168.40
STB-03	9m-ROW	0.30	3.10	0.14	21.20	1	4188.12
STB-04	9m-ROW	0.30	3.10	0.14	21.20	1	4621.40
STB-05	9m-ROW	0.30	3.10	0.14	21.20	1	4632.94
STB-06	9m-ROW	0.30	3.10	0.14	21.20	1	4632.94
STB-07	9m-ROW	0.30	3.10	0.14	21.20	1	4643.63
STB-08	9m-ROW	0.30	3.10	0.14	21.20	1	4562.20
STB-09	9m-ROW	0.30	3.10	0.14	21.20	1	4745.50
STB-10	9m-ROW	0.30	3.10	0.14	21.20	1	5040.78
STB-11	9m-ROW	0.30	3.10	0.14	21.20	1	5040.78
STB-12	9m-ROW	0.30	3.10	0.14	21.20	1	4158.17
STB-13	9m-ROW	0.30	3.10	0.14	21.20	1	4512.57

\*\*\*\*\*  
 Transect Summary  
 \*\*\*\*\*

**Transect 11m-ROW**

<b>Area:</b>					
	0.0003	0.0014	0.0031	0.0056	0.0087
	0.0126	0.0171	0.0224	0.0283	0.0350
	0.0423	0.0503	0.0591	0.0685	0.0787
	0.0895	0.1010	0.1133	0.1262	0.1398
	0.1542	0.1692	0.1849	0.2014	0.2185
	0.2374	0.2590	0.2824	0.3070	0.3324
	0.3586	0.3856	0.4133	0.4417	0.4710
	0.5009	0.5317	0.5632	0.5954	0.6284
	0.6622	0.6967	0.7319	0.7680	0.8047
	0.8423	0.8806	0.9196	0.9594	1.0000
<b>Hrad:</b>					
	0.0203	0.0406	0.0608	0.0811	0.1014
	0.1217	0.1420	0.1623	0.1825	0.2028
	0.2231	0.2434	0.2637	0.2840	0.3042
	0.3245	0.3448	0.3651	0.3854	0.4056
	0.4259	0.4462	0.4665	0.4868	0.5071
	0.4758	0.4659	0.4779	0.4994	0.5251
	0.5505	0.5757	0.6006	0.6253	0.6498
	0.6741	0.6982	0.7221	0.7459	0.7696
	0.7931	0.8165	0.8398	0.8630	0.8860
	0.9090	0.9319	0.9547	0.9774	1.0000
<b>Width:</b>					
	0.0171	0.0342	0.0512	0.0683	0.0854
	0.1025	0.1196	0.1366	0.1537	0.1708
	0.1879	0.2049	0.2220	0.2391	0.2562
	0.2733	0.2903	0.3074	0.3245	0.3416
	0.3587	0.3757	0.3928	0.4099	0.4270
	0.4954	0.5528	0.5880	0.6121	0.6305
	0.6490	0.6675	0.6860	0.7044	0.7229
	0.7414	0.7599	0.7783	0.7968	0.8153
	0.8337	0.8522	0.8707	0.8892	0.9076
	0.9261	0.9446	0.9631	0.9815	1.0000

**Transect 5m-ROW**  
 Area:

	0.0005	0.0019	0.0043	0.0076	0.0119
	0.0172	0.0234	0.0306	0.0387	0.0478
	0.0578	0.0688	0.0807	0.0934	0.1062
	0.1190	0.1318	0.1446	0.1574	0.1703
	0.1832	0.1961	0.2090	0.2220	0.2349
	0.2494	0.2668	0.2857	0.3058	0.3271
	0.3496	0.3732	0.3981	0.4241	0.4512
	0.4796	0.5091	0.5398	0.5717	0.6048
	0.6390	0.6744	0.7110	0.7487	0.7877
	0.8278	0.8691	0.9115	0.9552	1.0000
<b>Hrad:</b>					
	0.0225	0.0450	0.0676	0.0901	0.1126
	0.1351	0.1577	0.1802	0.2027	0.2252
	0.2477	0.2703	0.2928	0.3297	0.3737
	0.4176	0.4612	0.5047	0.5480	0.5912
	0.6342	0.6770	0.7197	0.7622	0.8045
	0.6979	0.6515	0.6566	0.6637	0.6724
	0.6826	0.6940	0.7064	0.7197	0.7339
	0.7487	0.7642	0.7802	0.7967	0.8137
	0.8310	0.8487	0.8668	0.8851	0.9037
	0.9225	0.9416	0.9609	0.9804	1.0000
<b>Width:</b>					
	0.0210	0.0421	0.0631	0.0842	0.1052
	0.1263	0.1473	0.1684	0.1894	0.2104
	0.2315	0.2525	0.2736	0.2809	0.2813
	0.2818	0.2822	0.2826	0.2831	0.2835
	0.2840	0.2844	0.2848	0.2853	0.2857
	0.3516	0.4042	0.4301	0.4560	0.4819
	0.5078	0.5338	0.5597	0.5856	0.6115
	0.6374	0.6633	0.6892	0.7151	0.7410
	0.7669	0.7928	0.8187	0.8446	0.8705
	0.8964	0.9223	0.9482	0.9741	1.0000

**Transect 9m-ROW**

<b>Area:</b>					
	0.0004	0.0015	0.0034	0.0061	0.0095
	0.0137	0.0186	0.0243	0.0308	0.0380
	0.0460	0.0548	0.0643	0.0746	0.0856
	0.0974	0.1099	0.1232	0.1373	0.1522
	0.1678	0.1841	0.2012	0.2189	0.2367
	0.2557	0.2768	0.2992	0.3223	0.3463
	0.3711	0.3968	0.4232	0.4505	0.4787
	0.5076	0.5374	0.5680	0.5995	0.6317
	0.6648	0.6988	0.7335	0.7691	0.8055
	0.8428	0.8808	0.9197	0.9594	1.0000
<b>Hrad:</b>					
	0.0203	0.0406	0.0609	0.0812	0.1015
	0.1218	0.1421	0.1624	0.1827	0.2031
	0.2234	0.2437	0.2640	0.2843	0.3046
	0.3249	0.3452	0.3655	0.3858	0.4061
	0.4264	0.4467	0.4670	0.5004	0.5401
	0.5171	0.5140	0.5356	0.5570	0.5784
	0.5998	0.6211	0.6423	0.6636	0.6848
	0.7059	0.7271	0.7482	0.7692	0.7903
	0.8114	0.8324	0.8534	0.8744	0.8953
	0.9163	0.9372	0.9582	0.9791	1.0000
<b>Width:</b>					
	0.0186	0.0371	0.0557	0.0743	0.0929
	0.1114	0.1300	0.1486	0.1671	0.1857
	0.2043	0.2229	0.2414	0.2600	0.2786
	0.2971	0.3157	0.3343	0.3529	0.3714
	0.3900	0.4086	0.4271	0.4336	0.4340
	0.4906	0.5350	0.5553	0.5755	0.5957
	0.6159	0.6361	0.6563	0.6765	0.6968
	0.7170	0.7372	0.7574	0.7776	0.7978
	0.8181	0.8383	0.8585	0.8787	0.8989
	0.9191	0.9394	0.9596	0.9798	1.0000

**Burnett Lands – 3370 Greenbank Road**  
**5-year Storm, 100-year Fixed Outlet Elevations**  
**Model Output**



\*\*\*\*\*  
Control Actions Taken  
\*\*\*\*\*

*****	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation .....	1.039	45.162
Evaporation Loss .....	0.000	0.000
Infiltration Loss .....	0.376	16.351
Surface Runoff .....	0.650	28.232
Final Surface Storage ....	0.012	0.542
Continuity Error (%) .....	0.082	

*****	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow .....	0.000	0.000
Wet Weather Inflow .....	0.650	6.496
Groundwater Inflow .....	0.000	0.000
RDII Inflow .....	0.000	0.000
External Inflow .....	0.006	0.055
External Outflow .....	0.659	6.592
Internal Outflow .....	0.000	0.000
Storage Losses .....	0.000	0.000
Initial Stored Volume ....	0.114	1.141
Final Stored Volume .....	0.114	1.138
Continuity Error (%) .....	-0.494	

\*\*\*\*\*  
Highest Continuity Errors  
\*\*\*\*\*  
Node 02+196 (99.64%)  
Node CB11-12ROAD (-32.99%)  
Node 04+078 (9.02%)  
Node 02+512 (8.12%)  
Node C100 (STM) (6.41%)

\*\*\*\*\*  
Time-Step Critical Elements  
\*\*\*\*\*  
None

\*\*\*\*\*  
Highest Flow Instability Indexes  
\*\*\*\*\*  
Link C100-208 (104)  
Link C110-216 (103)  
Link C108-214 (103)  
Link 206-208 (102)  
Link C104-210 (101)

\*\*\*\*\*  
Routing Time Step Summary  
\*\*\*\*\*  
Minimum Time Step : 1.82 sec  
Average Time Step : 4.99 sec  
Maximum Time Step : 5.00 sec  
Percent in Steady State : 0.00  
Average Iterations per Step : 6.82

\*\*\*\*\*

Subcatchment Runoff Summary  
\*\*\*\*\*

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
A-01	45.16	0.00	0.00	14.73	29.94	0.07	44.76	0.663
A-02	45.16	0.00	0.00	14.87	29.80	0.07	46.82	0.660
A-03	45.16	0.00	0.00	14.49	30.44	0.04	24.81	0.674
A-04	45.16	0.00	0.00	15.02	29.65	0.09	55.90	0.657
A-05	45.16	0.00	0.00	14.91	30.01	0.08	51.50	0.664
A-06	45.16	0.00	0.00	14.91	29.86	0.08	51.32	0.661
A-07	45.16	0.00	0.00	14.91	29.76	0.09	54.12	0.659
A-08	45.16	0.00	0.00	14.79	30.14	0.07	46.63	0.667
A-09	45.16	0.00	0.00	14.49	30.19	0.02	15.82	0.669
A-10	45.16	0.00	0.00	14.69	29.99	0.06	35.77	0.664
A-11	45.16	0.00	0.00	15.03	29.64	0.07	41.61	0.656
A-12	45.16	0.00	0.00	14.11	28.94	0.01	6.67	0.641
A-13	45.16	0.00	0.00	14.98	29.69	0.06	37.18	0.657
A-14	45.16	0.00	0.00	14.99	29.68	0.06	36.21	0.657
A-15	45.16	0.00	0.00	14.49	28.92	0.03	19.43	0.640
A-16	45.16	0.00	0.00	14.56	28.92	0.04	23.68	0.640
A-17	45.16	0.00	0.00	14.69	29.99	0.07	47.07	0.664
A-18	45.16	0.00	0.00	14.99	29.68	0.08	50.50	0.657
A-19	45.16	0.00	0.00	14.55	28.92	0.04	22.57	0.640
A-20	45.16	0.00	0.00	14.62	28.92	0.04	24.03	0.640
A-21	45.16	0.00	0.00	14.96	29.71	0.06	40.44	0.658
A-22	45.16	0.00	0.00	14.32	28.93	0.02	11.11	0.641
A-23	45.16	0.00	0.00	15.08	29.83	0.14	83.88	0.661
A-24	45.16	0.00	0.00	14.92	30.01	0.06	39.13	0.664
A-25	45.16	0.00	0.00	14.91	30.01	0.06	40.09	0.665
A-26	45.16	0.00	0.00	15.09	29.78	0.13	82.21	0.659
A-27	45.16	0.00	0.00	14.78	30.14	0.06	36.04	0.667
A-28	45.16	0.00	0.00	15.23	28.93	0.07	41.39	0.641
A-29	45.16	0.00	0.00	14.68	30.09	0.07	44.16	0.666
A-30	45.16	0.00	0.00	14.88	29.79	0.15	93.52	0.660
A-31	45.16	0.00	0.00	14.42	29.77	0.02	11.99	0.659
A-32	45.16	0.00	0.00	14.96	29.96	0.13	83.95	0.663
A-33	45.16	0.00	0.00	14.88	29.80	0.24	153.37	0.660
A-34	45.16	0.00	0.00	15.28	28.87	0.22	137.01	0.639
A-35	45.16	0.00	0.00	15.28	28.87	0.28	174.10	0.639
A-36	45.16	0.00	0.00	14.92	29.25	0.13	84.22	0.648
A-37	45.16	0.00	0.00	14.88	29.29	0.11	69.89	0.649
B-01	45.16	0.00	0.00	14.46	30.23	0.03	18.11	0.669
B-02	45.16	0.00	0.00	14.83	29.34	0.04	28.07	0.650
B-03	45.16	0.00	0.00	14.42	29.87	0.03	19.11	0.661
B-04	45.16	0.00	0.00	14.92	29.75	0.25	154.40	0.659
B-05	45.16	0.00	0.00	14.30	28.93	0.02	11.90	0.641
B-06	45.16	0.00	0.00	14.56	29.62	0.02	14.72	0.656
B-07	45.16	0.00	0.00	14.55	30.03	0.04	24.57	0.665
B-08	45.16	0.00	0.00	14.88	29.79	0.07	42.35	0.660
B-09	45.16	0.00	0.00	14.90	29.78	0.07	45.99	0.659
B-10	45.16	0.00	0.00	37.87	7.07	0.08	47.11	0.157
B-11	45.16	0.00	0.00	14.52	29.66	0.01	9.45	0.657
B-12	45.16	0.00	0.00	14.61	30.07	0.03	16.82	0.666
B-13	45.16	0.00	0.00	14.61	30.07	0.03	18.72	0.666
B-14	45.16	0.00	0.00	14.61	30.07	0.10	61.62	0.666
B-15	45.16	0.00	0.00	14.62	30.06	0.04	23.16	0.666
B-16	45.16	0.00	0.00	14.61	30.07	0.04	26.16	0.666
B-17	45.16	0.00	0.00	14.98	29.69	0.08	50.86	0.657
B-18	45.16	0.00	0.00	14.60	30.08	0.04	23.42	0.666
B-19	45.16	0.00	0.00	14.67	30.01	0.03	20.56	0.664
CaivanLands	45.16	0.00	0.00	15.86	28.74	2.33	1044.59	0.636

\*\*\*\*\*  
Node Depth Summary

**Burnett Lands – 3370 Greenbank Road  
5-year Storm, 100-year Fixed Outlet Elevations  
Model Output**

\*\*\*\*\*

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min
01+011	JUNCTION	0.00	0.00	93.48	0 00:00
01+051	JUNCTION	0.00	0.00	93.76	0 00:00
01+118	JUNCTION	0.00	0.00	93.72	0 00:00
01+143	JUNCTION	0.00	0.00	93.93	0 00:00
02+196	JUNCTION	0.00	0.00	93.67	0 01:30
02+288	JUNCTION	0.00	0.00	93.76	0 00:00
02+365	JUNCTION	0.00	0.00	93.84	0 00:00
02+432	JUNCTION	0.00	0.00	93.86	0 00:00
02+458	JUNCTION	0.00	0.00	93.77	0 00:00
02+512	JUNCTION	0.00	0.01	93.87	0 01:31
02+564	JUNCTION	0.00	0.00	94.40	0 00:00
03+026	JUNCTION	0.00	0.00	93.56	0 00:00
03+054	JUNCTION	0.00	0.00	93.61	0 00:00
03+109	JUNCTION	0.00	0.00	93.44	0 00:00
03+127	JUNCTION	0.00	0.00	93.37	0 00:00
03+200	JUNCTION	0.00	0.00	93.42	0 00:00
03+291	JUNCTION	0.00	0.00	93.34	0 00:00
03+384	JUNCTION	0.00	0.00	93.34	0 00:00
04+078	JUNCTION	0.00	0.01	93.66	0 01:39
04+089	JUNCTION	0.00	0.00	93.76	0 00:00
04+129	JUNCTION	0.00	0.00	93.64	0 00:00
04+207	JUNCTION	0.00	0.00	93.87	0 00:00
05+116	JUNCTION	0.00	0.00	93.93	0 00:00
05+200	JUNCTION	0.00	0.03	93.52	0 01:31
06+023	JUNCTION	0.00	0.00	93.64	0 00:00
06+043	JUNCTION	0.00	0.00	93.74	0 00:00
06+108	JUNCTION	0.00	0.00	93.60	0 00:00
06+194	JUNCTION	0.00	0.00	93.51	0 00:00
06+273	JUNCTION	0.00	0.00	93.40	0 00:00
06+309	JUNCTION	0.00	0.00	93.88	0 00:00
06+332	JUNCTION	0.00	0.00	94.51	0 00:00
06+355	JUNCTION	0.00	0.00	94.15	0 00:00
09+000	JUNCTION	0.00	0.00	93.36	0 00:00
09+097	JUNCTION	0.00	0.00	93.65	0 00:00
09+203	JUNCTION	0.00	0.00	93.89	0 00:00
10+011	JUNCTION	0.00	0.00	93.58	0 00:00
10+232	JUNCTION	0.00	0.00	94.36	0 00:00
102 (STM)	JUNCTION	0.47	1.21	92.04	0 01:29
104 (STM)	JUNCTION	0.63	1.37	92.04	0 01:30
106 (STM)	JUNCTION	0.73	1.47	92.04	0 01:30
106a	JUNCTION	0.80	1.52	92.02	0 01:30
108 (STM)	JUNCTION	0.90	1.61	92.01	0 01:30
108a	JUNCTION	0.96	1.64	91.98	0 01:30
11+000	JUNCTION	0.00	0.00	94.01	0 00:00
11+086	JUNCTION	0.00	0.00	94.26	0 00:00
110 (STM)	JUNCTION	1.06	1.71	91.95	0 01:30
110a	JUNCTION	1.13	1.72	91.89	0 01:30
200 (STM)	JUNCTION	0.27	1.04	92.37	0 01:24
202 (STM)	JUNCTION	0.35	1.12	92.37	0 01:24
202a	JUNCTION	0.45	1.17	92.31	0 01:24
204 (STM)	JUNCTION	0.58	1.25	92.26	0 01:24
204a	JUNCTION	0.68	1.30	92.21	0 01:30
206 (STM)	JUNCTION	0.82	1.36	92.13	0 01:24
208 (STM)	JUNCTION	0.90	1.37	92.06	0 01:24
208a	JUNCTION	0.94	1.34	91.99	0 01:24
210 (STM)	JUNCTION	1.06	1.38	91.91	0 01:24
212 (STM)	JUNCTION	1.18	1.41	91.81	0 01:24
214 (STM)	JUNCTION	1.28	1.44	91.74	0 01:24
216 (STM)	JUNCTION	1.57	1.64	91.65	0 01:28
304 (STM)	JUNCTION	0.43	0.87	91.73	0 01:30
306 (STM)	JUNCTION	0.64	1.07	91.72	0 01:30
306a (STM)	JUNCTION	0.77	1.19	91.71	0 01:30

308 (STM)	JUNCTION	0.88	1.30	91.71	0 01:30
310 (STM)	JUNCTION	1.22	1.60	91.67	0 01:29
312 (STM)	JUNCTION	1.73	2.09	91.65	0 01:30
314 (STM)	JUNCTION	1.80	2.10	91.58	0 01:30
316 (STM)	JUNCTION	1.92	2.13	91.49	0 01:29
324 (STM)	JUNCTION	1.93	2.06	91.41	0 01:30
326 (STM)	JUNCTION	1.98	2.06	91.36	0 01:30
328 (STM)	JUNCTION	2.37	2.39	91.30	0 01:28
330 (STM)	JUNCTION	1.96	2.06	91.38	0 01:28
402 (STM)	JUNCTION	0.39	1.12	92.03	0 01:30
402a (STM)	JUNCTION	0.46	1.18	92.02	0 01:30
404 (STM)	JUNCTION	0.63	1.35	92.02	0 01:30
404a	JUNCTION	0.67	1.38	92.01	0 01:30
406 (STM)	JUNCTION	0.84	1.75	92.20	0 01:22
408 (STM)	JUNCTION	0.94	1.68	92.04	0 01:30
608 (STM)	JUNCTION	0.75	1.45	92.00	0 01:30
608a	JUNCTION	0.80	1.49	91.98	0 01:30
610 (STM)	JUNCTION	1.41	1.92	91.80	0 01:30
610a	JUNCTION	1.45	1.90	91.74	0 01:29
612 (STM)	JUNCTION	1.17	1.52	91.64	0 01:30
614 (STM)	JUNCTION	1.07	1.49	91.71	0 01:30
616 (STM)	JUNCTION	0.76	1.19	91.72	0 01:29
618 (STM)	JUNCTION	0.93	1.43	91.80	0 01:29
902 (STM)	JUNCTION	0.59	1.13	91.83	0 01:30
904 (STM)	JUNCTION	0.71	1.26	91.84	0 01:30
906 (STM)	JUNCTION	0.81	1.34	91.83	0 01:30
908 (STM)	JUNCTION	1.02	1.54	91.81	0 01:30
908a	JUNCTION	1.10	1.62	91.81	0 01:30
C100 (STM)	JUNCTION	0.70	1.18	92.07	0 01:24
C102 (STM)	JUNCTION	0.74	1.14	91.99	0 01:24
C104 (STM)	JUNCTION	0.78	1.10	91.91	0 01:24
C106 (STM)	JUNCTION	0.83	1.07	91.82	0 01:24
C108 (STM)	JUNCTION	0.86	1.03	91.75	0 01:24
C110 (STM)	JUNCTION	0.95	2.24	92.91	0 01:31
CB01-02	JUNCTION	0.04	1.25	93.30	0 01:30
CB03-04	JUNCTION	0.04	1.33	93.22	0 01:30
CB05-06	JUNCTION	0.04	1.21	93.06	0 01:30
CB07-08	JUNCTION	0.05	1.24	92.96	0 01:31
CB09-10	JUNCTION	0.04	1.15	93.06	0 01:30
CB100	JUNCTION	0.04	1.41	94.01	0 01:30
CB102	JUNCTION	0.03	1.40	94.00	0 01:30
CB104	JUNCTION	0.03	1.40	94.00	0 01:30
CB106	JUNCTION	0.04	1.41	94.01	0 01:30
CB108	JUNCTION	0.03	1.40	94.00	0 01:30
CB110	JUNCTION	0.04	1.41	94.01	0 01:30
CB11-12	JUNCTION	0.06	0.79	93.17	0 01:31
CB11-12ROAD	JUNCTION	0.00	0.04	93.66	0 01:30
CB13-14	JUNCTION	0.00	0.00	92.64	0 00:00
CB13-14ROAD	JUNCTION	0.01	0.05	93.89	0 01:30
CB15-16	JUNCTION	0.08	1.31	93.31	0 01:33
CB17-18	JUNCTION	0.12	1.35	93.66	0 01:40
CB19	JUNCTION	0.03	1.08	93.32	0 01:30
CB20-21	JUNCTION	0.05	1.28	93.42	0 01:31
CB22	JUNCTION	0.05	1.25	93.54	0 01:31
CB23	JUNCTION	0.05	1.22	93.30	0 01:31
CB24-25	JUNCTION	0.04	1.20	93.25	0 01:30
CB26-27	JUNCTION	0.04	1.22	93.10	0 01:30
CB28	JUNCTION	0.00	0.32	92.10	0 01:30
CB29	JUNCTION	0.06	1.23	93.01	0 01:30
CB30-31	JUNCTION	0.13	1.38	93.49	0 01:42
CB32-33	JUNCTION	0.00	0.00	92.42	0 00:00
CB32-33 ROAD	JUNCTION	0.01	0.07	93.69	0 01:30
CB34-35	JUNCTION	0.00	0.00	92.80	0 00:00
CB34-35 ROAD	JUNCTION	0.01	0.05	94.05	0 01:30
CB36	JUNCTION	0.03	1.21	94.27	0 01:30
CB37	JUNCTION	0.04	1.22	94.05	0 01:30
CB39	JUNCTION	0.04	1.24	93.82	0 01:30
CB40-41	JUNCTION	0.04	1.24	93.39	0 01:30
CB42-43	JUNCTION	0.03	1.10	93.37	0 01:30





**Burnett Lands – 3370 Greenbank Road  
5-year Storm, 100-year Fixed Outlet Elevations  
Model Output**

```
B04_Stor          STORAGE          0.35         1.150        0.298
Caivan-Stor      STORAGE          0.21         0.690        0.320
```

```
*****
Node Flooding Summary
*****

No nodes were flooded.
```

```
*****
Storage Volume Summary
*****
```

Storage Unit	Average Volume 1000 m3	Avg Pcmt Full	E&I Pcmt Loss	Maximum Volume 1000 m3	Max Pcmt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
A30-STOR	0.000	0	0	0.001	0	0 01:30	92.73
A33-STOR	0.000	0	0	0.001	1	0 01:30	151.28
B04_Stor	0.000	0	0	0.001	0	0 01:30	153.25
Caivan-Stor	0.000	0	0	0.013	1	0 01:31	933.92

```
*****
Outfall Loading Summary
*****
```

Outfall Node	Flow Freq. Pcnt.	Avg. Flow LPS	Max. Flow LPS	Total Volume 10^6 ltr
1-HW-01 (STM)	99.84	59.31	1199.64	2.521
2-OverlandOutlet	0.00	0.00	0.00	0.000
3-HW-02 (STM)	99.77	79.66	1521.75	3.379
4-GreenbankOut	73.01	24.04	465.16	0.747
System	68.15	163.01	3181.45	6.647

```
*****
Link Flow Summary
*****
```

Link	Type	Maximum  Flow  LPS	Time of Max Occurrence days hr:min	Maximum  Veloc  m/sec	Max/ Full Flow	Max/ Full Depth
{STM}.102-104	CONDUIT	15.78	0 01:22	0.05	0.03	1.00
{STM}.104-106	CONDUIT	20.75	0 01:28	0.07	0.06	1.00
{STM}.106-106a	CONDUIT	109.95	0 01:30	0.38	0.37	1.00
{STM}.108-108a	CONDUIT	204.29	0 01:30	0.55	0.58	1.00
{STM}.110-110a	CONDUIT	400.00	0 01:30	0.88	0.78	1.00
{STM}.306-306a	CONDUIT	77.25	0 01:31	0.26	0.24	1.00
{STM}.308-310	CONDUIT	130.12	0 01:30	0.45	0.40	1.00
{STM}.310-312	CONDUIT	151.95	0 01:30	0.33	0.34	1.00
{STM}.312-314	CONDUIT	964.54	0 01:30	0.83	0.75	1.00
{STM}.314-316	CONDUIT	966.20	0 01:30	0.83	0.74	1.00
{STM}.316-324	CONDUIT	1200.47	0 01:30	1.03	0.94	1.00
{STM}.326-328	CONDUIT	1199.35	0 01:30	0.64	0.50	1.00
{STM}.328-HW-1	CONDUIT	1199.64	0 01:30	0.64	0.49	1.00
{STM}.330-336	CONDUIT	731.87	0 01:30	0.63	0.25	1.00
{STM}.402-402a	CONDUIT	32.64	0 01:38	0.12	0.12	1.00
{STM}.404-404a	CONDUIT	70.05	0 01:31	0.24	0.21	1.00
{STM}.406-408	CONDUIT	41.01	0 01:22	0.14	0.14	1.00

{STM}.408-106	CONDUIT	44.12	0 01:22	0.15	0.15	1.00
{STM}.608-608a	CONDUIT	130.29	0 01:31	0.45	0.46	1.00
{STM}.610-610a	CONDUIT	636.09	0 01:31	0.82	0.78	1.00
{STM}.612-316	CONDUIT	237.04	0 01:30	0.81	0.82	1.00
{STM}.614-312	CONDUIT	237.19	0 01:30	0.81	0.83	1.00
{STM}.616-614	CONDUIT	11.63	0 01:30	0.07	0.09	1.00
{STM}.618-610	CONDUIT	40.08	0 01:44	0.14	0.11	1.00
{STM}.902-904	CONDUIT	4.79	0 01:07	0.02	0.01	1.00
{STM}.904-906	CONDUIT	5.96	0 01:22	0.02	0.02	1.00
{STM}.906-908	CONDUIT	74.82	0 01:31	0.26	0.26	1.00
{STM}.908-908a	CONDUIT	75.53	0 01:33	0.20	0.22	1.00
106a-108	CONDUIT	159.11	0 01:30	0.54	0.56	1.00
108a-110	CONDUIT	241.16	0 01:29	0.65	0.67	1.00
110a-610	CONDUIT	410.64	0 01:30	0.90	0.80	1.00
200-202	CONDUIT	60.78	0 01:23	0.17	0.17	1.00
202-204	CONDUIT	209.14	0 01:30	0.57	0.61	1.00
202a-204	CONDUIT	234.88	0 01:30	0.64	0.68	1.00
204-206	CONDUIT	250.98	0 01:30	0.55	0.55	1.00
204a-206	CONDUIT	288.23	0 01:30	0.63	0.65	1.00
206-208	CONDUIT	361.97	0 01:30	0.79	0.79	1.00
208-208a	CONDUIT	386.43	0 01:30	0.85	1.02	1.00
208a-210	CONDUIT	404.11	0 01:30	0.89	1.09	1.00
210-212	CONDUIT	479.86	0 01:30	0.87	0.99	1.00
212-214	CONDUIT	504.58	0 01:30	0.77	1.06	1.00
214-216	CONDUIT	577.32	0 01:30	0.88	0.90	1.00
216-H2	CONDUIT	1521.75	0 01:30	0.81	0.55	1.00
304-306	CONDUIT	38.09	0 01:31	0.13	0.11	1.00
306a-308	CONDUIT	78.12	0 01:31	0.27	0.24	1.00
324-330	CONDUIT	729.67	0 01:30	0.63	0.29	1.00
402a-404	CONDUIT	61.28	0 01:43	0.21	0.22	1.00
404a-608	CONDUIT	128.74	0 01:31	0.44	0.39	1.00
608a-110	CONDUIT	135.82	0 01:31	0.46	0.47	1.00
610a-312	CONDUIT	692.07	0 01:31	0.90	0.82	1.00
908a-610	CONDUIT	148.29	0 01:31	0.40	0.44	1.00
C100-208	CONDUIT	16.52	0 09:00	0.06	0.07	1.00
C102-208a	CONDUIT	17.99	0 01:30	0.06	0.07	1.00
C104-210	CONDUIT	22.45	0 01:29	0.08	0.09	1.00
C106-212	CONDUIT	25.56	0 01:29	0.09	0.10	1.00
C108-214	CONDUIT	22.68	0 01:30	0.08	0.09	1.00
C110-216	CONDUIT	947.68	0 01:32	3.24	3.81	1.00
CB10-11ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.07
CB12-13ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB27-28ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB29-30ROADLINK	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB53-ROAD	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB57-ROAD	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CB59-ROAD	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
CST-01	CHANNEL	0.34	0 01:30	0.21	0.00	0.02
CST-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.07
CST-03	CHANNEL	0.07	0 01:30	0.00	0.00	0.01
CST-04	CHANNEL	0.32	0 01:30	0.43	0.00	0.02
CST-05	CHANNEL	0.00	0 00:00	0.00	0.00	0.01
CST-06	CHANNEL	2.36	0 01:30	0.39	0.00	0.04
RY-00	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-00a	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-01	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-02	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-03	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-04	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-05	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-06	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-07	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-08	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-09	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
RY-10	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
S01-00	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
S01-01	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
S01-02	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
S01-03	CHANNEL	0.00	0 00:00	0.00	0.00	0.00





**Burnett Lands – 3370 Greenbank Road**  
**5-year Storm, 100-year Fixed Outlet Elevations**  
**Model Output**



S09-01	1.14	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-02	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-03	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-04	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-05	1.08	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S09-06	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S10-01	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S10-02	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.37	0.0000
S10-03	1.00	0.00	0.00	0.00	0.07	0.00	0.00	0.93	0.68	0.0000
S10-04	1.96	0.92	0.08	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S10-05	1.76	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S11-01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S11-02	1.21	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S11-03	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST10-00	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST6-01a	1.66	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST6-01b	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
ST6-01c	1.21	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-00	1.27	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-01	1.00	0.99	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.0000
STB-02	1.00	0.00	0.99	0.00	0.00	0.01	0.00	0.00	0.01	0.0000
STB-03	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.46	0.0000
STB-04	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-05	1.12	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-06	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-07	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-08	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-09	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-10	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-11	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-12	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.0000
STB-13	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.0000

{STM}.906-908	12.00	12.00	12.00	0.01	0.01
{STM}.908-908a	12.00	12.00	12.00	0.01	0.01
106a-108	12.00	12.00	12.00	0.01	0.23
108a-110	12.00	12.00	12.00	0.01	0.12
110a-610	12.00	12.00	12.00	0.01	0.12
200-202	0.15	0.15	0.15	0.01	0.01
202-204	0.17	0.17	0.17	0.01	0.01
202a-204	0.21	0.21	0.21	0.01	0.01
204-206	0.24	0.24	0.24	0.01	0.01
204a-206	0.35	0.35	0.35	0.01	0.01
206-208	12.00	12.00	12.00	0.01	0.04
208-208a	12.00	12.00	12.00	0.04	0.26
208a-210	12.00	12.00	12.00	0.10	0.22
210-212	12.00	12.00	12.00	0.01	0.15
212-214	12.00	12.00	12.00	0.09	0.20
214-216	12.00	12.00	12.00	0.01	0.13
216-H2	12.00	12.00	12.00	0.01	0.01
304-306	0.20	0.20	0.20	0.01	0.01
306a-308	12.00	12.00	12.00	0.01	0.01
324-330	12.00	12.00	12.00	0.01	0.14
402a-404	0.33	0.33	0.33	0.01	0.01
404a-608	12.00	12.00	12.00	0.01	0.01
608a-110	12.00	12.00	12.00	0.01	0.01
610a-312	12.00	12.00	12.00	0.01	0.17
908a-610	12.00	12.00	12.00	0.01	0.07
C100-208	12.00	12.00	12.00	0.01	0.09
C102-208a	12.00	12.00	12.00	0.01	0.02
C104-210	12.00	12.00	12.00	0.01	0.01
C106-212	12.00	12.00	12.00	0.01	0.01
C108-214	12.00	12.00	12.00	0.01	0.01
C110-216	12.00	12.00	12.00	0.70	0.87

\*\*\*\*\*  
 Conduit Surcharge Summary  
 \*\*\*\*\*

Analysis begun on: Wed Nov 23 10:02:12 2016  
 Analysis ended on: Wed Nov 23 10:02:15 2016  
 Total elapsed time: 00:00:03

Conduit	----- Both Ends	Hours Full Upstream	----- Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
{STM}.102-104	0.32	0.32	0.32	0.01	0.01
{STM}.104-106	11.86	11.86	11.88	0.01	0.01
{STM}.106-106a	12.00	12.00	12.00	0.01	0.01
{STM}.108-108a	12.00	12.00	12.00	0.01	0.01
{STM}.110-110a	12.00	12.00	12.00	0.01	0.01
{STM}.306-306a	12.00	12.00	12.00	0.01	0.01
{STM}.308-310	12.00	12.00	12.00	0.01	0.01
{STM}.310-312	12.00	12.00	12.00	0.01	0.01
{STM}.312-314	12.00	12.00	12.00	0.01	0.11
{STM}.314-316	12.00	12.00	12.00	0.01	0.04
{STM}.316-324	12.00	12.00	12.00	0.01	1.21
{STM}.326-328	12.00	12.00	12.00	0.01	0.01
{STM}.328-HW-1	12.00	12.00	12.00	0.01	0.20
{STM}.330-336	12.00	12.00	12.00	0.01	0.14
{STM}.402-402a	0.27	0.27	0.27	0.01	0.01
{STM}.404-404a	10.02	10.02	10.85	0.01	0.01
{STM}.406-408	0.25	0.25	0.25	0.01	0.01
{STM}.408-106	11.99	11.99	11.99	0.01	0.01
{STM}.608-608a	12.00	12.00	12.00	0.01	0.01
{STM}.610-610a	12.00	12.00	12.00	0.01	0.16
{STM}.612-316	12.00	12.00	12.00	0.01	0.01
{STM}.614-312	12.00	12.00	12.00	0.01	0.12
{STM}.616-614	12.00	12.00	12.00	0.01	0.01
{STM}.618-610	12.00	12.00	12.00	0.01	0.01
{STM}.902-904	0.64	0.64	0.64	0.01	0.01
{STM}.904-906	12.00	12.00	12.00	0.01	0.01

**Burnett Lands – 3370 Greenbank Road**  
**100-year Storm, 5-year Fixed Outlet Elevations**  
**Model Output**



EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)

M:\2011\111117\CAD\Design\111117 - GP.DWG  
M:\2011\111117\CAD\Design\111117 - GP.DWG

\*\*\*\*\*  
NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.  
\*\*\*\*\*

\*\*\*\*\*  
Analysis Options  
\*\*\*\*\*

Flow Units ..... LPS  
Process Models:  
  Rainfall/Runoff ..... YES  
  Snowmelt ..... NO  
  Groundwater ..... NO  
  Flow Routing ..... YES  
  Ponding Allowed ..... YES  
  Water Quality ..... NO  
Infiltration Method ..... HORTON  
Flow Routing Method ..... DYNWAVE  
Starting Date ..... JAN-21-2016 00:00:00  
Ending Date ..... JAN-21-2016 12:00:00  
Antecedent Dry Days ..... 0.0  
Report Time Step ..... 00:01:00  
Wet Time Step ..... 00:01:00  
Dry Time Step ..... 01:00:00  
Routing Time Step ..... 5.00 sec

WARNING 03: negative offset ignored for Link CB10-11ROADLINK  
WARNING 04: minimum elevation drop used for Conduit CB12-13ROADLINK  
WARNING 04: minimum elevation drop used for Conduit CB27-28ROADLINK  
WARNING 04: minimum elevation drop used for Conduit CB29-30ROADLINK  
WARNING 02: maximum depth increased for Node CB07-08  
WARNING 02: maximum depth increased for Node CB09-10  
WARNING 02: maximum depth increased for Node CB13-14  
WARNING 02: maximum depth increased for Node CB13-14ROAD  
WARNING 02: maximum depth increased for Node CB32-33  
WARNING 02: maximum depth increased for Node CB32-33 ROAD  
WARNING 02: maximum depth increased for Node CB34-35  
WARNING 02: maximum depth increased for Node CB34-35 ROAD  
WARNING 02: maximum depth increased for Node CB57-58  
WARNING 02: maximum depth increased for Node CB60  
WARNING 02: maximum depth increased for Node CB61

\*\*\*\*\*  
Element Count  
\*\*\*\*\*  
Number of rain gages ..... 1

Number of subcatchments ... 57  
Number of nodes ..... 162  
Number of links ..... 220  
Number of pollutants ..... 0  
Number of land uses ..... 0

\*\*\*\*\*  
Raingage Summary  
\*\*\*\*\*

Name	Data Source	Data Type	Recording Interval
RAIN	C100-4	INTENSITY	10 min.

\*\*\*\*\*  
Subcatchment Summary  
\*\*\*\*\*

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
A-01	0.23	43.00	64.00	0.7500	RAIN	CB01-02
A-02	0.25	36.00	64.00	0.7500	RAIN	CB03-04
A-03	0.13	36.00	64.00	0.7500	RAIN	CB13-14ROAD
A-04	0.30	34.00	64.00	0.7500	RAIN	CB05-06
A-05	0.28	37.00	64.00	0.7500	RAIN	CB11-12ROAD
A-06	0.27	37.00	64.00	0.7500	RAIN	CB07-08
A-07	0.29	39.00	64.00	0.7500	RAIN	CB15-16
A-08	0.25	41.00	64.00	0.7500	RAIN	CB17-18
A-09	0.08	23.00	64.00	0.7500	RAIN	CB19
A-10	0.19	37.00	64.00	0.7500	RAIN	CB20-21
A-11	0.23	25.00	64.00	0.7500	RAIN	CB20-21
A-12	0.04	24.00	64.00	0.7500	RAIN	CB59
A-13	0.20	24.00	64.00	0.7500	RAIN	CB23
A-14	0.20	23.00	64.00	0.7500	RAIN	CB22
A-15	0.11	30.00	64.00	0.7500	RAIN	CB64
A-16	0.13	32.00	64.00	0.7500	RAIN	CB63
A-17	0.25	49.00	64.00	0.7500	RAIN	CB44-45
A-18	0.27	32.00	64.00	0.7500	RAIN	CB42-43
A-19	0.12	31.00	64.00	0.7500	RAIN	CB62
A-20	0.13	29.00	64.00	0.7500	RAIN	CB60
A-21	0.22	27.00	64.00	0.7500	RAIN	CB24-25
A-22	0.06	24.00	64.00	0.7500	RAIN	CB61
A-23	0.46	46.00	64.00	0.7500	RAIN	CB40-41
A-24	0.21	28.00	64.00	0.7500	RAIN	CB34-35 ROAD
A-25	0.21	29.00	64.00	0.7500	RAIN	CB32-33 ROAD
A-26	0.45	45.00	64.00	0.7500	RAIN	CB26-27
A-27	0.19	32.00	64.00	0.7500	RAIN	CB30-31
A-28	0.23	18.00	64.00	0.7500	RAIN	CB29
A-29	0.23	46.00	64.00	0.7500	RAIN	CB09-10
A-30	0.50	71.00	64.00	0.7500	RAIN	A30-STOR
A-31	0.06	20.00	64.00	0.7500	RAIN	CB66
A-32	0.45	56.00	64.00	0.7500	RAIN	CB67-68-69
A-33	0.82	117.00	64.00	0.7500	RAIN	A33-STOR
A-34	0.77	55.00	64.00	0.7500	RAIN	4-GreenbankOut
A-35	0.98	70.00	64.00	0.7500	RAIN	4-GreenbankOut
A-36	0.45	60.00	64.00	0.7500	RAIN	4-GreenbankOut
A-37	0.37	53.00	64.00	0.7500	RAIN	4-GreenbankOut
B-01	0.09	28.00	64.00	0.7500	RAIN	CB36
B-02	0.15	23.00	64.00	0.7500	RAIN	CB37
B-03	0.10	32.00	64.00	0.7500	RAIN	CB39
B-04	0.83	110.00	64.00	0.7500	RAIN	B04 Stor
B-05	0.06	27.00	64.00	0.7500	RAIN	CB-65
B-06	0.08	19.00	64.00	0.7500	RAIN	CB46-47
B-07	0.13	32.00	64.00	0.7500	RAIN	CB48-49
B-08	0.23	32.00	64.00	0.7500	RAIN	CB50-51
B-09	0.25	34.00	64.00	0.7500	RAIN	CB52-53
B-10	1.12	149.00	14.00	0.1000	RAIN	CB52-53
B-11	0.05	13.00	64.00	0.7500	RAIN	CB54

**Burnett Lands – 3370 Greenbank Road**  
**100-year Storm, 5-year Fixed Outlet Elevations**  
**Model Output**

B-12	0.09	20.00	64.00	0.7500	RAIN
B-13	0.10	22.00	64.00	0.7500	RAIN
B-14	0.32	73.00	64.00	0.7500	RAIN
B-15	0.12	27.00	64.00	0.7500	RAIN
B-16	0.14	31.00	64.00	0.7500	RAIN
B-17	0.27	33.00	64.00	0.7500	RAIN
B-18	0.12	28.00	64.00	0.7500	RAIN
B-19	0.11	22.00	64.00	0.7500	RAIN
CaivanLands	8.10	200.00	64.00	0.5000	RAIN

CB100
CB102
CB55-56
CB104
CB106
CB57-58
CB108
CB110
Caivan-Stor

206 (STM)	JUNCTION	90.77	2.85	0.0
208 (STM)	JUNCTION	90.69	3.13	0.0
208a	JUNCTION	90.65	3.02	0.0
210 (STM)	JUNCTION	90.53	2.99	0.0
212 (STM)	JUNCTION	90.40	3.33	0.0
214 (STM)	JUNCTION	90.30	3.17	0.0
216 (STM)	JUNCTION	90.01	3.36	0.0
304 (STM)	JUNCTION	90.86	2.51	0.0
306 (STM)	JUNCTION	90.65	2.93	0.0
306a (STM)	JUNCTION	90.52	2.96	0.0
308 (STM)	JUNCTION	90.41	3.00	0.0
310 (STM)	JUNCTION	90.07	3.39	0.0
312 (STM)	JUNCTION	89.56	3.80	0.0
314 (STM)	JUNCTION	89.49	3.15	0.0
316 (STM)	JUNCTION	89.36	3.90	0.0
324 (STM)	JUNCTION	89.35	3.30	0.0
326 (STM)	JUNCTION	89.30	3.24	0.0
328 (STM)	JUNCTION	88.91	3.25	0.0
330 (STM)	JUNCTION	89.33	3.21	0.0
402 (STM)	JUNCTION	90.91	2.91	0.0
402a (STM)	JUNCTION	90.84	2.97	0.0
404 (STM)	JUNCTION	90.67	3.13	0.0
404a	JUNCTION	90.63	3.13	0.0
406 (STM)	JUNCTION	90.45	3.25	0.0
408 (STM)	JUNCTION	90.36	3.44	0.0
608 (STM)	JUNCTION	90.55	3.16	0.0
608a	JUNCTION	90.49	3.13	0.0
610 (STM)	JUNCTION	89.88	3.42	0.0
610a	JUNCTION	89.84	3.48	0.0
612 (STM)	JUNCTION	90.12	4.03	0.0
614 (STM)	JUNCTION	90.22	3.50	0.0
616 (STM)	JUNCTION	90.53	3.74	0.0
618 (STM)	JUNCTION	90.36	3.28	0.0
902 (STM)	JUNCTION	90.70	3.27	0.0
904 (STM)	JUNCTION	90.58	3.18	0.0
906 (STM)	JUNCTION	90.49	3.04	0.0
908 (STM)	JUNCTION	90.27	3.30	0.0
908a	JUNCTION	90.19	3.17	0.0
C100 (STM)	JUNCTION	90.89	2.61	0.0
C102 (STM)	JUNCTION	90.85	2.65	0.0
C104 (STM)	JUNCTION	90.81	2.69	0.0
C106 (STM)	JUNCTION	90.75	2.75	0.0
C108 (STM)	JUNCTION	90.72	2.78	0.0
C110 (STM)	JUNCTION	90.67	2.83	0.0
CB01-02	JUNCTION	92.05	1.50	0.0
CB03-04	JUNCTION	91.89	1.59	0.0
CB05-06	JUNCTION	91.85	1.50	0.0
CB07-08	JUNCTION	91.72	1.65	0.0
CB09-10	JUNCTION	91.91	1.40	0.0
CB100	JUNCTION	92.60	1.75	0.0
CB102	JUNCTION	92.60	1.75	0.0
CB104	JUNCTION	92.60	1.75	0.0
CB106	JUNCTION	92.60	1.75	0.0
CB108	JUNCTION	92.60	1.75	0.0
CB110	JUNCTION	92.60	1.75	0.0
CB11-12	JUNCTION	92.38	1.54	0.0
CB11-12ROAD	JUNCTION	93.62	0.30	0.0
CB13-14	JUNCTION	92.64	1.61	0.0
CB13-14ROAD	JUNCTION	93.84	0.41	0.0
CB15-16	JUNCTION	92.00	1.50	0.0
CB17-18	JUNCTION	92.31	1.50	0.0
CB19	JUNCTION	92.24	1.50	0.0
CB20-21	JUNCTION	92.14	1.50	0.0
CB22	JUNCTION	92.29	1.50	0.0
CB23	JUNCTION	92.08	1.50	0.0
CB24-25	JUNCTION	92.05	1.50	0.0
CB26-27	JUNCTION	91.88	1.50	0.0
CB28	JUNCTION	91.78	1.51	0.0
CB29	JUNCTION	91.78	1.50	0.0

\*\*\*\*\*  
 Node Summary  
 \*\*\*\*\*

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
01+011	JUNCTION	93.48	0.30	0.0	
01+051	JUNCTION	93.76	0.30	0.0	
01+118	JUNCTION	93.72	0.30	0.0	
01+143	JUNCTION	93.93	0.30	0.0	
02+196	JUNCTION	93.67	0.30	0.0	
02+288	JUNCTION	93.76	0.30	0.0	
02+365	JUNCTION	93.84	0.30	0.0	
02+432	JUNCTION	93.86	0.30	0.0	
02+458	JUNCTION	93.77	0.30	0.0	
02+512	JUNCTION	93.86	0.30	0.0	
02+564	JUNCTION	94.40	0.30	0.0	
03+026	JUNCTION	93.56	0.30	0.0	
03+054	JUNCTION	93.61	0.30	0.0	
03+109	JUNCTION	93.44	0.30	0.0	
03+127	JUNCTION	93.37	0.30	0.0	
03+200	JUNCTION	93.42	0.30	0.0	
03+291	JUNCTION	93.34	0.30	0.0	
03+384	JUNCTION	93.34	0.30	0.0	
04+078	JUNCTION	93.65	0.30	0.0	
04+089	JUNCTION	93.76	0.30	0.0	
04+129	JUNCTION	93.64	0.30	0.0	
04+207	JUNCTION	93.87	0.30	0.0	
05+116	JUNCTION	93.93	0.30	0.0	
05+200	JUNCTION	93.49	0.30	0.0	
06+023	JUNCTION	93.64	0.30	0.0	
06+043	JUNCTION	93.74	0.30	0.0	
06+108	JUNCTION	93.60	0.30	0.0	
06+194	JUNCTION	93.51	0.30	0.0	
06+273	JUNCTION	93.40	0.30	0.0	
06+309	JUNCTION	93.88	0.30	0.0	
06+332	JUNCTION	94.51	0.30	0.0	
06+355	JUNCTION	94.15	0.30	0.0	
09+000	JUNCTION	93.36	0.30	0.0	
09+097	JUNCTION	93.65	0.30	0.0	
09+203	JUNCTION	93.89	0.30	0.0	
10+011	JUNCTION	93.58	0.30	0.0	
10+232	JUNCTION	94.36	0.30	0.0	
102 (STM)	JUNCTION	90.82	2.95	0.0	
104 (STM)	JUNCTION	90.67	3.27	0.0	
106 (STM)	JUNCTION	90.57	3.30	0.0	
106a	JUNCTION	90.50	3.29	0.0	
108 (STM)	JUNCTION	90.40	3.35	0.0	
108a	JUNCTION	90.34	3.21	0.0	
11+000	JUNCTION	94.01	0.30	0.0	
11+086	JUNCTION	94.26	0.30	0.0	
110 (STM)	JUNCTION	90.24	3.22	0.0	
110a	JUNCTION	90.17	3.21	0.0	
200 (STM)	JUNCTION	91.33	3.21	0.0	
202 (STM)	JUNCTION	91.25	2.89	0.0	
202a	JUNCTION	91.14	2.87	0.0	
204 (STM)	JUNCTION	91.01	2.93	0.0	
204a	JUNCTION	90.91	2.90	0.0	









**Burnett Lands – 3370 Greenbank Road**  
**100-year Storm, 5-year Fixed Outlet Elevations**  
**Model Output**



\*\*\*\*\*  
Control Actions Taken  
\*\*\*\*\*

```

*****
Volume      Depth
Runoff Quantity Continuity hectare-m      mm
-----
Total Precipitation .....    1.749    76.002
Evaporation Loss .....         0.000      0.000
Infiltration Loss .....        0.486    21.107
Surface Runoff .....           1.245    54.106
Final Surface Storage ....       0.012     0.543
Continuity Error (%) .....       0.323

```

```

*****
Volume      Volume
Flow Routing Continuity hectare-m      10^6 ltr
-----
Dry Weather Inflow .....         0.000      0.000
Wet Weather Inflow .....        1.245    12.450
Groundwater Inflow .....         0.000      0.000
RDII Inflow .....               0.000      0.000
External Inflow .....           0.002     0.025
External Outflow .....          1.264    12.642
Internal Outflow .....           0.000      0.000
Storage Losses .....            0.000      0.000
Initial Stored Volume ....        0.070     0.696
Final Stored Volume .....         0.070     0.696
Continuity Error (%) .....       -1.268

```

\*\*\*\*\*  
Highest Continuity Errors  
\*\*\*\*\*  
Node 06+194 (-22.16%)  
Node CB11-12ROAD (-16.70%)  
Node 02+196 (-12.32%)  
Node 02+512 (-3.77%)  
Node 316 (STM) (-2.66%)

\*\*\*\*\*  
Time-Step Critical Elements  
\*\*\*\*\*  
Link 208-208a (2.63%)  
Link C110-216 (1.39%)

\*\*\*\*\*  
Highest Flow Instability Indexes  
\*\*\*\*\*  
Link VORTECHS (114)  
Link 324-330 (109)  
Link {STM}.330-336 (96)  
Link {STM}.316-324 (90)  
Link {STM}.314-316 (88)

\*\*\*\*\*  
Routing Time Step Summary  
\*\*\*\*\*  
Minimum Time Step : 0.50 sec  
Average Time Step : 4.76 sec  
Maximum Time Step : 5.00 sec  
Percent in Steady State : 0.00  
Average Iterations per Step : 6.68

\*\*\*\*\*  
Subcatchment Runoff Summary  
\*\*\*\*\*

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
A-01	76.00	0.00	0.00	17.96	57.58	0.14	86.67	0.758
A-02	76.00	0.00	0.00	18.19	57.34	0.14	89.11	0.755
A-03	76.00	0.00	0.00	17.62	58.18	0.07	49.91	0.766
A-04	76.00	0.00	0.00	18.48	57.05	0.17	104.88	0.751
A-05	76.00	0.00	0.00	18.27	57.52	0.16	97.73	0.757
A-06	76.00	0.00	0.00	18.26	57.37	0.16	97.29	0.755
A-07	76.00	0.00	0.00	18.27	57.27	0.17	102.56	0.753
A-08	76.00	0.00	0.00	18.05	57.74	0.14	89.84	0.760
A-09	76.00	0.00	0.00	17.62	57.93	0.05	31.75	0.762
A-10	76.00	0.00	0.00	17.89	57.65	0.11	69.70	0.759
A-11	76.00	0.00	0.00	18.50	57.03	0.13	78.02	0.750
A-12	76.00	0.00	0.00	17.23	48.71	0.02	11.43	0.641
A-13	76.00	0.00	0.00	18.40	57.13	0.11	69.98	0.752
A-14	76.00	0.00	0.00	18.42	57.11	0.11	68.09	0.751
A-15	76.00	0.00	0.00	17.62	48.69	0.05	33.33	0.641
A-16	76.00	0.00	0.00	17.71	48.68	0.06	40.62	0.641
A-17	76.00	0.00	0.00	17.88	57.66	0.14	91.77	0.759
A-18	76.00	0.00	0.00	18.43	57.10	0.16	94.95	0.751
A-19	76.00	0.00	0.00	17.70	48.68	0.06	38.72	0.641
A-20	76.00	0.00	0.00	17.79	48.68	0.06	41.25	0.641
A-21	76.00	0.00	0.00	18.36	57.17	0.12	76.26	0.752
A-22	76.00	0.00	0.00	17.43	48.69	0.03	19.05	0.641
A-23	76.00	0.00	0.00	18.62	57.15	0.26	156.84	0.752
A-24	76.00	0.00	0.00	18.27	57.51	0.12	74.24	0.757
A-25	76.00	0.00	0.00	18.26	57.52	0.12	76.12	0.757
A-26	76.00	0.00	0.00	18.63	57.10	0.26	153.65	0.751
A-27	76.00	0.00	0.00	18.04	57.75	0.11	69.48	0.760
A-28	76.00	0.00	0.00	18.99	56.02	0.13	76.79	0.737
A-29	76.00	0.00	0.00	17.88	57.76	0.13	86.15	0.760
A-30	76.00	0.00	0.00	18.20	57.33	0.29	177.82	0.754
A-31	76.00	0.00	0.00	17.53	57.52	0.03	24.31	0.757
A-32	76.00	0.00	0.00	18.36	57.42	0.26	158.56	0.756
A-33	76.00	0.00	0.00	18.20	57.33	0.47	291.72	0.754
A-34	76.00	0.00	0.00	19.13	55.88	0.43	254.07	0.735
A-35	76.00	0.00	0.00	19.13	55.88	0.55	322.85	0.735
A-36	76.00	0.00	0.00	18.28	56.75	0.26	159.31	0.747
A-37	76.00	0.00	0.00	18.21	56.82	0.21	132.74	0.748
B-01	76.00	0.00	0.00	17.58	57.98	0.05	36.54	0.763
B-02	76.00	0.00	0.00	18.12	56.91	0.08	53.60	0.749
B-03	76.00	0.00	0.00	17.53	57.63	0.06	38.76	0.758
B-04	76.00	0.00	0.00	18.28	57.25	0.47	292.34	0.753
B-05	76.00	0.00	0.00	17.41	48.70	0.03	20.40	0.641
B-06	76.00	0.00	0.00	17.71	57.34	0.04	29.20	0.754
B-07	76.00	0.00	0.00	17.70	57.75	0.07	48.83	0.760
B-08	76.00	0.00	0.00	18.21	57.32	0.13	80.50	0.754
B-09	76.00	0.00	0.00	18.24	57.30	0.14	87.29	0.754
B-10	76.00	0.00	0.00	51.72	24.07	0.27	97.39	0.317
B-11	76.00	0.00	0.00	17.66	57.39	0.03	18.84	0.755
B-12	76.00	0.00	0.00	17.77	57.78	0.05	33.18	0.760
B-13	76.00	0.00	0.00	17.78	57.77	0.06	36.88	0.760
B-14	76.00	0.00	0.00	17.77	57.77	0.18	121.50	0.760
B-15	76.00	0.00	0.00	17.79	57.76	0.07	45.60	0.760
B-16	76.00	0.00	0.00	17.77	57.77	0.08	51.58	0.760
B-17	76.00	0.00	0.00	18.39	57.13	0.16	95.77	0.752
B-18	76.00	0.00	0.00	17.77	57.78	0.07	46.21	0.760
B-19	76.00	0.00	0.00	17.86	57.68	0.06	40.18	0.759
CaivanLands	76.00	0.00	0.00	21.68	53.76	4.36	2057.72	0.707

\*\*\*\*\*





**Burnett Lands – 3370 Greenbank Road**  
**100-year Storm, 5-year Fixed Outlet Elevations**  
**Model Output**



ID	Type	Flow	Elevation	Time	Surcharge	Depth
C108 (STM)	JUNCTION	0.00	38.69	0 02:03	0.000	0.062
C110 (STM)	JUNCTION	0.00	1077.82	0 01:39	0.000	4.405
CB01-02	JUNCTION	86.67	86.67	0 01:30	0.135	0.135
CB03-04	JUNCTION	89.11	90.06	0 01:30	0.143	0.201
CB05-06	JUNCTION	104.87	104.87	0 01:30	0.172	0.172
CB07-08	JUNCTION	97.28	97.28	0 01:30	0.157	0.157
CB09-10	JUNCTION	86.15	145.07	0 01:30	0.133	0.189
CB100	JUNCTION	33.18	33.18	0 01:30	0.050	0.050
CB102	JUNCTION	36.88	36.88	0 01:30	0.056	0.056
CB104	JUNCTION	45.59	45.59	0 01:30	0.069	0.069
CB106	JUNCTION	51.57	51.57	0 01:30	0.078	0.078
CB108	JUNCTION	46.20	46.20	0 01:30	0.070	0.070
CB110	JUNCTION	40.18	40.18	0 01:30	0.062	0.062
CB11-12	JUNCTION	0.00	30.01	0 01:30	0.000	0.129
CB11-12ROAD	JUNCTION	97.73	97.73	0 01:30	0.158	0.158
CB13-14	JUNCTION	0.00	0.00	0 00:00	0.000	0.000
CB13-14ROAD	JUNCTION	49.90	49.90	0 01:30	0.074	0.074
CB15-16	JUNCTION	102.55	149.94	0 01:30	0.165	0.224
CB17-18	JUNCTION	89.83	137.38	0 01:30	0.142	0.218
CB19	JUNCTION	31.75	62.74	0 01:29	0.047	0.059
CB20-21	JUNCTION	147.70	147.70	0 01:30	0.236	0.241
CB22	JUNCTION	68.09	68.09	0 01:30	0.111	0.111
CB23	JUNCTION	69.98	70.02	0 01:30	0.114	0.114
CB24-25	JUNCTION	76.25	76.25	0 01:30	0.124	0.124
CB26-27	JUNCTION	153.65	153.65	0 01:30	0.256	0.256
CB28	JUNCTION	0.00	78.41	0 01:37	0.000	0.089
CB29	JUNCTION	76.79	106.76	0 01:37	0.129	0.191
CB30-31	JUNCTION	69.48	207.92	0 01:30	0.110	0.354
CB32-33	JUNCTION	0.00	0.00	0 00:00	0.000	0.000
CB32-33 ROAD	JUNCTION	76.12	145.75	0 01:30	0.123	0.243
CB34-35	JUNCTION	0.00	0.00	0 00:00	0.000	0.000
CB34-35 ROAD	JUNCTION	74.23	74.23	0 01:30	0.120	0.120
CB36	JUNCTION	36.54	36.54	0 01:30	0.054	0.054
CB37	JUNCTION	53.60	73.11	0 01:30	0.085	0.093
CB39	JUNCTION	38.75	50.49	0 01:26	0.056	0.060
CB40-41	JUNCTION	156.84	161.92	0 01:30	0.261	0.264
CB42-43	JUNCTION	94.94	114.29	0 01:30	0.155	0.168
CB44-45	JUNCTION	91.76	91.76	0 01:30	0.142	0.143
CB46-47	JUNCTION	29.20	73.72	0 01:30	0.044	0.072
CB48-49	JUNCTION	48.83	48.83	0 01:30	0.073	0.073
CB50-51	JUNCTION	80.49	80.49	0 01:30	0.129	0.130
CB52-53	JUNCTION	184.67	184.67	0 01:30	0.409	0.409
CB54	JUNCTION	18.84	36.44	0 01:30	0.028	0.036
CB55-56	JUNCTION	121.49	178.44	0 01:30	0.185	0.210
CB57-58	JUNCTION	95.76	162.61	0 01:30	0.156	0.188
CB59	JUNCTION	11.43	11.43	0 01:29	0.018	0.018
CB60	JUNCTION	41.25	41.25	0 01:29	0.063	0.063
CB61	JUNCTION	19.05	19.05	0 01:29	0.029	0.029
CB62	JUNCTION	38.72	38.72	0 01:29	0.059	0.059
CB63	JUNCTION	40.62	40.62	0 01:29	0.062	0.062
CB64	JUNCTION	33.33	33.33	0 01:29	0.051	0.051
CB-65	JUNCTION	20.40	20.40	0 01:29	0.031	0.044
CB66	JUNCTION	24.30	24.30	0 01:30	0.035	0.035
CB67-68-69	JUNCTION	158.55	158.55	0 01:30	0.259	0.259
HP-01	JUNCTION	0.00	0.00	0 00:00	0.000	0.000
HP-03	JUNCTION	0.00	1.43	0 01:30	0.000	0.000
RYP-01	JUNCTION	0.00	0.00	0 00:00	0.000	0.000
RYP-02	JUNCTION	0.00	0.00	0 00:00	0.000	0.000
RYP-03	JUNCTION	0.00	2.70	0 01:30	0.000	0.000
RYP-04	JUNCTION	0.00	0.00	0 00:00	0.000	0.000
1-HW-01 (STM)	OUTFALL	0.00	1303.10	0 01:36	0.000	4.758
2-OverlandOutlet	OUTFALL	0.00	4.17	0 01:36	0.000	0.002
3-HW-02 (STM)	OUTFALL	0.00	1672.18	0 01:37	0.000	6.459
4-GreenbankOut	OUTFALL	868.92	868.92	0 01:30	1.447	1.447
A30-STOR	STORAGE	177.81	177.81	0 01:30	0.285	0.285
A33-STOR	STORAGE	291.70	291.70	0 01:30	0.468	0.468
B04_Stor	STORAGE	292.32	292.32	0 01:30	0.472	0.472
Caivan-Stor	STORAGE	2057.67	2057.67	0 01:30	4.355	4.355

\*\*\*\*\*  
Node Surcharge Summary  
\*\*\*\*\*

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
102 (STM)	JUNCTION	0.12	0.102	2.242
104 (STM)	JUNCTION	0.28	0.223	2.396
106 (STM)	JUNCTION	0.40	0.309	2.337
106a	JUNCTION	0.50	0.408	2.272
108 (STM)	JUNCTION	0.51	0.408	2.258
108a	JUNCTION	0.57	0.430	2.094
110 (STM)	JUNCTION	0.58	0.418	2.041
110a	JUNCTION	0.62	0.416	2.036
206 (STM)	JUNCTION	0.22	0.036	2.052
208 (STM)	JUNCTION	0.20	0.025	2.343
306a (STM)	JUNCTION	0.02	0.002	2.348
308 (STM)	JUNCTION	0.31	0.093	2.284
310 (STM)	JUNCTION	0.59	0.248	2.376
312 (STM)	JUNCTION	0.67	0.287	2.292
314 (STM)	JUNCTION	0.73	0.266	1.653
316 (STM)	JUNCTION	12.00	0.262	2.389
324 (STM)	JUNCTION	0.56	0.084	1.868
326 (STM)	JUNCTION	0.72	0.061	1.829
328 (STM)	JUNCTION	12.00	0.199	1.503
330 (STM)	JUNCTION	12.00	0.206	1.780
402a (STM)	JUNCTION	0.09	0.046	2.314
404 (STM)	JUNCTION	0.24	0.158	2.314
404a	JUNCTION	0.40	0.241	2.276
406 (STM)	JUNCTION	0.05	0.028	2.171
408 (STM)	JUNCTION	0.32	0.236	2.266
608 (STM)	JUNCTION	0.43	0.278	2.244
608a	JUNCTION	0.50	0.346	2.170
610 (STM)	JUNCTION	0.63	0.369	2.055
610a	JUNCTION	0.65	0.339	2.150
612 (STM)	JUNCTION	0.57	0.323	3.050
614 (STM)	JUNCTION	0.55	0.413	2.474
616 (STM)	JUNCTION	0.40	0.740	2.544
618 (STM)	JUNCTION	0.52	0.269	2.405
904 (STM)	JUNCTION	0.19	0.083	2.482
906 (STM)	JUNCTION	0.37	0.167	2.256
908 (STM)	JUNCTION	0.54	0.303	2.314
908a	JUNCTION	0.62	0.373	2.106
C110 (STM)	JUNCTION	0.70	1.452	0.638
A30-STOR	STORAGE	0.79	1.321	0.184
A33-STOR	STORAGE	0.70	1.400	0.050
B04_Stor	STORAGE	0.70	1.275	0.173
Caivan-Stor	STORAGE	0.68	0.844	0.166

\*\*\*\*\*  
Node Flooding Summary  
\*\*\*\*\*

No nodes were flooded.

\*\*\*\*\*  
Storage Volume Summary  
\*\*\*\*\*

Average Volume	Avg Pcnt	E&I Pcnt	Maximum Volume	Max Pcnt	Time of Max Occurrence	Maximum Outflow
----------------	----------	----------	----------------	----------	------------------------	-----------------







**Burnett Lands – 3370 Greenbank Road**  
**100-year Storm, 5-year Fixed Outlet Elevations**  
**Model Output**

STB-00	1.27	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-01	1.00	0.96	0.00	0.00	0.03	0.00	0.00	0.00	0.02	0.0000
STB-02	1.00	0.00	0.96	0.00	0.03	0.01	0.00	0.00	0.03	0.0000
STB-03	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.43	0.0000
STB-04	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.81	0.0000
STB-05	1.12	0.00	0.00	0.00	0.95	0.05	0.00	0.00	0.68	0.0000
STB-06	1.00	0.00	0.00	0.00	0.07	0.00	0.00	0.93	0.38	0.0000
STB-07	1.00	0.93	0.07	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-08	1.00	0.87	0.13	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
STB-09	1.00	0.00	0.00	0.00	0.13	0.00	0.00	0.87	0.37	0.0000
STB-10	1.00	0.00	0.00	0.00	0.02	0.00	0.00	0.98	0.48	0.0000
STB-11	1.00	0.90	0.06	0.00	0.02	0.00	0.03	0.00	0.00	0.0000
STB-12	1.00	0.00	0.00	0.00	0.10	0.00	0.00	0.90	0.88	0.0000
STB-13	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	1.00	0.0000

Analysis ended on: Wed Nov 23 10:04:49 2016  
 Total elapsed time: 00:00:04

\*\*\*\*\*  
 Conduit Surcharge Summary  
 \*\*\*\*\*

Conduit	Hours Full			Hours	Hours
	Both Ends	Upstream	Dnstream	Above Full Normal Flow	Capacity Limited
{STM}.102-104	0.12	0.12	0.12	0.01	0.01
{STM}.104-106	0.35	0.35	0.35	0.01	0.01
{STM}.106-106a	0.44	0.44	0.44	0.01	0.01
{STM}.108-108a	0.51	0.51	0.52	0.01	0.01
{STM}.110-110a	0.58	0.58	0.58	0.01	0.05
{STM}.308-310	0.33	0.33	0.33	0.01	0.01
{STM}.310-312	0.59	0.59	0.59	0.01	0.01
{STM}.312-314	0.67	0.67	0.67	0.01	0.39
{STM}.314-316	0.76	0.76	0.76	0.01	0.31
{STM}.316-324	12.00	12.00	12.00	0.11	0.90
{STM}.326-328	12.00	12.00	12.00	0.01	0.01
{STM}.328-HW-1	12.00	12.00	12.00	0.01	0.64
{STM}.330-336	12.00	12.00	12.00	0.01	0.39
{STM}.404-404a	0.35	0.35	0.36	0.01	0.01
{STM}.406-408	0.05	0.05	0.05	0.01	0.01
{STM}.408-106	0.37	0.37	0.37	0.01	0.01
{STM}.608-608a	0.46	0.46	0.46	0.01	0.01
{STM}.610-610a	0.63	0.63	0.63	0.01	0.52
{STM}.612-316	0.66	0.66	0.66	0.01	0.32
{STM}.614-312	0.55	0.55	0.56	0.01	0.44
{STM}.616-614	0.40	0.40	0.41	0.01	0.01
{STM}.618-610	0.52	0.52	0.52	0.01	0.01
{STM}.904-906	0.21	0.21	0.21	0.01	0.01
{STM}.906-908	0.40	0.40	0.40	0.01	0.01
{STM}.908-908a	0.54	0.54	0.54	0.01	0.01
106a-108	0.50	0.50	0.50	0.01	0.01
108a-110	0.57	0.57	0.57	0.01	0.10
110a-610	0.62	0.62	0.62	0.01	0.21
206-208	0.20	0.20	0.20	0.01	0.20
208-208a	0.01	0.01	0.01	0.33	0.01
208a-210	0.01	0.01	0.01	0.39	0.01
210-212	0.01	0.01	0.01	0.17	0.01
212-214	0.01	0.01	0.01	0.33	0.01
306a-308	0.02	0.02	0.02	0.01	0.01
324-330	12.00	12.00	12.00	0.01	0.40
402a-404	0.09	0.09	0.09	0.01	0.01
404a-608	0.40	0.40	0.40	0.01	0.01
608a-110	0.50	0.50	0.50	0.01	0.01
610a-312	0.65	0.65	0.65	0.01	0.54
908a-610	0.62	0.62	0.62	0.01	0.01
C110-216	0.65	0.65	0.65	1.25	0.65

Analysis begun on: Wed Nov 23 10:04:45 2016

## **Appendix E**

### Burnett Municipal Drain Analysis

# MEMORANDUM

---

**DATE:** JUNE 10, 2016  
**TO:** BOB DOWDELL, E.I.T. & EDSON DONNELLY, C.E.T.  
**FROM:** CONRAD STANG, M.A.SC., P.ENG.  
**RE:** SOUTH NEPEAN COLLECTOR CULVERT CROSSINGS  
**CC:** MIKE PETEPIECE, P.ENG.

---

This technical memorandum provides details on the sizing and location of the proposed access road culvert crossings which will be installed as part of Phase 2 of the South Nepean Collector (SNC). It is anticipated that the proposed culverts will remain in place until such time that the subject lands are developed. The location of the proposed culvert crossings and corresponding drainage areas are shown on the attached figure (DSK54):

- Culvert C1: Burnett Municipal Drain
- Culvert C2: Ditch draining Mion property west of K-B SWMF
- Culvert C3: Ditch adjacent the K-B SWMF

## Design Criteria

The culverts are to convey the 10-year peak flows from their respective upstream drainage areas without overtopping the access road, as per Section 6.4.2 of the City of Ottawa Sewer Design Guidelines (October, 2012). For the 600mm culverts this corresponds to a Headwater / Depth (HW/D) of 1.5 (300mm cover), which corresponds to the maximum HW/D ratio as recommended by MTO. Excess flows will overtop the access road and graded back towards the downstream watercourse, as shown in the detail on the attached figure (DSK54).

## Design Flows

The culvert crossings have been designed based on current City of Ottawa standards and rainfall data. Peak flows were estimated using the Visual Otthymo hydrologic model; modeling parameters and results are attached. The 12-hour SCS distribution generated the highest peak flows and was selected as the critical storm distribution for sizing the proposed culverts. Simulated peak flows at the proposed culvert crossings are provided in Table 1.

**Table 1: Simulated Peak Flows at Proposed Culvert Crossings**

Culvert	Culvert Dimensions	Return Period (years)	Peak Flow (m <sup>3</sup> /s)	HW/D (m)	Freeboard (m)
Culvert C1 Burnett Municipal Drain (29.27 ha)	3x 600mm Dia. CSP Culverts L = 8.0m S = 0.75% *Inv. = 91.10m	2-year	0.63	0.82	0.41
		5-year	1.10	1.13	0.22
		10-year	1.44	1.37	0.08
		25-year	1.90	1.65	0.09 Overtopping
		50-year	2.26	1.75	0.15 Overtopping
		100-year	2.66	1.84	0.20 Overtopping
Culvert C2 Mion/Pavic Ditch (5.11 ha)	1x 600mm Dia. CSP Culvert L = 8.0m S = 0.50% *Inv. = 92.30m	2-year	0.09	0.46	0.63
		5-year	0.16	0.64	0.51
		10-year	0.21	0.77	0.44
		25-year	0.28	0.93	0.34
		50-year	0.34	1.07	0.26
		100-year	0.40	1.21	0.17
Culvert C3 KB-SWMF Ditch (1.36 ha)	1x 1000mm Dia. CSP Culvert L = 12.0m S = 0.60% *Inv. = 92.35m	2-year	0.04	0.15	1.85
		5-year	0.07	0.20	1.80
		10-year	0.09	0.23	1.77
		25-year	0.12	0.26	1.74
		50-year	0.15	0.30	1.70
		100-year	0.17	0.32	1.68

\*Inverts to be confirmed in the field.

### Culvert Crossings

The proposed culvert crossings were designed using Autodesk Hydraflow Express culvert sizing software – supporting calculations are attached. A summary of the proposed culvert crossings is provided below.

#### Culvert C1: Burnett Municipal Drain (3x 600mm CSP Culverts)

The Burnett Municipal Drain was established in the late 1960's after the passing of By-Law No. 107-68 (Township of Nepean). A copy of the Burnett Municipal Drain By-Law and October 16<sup>th</sup>, 1968 Engineers report is attached. Land use in the watershed has changed substantially since the adoption of the By-Law and a significant portion of the upstream drainage area has been redirected to the Kennedy-Burnett SWM Facility. It is anticipated that this drain will be abandoned in the near future as development proceeds within the remaining undeveloped areas south of Strandherd Drive.

The Burnett Municipal Drain is a trapezoidal channel with a 3m bottom width, 0.60m depth and side slopes ranging from 2:1 to 4:1. Based on Manning's equation, the Burnett Municipal Drain has a bankfull capacity of 3.88 m<sup>3</sup>/s, which is sufficient to convey the 100-year peak flow from the current upstream drainage area (which is considerably smaller than the original drainage area from the 1968 Engineer's Report).

As per the 1968 by-law, an existing 900mm diameter CSP culvert was installed downstream of the proposed SNC crossing to provide access to the Kelvin Burnett property. The proposed crossing should therefore provide at least a similar flow capacity (approximately 900 L/s).

Based on current design standards, the proposed culvert crossing should be three (3) 600mm diameter CSP culverts, which will provide capacity for a 5-year return period flowing full and a 10-

year return period without over topping the access road. Refer to the attached detailed calculations. Storm events greater than the 10-year return period will overtop the access road, but the grading will confine excess flows to the downstream ditch. The middle 600mm CSP culvert will be countersunk 0.10m.

*Culvert C2: Mion Ditch (600mm CSP Culvert)*

The west watercourse is an intermittent ditch that runs north-to-south through the Mion property. The ditch is a V-bottom ditch with a 0.30m depth and 3:1 / 6:1 side slopes. Based on Manning's equation, the capacity of the ditch is 0.23 m<sup>3</sup>/s, which corresponds to a 10-year storm.

A 600mm diameter CSP culvert has capacity to convey storm events up-to and including the 100-year storm event (0.40 m<sup>3</sup>/s) without overtopping the access road. Refer to the attached detailed calculations. The 600mm CSP culvert will be countersunk 0.10m.

*Culvert C3: KB-SWMF Ditch (1000mm CSP Culvert)*

The watercourse west of the KB-SWMF is a deep intermittent ditch that runs north-to-south adjacent the K-B SWMF. The ditch is a V-bottom ditch with a 2.0m depth and 2.5:1 side slopes. Based on Manning's equation, the capacity of the ditch is 21.06 m<sup>3</sup>/s. The estimated 100-year peak flows from the 1.36 ha catchment are 0.17 m<sup>3</sup>/s; therefore, the ditch has ample capacity to convey large peak flows.

Due to the dimensions of the ditch, a 1000mm diameter CSP culvert is recommended. The capacity of the culvert (HW/D=1.0) is 1.10 m<sup>3</sup>/s. Flows should not overtop the culvert, but if they do they will spill into the KB-SWMF. Refer to the attached detailed calculations.

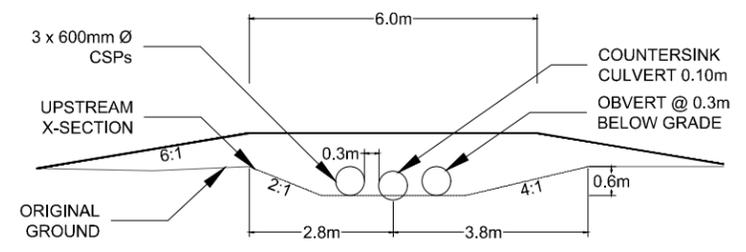
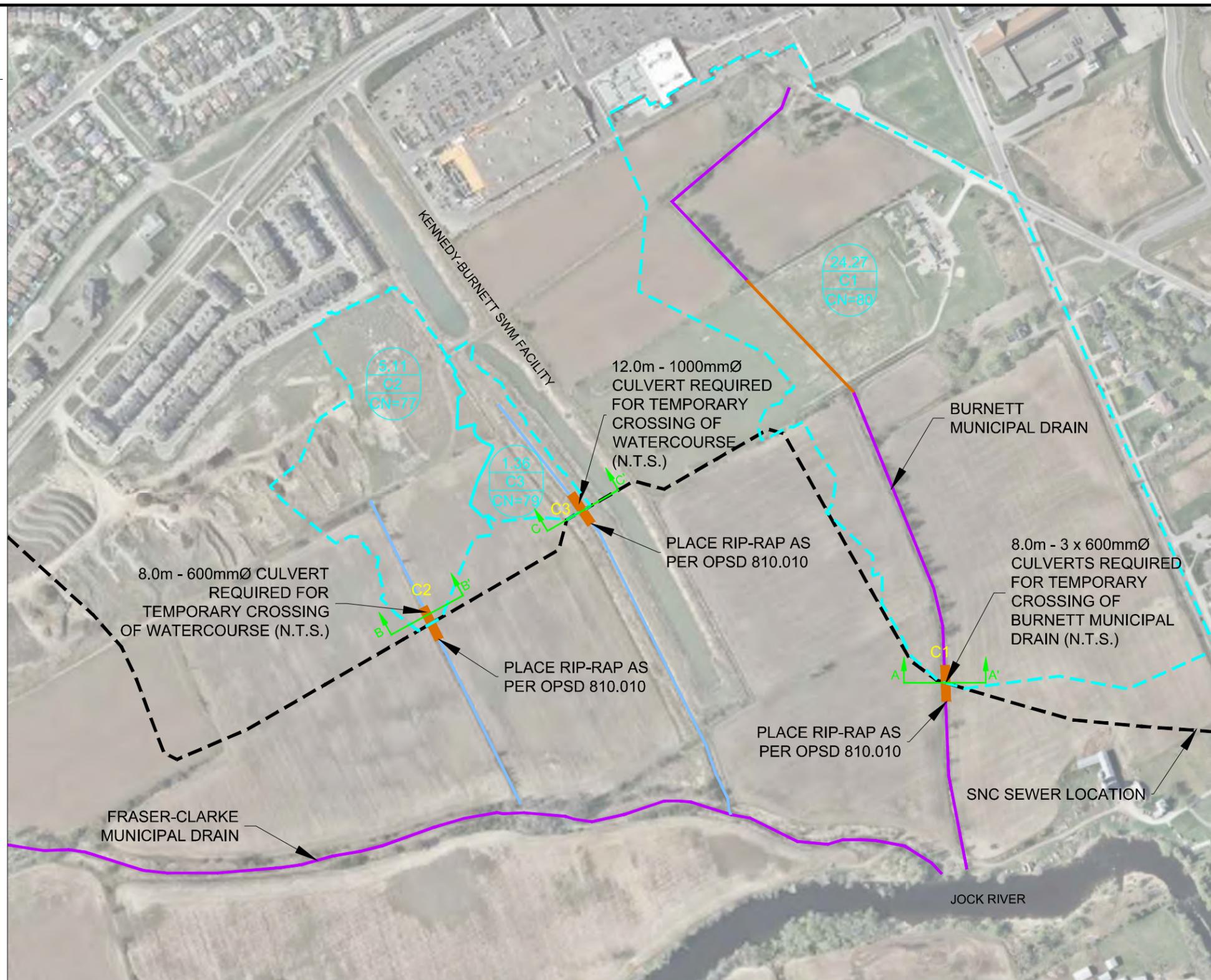
**Access Road**

For culvert crossings C1 and C2, the 6.0m wide access road will be graded adjacent the culvert crossing in order to have two (2) 3.0m wide depressions, which will act as a weir for flows in excess of the culvert capacity. Refer to the cross-section detail on the attached Figure (DSK54).

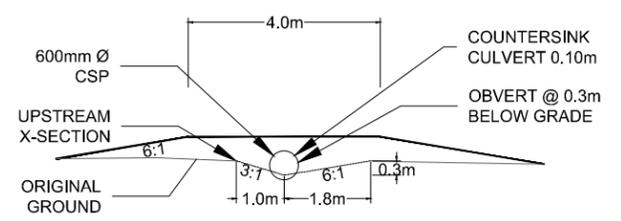
**Attachments:**

- Culvert Locations and Sizes (Figure DSK54)
- Visual Otthymo Modeling Parameters and Results
- Detailed Culvert and Ditch Calculations
- Burnett Municipal Drain By-Law

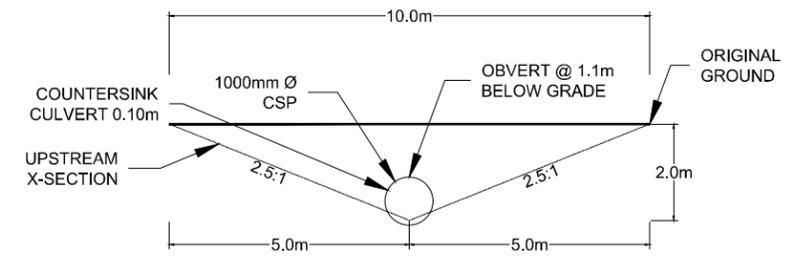




**TEMPORARY CROSSING  
CROSS SECTION A-A' (C1)**  
SCALE: 1:150



**TEMPORARY CROSSING  
CROSS SECTION B-B' (C2)**  
SCALE: 1:150



**TEMPORARY CROSSING  
CROSS SECTION C-C' (C3)**  
SCALE: 1:150

M:\2016\115075\CAD\Design\Figures\DSK54\_Culverts.dwg, SNC, Jun 07, 2016 - 10:01am, nsmit

**LEGEND**

- CULVERT
  - WATERCOURSE
  - MUNICIPAL DRAIN
  - PROPOSED ALIGNMENT OF THE SOUTH NEPEAN COLLECTOR (SNC)
  - DRAINAGE AREA BOUNDARY
- 
- 24.27  
C1  
CN=80 CATCHMENT AREA (ha)
  - AREA ID
  - RUNOFF CURVE NUMBER



Engineers, Planners & Landscape Architects  
Suite 200, 240 Michael Cowpland Drive  
Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643  
Facsimile (613) 254-5867  
Website www.novatech-eng.com

**SOUTH NEPEAN COLLECTOR  
SEWER PHASE 2**

**REQUIRED CULVERT  
LOCATIONS AND SIZES**



DATE JUN 2016	JOB 115075	FIGURE DSK54
------------------	---------------	-----------------

**Time to Peak Calculations**  
 (Uplands Overland Flow Method)

Area ID	Area (ha)	CN	Ia	Overland Flow						Concentrated Overland Flow						Channel Flow				Overall			
				Length (m)	Elevation U/S (m)	Elevation D/S (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Length (m)	Elevation U/S (m)	Elevation D/S (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Length (m)	Slope (%)	Velocity (m/s)	Travel Time (min)	Time of Concentration (min)	Time to Peak (min)	Time to Peak (min)	Time to Peak (hrs)
C1: Burnett MD	29.27	80	6.4	100	95.50	94.50	1.0%	0.28	5.95	150	94.50	92.75	1.2%	0.30	8.33	400	0.7%	0.38	18	32	21	21	0.36
C2: West Ditch	5.11	77	7.6	100	93.30	93.00	0.3%	0.15	11.11	125	93.00	92.75	0.2%	0.13	16.03	150	0.5%	0.32	8	35	23	23	0.39
C3: KB SWMF Ditch	1.36	79	6.8	50	93.35	93.15	0.4%	0.17	4.90	25	93.15	93.05	0.4%	0.16	2.60	150	0.6%	0.35	7	15	10	10	0.17

$Ia = 0.10 \times S$

**Model Results: 12-hour SCS Storm Distribution**

Area ID	Peak Flow (m <sup>3</sup> /s) (Return Period)					
	2-year	5-year	10-year	25-year	50-year	100-year
C1: Burnett MD	0.63	1.10	1.44	1.90	2.26	2.66
C2: West Ditch	0.09	0.16	0.21	0.28	0.34	0.40
C3: KB SWMF Ditch	0.04	0.07	0.09	0.12	0.15	0.17

# Culvert Report

2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

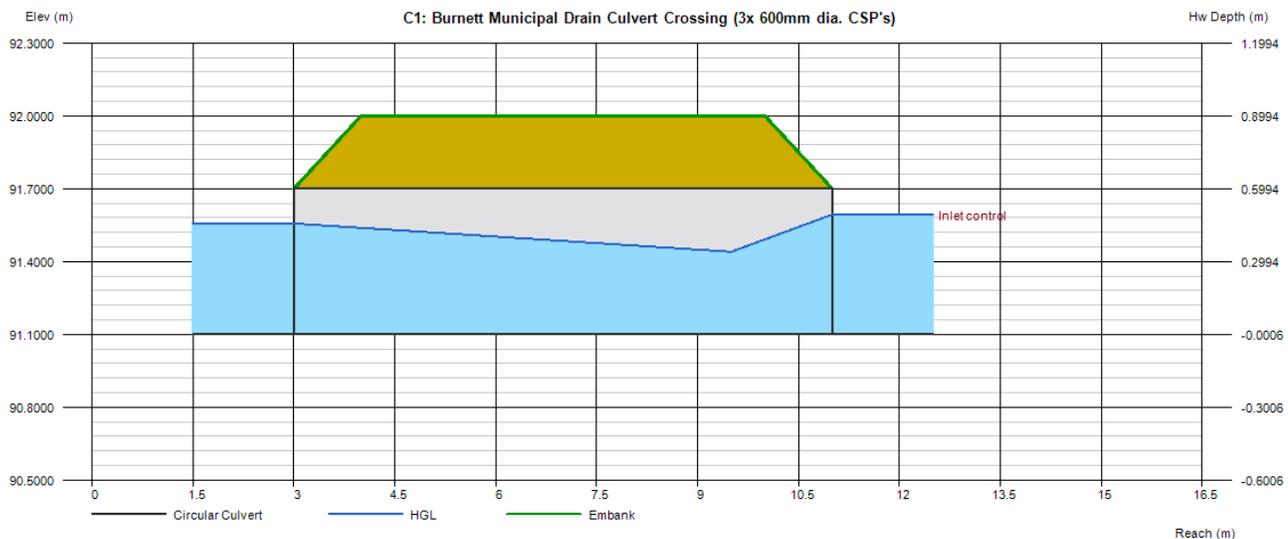
## C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m) = 91.1000  
 Pipe Length (m) = 8.0000  
 Slope (%) = 0.0076  
 Invert Elev Up (m) = 91.1006  
 Rise (mm) = 600.0  
 Shape = Circular  
 Span (mm) = 600.0  
 No. Barrels = 3  
 n-Value = 0.016  
 Culvert Type = Circular Corrugate Metal Pipe  
 Culvert Entrance = Projecting  
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

**Embankment**  
 Top Elevation (m) = 92.0000  
 Top Width (m) = 6.0000  
 Crest Width (m) = 6.0000

**Calculations**  
 Qmin (cms) = 0.0000  
 Qmax (cms) = 2.0000  
 Tailwater Elev (m) = (dc+D)/2

**Highlighted**  
 Qtotal (cms) = 0.7000  
 Qpipe (cms) = 0.7000  
 Qovertop (cms) = 0.0000  
 Veloc Dn (m/s) = 1.0109  
 Veloc Up (m/s) = 1.5645  
 HGL Dn (m) = 91.5565  
 HGL Up (m) = 91.4135  
 Hw Elev (m) = 91.5931  
 Hw/D (m) = 0.8208  
 Flow Regime = Inlet Control



# Culvert Report

5-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

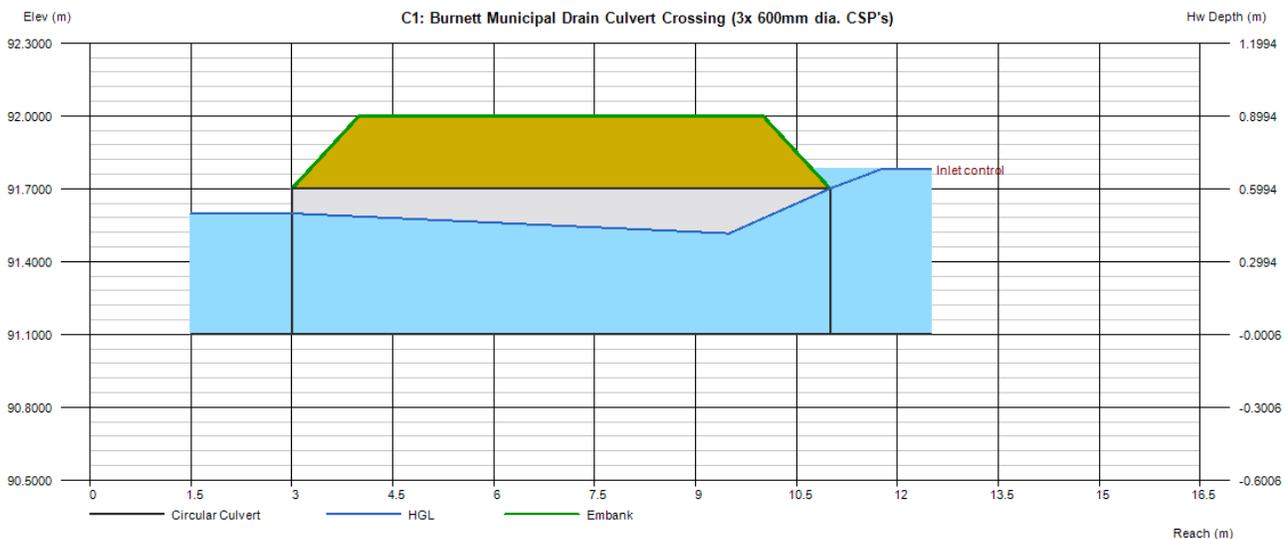
## C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 2.0000
Tailwater Elev (m)	= (dc+D)/2

<b>Highlighted</b>	
Qtotal (cms)	= 1.1000
Qpipe (cms)	= 1.1000
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 1.4614
Veloc Up (m/s)	= 1.8514
HGL Dn (m)	= 91.5981
HGL Up (m)	= 91.4967
Hw Elev (m)	= 91.7793
Hw/D (m)	= 1.1311
Flow Regime	= Inlet Control

<b>Embankment</b>	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000



# Culvert Report

10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Apr 29 2016

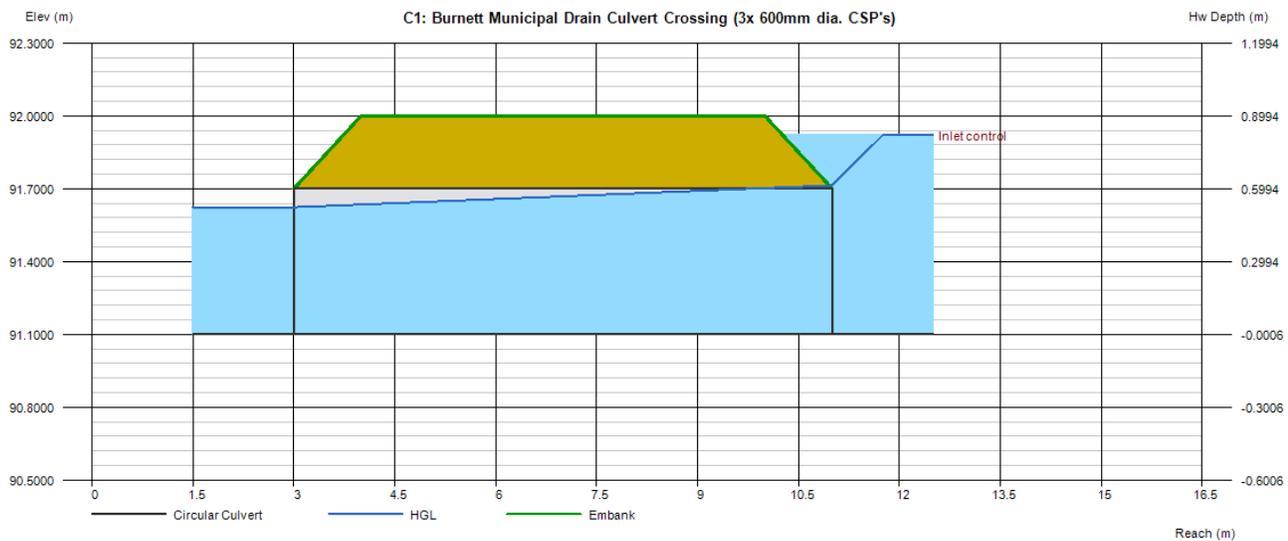
## C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m) = 91.1000  
 Pipe Length (m) = 8.0000  
 Slope (%) = 0.0076  
 Invert Elev Up (m) = 91.1006  
 Rise (mm) = 600.0  
 Shape = Circular  
 Span (mm) = 600.0  
 No. Barrels = 3  
 n-Value = 0.016  
 Culvert Type = Circular Corrugate Metal Pipe  
 Culvert Entrance = Projecting  
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

**Embankment**  
 Top Elevation (m) = 92.0000  
 Top Width (m) = 6.0000  
 Crest Width (m) = 6.0000

**Calculations**  
 Qmin (cms) = 0.0000  
 Qmax (cms) = 2.0000  
 Tailwater Elev (m) = (dc+D)/2

**Highlighted**  
 Qtotal (cms) = 1.4000  
 Qpipe (cms) = 1.4000  
 Qovertop (cms) = 0.0000  
 Veloc Dn (m/s) = 1.7823  
 Veloc Up (m/s) = 1.6505  
 HGL Dn (m) = 91.6237  
 HGL Up (m) = 91.7136  
 Hw Elev (m) = 91.9199  
 Hw/D (m) = 1.3655  
 Flow Regime = Inlet Control



# Culvert Report

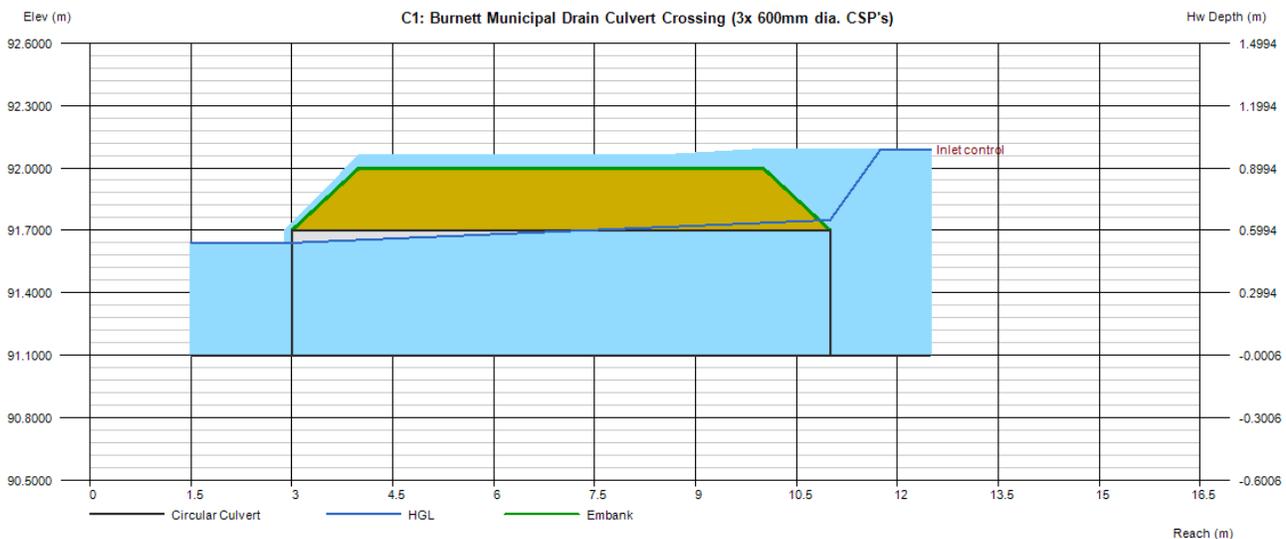
25-year 12-hour SCS Storm

## C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 2.0000
Tailwater Elev (m)	= (dc+D)/2
<b>Highlighted</b>	
Qtotal (cms)	= 1.9000
Qpipe (cms)	= 1.6221
Qovertop (cms)	= 0.2779
Veloc Dn (m/s)	= 2.0174
Veloc Up (m/s)	= 1.9123
HGL Dn (m)	= 91.6400
HGL Up (m)	= 91.7494
Hw Elev (m)	= 92.0880
Hw/D (m)	= 1.6457
Flow Regime	= Inlet Control



# Culvert Report

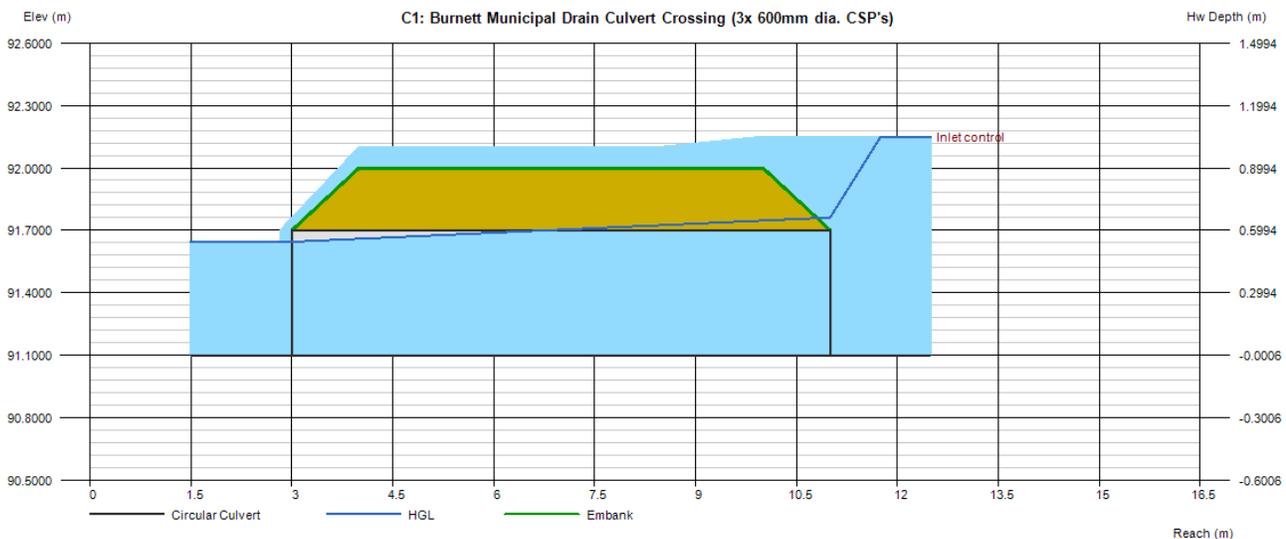
50-year 12-hour SCS Storm

## C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 2.0000
Qmax (cms)	= 3.0000
Tailwater Elev (m)	= (dc+D)/2
<b>Highlighted</b>	
Qtotal (cms)	= 2.3000
Qpipe (cms)	= 1.6952
Qovertop (cms)	= 0.6048
Veloc Dn (m/s)	= 2.0950
Veloc Up (m/s)	= 1.9985
HGL Dn (m)	= 91.6448
HGL Up (m)	= 91.7612
Hw Elev (m)	= 92.1492
Hw/D (m)	= 1.7477
Flow Regime	= Inlet Control



# Culvert Report

100-year 12-hour SCS Storm

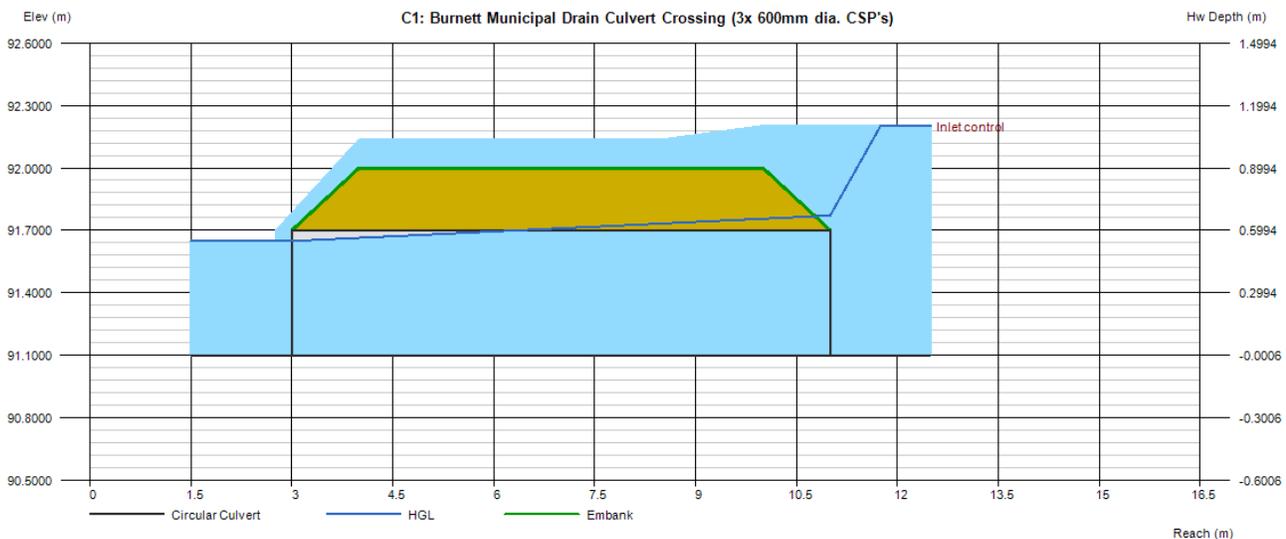
## C1: Burnett Municipal Drain Culvert Crossing (3x 600mm dia. CSP's)

Invert Elev Dn (m)	= 91.1000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0076
Invert Elev Up (m)	= 91.1006
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 3
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 92.0000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 2.0000
Qmax (cms)	= 3.0000
Tailwater Elev (m)	= (dc+D)/2

<b>Highlighted</b>	
Qtotal (cms)	= 2.7000
Qpipe (cms)	= 1.7556
Qovertop (cms)	= 0.9444
Veloc Dn (m/s)	= 2.1591
Veloc Up (m/s)	= 2.0697
HGL Dn (m)	= 91.6486
HGL Up (m)	= 91.7709
Hw Elev (m)	= 92.2017
Hw/D (m)	= 1.8352
Flow Regime	= Inlet Control



# Culvert Report

2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

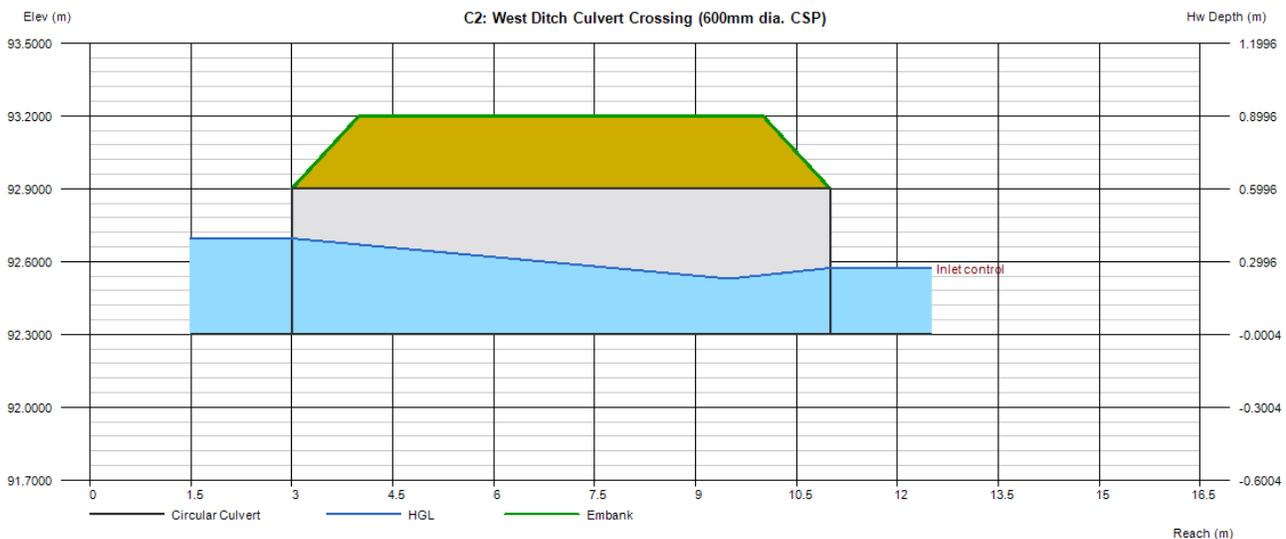
## C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0049
Invert Elev Up (m)	= 92.3004
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 1
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 93.2000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.4000
Tailwater Elev (m)	= (dc+D)/2

<b>Highlighted</b>	
Qtotal (cms)	= 0.0900
Qpipe (cms)	= 0.0900
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.4555
Veloc Up (m/s)	= 1.1656
HGL Dn (m)	= 92.6953
HGL Up (m)	= 92.4910
Hw Elev (m)	= 92.5734
Hw/D (m)	= 0.4550
Flow Regime	= Inlet Control



# Culvert Report

5-year 12-hour SCS Storm

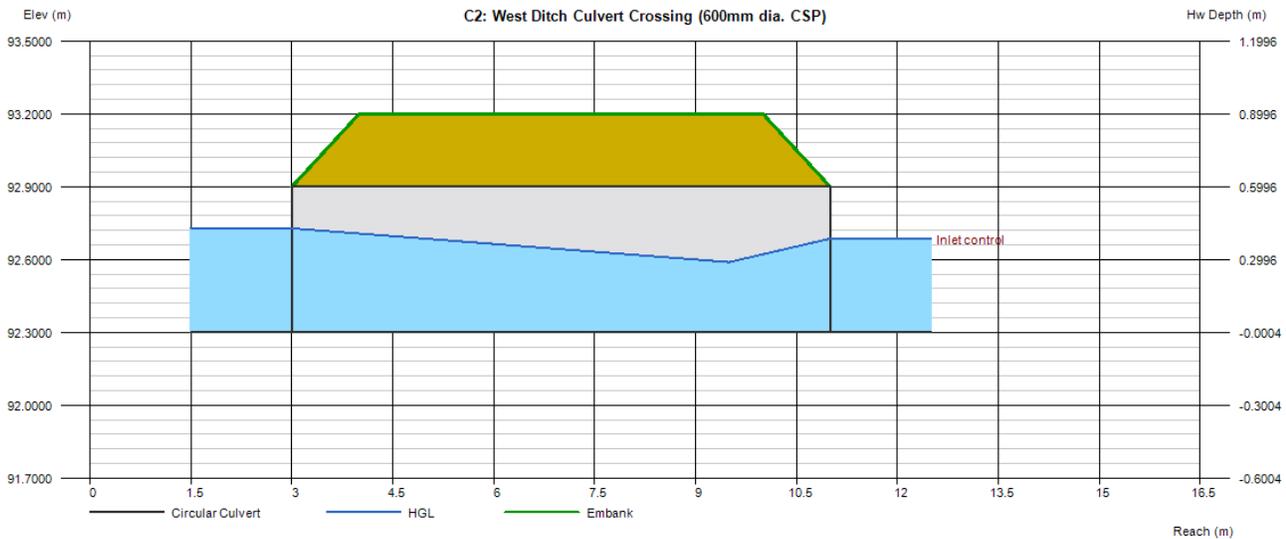
## C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0049
Invert Elev Up (m)	= 92.3004
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 1
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 93.2000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.4000
Tailwater Elev (m)	= (dc+D)/2

<b>Highlighted</b>	
Qtotal (cms)	= 0.1600
Qpipe (cms)	= 0.1600
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.7407
Veloc Up (m/s)	= 1.3844
HGL Dn (m)	= 92.7284
HGL Up (m)	= 92.5573
Hw Elev (m)	= 92.6860
Hw/D (m)	= 0.6428
Flow Regime	= Inlet Control



# Culvert Report

10-year 12-hour SCS Storm

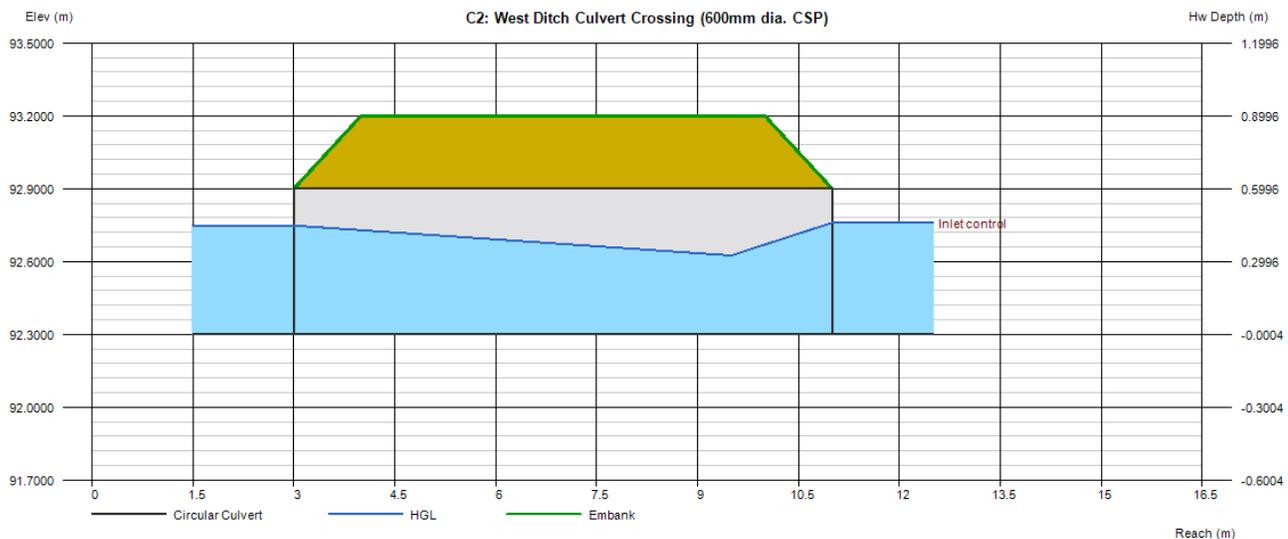
## C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m)	= 92.3000
Pipe Length (m)	= 8.0000
Slope (%)	= 0.0049
Invert Elev Up (m)	= 92.3004
Rise (mm)	= 600.0
Shape	= Circular
Span (mm)	= 600.0
No. Barrels	= 1
n-Value	= 0.016
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 93.2000
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.4000
Tailwater Elev (m)	= (dc+D)/2

<b>Highlighted</b>	
Qtotall (cms)	= 0.2100
Qpipe (cms)	= 0.2100
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.9273
Veloc Up (m/s)	= 1.5101
HGL Dn (m)	= 92.7481
HGL Up (m)	= 92.5966
Hw Elev (m)	= 92.7595
Hw/D (m)	= 0.7652
Flow Regime	= Inlet Control



# Culvert Report

25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

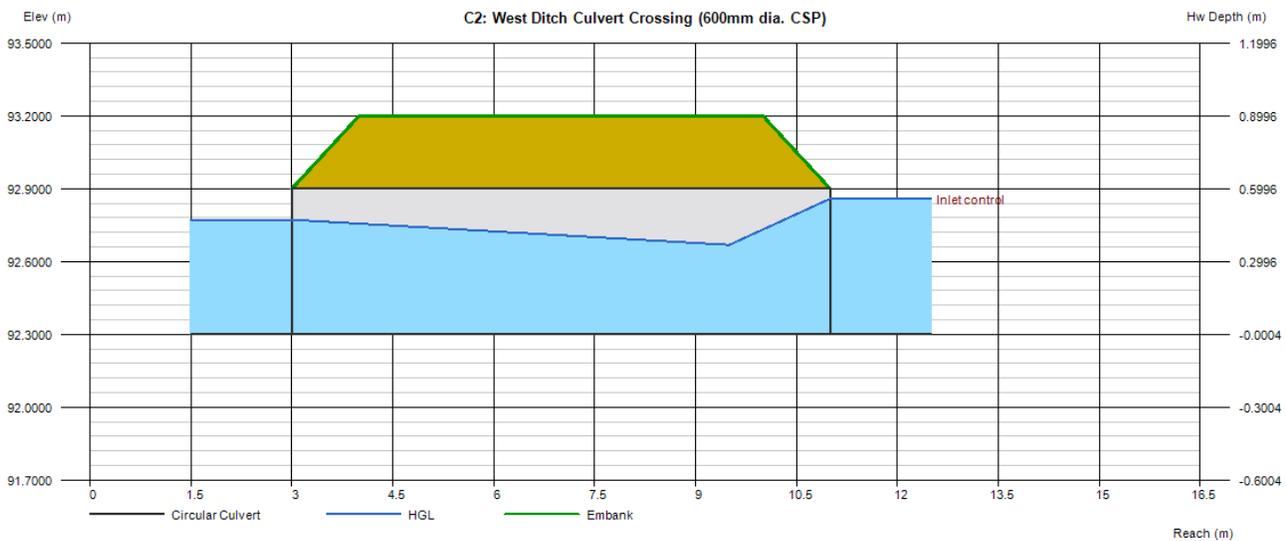
## C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000  
 Pipe Length (m) = 8.0000  
 Slope (%) = 0.0049  
 Invert Elev Up (m) = 92.3004  
 Rise (mm) = 600.0  
 Shape = Circular  
 Span (mm) = 600.0  
 No. Barrels = 1  
 n-Value = 0.016  
 Culvert Type = Circular Corrugate Metal Pipe  
 Culvert Entrance = Projecting  
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

**Embankment**  
 Top Elevation (m) = 93.2000  
 Top Width (m) = 6.0000  
 Crest Width (m) = 6.0000

**Calculations**  
 Qmin (cms) = 0.0000  
 Qmax (cms) = 0.4000  
 Tailwater Elev (m) = (dc+D)/2

**Highlighted**  
 Qtotal (cms) = 0.2800  
 Qpipe (cms) = 0.2800  
 Qovertop (cms) = 0.0000  
 Veloc Dn (m/s) = 1.1731  
 Veloc Up (m/s) = 1.6682  
 HGL Dn (m) = 92.7722  
 HGL Up (m) = 92.6447  
 Hw Elev (m) = 92.8585  
 Hw/D (m) = 0.9302  
 Flow Regime = Inlet Control



# Culvert Report

50-year 12-hour SCS Storm

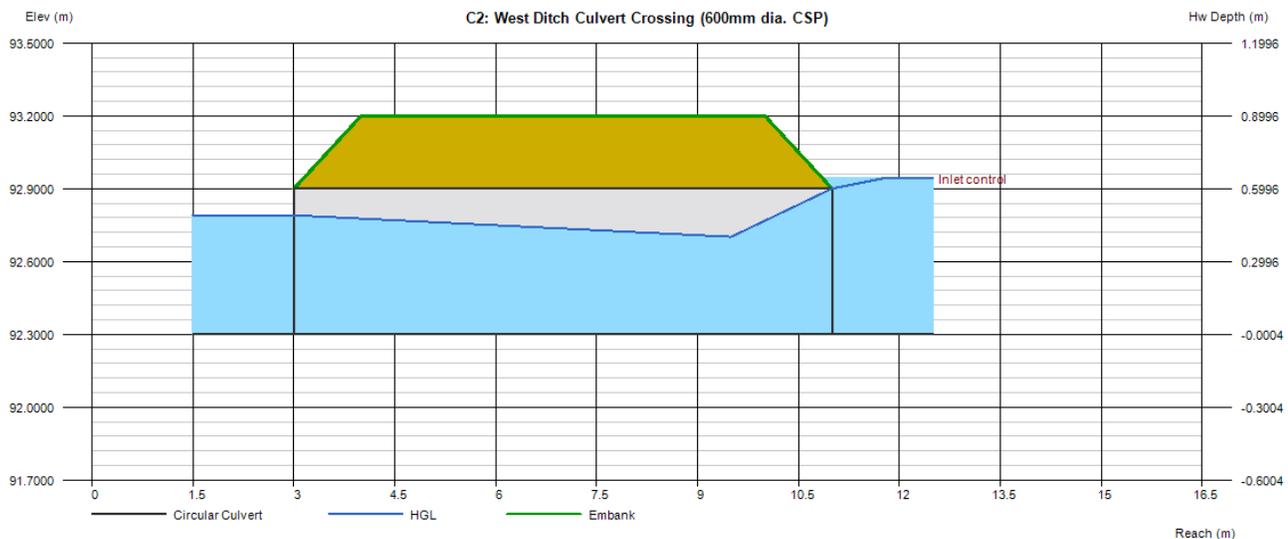
## C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000  
 Pipe Length (m) = 8.0000  
 Slope (%) = 0.0049  
 Invert Elev Up (m) = 92.3004  
 Rise (mm) = 600.0  
 Shape = Circular  
 Span (mm) = 600.0  
 No. Barrels = 1  
 n-Value = 0.016  
 Culvert Type = Circular Corrugate Metal Pipe  
 Culvert Entrance = Projecting  
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

**Embankment**  
 Top Elevation (m) = 93.2000  
 Top Width (m) = 6.0000  
 Crest Width (m) = 6.0000

**Calculations**  
 Qmin (cms) = 0.0000  
 Qmax (cms) = 0.4000  
 Tailwater Elev (m) = (dc+D)/2

**Highlighted**  
 Qtotal (cms) = 0.3400  
 Qpipe (cms) = 0.3400  
 Qovertop (cms) = 0.0000  
 Veloc Dn (m/s) = 1.3741  
 Veloc Up (m/s) = 1.7957  
 HGL Dn (m) = 92.7905  
 HGL Up (m) = 92.6813  
 Hw Elev (m) = 92.9420  
 Hw/D (m) = 1.0693  
 Flow Regime = Inlet Control



# Culvert Report

100-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Thursday, Jun 9 2016

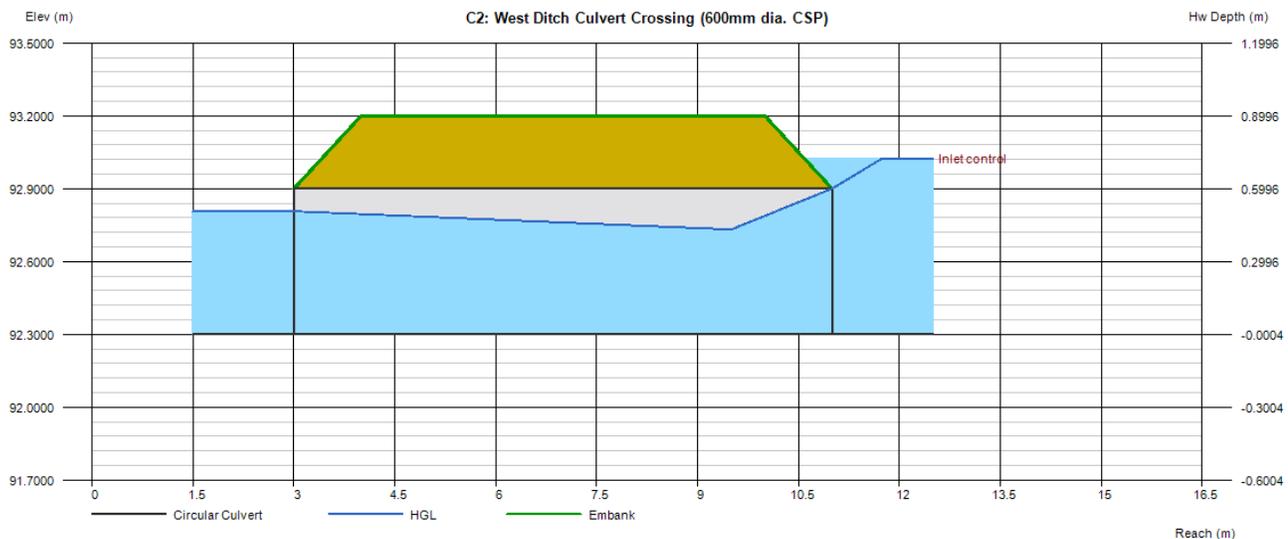
## C2: West Ditch Culvert Crossing (600mm dia. CSP)

Invert Elev Dn (m) = 92.3000  
Pipe Length (m) = 8.0000  
Slope (%) = 0.0049  
Invert Elev Up (m) = 92.3004  
Rise (mm) = 600.0  
Shape = Circular  
Span (mm) = 600.0  
No. Barrels = 1  
n-Value = 0.016  
Culvert Type = Circular Corrugate Metal Pipe  
Culvert Entrance = Projecting  
Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

**Embankment**  
Top Elevation (m) = 93.2000  
Top Width (m) = 6.0000  
Crest Width (m) = 6.0000

**Calculations**  
Qmin (cms) = 0.0000  
Qmax (cms) = 0.4000  
Tailwater Elev (m) = (dc+D)/2

**Highlighted**  
Qtotal (cms) = 0.4000  
Qpipe (cms) = 0.4000  
Qovertop (cms) = 0.0000  
Veloc Dn (m/s) = 1.5694  
Veloc Up (m/s) = 1.9217  
HGL Dn (m) = 92.8071  
HGL Up (m) = 92.7145  
Hw Elev (m) = 93.0256  
Hw/D (m) = 1.2087  
Flow Regime = Inlet Control



# Culvert Report

2-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

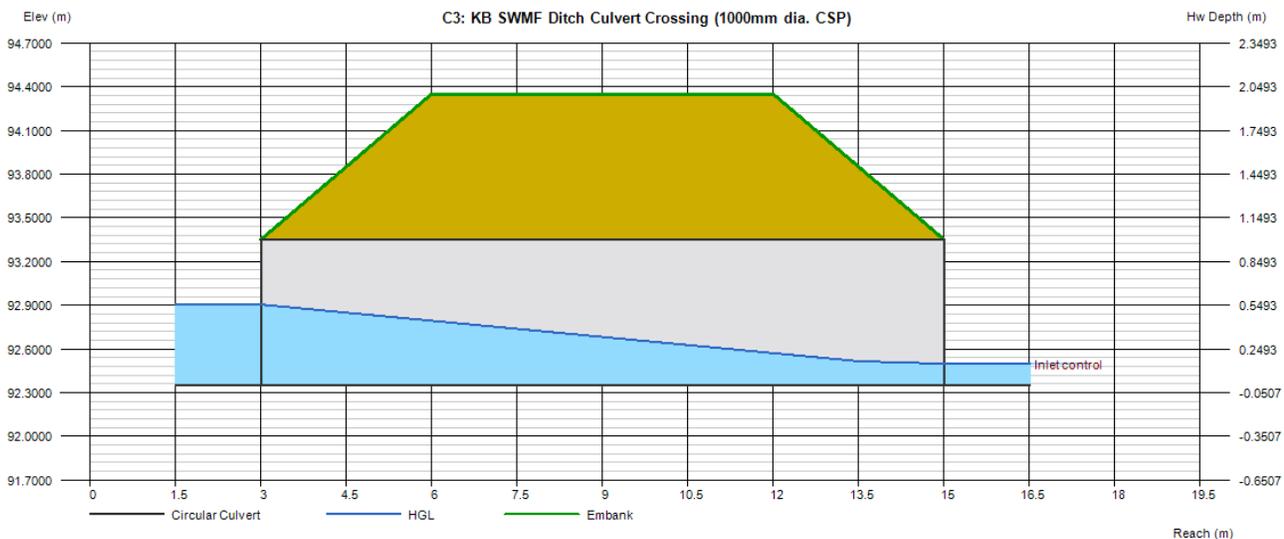
Friday, Jun 10 2016

## C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2
<b>Highlighted</b>	
Qtotal (cms)	= 0.0400
Qpipe (cms)	= 0.0400
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.0894
Veloc Up (m/s)	= 0.8567
HGL Dn (m)	= 92.9048
HGL Up (m)	= 92.4602
Hw Elev (m)	= 92.4986
Hw/D (m)	= 0.1479
Flow Regime	= Inlet Control



# Culvert Report

5-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

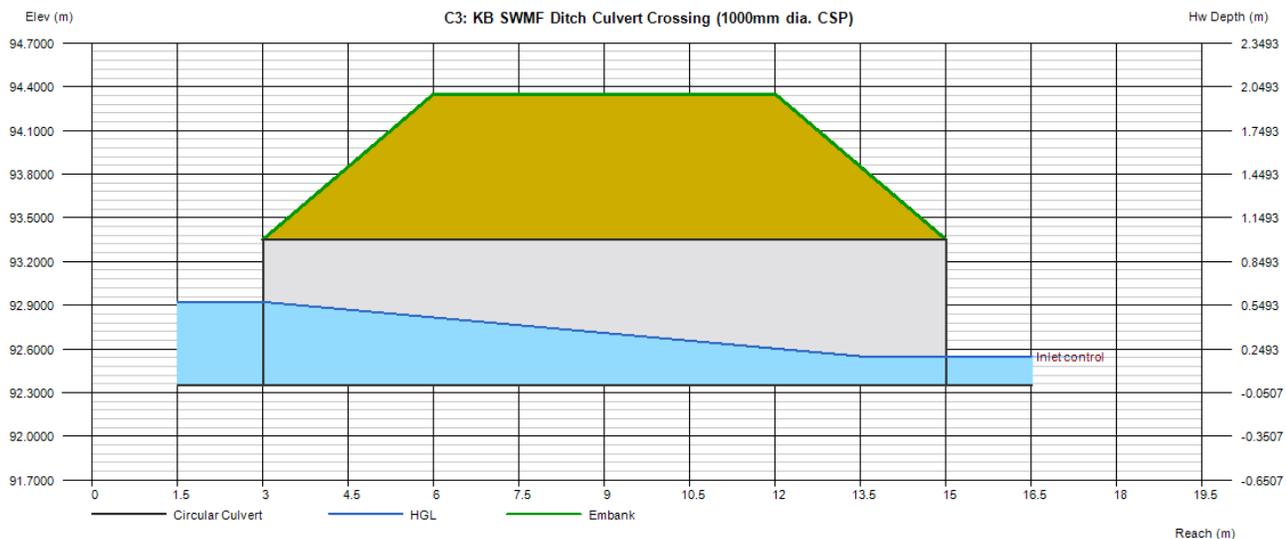
Friday, Jun 10 2016

## C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2
<b>Highlighted</b>	
Qtotal (cms)	= 0.0700
Qpipe (cms)	= 0.0700
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.1505
Veloc Up (m/s)	= 0.9911
HGL Dn (m)	= 92.9227
HGL Up (m)	= 92.4961
Hw Elev (m)	= 92.5484
Hw/D (m)	= 0.1977
Flow Regime	= Inlet Control



# Culvert Report

10-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

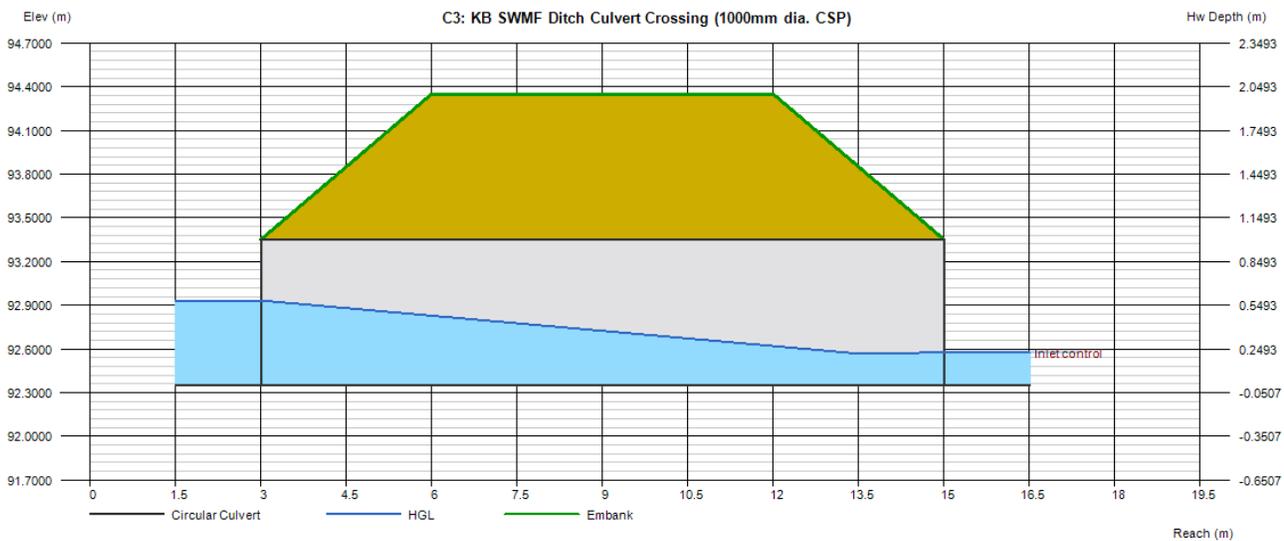
Friday, Jun 10 2016

## C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2
<b>Highlighted</b>	
Qtotal (cms)	= 0.0900
Qpipe (cms)	= 0.0900
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.1895
Veloc Up (m/s)	= 1.0598
HGL Dn (m)	= 92.9326
HGL Up (m)	= 92.5159
Hw Elev (m)	= 92.5763
Hw/D (m)	= 0.2256
Flow Regime	= Inlet Control



# Culvert Report

25-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

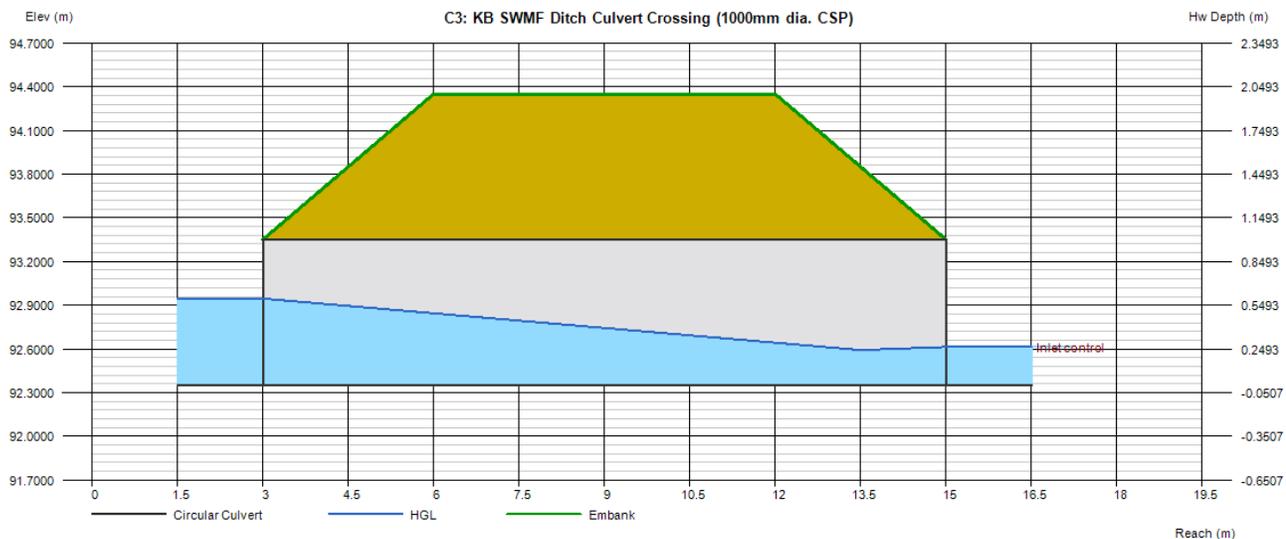
Friday, Jun 10 2016

## C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2
<b>Highlighted</b>	
Qtotal (cms)	= 0.1200
Qpipe (cms)	= 0.1200
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.2461
Veloc Up (m/s)	= 1.1449
HGL Dn (m)	= 92.9456
HGL Up (m)	= 92.5419
Hw Elev (m)	= 92.6136
Hw/D (m)	= 0.2629
Flow Regime	= Inlet Control



# Culvert Report

50-year 12-hour SCS Storm

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

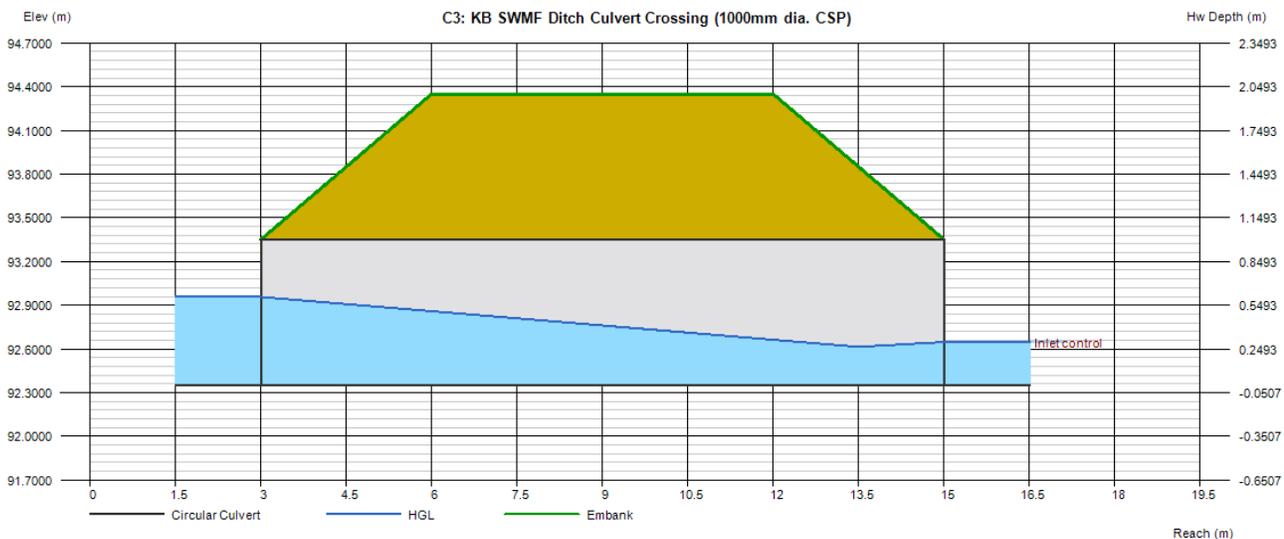
## C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2

<b>Highlighted</b>	
Qtotal (cms)	= 0.1500
Qpipe (cms)	= 0.1500
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.3006
Veloc Up (m/s)	= 1.2156
HGL Dn (m)	= 92.9571
HGL Up (m)	= 92.5650
Hw Elev (m)	= 92.6472
Hw/D (m)	= 0.2965
Flow Regime	= Inlet Control



# Culvert Report

100-year 12-hour SCS Storm

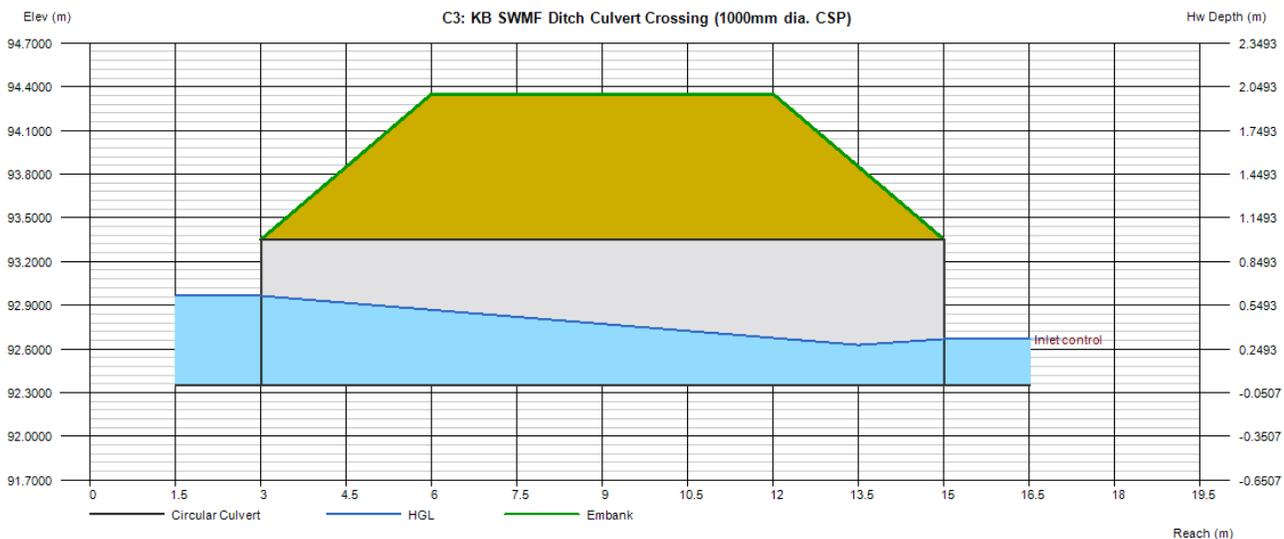
## C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m)	= 92.3500
Pipe Length (m)	= 12.0000
Slope (%)	= 0.0058
Invert Elev Up (m)	= 92.3507
Rise (mm)	= 1000.0
Shape	= Circular
Span (mm)	= 1000.0
No. Barrels	= 1
n-Value	= 0.020
Culvert Type	= Circular Corrugate Metal Pipe
Culvert Entrance	= Projecting
Coeff. K,M,c,Y,k	= 0.034, 1.5, 0.0553, 0.54, 0.9

<b>Embankment</b>	
Top Elevation (m)	= 94.3500
Top Width (m)	= 6.0000
Crest Width (m)	= 6.0000

<b>Calculations</b>	
Qmin (cms)	= 0.0000
Qmax (cms)	= 0.2000
Tailwater Elev (m)	= (dc+D)/2

<b>Highlighted</b>	
Qtotal (cms)	= 0.1700
Qpipe (cms)	= 0.1700
Qovertop (cms)	= 0.0000
Veloc Dn (m/s)	= 0.3360
Veloc Up (m/s)	= 1.2582
HGL Dn (m)	= 92.9642
HGL Up (m)	= 92.5791
Hw Elev (m)	= 92.6682
Hw/D (m)	= 0.3175
Flow Regime	= Inlet Control



# Culvert Report

Full Flow Capacity (HW/D = 1.0)

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jun 10 2016

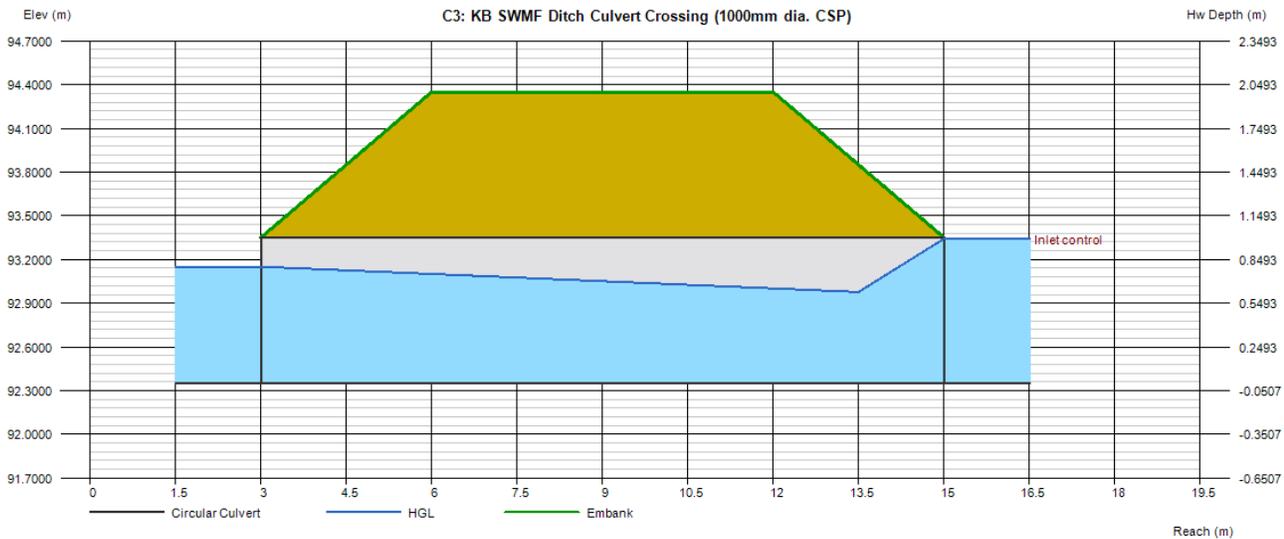
## C3: KB SWMF Ditch Culvert Crossing (1000mm dia. CSP)

Invert Elev Dn (m) = 92.3500  
 Pipe Length (m) = 12.0000  
 Slope (%) = 0.0058  
 Invert Elev Up (m) = 92.3507  
 Rise (mm) = 1000.0  
 Shape = Circular  
 Span (mm) = 1000.0  
 No. Barrels = 1  
 n-Value = 0.020  
 Culvert Type = Circular Corrugate Metal Pipe  
 Culvert Entrance = Projecting  
 Coeff. K,M,c,Y,k = 0.034, 1.5, 0.0553, 0.54, 0.9

**Embankment**  
 Top Elevation (m) = 94.3500  
 Top Width (m) = 6.0000  
 Crest Width (m) = 6.0000

**Calculations**  
 Qmin (cms) = 0.0000  
 Qmax (cms) = 1.3000  
 Tailwater Elev (m) = (dc+D)/2

**Highlighted**  
 Qtotal (cms) = 1.1000  
 Qpipe (cms) = 1.1000  
 Qovertop (cms) = 0.0000  
 Veloc Dn (m/s) = 1.6315  
 Veloc Up (m/s) = 2.2284  
 HGL Dn (m) = 93.1508  
 HGL Up (m) = 92.9523  
 Hw Elev (m) = 93.3430  
 Hw/D (m) = 0.9923  
 Flow Regime = Inlet Control



**South Nepean Collector Phase 2: Culvert Crossings  
Ditch Capacities (Manning's Equation)**



<b>C1: Burnett Municipal Drain</b>		
<b>Parameter</b>	<b>Units</b>	<b>Value</b>
Depth	m	0.60
Bottom Width	m	3.00
Side slope (L)	1 to X	2.0
Side slope (R)	1 to X	4.0
Top Width (L)	m	1.20
Top Width (R)	m	2.40
Area	m <sup>2</sup>	2.880
Perimeter	m	6.82
R=A/P	m	0.42
n		0.035
Slope	m/m	0.007
Q <sub>max</sub>	m <sup>3</sup> /s	3.877
V <sub>max</sub>	m/s	1.346

Trapezoidal Channel (different side slopes)

<b>C2: West Ditch</b>		
<b>Parameter</b>	<b>Units</b>	<b>Value</b>
Depth	m	0.30
Side slope (L)	1 to X	3.0
Side slope (R)	1 to X	6.0
Top Width (L)	m	0.90
Top Width (R)	m	1.80
Area	m <sup>2</sup>	0.405
Perimeter	m	2.77
R=A/P	m	0.15
n		0.035
Slope	m/m	0.005
Q <sub>max</sub>	m <sup>3</sup> /s	0.227
V <sub>max</sub>	m/s	0.560

V-bottom ditch (different side slopes)

<b>C3: KB SWMF Ditch</b>		
<b>Parameter</b>	<b>Units</b>	<b>Value</b>
Depth	m	2.00
Side slope (L)	1 to X	2.5
Side slope (R)	1 to X	2.5
Top Width (L)	m	5.00
Top Width (R)	m	5.00
Area	m <sup>2</sup>	10.000
Perimeter	m	10.77
R=A/P	m	0.93
n		0.035
Slope	m/m	0.006
Q <sub>max</sub>	m <sup>3</sup> /s	21.063
V <sub>max</sub>	m/s	2.106

V-bottom ditch (same side slopes)

CORPORATION OF THE TOWNSHIP OF NEPEAN

By-Law No. 107-68

(The Drainage Act, 1962-63, Section 27, Form 4)

A By-law to provide for Drainage Work in the Township of Nepean, in the County of Carleton and for borrowing on the credit of the Municipality the sum of Five Thousand, Six Hundred and Thirty-Three (\$5,633.00) Dollars, for completing the same.

-----

WHEREAS the requisite number of owners, as shown by the last revised Assessment Roll, of the property hereinafter set forth requiring drainage have petitioned the Council of the said Township of Nepean praying that the following lands and roads may be drained by a drainage works.

Plan and Profile:

The accompanying plan, profile and specifications dated October 16th, 1968 are to form a part of this report. The plan will show the area of the watershed, and the drain. The profile and specifications will show the extent of work, the bench marks, grades and disposal of materials, etc.

Recommendation:

It is our recommendation that this drain be reconstructed from station 0 + 00 to station 15 + 31 where it enters the present road ditch. At this location the present road ditch will carry the water to the south side of the forced road where construction is to continue from station 19 + 98. The drain then flows east to the present railway culvert which is of sufficient size and the drain has been designed to flow through it at its present elevation. From this location the drain still flows easterly to the Township Road between lots 15 and 16, Concession 3 at station 43 + 70. The present culvert through this road is to be lowered to the designed grade for this location and from here the drain flows east to its outlet in the Jock River at station 86 + 25.

AND WHEREAS the Council has procured a report made by Alex J. Graham, C.E., hereto attached and marked Schedule "C" of By-Law No. 107-68.

AND WHEREAS the Council is of opinion that the drainage of the area described is desirable:

THEREFORE the Council of the Township of Nepean pursuant to The Drainage Act, 1962-63, enacts as follows:

1. The report is hereby adopted, and the drainage works as therein indicated and set forth are hereby authorized and shall be completed in accordance therewith.

2. The Corporation of the Township of Nepean may borrow on the credit of the Corporation the sum of \$5,633.00, being the funds necessary for the drainage works, not otherwise provided for, provided that such sum shall be reduced by the amount of grants and commuted payments with respect to the land and roads assessed, and may issue Debentures of the Corporation to that amount in sums of not less than Fifty Dollars each, and payable within ten years from the date of the said Debentures, with interest at the rate of 8 per centum per annum, that is to say annually with coupons.

3. For paying the sum of \$2,914.65 the amount charged against such lands and roads for benefit, and the sum of \$2,718.35 the amount charged against such lands and roads for outlet liability, and the sum of \$Nil the amount charged against such lands and roads for injuring liability, and the sum of Nil the amount charged against lands and roads for improvement, apart from lands and roads belonging to or controlled by the Municipality, and for covering interest thereon for ten years at the rate of 8 per centum per annum, the following total special rates, over and above all other rates, shall be assessed, levied, and collected (in the same manner and at the same time as other taxes are levied and collected) upon and from the under-mentioned parcels of land and parts of parcels and roads, and the amount of the total special rates and interest against each parcel or part of parcel respectively shall be divided into ten equal parts and one such part shall be assessed, levied and collected as aforesaid, in each year, for ten years after the passing of this by-law during which the Debentures have to run, provided that no greater amount shall be levied than is required after taking into account and crediting the amount of grants under subsection 3 of section 64 of The Drainage Act, 1962-63, the amount of moneys paid under a by-law passed under subsection 4 of section 40 of that Act and commuted payments with respect to lands and roads assessed.

4. For paying the sum of \$590.43 the amount assessed against such roads and lands of the Municipality, and for covering interest thereon for ten years at the rate of 8 per cent per annum, a special rate, sufficient to produce the required yearly amount therefor, shall, over and above all other rates, be levied and collected (in the same manner and at the same time as other taxes are levied and collected) upon and from the whole rateable property in the Township of Hopeen in each year for ten years, after the passing of this by-law, during which the Debentures have to run.

5. This by-law comes into force on the passing thereof and may be cited as the "Burnett Municipal Drain By-Law".

First Reading December 9th, 1968.

Second Reading December 9th, 1968.

Third Reading January 6th, 1969.

Enacted this 6th day of January, 1969.

D. A. Moodie  
D. A. Moodie                      Reeve

D. E. Hobbs  
D. E. Hobbs                      Clerk

TOWNSHIP OF NEPEAN  
BURNETT MUNICIPAL DRAIN  
SCHEDULE OF ASSESSMENT

no.	Lot	Name	Acres Assessed	Main Drain Benefit	Outlet	Total	Esti- mated Grant	Balance Payable	To Cover Interest For 10 yrs. at 8%	Total Special Rate	Annual Assessment During Each Year For 10 years
S $\frac{1}{2}$	Lot 19	Don Fraser	13		198.18	198.18	132.13	66.05	22.36	98.41	9.84
N $\frac{1}{2}$ S. pt.	Lot 18	Lorne Burnett	43	549.77	517.25	1,067.02	721.31	355.71	174.29	530.00	53.00
S Pt.	Lot 18	Robert Mowat	6		74.28	74.28	49.54	24.74	12.13	36.87	3.69
Pt. W $\frac{3}{4}$ N $\frac{1}{4}$	Lot 17	Mrs. E. Monk	8	96.25	99.84	196.09	130.72	65.37	30.05	97.42	9.74
Pt. W $\frac{3}{4}$ N $\frac{1}{4}$	Lot 17	Sals & Riccio	11	76.23	136.18	212.41	141.62	70.79	34.69	105.48	10.55
S $\frac{1}{2}$	Lot 17	Carl Fraser	20	160.93	149.37	310.50	206.99	103.51	50.72	154.23	15.42
N $\frac{1}{2}$ & Pt S $\frac{2}{3}$ L16	Patrick Moloughney	15	249.10	223.80	472.90	315.27	157.63	77.24	234.87	23.49	
S $\frac{2}{3}$ Less 20ac L16	Fergus Houlshen	49	281.15	492.26	872.51	582.30	291.21	142.69	433.90	43.39	
E $\frac{2}{3}$ N $\frac{1}{2}$	Lot 15	Carl Fraser	19	204.05	107.16	311.21	207.47	103.74	50.83	154.57	15.46
S $\frac{1}{2}$	Lot 15	Wm. Clark	18	204.05	73.98	278.03	185.35	92.68	45.41	138.09	13.81
Pt. E $\frac{7}{15}$ N $\frac{3}{8}$ L.14	Mrs. H. Houlshen	16	233.70	50.72	284.42	189.63	94.79	46.45	141.24	14.12	
S. 5/8	Lot 14	John Houlshen	13	294.52	34.71	329.23	219.49	109.74	53.77	163.51	16.35
Pt. E. $\frac{1}{2}$	Lot 13	Kelvin Burnett	9	208.10	--	208.10	138.76	69.34	33.98	103.32	10.33
		Forced Road in Concession 3	--	122.82	254.64	377.36	--	--	--	--	--
		Township Road Between Lots 15 & 16	--	76.23	136.94	213.07	--	--	--	--	--
		Canadian National Railways	--	57.75	168.94	226.69	--	--	--	--	--
Totals:			\$2,912.65	\$2,718.35	\$5,633.00	\$3,210.58	\$1,605.30	\$784.71	\$2,391.91	\$239.19	

ENGINEER'S REPORT  
BURNETT MUNICIPAL DRAIN  
TOWNSHIP OF NEPEAN

Graham, Norman and Associates Ltd.,  
Consulting Engineers,  
Ottawa 8, Ontario.

October 16th, 1968.

---

*Schedali C. By-Law 107-68*

October 16th, 1968.

The Reeve and Members of Council,  
Township of Nepean,  
3895 Richmond Road,  
Ottawa 6, Ontario.

Gentlemen:

In answer to the prayer of a petition of over half the property owners concerned, requesting that the Burnett Acre Ditch be reconstructed to relieve the present flooding conditions and provide adequate outlet for tile drains. This drain is to be reconstructed under section 3 of the Drainage Act 1962-63 and by reason of a motion passed in Council, I have caused a survey to be made, prepared plan, profile and schedule of assessment, and beg to report as follows:

The drainage works shall be known as the Burnett Municipal Drain.

Plan and Profile:

The accompanying plan, profile and specifications dated October 16th, 1968 are to form a part of this report. The plan will show the area of the watershed, and the drain. The profile and specifications will show the extent of work, the bench marks, grades and disposal of materials, etc.

Recommendation:

It is our recommendation that this drain be reconstructed from station 0 + 00 to station 15 + 31 where it enters the present road ditch. At this location the present road ditch will carry the water to the south side of the forced road where construction is to continue from station 19 + 98. The drain then flows east to the present railway culvert which is of sufficient size and the drain has been designed to flow through it at its present elevation. From this location the drain still flows easterly to the Township Road between lots 15 and 16, Concession 3 at station 13 + 70. The present culvert through this road is to be lowered to the designed grade for this location and from here the drain flows east to its outlet in the Jack River at station 86 + 25.

Farm Crossings:

A Severance Allowance is made in this report under Section 3 (6) of the Drainage Act 1962-63, culvert dimensions are stated in the following Schedule of Allowance.

Township of Nepean:

Concession 3:

M <sub>2</sub>	Lot 18	Lorne Burnett	1-24" x 16' x 16 gauge CMP	\$81.52
S <sub>1</sub>	Lot 18	Lorne Burnett	1-24" x 16' x 16 gauge CMP	81.52
Pt. W3/4 N <sub>2</sub>	Lot 17	Mrs. E. Monk	1-24" x 16' x 16 gauge CMP	81.52
W3/4 N <sub>2</sub>	Lot 17	Dale & Macleod	1-24" x 16' x 16 gauge CMP	81.52
S <sub>2</sub>	Lot 17	Clerk Fraser	1-24" x 16' x 16 gauge CMP	81.52
H 1/3	Lot 16	Patrick McLoughney	1-30" x 16' x 16 gauge CMP	\$2.08
PT S2/3	Lot 16	Fergus Houlahan	1-30" x 16' x 16 gauge CMP	92.08
E2/3 N <sub>1</sub>	Lot 15	Carl Fraser	1-36" x 16' x 16 gauge CMP	104.88
S <sub>2</sub>	Lot 15	Mr. Clark	1-36" x 16' x 16 gauge CMP	104.88

Pt E7/15	Lot 14	Mrs. H. Houlshen	1-36" x 16' x 16 gauge CMP	\$104.88
S 5/8	Lot 14	John Houlshen	1-36" x 16' x 16 gauge CMP	104.88
E Pt	Lot 13	Kelvin Burnett	1-36" x 16' x 16 gauge CMP	104.88
Total				<u>\$1,116.16</u>

These allowances will, in my opinion, adequately compensate the above property owners for the cost and installation of the above culverts.

Road Bridges and Culverts:

The present 24" x 48' CMP through the forced road in Concession 3 is sufficient in size, and the drain has been designed to flow through it at its present grade.

The present 30" x 26' CMP through the Township Road between lots 15 and 16, Concession 3 is sufficient in size and requires only to be lowered to the designed grade at this location.

Estimated Costs      \$203.00

These estimated costs have not been included in the Estimated cost of construction as it is expected that the Township of Menan's Road Department will accept this responsibility as part of their maintenance program.

Railroad Culvert:

The present culvert through the Canadian National Railway which consists of a 24" CMP on the south side, and a 2' x 2.75' timber culvert on the north side of the tracks is sufficient in size, and the drain has been designed to flow through at the present grade.

Land Damage:

The amounts shown in the following allowances will, in my opinion, adequately compensate the property owners indicated for land or crop damage (if any) under Section 2 (1) of the Drainage Act 1962-63.

N $\frac{1}{2}$	Lot 18	Lorne Burnett		\$66.00
S $\frac{1}{2}$	Lot 18	Lorne Burnett		62.00
Pt W3/4				
N $\frac{1}{2}$	Lot 17	Mrs. E. Monk		9.00
W3/4 N $\frac{1}{2}$	Lot 17	Sole & Nicolvto		18.00
S $\frac{1}{2}$	Lot 17	Clark Fraser		38.00
N 1/3	Lot 16	Patrick Keloughney		59.00
Pt E2/3	Lot 16	Fergus Houlshen		90.00
E2/3 N $\frac{1}{2}$	Lot 15	Carl Fraser		81.00
S $\frac{1}{2}$	Lot 15	Ms. Clark		61.00
Pt E7/15 & N 3/8	Lot 14	Mrs. Helen Houlshen		63.00
S 5/8	Lot 14	John Houlshen		63.00
E Pt	Lot 13	Kelvin Burnett		96.00
Total				<u>\$725.00</u>

Distribution of Costs:

The estimated costs for this construction are apportioned to the properties responsible for benefit and outlet as determined by their areas, locations, and run-off.

In my opinion, no liability for injury will exist because of this construction, and for this reason no injury liability column will appear in the Schedule of Assessment, this being in my estimation a fair distribution of costs.

Future Maintenance:

This drain will be maintained by the Township of Nepean, and the costs of such future maintenance will be apportioned to the property owners in the same proportions to the property owners in the same proportions as in the attached Schedule of Assessment.

The "Estimated Costs" and incidental expenses are as follows:

Earth Excavation and Spreading of 1,992 c.y. @ .60¢	\$1,195.20
Hard Pan Excavation 282 c.y. @ \$1.60	451.20
Brushing	40.00
Fern Crossings	1,116.16
Land Damage Section 8 (1) of the Drainage Act 1962-63	726.00
Engineer's Fees and Expenses	884.44
Contingencies	470.00
Clerk's Fees	200.00
Printing and Publishing By-Law	125.00
Advertising and Letting Contract	85.00
Court of Revision	150.00
Supervision of Construction	190.00
Total Estimated Cost	<u>\$5,633.00</u>

Grants:

Under Section 61, 63 and 64 of the Drainage Act 1962-63, a Provincial Grant of 33 1/3% of the cost of construction for Agricultural Lands may be obtained.

A subsequent grant by Federal A.R.D.A. through the Provincial grant administration media will further reduce Agricultural assessments by another one-third.

The assessments are then payable two-thirds by grant, and one-third by property assessment.

Respectfully submitted this 16th day of October, 1968.



GRAHAM, BERMAN AND ASSOCIATES LTD.

*John S. Morrison*  
John S. Morrison,  
Drainage Manager.

## BURNETT MUNICIPAL DRAIN

## TOWNSHIP OF NEPEAN

Conc.	Lot	Name	Acresage Assessed	Main Drain Benefit	Outlet	Total
3	S $\frac{1}{2}$ Lot 19	Don Fraser	13		198.18	198.18
	N $\frac{1}{2}$ Spt Lot 18	Lorne Burnett	43	549.77	517.25	1,067.02
	S Pt. Lot 18	Robert Nowat	6		74.28	74.28
	Pt W3/4 N $\frac{1}{2}$ L.17	Mrs. E. Monk	8	96.25	99.84	196.09
	Pt W3/4 N $\frac{1}{2}$ Lot 17	Sala & Riccivto	11	76.23	136.18	212.41
	S $\frac{1}{2}$ Lot 17	Carl Fraser	20	160.93	149.37	310.50
	W1/3 & Pt S2/3 L.16	Patrick Maloughney	15	249.10	223.60	472.70
	S2/3 Less 30ac L. 16	Fergus Houlehan	49	381.15	492.36	873.51
	E2/3 N $\frac{1}{2}$ Lot 15	Carl Fraser	19	204.05	107.16	311.21
	S $\frac{1}{2}$ Lot 15	Mr. Clerk	18	204.05	73.98	278.03
	Pt E7/15 N3/8 L.14	Mrs. H Houlehan	16	233.70	50.72	284.42
	S5/8 Lot 14	John Houlehan	13	294.52	34.71	329.23
	Pt E $\frac{1}{2}$ Lot 13	Kelvin Barnett	9	208.10		208.10
	Forced Road in Concession 3			122.82	254.64	377.46
	Township Road Between Lots 15 and 16			76.23	136.94	213.07
	Canadian National Railways			57.75	168.94	226.69
Totals				\$2,914.65	\$2,718.35	\$5,633.00

SUMMARY OF ASSESSMENT

LANDS:

Township Roads	\$ 590.43
Non Agricultural Lands	\$ 226.69
Land used for Agricultural Purposes	\$4,815.88

GRANTS ON AGRICULTURAL LANDS

Estimated Provincial Grant of 33 1/3%	\$1,605.29
Estimated Federal A.R.D.A. Grant of 33 1/3%	\$1,605.29
Estimated Property Assessments	\$1,605.30

SUMMARY OF ASSESSMENT

LANDS

Township roads	\$ 590.43
Non Agricultural Lands	422.78
Land used for Agricultural Purposes	\$4,619.79
<u>Grants on Agricultural Lands</u>	
Estimated Provincial Grant of 33 1/3 %	\$1,539.93
Estimated Federal A.R.D.A. Grant of 33 1/3%	\$1,539.93
Estimated Property Assessments	\$1,539.93

SUPPLEMENT TO THE GENERAL SPECIFICATIONS

BURNETT MUNICIPAL DRAIN

TOWNSHIP OF NEPEAN

Graham, Borden and Associates Ltd.,  
Consulting Engineers,  
Ottawa 8, Ontario.

October 16th, 1968.

Meaning of Terms:

- "Municipal Council" - shall mean the Municipal Council of Nereen Township.
- "Reeve" - shall mean the Reeve of Nereen Township.
- "Engineer" - shall mean the Engineer in charge of the works, or his authorized representatives.
- "Contractor" - shall mean the Contractor or Contractors performing the work, or their foreman on the grounds.

Extent of Work:

The accompanying plan, profile and specifications dated October 16th, 1968 apply to and govern this construction.

8,625 lineal feet of open drain as follows:

Earth Excavation and Spreading of 1,992 c.y.

Hardpan Excavation and Spreading of 282 c.y.

Brushing

A Severance Allowance under Section 8 (6) of the Drainage Act 1962-63 is applied on this drain.

Center-line:

The present watercourse is to be the center-line of construction.

Important:

- (a) The Engineer must be notified at least 5 days prior to the starting of work on this contract.
- (b) Fences may be opened to allow construction equipment to go through them, and are to be closed immediately after that piece of equipment passes through, if fences are found to be left opened, they will be closed at the "Contractor's Expense".

GENERAL SPECIFICATIONS

Municipal Drainage (Open-Drains)

Graham, Barman & Associates Ltd.  
St. Thomas & Ottawa, Ontario.

1. These specifications are drawn up to cover the work as outlined in the Engineer's Report on the drain, and as further outlined in the supplement to General Specifications.

Where there is any doubt as to the meaning or intention of the specifications, it shall be the Contractor's duty to obtain a ruling in writing from the Engineer before proceeding with the work.

2. Supply of Labour and Materials:

The Contractor shall supply all materials, labour, equipment, tools, machinery etc. for the full and proper completion of this work in accordance with the specifications, plan and profile. All work must be done in a neat and workmanlike manner, and to the satisfaction of the Engineer.

3. Roads to be kept open:

All roads, public and private are to be kept open and in passable condition during the continuance of this work.

4. Relief Ditches:

Should the Contractor deem it necessary to dig relief ditches on any part of this work, he shall do so and refill same entirely at his own expense.

5. Damages:

a) In case of damage being done to any farm or other property along the line of work by blasting or other operations, the Contractor shall be held liable for such damage.

b) The Owner or Occupant of the property on which the drainage works is located shall be responsible for the protection of all livestock on said property during construction, and shall be liable for any damages caused to or by such livestock.

c) The Contractor shall also rebuild and leave in as good condition as before construction, all fences removed in order to execute this work.

i) Fences crossing the drainage works must be closed immediately after the construction equipment has passed through them.

ii) Fences parallel to the drain must be replaced immediately after the work is completed in each section.

d) When hay or other produce is growing on lands adjacent to the proposed work, the Contractor must give the Owner ten days' notice

notice in writing to remove the same before he begins work on that portion, otherwise he shall be held liable for any damage caused.

6. Clearing and Grubbing:

a) Trees or brush growing in or on the banks of the drain are to be grubbed out clean. Trees having a stump diameter of 6" or over are to be cut into log lengths and piled clear of the spread materials; The stumps are to be piled in a corner of the field from which they were taken, adjacent to, but not closer than 4 feet to the edge of the drain. Branches and brush under 6" in diameter are to be piled on the excavated materials for disposal by the Property Owner.

b) At locations where the drain passes through brush or wood-lots, it is necessary that a strip of land be cleared along one side of the drain; the locations and dimensions will be given in the Supplement Specifications. However it is not intended that large trees growing within this specified area should be cut unless it is apparent that excessive damage will be caused to them. Stumps are not to exceed 1 ft. in height, and brush and branches are to be disposed of as in section (a) above.

Payment for this work will be made under the lump sum tender item "Cutting Brush".

7. Disposal of Materials:

The excavated materials shall be disposed of so as to do as little damage to lands and crops as possible.

a) Earth excavated from the drain is to be taken back a distance of 10 feet, leaving a clean berm 10 feet wide along the edge of the drain; and to be spread over the adjoining lands in such a manner that the elevation of the completed work does not exceed the elevation of the adjoining lands by more than 6" on cultivated lands and 12" on unworked or bush lands.

The completed work is to have a neat appearance and to be comparatively smooth.

b) Hard Pan and Rock excavated from the drain is to be taken back 4 feet from the edge of the drain and left in a pile so that it may be disposed of by the Property Owner.

c) Water Courses where necessary will be cut through the spread materials every 200 feet or in the low spots along the drain to allow surface drainage of the surrounding areas.

d) Re-location of Drain: At locations where the drain is to be removed from a road allowance, materials excavated from the new drain may be used to fill the road ditch in such a manner so as to allow the water from the road to enter the new drain. Excess materials are to be spread on the adjoining lands as above.

e) Straightening: At locations where straightening occurs the upper end of the old water course is to be filled level to the shoulders for a distance of 20 feet, and the excavated material put in the old drain when the intervening distance does not exceed 100 feet. Where the distance exceeds 100, the shoulders / are

are to be pushed into the bottom of the remainder of the abandoned watercourse, and so shaped that the water will run out of the low end, and in such a manner as to allow the Property Owner to travel through the drain and gradually reclaim the land.

f) Boulders: All boulders having a cubic content of 1 cubic foot or more are to be neatly placed along the edge of the drain at a distance of approximately 4 feet from it.

The price for the above materials disposal is to be included in the bid price per cubic yard for excavation.

8. Description of Ditch:

The ditch is to be constructed to the grades, widths and side slopes as shown on the accompanying profile.

The bottom width shall not be increased without maintaining the specified side slopes.

The grade is to be constructed to provide a constant slope to the end of the ditch so that no water will be stagnant therein.

9. Centre Line:

The Centre Line shall be the Centre Line of the present ditch, provided the fences are far enough back from the shoulders to allow for the proper width of bottom and side slope as shown on the accompanying profile. In locations where fences are too close, the Centre Line may be moved away from the fence a sufficient distance so that bottom widths and side slopes may be maintained.

At locations where the drain is to be removed from a road allowance, the centre line will be staked by the Engineer to allow for a clear berm of 3 feet between the property line and the edge of the drain.

At locations where excessive meanderings of the present stream take place, straightening may be required; in such cases the Centre Line will be staked in the field by the Engineer.

10. Grades and Centre Line:

Grades and Centre Line will be given by the Engineer upon receiving 5 clear days notice in writing that such Grades and Centre Line are required.

The Contractor will take precautions to ensure that Grades and Line so set will not be disturbed during construction.

Any subsequent setting of Line and Grade on the project will be charged to the Contractor.

11. Inspection:

Upon completion of the work there should be a continuous ditch or water course of the size and dimensions according to the plan, profile and specifications.

Any foreign material accumulated in the drain will be removed by the Contractor, unless he can clearly show that he is not responsible for the foreign material being in the drain.

The Contractor, when he considers all work to have been so completed, must notify the Engineer that he requires an inspection of the works to be made and he, or his representative, will accompany the Engineer on this inspection.

12. Classification of Materials:

Earth shall mean clay, loam, sand, small stones, gravel and muck, etcetera.

Hard Pan shall mean materials other than rock that require the use of picks, bars, dynamite, etc. for their removal.

Rock shall mean strata rock or boulders having a cubic content of 1/4 cubic feet or more.

Prices are to be submitted separately to cover each type of material.

13. Farm Bridges and Culverts:

The Contractor shall, as part of this contract, clean out or lower Farm Bridges and Culverts considered to be large enough for their locations.

The Contractor shall notify the Owner four clear days in writing to remove farm bridges that are not large enough to meet specifications; if the bridges are not removed, the Contractor may, after four days, remove same, exercising normal caution so as not to unduly damage the materials, piling same neatly 15 feet from the edge of the drain.

The Contractor may remove the flooring to clean out under a bridge that is of sufficient size; however, he must replace this flooring in as good condition as before it was removed.

The responsibility for the replacement of culverts or the construction of new ones is set forth in the Supplement Specifications.

Payment for the above work is to be included in the bid price per cubic yard for earth excavation.

14. Road Bridges and Culverts:

The Contractor shall notify the road Superintendent concerned as to the date and time the excavating equipment will be at the site of the road bridge or culvert.

The Contractor will construct the ditch to the ends of the present bridge or culvert.

The Contractor will clean all culverts that are of sufficient size and set at the required grade.

The Contractor will lower the grade and otherwise clear out under bridges having sufficient size for their locations. Payment for this work is deemed to be included in the bid price per cubic yard for earth excavation.

14. Road Bridges and Culverts: (Continued)

Work other than the above that may be required by the Road Superintendent will be paid for on an hourly basis by the Road Department responsible for the upkeep of this section of road.

15. Right to Increase or Decrease:

The Municipal Council reserves the right to increase the work as it deems necessary and the contract price per cubic yard shall remain the same.

16. Sub-Letting:

No portion of the work is to be sub-let without the consent of the Municipal Council and the Engineer.

17. Deposit:

A cash deposit or certified cheque on a chartered bank in the amount of 10% of the tender price must accompany each tender, such deposit will be returned to the unsuccessful bidders within 7 days of tender closing.

18. Date of Completion:

The whole work shall be completed on or before the day of \_\_\_\_\_, 196\_\_\_\_ and when the Contractor considers that the work is completed, he must notify the Engineer in writing that he requires a final inspection thereof.

19. Payment:

Cash payment will be made monthly equal to 90% of the value of the work completed on the certificate of the Engineer, when the value of work completed within the month amounts to Three Hundred Dollars (\$300.00) or more. The remaining 10% will be retained until 60 days after the whole work has been accepted as finished.

20. The Municipal Council reserves the right to reject any and all tenders.

21. The Contractor shall comply with the regulations of the Workmen's Compensation Board of Ontario.

22. Damages caused to public utilities installation shall be the responsibility of the General Contractor.

Prior to starting this work, he shall obtain from the public utilities (i.e. telephone, hydro, gas) the locations, if any, of all their installations along these works.

TENDER FORM

I/WE OF  
do hereby tender and agree to construct the  
in accordance with the attached specifications and drawings.

I/WE have examined the site of the above work and are thoroughly  
familiar with the work that is to be done.

I/WE tender and agree to perform the above mentioned work for the  
following prices:

EXTENT OF WORK:

TOTAL PRICE

Earth Excavation and Spreading of 1,992 c.y. ....

Hardpan Excavation and Spreading of 282 c.y. ....

Brushing .....

TOTAL CONTRACT PRICE \_\_\_\_\_

I/WE guarantee that the above work will be completed on the  
day of \_\_\_\_\_, 196 \_\_\_\_\_.

I/WE enclose a certified cheque or cash deposit for the sum of  
(\$ \_\_\_\_\_), being 10% of the tender  
price, and further agree to furnish a suitable bond for 100% of the  
contract price within 7 days of notification of acceptance of tender,  
if so requested. In such case, the deposit will be returned on the  
signing of the contract.

The deposit or bonds of the successful bidder may be retained by the  
Clerk until 60 days after the above work has been completed and accepted,  
or in the event the tender is unsuccessful, it shall be returned within  
7 days.

Offered on behalf of Contractor:

Accepted on behalf of the Municipality

NAME: \_\_\_\_\_

RESUME: \_\_\_\_\_

ADDRESS: \_\_\_\_\_

CLERK: \_\_\_\_\_

DATE: \_\_\_\_\_

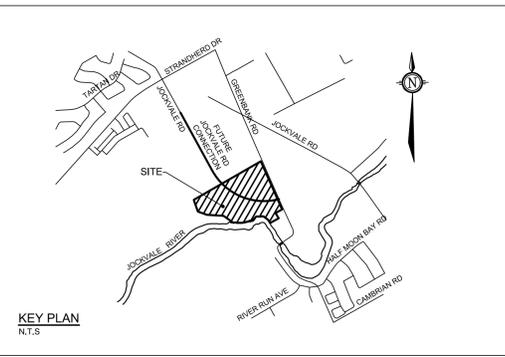
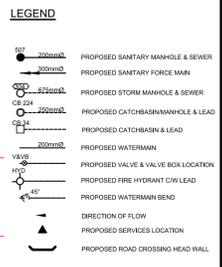
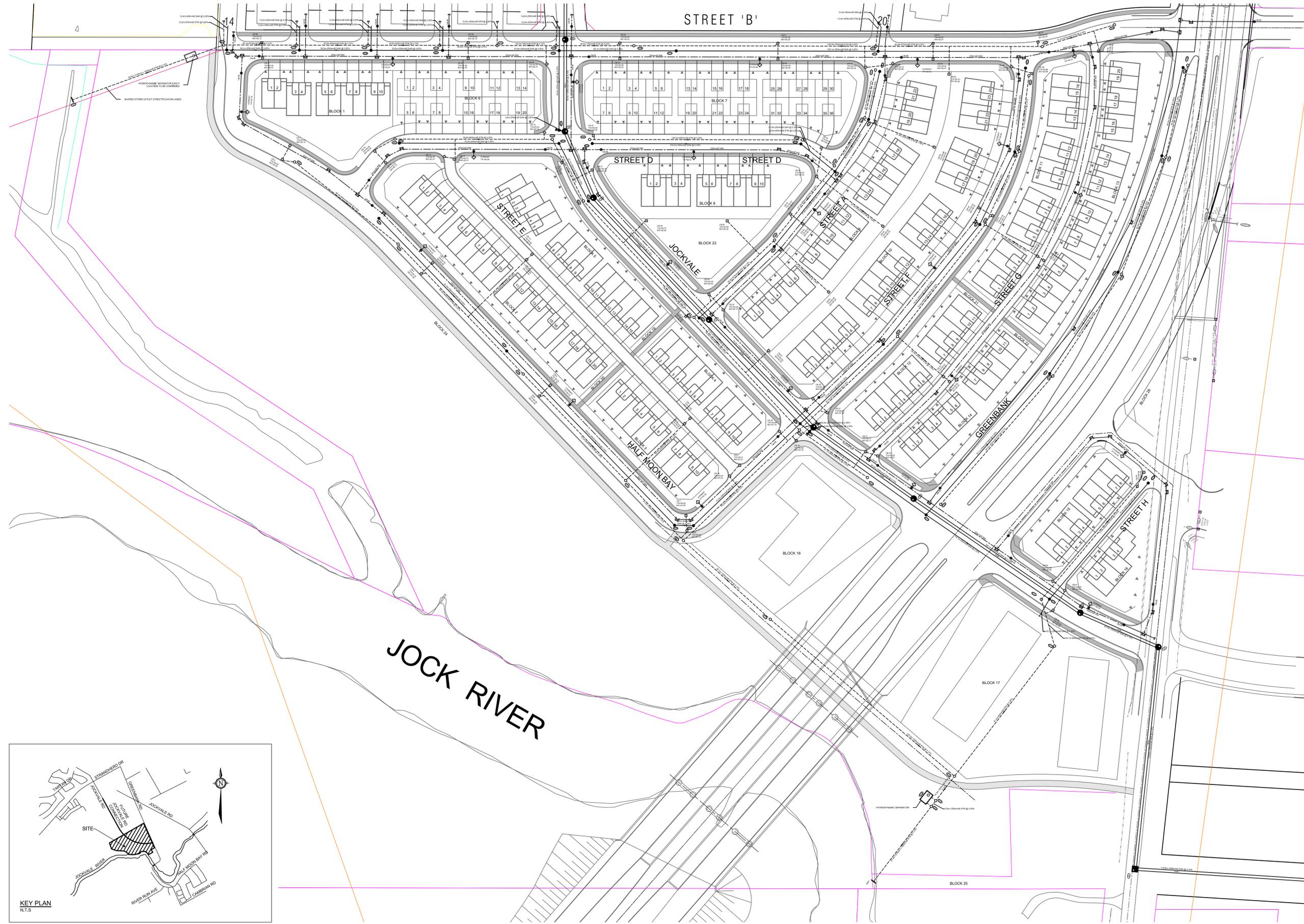
POSITION: \_\_\_\_\_

DATE: \_\_\_\_\_

## **Appendix F**

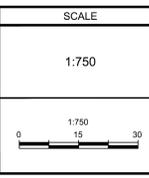
### Engineering Drawings

<i>General Plan of Services</i>	<i>111117-GP</i>	<i>(revision 1)</i>
<i>Grading Plan</i>	<i>111117-GR</i>	<i>(revision 1)</i>
<i>Sanitary Drainage Area Plan</i>	<i>111117-SAN</i>	<i>(revision 1)</i>
<i>Storm Drainage Area Plan</i>	<i>111117-STM</i>	<i>(revision 1)</i>

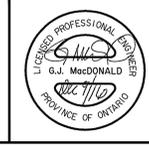


**NOTE:**  
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	REVISION	DATE	BY
1	ISSUED WITH DRAFT PLAN OF SUBDIVISION	DEC 6/16	GJM



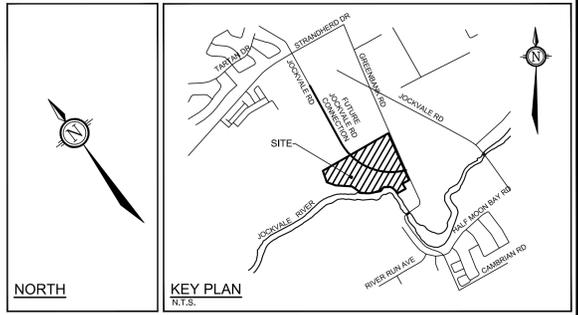
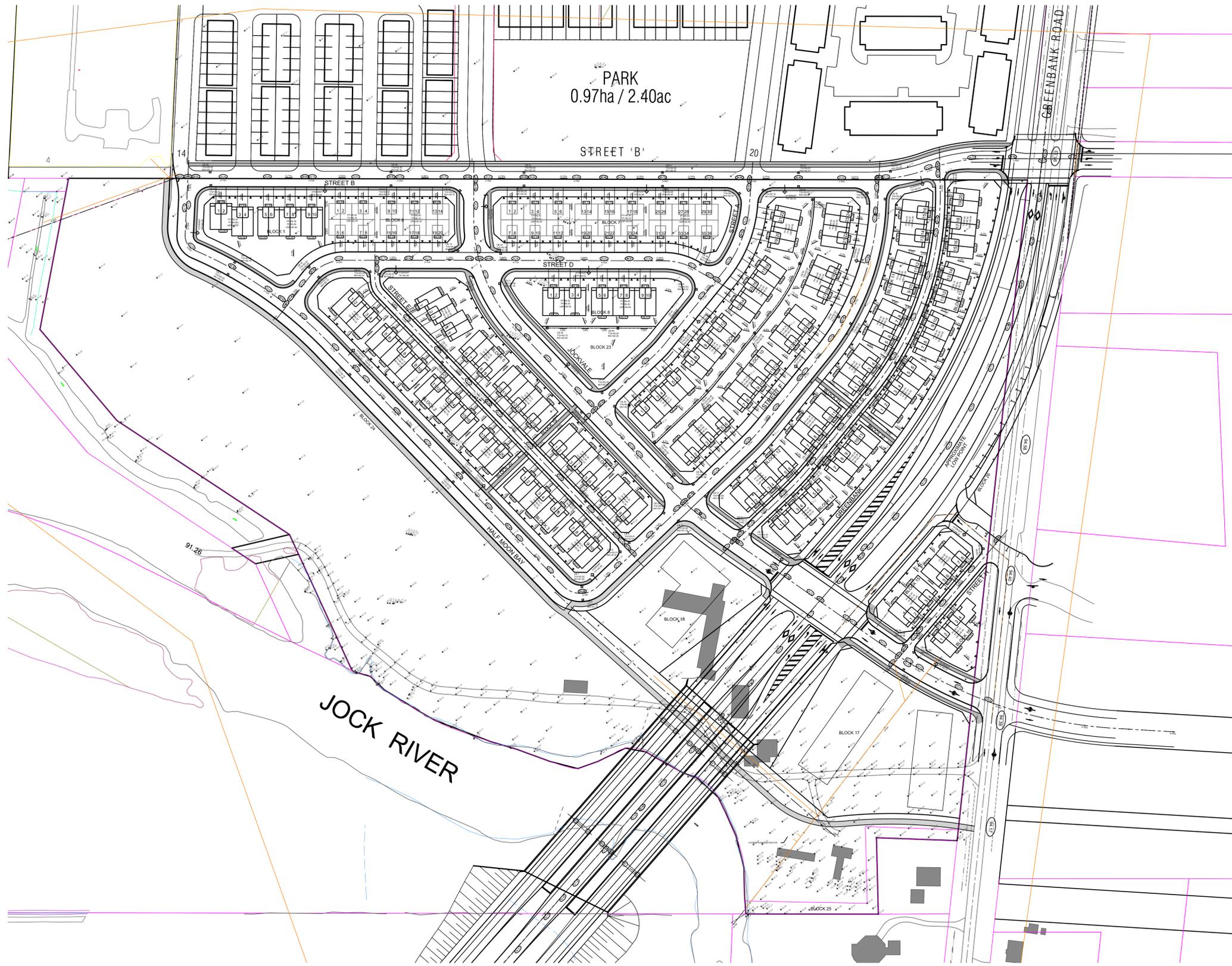
DESIGN	GJM/LSC
CHECKED	GJM
DRAWN	LSC
CHECKED	GJM
APPROVED	GJM



**NOVATECH**  
Engineers, Planners & Landscape Architects  
Suite 200, 240 Michael Cowpland Drive  
Ottawa, Ontario, Canada K2M 1P6  
Telephone: (613) 254-9643  
Facsimile: (613) 254-5667  
Website: www.novatech-eng.com

LOCATION CITY OF OTTAWA 3370 GREENBANK ROAD	DRAWING NAME <b>GENERAL PLAN OF SERVICES</b>	PROJECT No. 111117
		REV #1
		DRAWING No. 111117-GP

M:\2018\1117\CAD\Drawings\111117-GP.DWG, OVERALL, Dec 13, 2018 - 11:07am, mllong

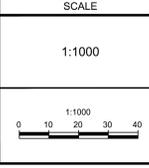


**LEGEND**

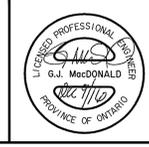
- IRON BAR & PROPERTY LINE
- LEGAL ADJACENT
- PROPOSED ROAD ELEVATION
- PROPOSED GARAGE ELEVATION
- PROPOSED TERRACE ELEVATION
- EXISTING TOP OF SURFACE ELEVATION
- PROPOSED SLOPE
- PROPOSED FINISHED FLOOR ELEVATION
- PROPOSED TOP OF FOUNDATION ELEVATION
- PROPOSED UNDERSIDE OF FOOTING ELEVATION

NOTE:  
 THE POSITION OF ALL POLE LINES, CONDUITS,  
 WATERMANS, SEWERS AND OTHER  
 UNDERGROUND AND OVERGROUND UTILITIES AND  
 STRUCTURES IS NOT NECESSARILY SHOWN ON  
 THE CONTRACT DRAWINGS, AND WHERE SHOWN,  
 THE ACCURACY OF THE POSITION OF SUCH  
 UTILITIES AND STRUCTURES IS NOT GUARANTEED.  
 BEFORE STARTING WORK, DETERMINE THE EXACT  
 LOCATION OF ALL SUCH UTILITIES AND  
 STRUCTURES AND ASSUME ALL LIABILITY FOR  
 DAMAGE TO THEM.

No.	REVISION	DATE	BY
1	ISSUED WITH DRAFT PLAN OF SUBDIVISION	DEC 6/16	GJM



DESIGN	GJMLSC
CHECKED	GJM
DRAWN	LSC
CHECKED	GJM
APPROVED	GJM



**NOVATECH**  
 Engineers, Planners & Landscape Architects  
 Suite 200, 240 Michael Cowpland Drive  
 Ottawa, Ontario, Canada K2M 1P6  
 Telephone: (613) 254-9643  
 Facsimile: (613) 254-5667  
 Website: www.novatech-eng.com

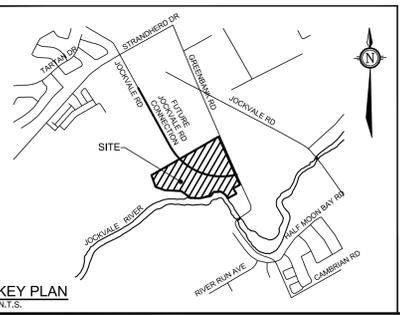
LOCATION CITY OF OTTAWA 3370 GREENBANK ROAD	PROJECT No. 111117
DRAWING NAME GRADING PLAN	REV #1
	DRAWING No. 111117-GR

M:\2016\1117\CAD\DWG\11117-GR.DWG OVERALL Dec 12, 2016 - 3:55pm mfbp



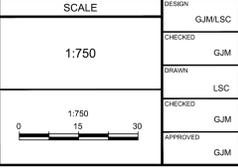
**LEGEND**

0.17	— DRAINAGE AREA (HECTARES)
507-509	— DRAINAGE AREA NUMBER
8 11	— POPULATION EQUIVALENT
—	— NUMBER OF UNITS
—	— SANITARY DRAINAGE AREA BOUNDARY
—	— PROPOSED SANITARY MANHOLE & SEWER
—	— PROPOSED SANITARY MANHOLE & TRUNK SEWER
—	— EXISTING SANITARY MANHOLE & SEWER
—	— FUTURE DEVELOPMENT BY OTHERS



**NOTE:**  
CONTRACTOR TO CONFIRM ELEVATIONS OF INFRASTRUCTURE IN THE STREET PRIOR TO EXTENDING SERVICES INTO THE SITE AND SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES IMMEDIATELY.

No.	REVISION	DATE	BY
1	ISSUED WITH DRAFT PLAN OF SUBDIVISION	DEC 6/16	GJM



**FOR REVIEW ONLY**

DESIGN	GJM/LSC
CHECKED	GJM
DRAWN	LSC
CHECKED	GJM
APPROVED	GJM



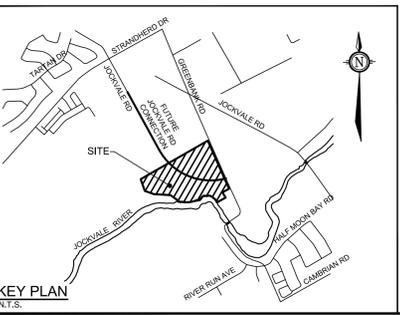
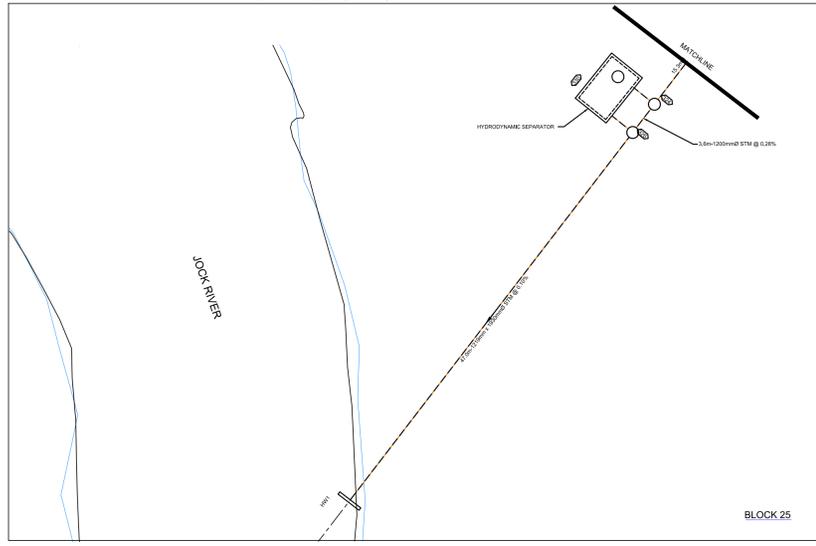
LOCATION  
CITY OF OTTAWA  
3370 GREENBANK ROAD

DRAWING NAME  
**SANITARY DRAINAGE AREA PLAN**

PROJECT NO.	11117
REV	REV #1
DRAWING NO.	11117-SAN1



- LEGEND**
- PROPOSED STORM MANHOLE & SEWER
  - PROPOSED CATCHBASIN MANHOLE & LEAD
  - PROPOSED REAR YARD CATCHBASIN & SUBDRAIN
  - PROPOSED SERVICES LOCATION
  - PROPOSED ROAD CROSSING HEAD WALL
  - FUTURE DEVELOPMENT BY OTHERS



**NOTE:**  
 CONTRACTOR TO CONFIRM ELEVATIONS OF INFRASTRUCTURE IN THE STREET PRIOR TO EXTENDING SERVICES INTO THE SITE AND SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES IMMEDIATELY.

No.	REVISION	DATE	BY
1	ISSUED WITH DRAFT PLAN OF SUBDIVISION	DEC 6/16	GJM

SCALE	DESIGN
1:750	GJM/LSC
1:750	GJM
1:750	LSC
1:750	GJM
1:750	GJM

**FOR REVIEW ONLY**

PROFESSIONAL ENGINEER  
 G.J. MacDONALD  
 PROVINCE OF ONTARIO

**NOVATECH**  
 Engineers, Planners & Landscape Architects  
 Suite 200, 240 Michael Cowpland Drive  
 Ottawa, Ontario, Canada K2M 1P6  
 Telephone: (613) 254-9643  
 Facsimile: (613) 254-5867  
 Website: www.novatech-eng.com

LOCATION  
 CITY OF OTTAWA  
 3370 GREENBANK ROAD

DRAWING NAME  
 STORM DRAINAGE AREA PLAN

PROJECT NO.  
 111117

REV #1

DRAWING NO.  
 111117-STM1

M:\031117\17\CAD\Design\1117-STM1.DWG, STM, Dec 13, 2016 - 2:27pm, mfg  
 DVIEW 51.17