6038 Ottawa Street Transportation Impact Assessment

Step 1 Screening Report
Step 2 Scoping Report
Step 3 Strategy Report

Prepared for:

Tamarack Homes & Taggart Investments 3187 Albion Road South Ottawa ON K1V 8Y3

Prepared by:



6 Plaza Court Ottawa, ON K2H 7W1

December 2024

PN: 2023-080

Table of Contents

L		Screening	1
2		Existing and Planned Conditions	1
	2.1	Proposed Development	1
	2.2	Existing Conditions	3
	2.2	2.1 Area Road Network	3
	2.2	2.2 Existing Intersections	3
	2.2	2.3 Existing Driveways	3
	2.2	2.4 Cycling and Pedestrian Facilities	4
	2.2	2.5 Existing Transit	7
	2.2	2.6 Existing Area Traffic Management Measures	8
	2.2	2.7 Existing Peak Hour Travel Demand	8
	2.2	2.8 Collision Analysis	. 10
	2.3	Planned Conditions	. 11
	2.3		
	2.3	3.2 Other Study Area Developments	. 11
3		Study Area and Time Periods	. 11
	3.1	Study Area	. 11
	3.2	Time Periods	. 12
	3.3	Horizon Years	. 12
1		Development-Generated Travel Demand	. 12
	4.1	Mode Shares	. 12
	4.2	Trip Generation	. 13
	4.3	Trip Distribution	. 14
	4.4	Trip Assignment	
5		Exemption Review	
ŝ		Development Design	
	6.1	Design for Sustainable Modes	
	6.2	New Street Networks	. 18
7		Boundary Street Design	. 19
3		Transportation Demand Management	
	8.1	Context for TDM	
	8.2	Need and Opportunity	
	8.3	TDM Program	
9		Background Network Travel Demands	
	9.1	Transportation Network Plans	
	9.2	Background Growth	
	9.3	Other Developments	
1(Demand Rationalization	
	10.1	2032 Future Background Intersection Operations	
	10.2	2037 Future Background Intersection Operations	
	10.3	Modal Share Sensitivity	
11	L	Transit	. 25



11.1	Route Capacity	25
11.2	Transit Priority	25
12	Intersection Design	25
12.1	Intersection Control	25
12.2		
1	2.2.1 2032 Future Total Intersection Operations	25
1	2.2.2 2037 Future Total Intersection Operations	
1	2.2.3 Intersection MMLOS	
1	2.2.4 Recommended Design Elements	
13	Summary of Improvements Indicated and Modifications Options	
14	Conclusion	33
List o	of Figures	
Figure	1: Area Context Plan	1
•	2: Concept Plan	
Figure	3: Existing Area Driveways	4
Figure	4: Study Area Pedestrian Facilities	5
Figure	5: Study Area Cycling Facilities	5
Figure	6: Existing Pedestrian Volumes	6
Figure	7: Existing Cyclist Volumes	6
Figure	8: Existing Study Area Transit Service	7
Figure	9: Existing Study Area Transit Stops	8
Figure	10: Existing Traffic Counts	9
Figure	11: Study Area Collision Records	10
Figure	12: New Site Generation Auto Volumes	15
_	13: Concept Pedestrian and Cycling Network	
Figure	14: Conceptual Transit Stops	18
Figure	15: Concept Traffic Calming Plan	19
•	16: 2032 Future Background Volumes	
•	17: 2037 Future Background Volumes	
_	18: 2032 Future Total Volumes	
Figure	19: 2037 Future Total Volumes	28
Table	e of Tables	
Table 1	1: Intersection Count Date	8
Table 2	2: Existing Intersection Operations	9
Table 3	3: Study Area Collision Summary, 2018-2022	10
Table 4	4: Summary of Collision Locations, 2018-2022	10
Table 5	5: TRANS Trip Generation Manual Recommended Mode Shares – Rural Districts Including Southwest	12
Table 6	5: Proposed Development Mode Shares – Rural Districts Including Southwest	13
Table 7	7: Trip Generation Person Trip Rates by Peak Period	13
Table 8	8: Total Residential Person Trip Generation by Peak Period	13



Table 9: Trip Generation by Mode	13
Table 10: OD Survey Distribution – Rural Southwest	14
Table 11: Exemption Review	15
Table 12: Boundary Street MMLOS Analysis	19
Table 13: TRANS Regional Model Projections – Study Area Growth Rates	20
Table 14: Applied Annual Growth Rates	21
Table 15: 2032 Future Background Intersection Operations	22
Table 16: 2037 Future Background Intersection Operations	24
Table 17: Trip Generation by Transit Mode	25
Table 18: 2032 Future Total Intersection Operations	26
Table 19: 2037 Future Total Intersection Operations	28
Table 20: Left-Turn Lane Warrant Results	29

List of Appendices

- Appendix A TIA Screening Form and Certification Form
- Appendix B Turning Movement Count Data
- Appendix C Synchro Intersection Worksheets Existing Conditions
- Appendix D Collision Data
- Appendix E MMLOS Analysis
- Appendix F TDM Checklist
- Appendix G TRANS Model Plots
- Appendix H Background Development Volumes
- Appendix I Synchro Intersection Worksheets 2032 Future Background Conditions
- Appendix J Synchro Intersection Worksheets Signal Warrants
- Appendix K Synchro Intersection Worksheets Left-Turn Lane Warrants
- Appendix L Synchro Intersection Worksheets 2037 Future Background Conditions
- Appendix M Synchro Intersection Worksheets 2032 Future Total Conditions
- Appendix N Synchro Intersection Worksheets 2037 Future Total Conditions



Screening

This study has been prepared according to the City of Ottawa's 2017 Transportation Impact Assessment (TIA) Guidelines, incorporating the 2023 Revision to Transportation Impact Assessment Guidelines. Accordingly, a Step 1 Screening Form has been prepared and is included as Appendix A, along with the Certification Form for the TIA Study PM. As shown in the Screening Form, a TIA is required, and this study has been prepared to support a draft plan of subdivision.

Existing and Planned Conditions

Proposed Development

The proposed site is located at 6038 Ottawa Street, currently zoned as Rural General Industrial Zone 3 (RG3). The proposed residential plan of subdivision includes a total of 1,173 homes, split between 535 single family homes, 144 semi-detached homes, and 494 townhomes. A new collector road between Eagleson Road and McBean Street is proposed via minor stop-controlled all-movement intersections at each road. The existing site is farm fields. The anticipated full build-out and occupancy horizon is 2032, and no phasing has been confirmed. The site is within the Village of Richmond Community Design Plan Area. Figure 1 illustrates the study area context. Figure 2 illustrates the proposed concept plan.

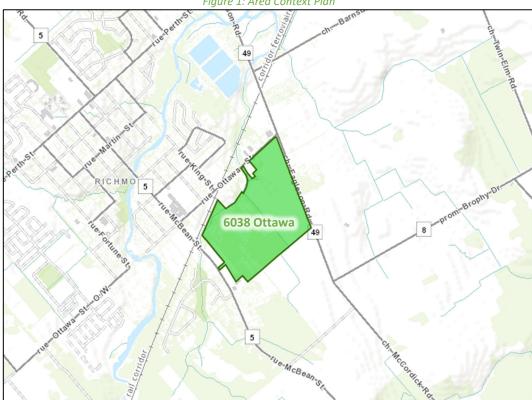
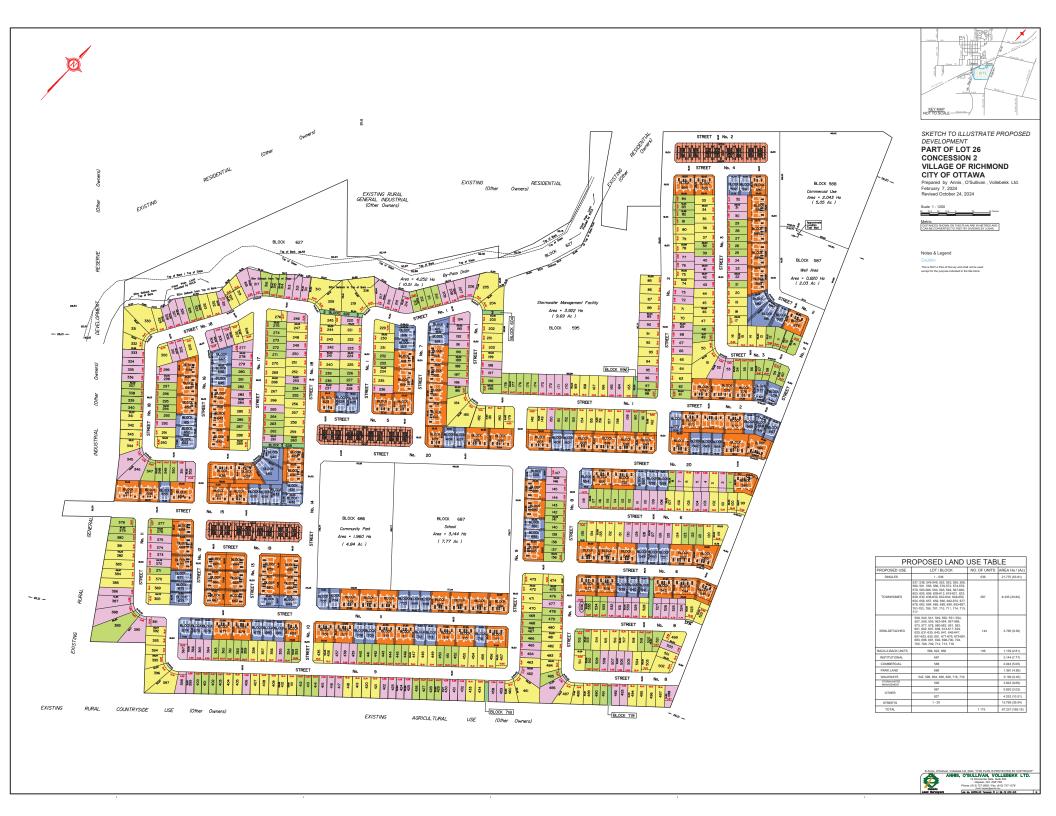


Figure 1: Area Context Plan

Source: http://maps.ottawa.ca/geoOttawa/ Accessed: September 10, 2024





2.2 Existing Conditions

2.2.1 Area Road Network

Eagleson Road: Eagleson Road is a City of Ottawa arterial road with a two-lane rural cross-section with gravel shoulders and a posted speed limit of 80 km/h. The measured right-of-way varies from 26.0 to 27.0 metres. Eagleson Road is a truck route north of Brophy Drive.

McBean Street: McBean Street is a City of Ottawa arterial road with a two-lane rural cross-section with paved shoulders north of the rail tracks and gravel shoulders to the south. The posted speed limit is 50 km/h north of the rail tracks and 70 km/h to the south. The City-protected right-of-way is 23.0 metres north of Ottawa Street and the measured right-of-way varies from 26.0-30.0 metres south of Ottawa Street. McBean Street is a truck route.

Brophy Drive: Brophy Drive is a City of Ottawa arterial road with a two-lane rural cross-section with gravel shoulders and a posted speed limit of 80 km/h. The measured right-of-way is 40.0 metres. Brophy Drive is a truck route.

Ottawa Street: Ottawa Street is a City of Ottawa collector road with a two-lane rural cross-section with a posted speed limit of 50 km/h. The measured right-of-way is 20.0 metres to the west of the rail tracks and 26.0 metres to the east.

King Street: King Street is a City of Ottawa collector road with a two-lane rural cross-section with gravel shoulders and an unposted speed limit of 50km/h. The measured right-of-way is 20.0 metres.

2.2.2 Existing Intersections

Eagleson Road & Brophy Drive

McBean Street & Ottawa Street

The key area intersections adjacent to the proposed site and additional signalized intersections within one kilometre of the site have been summarized below:

Eagleson Road & Ottawa Street	The intersection of Eagleson Road and Ottawa Street is an unsignalized intersection with stop-control on Ottawa Street. The northbound
	approach consists of a shared left-turn/through lane, the southbound approach consists of a shared through/right-turn lane, and the
	eastbound approach consists of a shared left-turn/right-turn lane. No
	turn restrictions are noted.

The intersection of Eagleson Road and Brophy Drive is an all-way stop-controlled intersection. The northbound approach consists of a shared through/right-turn lane, the southbound approach consists of a shared left-turn/through lane, the westbound approach consists of a shared left-turn/right-turn lane, and the eastbound approach is a private driveway. No turn restrictions are noted.

The intersection of McBean Street and Ottawa Street is an unsignalized intersection with stop control on Ottawa Street. All approaches consist of shared all-movement lanes. No turn restrictions are noted.

2.2.3 Existing Driveways

Within 200 metres of the proposed site, a number of private accesses are located along Eagleson Road and McBean Street. Along the east side of Eagleson Road, one private access for a farm is present, and along the east side of McBean Street, private accesses for residential and light industrial land uses are present. Figure 3 illustrates the area driveways within 200 metres of the proposed site accesses.





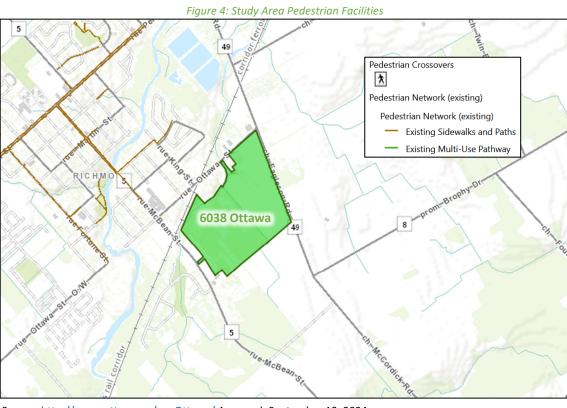
Figure 3: Existing Area Driveways

Cycling and Pedestrian Facilities

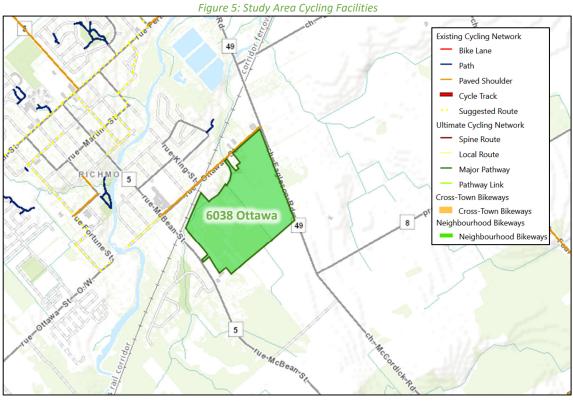
Figure 4 illustrates the pedestrian facilities in the study area and Figure 5 illustrates the cycling facilities.

Sidewalks are provided along the east side of McBean Street to the north of the South Carleton High School and on a few local streets to the north of the study area. Per the geoOttawa layers, Ottawa Street depicts paved shoulders between McBean Street, however these do not exist and the City is studying the need to provide facilities in this location.





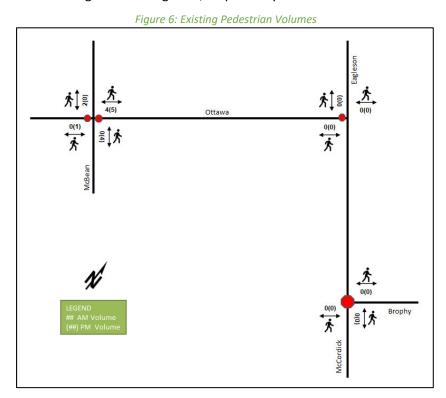
Source: http://maps.ottawa.ca/geoOttawa/ Accessed: September 10, 2024



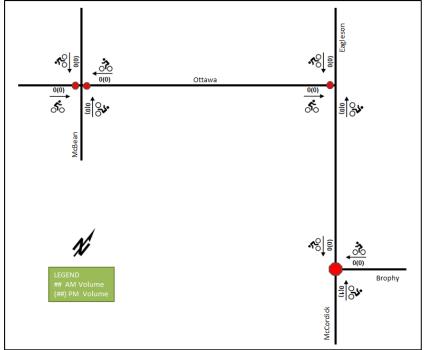
Source: http://maps.ottawa.ca/geoOttawa/ Accessed: September 10, 2024



Pedestrian and cyclist volumes included in study area intersection counts, presented in Section 2.2.7, have been compiled and are illustrated in Figure 6 and Figure 7, respectively.









2.2.5 Existing Transit

Figure 8 illustrates the transit system map in the study area and Figure 9 illustrates nearby transit stops. All transit information is from September 10, 2024, and is included for general information purposes and context to the surrounding area.

Within the study area, the routes #283 and #301 travel along McBean Street, Ottawa Street, and King Street. Stops are located on Ottawa Street at McBean Street and Cockburn Street and on King Street at Burke Street and Royal York Street. The frequency of these routes within proximity of the proposed site based on September 10, 2024 service levels are:

- Route #283 Three AM buses to Tunney's Pasture and four PM return buses
- Route #301 One AM bus to Carlingwood and one PM return bus Mondays only

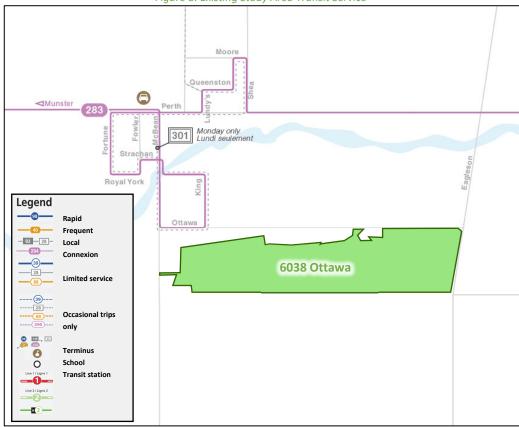


Figure 8: Existing Study Area Transit Service

Source: http://www.octranspo.com/ Accessed: September 10, 2024



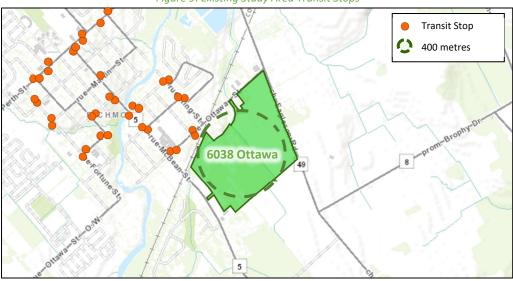


Figure 9: Existing Study Area Transit Stops

Source: http://www.octranspo.com/ Accessed: September 10, 2024

2.2.6 Existing Area Traffic Management Measures

No traffic calming measures are noted in the study area. McBean Street and Ottawa Street are signed school zones in the vicinity of South Carleton High School.

2.2.7 Existing Peak Hour Travel Demand

Existing turning movement counts were acquired from the City of Ottawa for the existing study area intersections. Table 1 summarizes the intersection count dates.

Intersection Count Date

Eagleson Road & Ottawa Street Tuesday, March 7, 2023

Eagleson Road/McCordick Road & Brophy Drive Tuesday, April 30, 2019

McBean Street & Ottawa Street Thursday, March 12, 2020

Table 1: Intersection Count Date

Figure 10 illustrates the existing traffic counts and Table 2 summarizes the existing intersection operations. The level of service is based on HCM 2010 delay for unsignalized intersections. Detailed turning movement count data is included in Appendix B and the Synchro worksheets are provided in Appendix C.



40(30) 0(15) 19(15) Ottawa 3(2) 11(4) 2(1) 107(56) 48(30) 69(168) 149(140) 153(191) 4(35) Brophy 30(14) 165(83) McCordick

Figure 10: Existing Traffic Counts

Table 2: Existing Intersection Operations

			AM Pe	ak Hour			PM Pe	ak Hour	
Intersection Eagleson Road & Ottawa Street Unsignalized Eagleson Road & Brophy Drive Unsignalized	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
5 1 5 10	EB	В	0.29	13.5	9.0	В	0.17	12.4	4.5
-	NB	Α	0.05	7.8	0.8	Α	0.03	7.9	0.8
	SB	-	-	-	-	-	-	-	-
Unsignalizea	Overall	Α	-	4.4	-	Α	-	2.5	-
Farlage Dand C	WB	Α	0.21	8.6	6.0	Α	0.32	9.7	10.5
J	NB	Α	0.27	9.2	8.3	Α	0.15	8.7	3.8
	SB	Α	0.32	9.9	10.5	В	0.45	11.6	17.3
Onsignanzea	Overall	Α	-	9.3	-	В	-	10.5	-
	EB	В	0.04	13.2	0.8	В	0.01	10.6	0.0
McBean Street &	WB	В	0.10	10.8	2.3	В	0.09	10.1	2.3
Ottawa Street	NB	Α	0.00	7.4	0.0	Α	0.00	7.4	0.0
Unsignalized	SB	Α	0.10	7.7	2.3	Α	0.02	7.5	0.8
	Overall	Α	-	4.9	-	Α	-	3.3	-

Saturation flow rate of 1800 veh/h/lane Notes:

Queue is measured in metres Peak Hour Factor = 0.90

Delay = average vehicle delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity



During both peak hours, the study area intersections operate well with LOS B or better on all movements. No capacity issues are noted.

2.2.8 Collision Analysis

Collision data have been acquired from the City of Ottawa open data website (data.ottawa.ca) for five years prior to the commencement of this TIA for the surrounding study area road network. Table 3 summarizes the collision types and conditions in the study area, Figure 11 illustrates the intersections and segments analyzed, and Table 4 summarizes the total collisions for each of these locations. Collision data are included in Appendix D.

% Number **Total Collisions** 7 100% 0 **Fatality** 0% Classification Non-Fatal Injury 4 57% **Property Damage Only** 3 43% **Angle** 2 29% **Initial Impact Type** Rear end 14% 1 **SMV Other** 4 57% 7 **Road Surface Condition** Dry 100%

Table 3: Study Area Collision Summary, 2018-2022 **Pedestrian Involved** 0 0% Cyclists Involved

0% Figure 11: Study Area Collision Records



Table 4: Summary of Collision Locations, 2018-2022

	Number	%
Intersections / Segments	7	100%
Eagleson Rd Btwn Brophy Dr & Ottawa St	3	4%
Eagleson Rd/McCordick Rd @ Brophy Dr	2	3%
King St @ Ottawa St	1	1%



	Number	%
Intersections / Segments	7	100%
McBean St @ Ottawa St	1	1%

Within the study area, no locations are noted to have experienced an average of one collision per year. No further review of collisions is required as part of this study.

2.3 Planned Conditions

2.3.1 Changes to the Area Transportation Network

2.3.1.1 Transportation Master Plan (2013)

No roadway or transit improvements are included in the City of Ottawa's Transportation Master Plan (TMP) (2013) within the study area.

2.3.1.2 Transportation Master Plan – Part 1

The City of Ottawa's Transportation Master Plan – Part 1 includes a list of planned Active Transportation projects for implementation by 2046. Separated cycling facilities are planned along McBean Street north of Ottawa Street.

2.3.1.3 Transportation Master Plan – Part 2

The City of Ottawa's Transportation Master Plan – Part 2 will recommend road and transit projects up to 2046 and is currently in the consultation phase. No recommendations, planned projects, or timing of previously planned projects is currently available as part of this forthcoming document.

2.3.1.4 Village of Richmond CDP

The Village of Richmond CDP identifies a collector road between McBean Street and Eagleson Road, a gateway feature to the southeast corner of the development lands on Eagleson Road, and local road connections to Ottawa Street and Eagleson Road. The collector road is noted to have a rural cross-section with a sidewalk on a single side. McBean Street is classified as a rural arterial with a sidewalk on a single side, transitioning to a village arterial north of the rail tracks to transition to an urban cross-section with sidewalks on both sides, on-street parking during the off-peak hours and trees in the boulevards. Eagleson Road is shown to remain an existing rural arterial.

2.3.1.5 Ottawa's Construction and Infrastructure Projects Portal

From the Planned Construction Projects section of Ottawa's Construction and Infrastructure Projects online portal, area traffic management measures are planned along Ottawa Street in front of South Carleton High School to commence construction this year.

2.3.2 Other Study Area Developments

2780 Eagleson Road

The development is an extension of Cardel Homes Creekside and is proposed to include 251 single detached dwellings, 70 semi-detached dwellings, and 135 townhouses. Two accesses onto Eagleson Road are proposed north of Richmond Road. The development is anticipated to be built out by 2027 and to generate 310 new AM and 320 new PM peak hour two-way auto trips. (CGH, 2024)

3 Study Area and Time Periods

3.1 Study Area

The study area will include the following intersections:

- Eagleson Road at:
 - Ottawa Street



- New Road
- o Brophy Drive
- McBean Street at:
 - Ottawa Street
 - New Road

The King Street and Ottawa Street intersection is not considered a study area intersection of note and will only be noted if the cycling route along Ottawa Street has any cycling facility recommendations resulting from this study.

The boundary roads are Eagleson Road and Ottawa Street. No screenlines are present near the proposed site and none will be reviewed as part of this study.

3.2 Time Periods

As the proposed development is composed primarily of residential units, the weekday AM and PM peak hours will be examined.

3.3 Horizon Years

The anticipated build-out year is 2032. As a result, the full build-out plus five years horizon year is 2037.

4 Development-Generated Travel Demand

4.1 Mode Shares

Examining the mode shares recommended in the TRANS Trip Generation Manual (2020) for the subject district, derived from the most recent National Capital Region Origin-Destination survey (OD Survey), the existing average district mode shares by land use for Rural Districts Including Southwest have been summarized in Table 5.

Travel Mode	Single-D	etached	Multi-Unit (Low-Rise)		
Travel Mode	AM	PM	AM	PM	
Auto Driver	60%	67%	66%	62%	
Auto Passenger	14%	17%	13%	19%	
Transit	24%	14%	21%	16%	
Cycling	2%	2%	1%	3%	
Walking	0%	0%	0%	0%	
Total	100%	100%	100%	100%	

Table 5: TRANS Trip Generation Manual Recommended Mode Shares – Rural Districts Including Southwest

OC Transpo has indicated through other development applications in Richmond Village that no intent to increase the number or diversity of bus routes is currently planned by area development build-out horizons. As there are no inbound transit routes to village in the AM peak period and no outbound transit routes from the village in the PM peak period, the off-peak directional transit mode shares will be reassigned to the auto mode shares. Consistent with City feedback and other studies in the area, the peak directional transit mode shares will be reduced and reassigned to the auto mode shares. Table 6 summarizes the directional mode share targets to be applied to the subject development by peak hour.



Table 6: Proposed Development Mode Shares – Rural Districts Including Southwest

		Single-D	etached		Multi-Unit (Low-Rise)				
Travel Mode	AM		PM		AM		PM		
	In	Out	In	Out	In	Out	In	Out	
Auto Driver	84%	79%	76%	81%	87%	82%	73%	78%	
Auto Passenger	14%	14%	17%	17%	13%	13%	19%	19%	
Transit	0%	5%	5%	0%	0%	5%	5%	0%	
Bicycle	2%	2%	2%	2%	1%	1%	3%	3%	
Walk	0%	0%	0%	0%	0%	0%	0%	0%	
Total	100%	100%	100%	100%	100%	100%	100%	100%	

4.2 Trip Generation

This TIA has been prepared using the vehicle and person trip rates for the residential dwellings using the TRANS Trip Generation Manual (2020). It is noted that a school and a commercial component each of unknown size are proposed within the development lands. No information or timelines are available for these components, and each will be subject to eventual individual site plan applications. Therefore, no trip generation will be undertaken for these future land uses. Table 7 summarizes the person trip rates for the proposed residential land uses for each peak period.

Table 7: Trip Generation Person Trip Rates by Peak Period

Land Use	Land Use Code	Peak Period	Person Trip Rates	
Cinala Datashad	210	AM	2.05	
Single-Detached	(TRANS)	PM	2.48	
Multi-Unit (Low-Rise)	220	AM	1.35	
	(TRANS)	PM	1.58	

Using the above person trip rates, the total person trip generation has been estimated. Table 8 summarizes the total person trip generation for the residential land uses.

Table 8: Total Residential Person Trip Generation by Peak Period

Lond Hoo	l luita	Δ.	AM Peak Perio	d	PM Peak Period			
Land Use	Units	In	Out	Total	In	Out	Total	
Single-Detached	535	329	768	1097	823	504	1327	
Multi-Unit (Low-Rise)	638	258	603	861	564	444	1008	

Using the above mode share targets and the person trip rates, the person trips by mode have been projected. Trip generation by peak hour has been forecasted using the prescribed peak period conversion factors presented in the TRANS Trip Generation Manual (2020) for the residential component. Table 9 summarizes the residential trip generation by mode and peak hour.

Table 9: Trip Generation by Mode

		А	M Peak F	PM Peak Hour					
Travel Mode		Mode Share	In	Out	Total	Mode Share	In	Out	Total
D	Auto Driver	84%/79%	134	291	425	76%/81%	275	181	456
etached	Auto Passenger	14%	22	52	74	17%	61	38	99
eta	Transit	0%/5%	0	21	21	5%/0%	19	0	19
<u>-</u>	Cycling	2%	4	9	13	2%	8	5	13
Single-De	Walking	0%	0	0	0	0%	0	0	0
Si	Total	100%	160	373	533	100%	363	224	587



		A	M Peak F	lour		PI	M Peak H	lour	
7	Travel Mode	Mode Share	In	Out	Total	Mode Share	In	Out	Total
	Auto Driver	87%/82%	109	237	346	73%/78%	181	154	335
e) #	Auto Passenger	13%	16	38	54	19%	47	37	84
Multi-Unit (Low-Rise)	Transit	0%/5%	0	17	17	5%/0%	13	0	13
e EE	Cycling	1%	2	4	6	3%	8	6	14
ΣŠ	Walking	0%	0	0	0	0%	0	0	0
	Total	100%	127	296	422	100%	249	197	446
	Auto Driver	-	243	528	771	-	456	335	791
	Auto Passenger	-	38	90	128	-	108	75	183
Total	Transit	-	0	38	38	-	32	0	32
P	Cycling	-	6	13	19	-	16	11	27
	Walking	-	0	0	0	-	0	0	0
	Total	-	287	669	955	-	612	421	1033

As shown above, a total of 771 AM and 791 PM new peak hour two-way vehicle trips are projected as a result of the proposed development.

4.3 Trip Distribution

To understand the travel patterns of the subject development, the OD Survey has been reviewed to determine the travel for the residential development, and these patterns were applied based on the build-out of Richmond Village and the Rural Southwest district. Table 10 below summarizes the distributions.

Table 10: OD Survey Distribution – Rural Southwest

To/From	% of Trips	Via
North	55%	30% Eagleson Rd, 25% McBean St
South	5%	3% Eagleson Rd, 2% McBean St
East	25%	10% Brophy Dr, 15% Eagleson Rd (north)
West	15%	McBean St (north)
Total	100%	-

4.4 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the study area road network. Figure 12 illustrates the new site generated volumes.



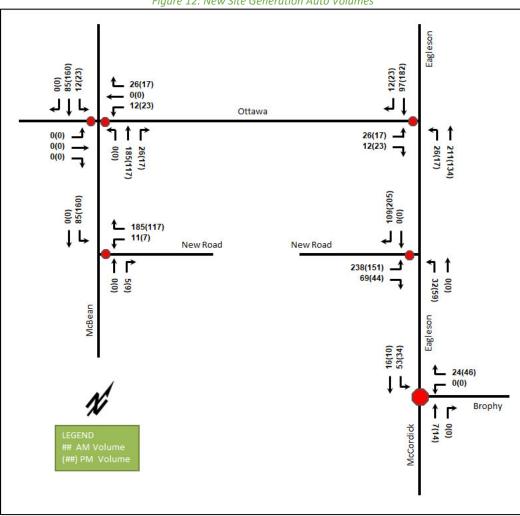


Figure 12: New Site Generation Auto Volumes

5 Exemption Review

Table 11 summarizes the exemptions for this TIA.

Table 11: Exemption Review

Module	Element	Explanation	Exempt/Required
Site Design and TDM			
4.1 Development Design	4.1.2 Circulation and Access	Only required for site plan and zoning by- law applications	Required
	4.1.3 New Street Networks	Only required for plans of subdivision	Required
4.2 Parking	4.2.1 Parking Supply	Only required for site plan and zoning by- law applications	Exempt
4.3 Boundary Street Design		All applications	Required
4.5 Transportation Demand Management	All Elements	Only required when the development generates more than 60 person-trips	Required
Network Impact			



Module	Element	Explanation	Exempt/Required
3.2 Background Network Travel Demand	All Elements	Only required when one or more other Network Impact Modules are triggered	Required
3.3 Demand Rationalization		Only required when one or more other Network Impact Modules are triggered	Required
4.6 Neighbourhood Traffic Calming	4.7.1 Transit Route	If the development meets all of the following criteria along the route(s) site generated traffic is expected to utilize between an arterial road and the site's access: 1. Access to Collector or Local; 2. "Significant sensitive land use presence" exists, where there is at least two of the following adjacent to the subject street segment: • School (within 250m walking distance); • Park; • Retirement / Older Adult Facility (i.e. long-term care and retirement homes); • Licenced Child Care Centre; • Community Centre; or • 50%, or greater, of adjacent property along the route(s) is occupied by residential lands and a minimum of 10 occupied residential units are present on the route. 3. Application is for Zoning By-Law Amendment or Draft Plan of Subdivision; 4. At least 75 site-generated auto trips; 5. Site Trip Infiltration is expected. Site traffic will increase peak hour vehicle volumes along the route by 50% or more. Only required when the development	Exempt
4.7 Transit	Capacity 4.7.2 Transit Priority Requirements	generates more than 75 transit trips Only required when the development generates more than 75 auto trips	Exempt
4.8 Network Concept		Only required when proposed development generates more than 200 person-trips during the peak hour in excess of equivalent volume permitted by established zoning	Exempt
4.9 Intersection Design	4.9.1 Intersection Control	Only required when the development generates more than 75 auto trips	Required



Module	Element	Explanation	Exempt/Required
	4.9.2 Intersection	Only required when the development	Required
	Design	generates more than 75 auto trips	

6 Development Design

6.1 Design for Sustainable Modes

The proposed development is a residential subdivision where each dwelling will include a driveway and garage providing vehicular parking. Bicycle parking is assumed to be within the individual units. The Richmond CDP and Secondary Plan show a pathway from the subject lands through the north of the site continuing along the railway line terminating at Ottawa Street. The sidewalks and pathways proposed along the northern extent of the site satisfy this connectivity. It is anticipated that as the development builds out, transit stops will be located along the internal collector road. Figure 13 illustrates the planning context of the pedestrian and cycling network for the subdivision, and Figure 14 illustrates the conceptual transit stop locations, subject to OC Transpo requirements.

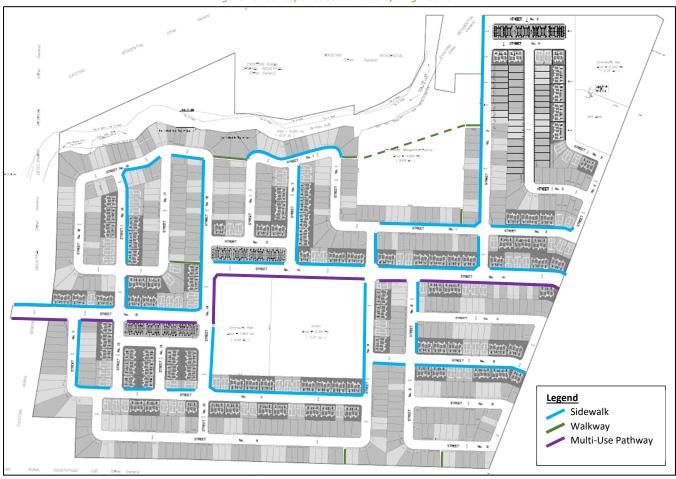


Figure 13: Concept Pedestrian and Cycling Network





Figure 14: Conceptual Transit Stops

6.2 New Street Networks

The planned street network will include 18.0-metre local roads with an 8.5-metre pavement width and 16.30-metre window streets (based on a 14.0 metre standard) with 8.5-metre pavement widths. The local roads are proposed to be posted as 30 km/h, consistent with Ottawa's 30 km/h Policy framework. A 24.0-metre collector road with a sidewalk on one side and a MUP on the other is proposed through the subdivision, connecting Eagleson Road and McBean Street and is consistent with the Ottawa's 40 km/h collector road standard. West of the residential frontages to McBean Street, the right-of-way reduces to 22.0 metres given property constraints.

Traffic calming elements are recommended at the internal intersections, including bulb-outs to narrow each approach to the intersection (e.g. reduced crossing distance). Figure 15 illustrates the concept traffic calming plan.

The internal road intersections are recommended to be stop-controlled on the minor approaches of all intersections.





Figure 15: Concept Traffic Calming Plan

7 Boundary Street Design

Table 12 summarizes the MMLOS analysis for the boundary streets of Eagleson Road, and Ottawa Street. The existing and future conditions will be the same and are considered in one row. The boundary street analysis is based on the policy area of Village. The MMLOS worksheets has been provided in Appendix E.

Table 12: Boundary Street MMLOS Analysis

Cogmont	Pedesti	rian LOS	Bicyc	le LOS	Trans	it LOS	Truc	k LOS
Segment	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target
Eagleson Road	F	С	F	D	N/A	N/A	В	D
Ottawa Street	F	С	E	В	N/A	N/A	N/A	N/A

Pedestrian and bicycle LOS targets are not being met along the boundary streets due partly to the operating speeds associated with rural arterial and collector roads and partly to the absence of dedicated active facilities.

Eagleson Road cannot meet pedestrian LOS targets due to traffic volumes and operating speeds and Ottawa Street could meet pedestrian LOS targets with a 1.8-metre sidewalk.

To meet bicycle LOS targets, Eagleson Road would require physically separated facilities due to operating speeds and Ottawa Street would require a bike lane.



GeoOttawa depicts paved shoulders along Ottawa street, and the City has indicated that City staff are studying the provision of these facilities along the roadway and no additional facilities are proposed. Active transportation along Eagleson Road will be subject to City plans for the arterial corridor.

Transportation Demand Management

Context for TDM

The mode shares used within the TIA represent a reduction to transit from the typical rural districts mode shares in the peak direction and with no transit trips in the off-peak directions. Overall, these modal shares are likely to be achieved, however, given the stated intention of not expanding bus service in the Village, limited opportunity to provide supporting TDM measures exists to encourage shifts towards sustainable modes.

The subject site is within the Richmond Village CDP area. The total bedroom count within the development is subject to the final unit count and layout selections by purchasers. No age restrictions are noted.

8.2 Need and Opportunity

The subject site has been assumed to rely predominantly on auto travel, and assumptions have been carried through the analysis. Given the high auto mode share, there is negligible risk of not meeting the applied modal shares.

8.3 TDM Program

The "suite of post occupancy TDM measures" has been summarized in the TDM checklist and the checklist is provided in Appendix F. Given the transit limitations previously discussed, the only TDM measure recommended is the provision of a multimodal travel option information package to new residents. It is recommended that the City revisit its transit strategy in the Village of Richmond given the forecasted population increase and therefore potential ridership base.

Background Network Travel Demands

9.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3. Neither the listed CDP features nor the TMP – Part 1 project are expected to have an impact on the study area traffic volumes and travel patterns.

9.2 Background Growth

A review of the background projections from the City's TRANS Regional Model for the 2011 and 2031 horizons was completed to determine the background growth for each of the study area roadways. Table 13 summarizes the results of the model and the projections are provided in Appendix G.

Table 13: TRANS Regional Model Projections – Study Area Growth Rates

Chuoch	Direction Growth % from 2011 to 2031					
Street	Eastbound	Westbound				
Ottawa St	-	-				
Brophy Dr	1.48%	-0.13%				
	Northbound	Southbound				
Eagleson Rd	0.06%	1.70%				
McBean St	-0.24%	-0.12%				



Annual growth rates derived from the TRANS model plots rounded to the nearest 0.25% will be applied to the identified roadway mainline movements and major turning movements at intersections in the AM peak hour and reversed in the PM peak hour. Table 10 summarizes the applied annual growth rates for each peak hour.

Table 14: Applied Annual Growth Rates

Chunch	AM Pea	ak Hour	PM Peak Hour			
Street	Eastbound	Westbound	Eastbound	Westbound		
Ottawa St	-	-	-	-		
Brophy Dr	1.50%	-	-	1.50%		
	Northbound	Southbound	Northbound	Southbound		
Eagleson Rd	Eagleson Rd 0.25%		1.75%	0.25%		
McBean St	-	-	-	-		

9.3 Other Developments

As the only study area development, the volumes from the 2780 Eagleson Road TIA will be considered explicitly in the background conditions. The background development volumes have been provided in Appendix H.

10 Demand Rationalization

10.1 2032 Future Background Intersection Operations

Figure 16 illustrates the 2032 background volumes and Table 15 summarizes the 2032 background intersection operations. The level of service is based on HCM 2010 delay for unsignalized intersections. The synchro worksheets for the 2032 future background horizon are provided in Appendix I. Signal warrants using OTM Justification 7 for the forecasted volumes at the intersections of Eagleson Road at Ottawa Street, Eagleson Road at Brophy Drive, and McBean Street at Ottawa Street are not met at this horizon, and the warrants are provided in Appendix J. Left-turn lane warrants were evaluated for the northbound left-turn movement at the intersection of Eagleson Road at Ottawa Street, and the southbound left-turn movement at the intersections of Eagleson Road at Brophy Drive and McBean Street at Ottawa Street at this horizon to indicate where left-turn lanes should be considered, and are provided in Appendix K. The left turn warrants are met on the southbound approach of Eagleson Road at Brophy Drive and the northbound approach of Eagleson Road at Ottawa Street. Operations will be assessed without these turn lanes to help inform whether they should be provided and a discussion on the appropriateness of providing turn lanes within the study area is provided in Section 12.2.4.



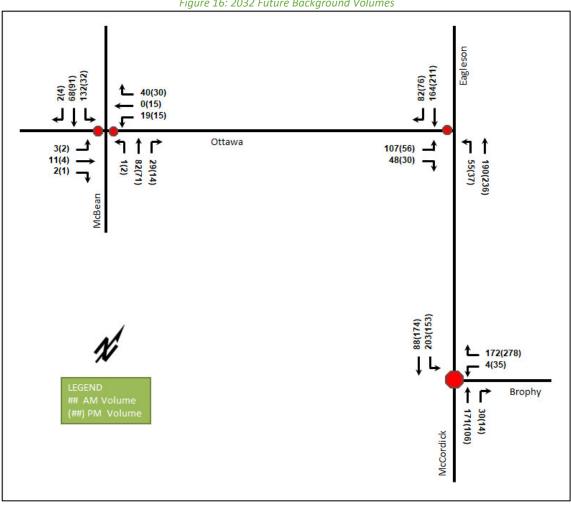


Figure 16: 2032 Future Background Volumes

Table 15: 2032 Future Background Intersection Operations

lusta va a ati a va			AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Lane	LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
5 1 5 10	EBL/R	В	0.27	13.6	8.3	В	0.16	12.7	4.5
Eagleson Road &	NBL/T	Α	0.04	7.8	0.8	Α	0.03	7.9	0.8
Ottawa Street	SBT/R	-	-	-	-	-	-	-	-
Unsignalized	Overall	Α	-	3.9	-	Α	-	2.2	-
Faslasan Dand C	WBL/R	Α	0.22	8.8	6.0	В	0.39	10.5	14.3
Eagleson Road &	NBT/R	Α	0.26	9.2	7.5	Α	0.17	9.1	4.5
Brophy Drive <i>Unsignalized</i>	SBT/L	В	0.38	10.6	13.5	В	0.45	11.9	17.3
Onsignanzea	Overall	Α	-	9.7	-	В	-	10.9	-
	EB	В	0.03	12.5	0.8	В	0.01	10.4	0.0
McBean Street &	WB	В	0.08	10.4	2.3	Α	0.08	9.9	1.5
Ottawa Street Unsignalized	NB	Α	0.00	7.4	0.0	Α	0.00	7.4	0.0
	SB	Α	0.09	7.7	2.3	Α	0.02	7.4	0.8
	Overall	Α	-	4.7	-	Α	-	3.3	-

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres Peak Hour Factor = 1.00

Delay = average vehicle delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity



During both the AM and PM peak hours, the study area intersections operate similarly to existing conditions. No capacity issues are noted, and the intersection of Eagleson Road at Ottawa Street operates well without the inclusion of an auxiliary northbound left-turn lane with negligible delays and queues on the northbound approach. The intersection of Eagleson Road at Brophy Drive on the southbound approach, with the movement operating with LOS B and with queues of less than three vehicles.

10.2 2037 Future Background Intersection Operations

Figure 17 illustrates the 2037 background volumes and Table 16 summarizes the 2037 background intersection operations. The level of service is based on HCM 2010 delay for unsignalized intersections. The synchro worksheets for the 2037 future background horizon are provided in Appendix L. Signal warrants using OTM Justification 7 for the forecasted volumes at the intersections of Eagleson Road at Ottawa Street, Eagleson Road at Brophy Drive, and McBean Street at Ottawa Street are not met at this horizon, and the warrants are provided in Appendix J. Left-turn lane warrants were evaluated for the northbound left-turn movement at the intersection of Eagleson Road at Ottawa Street, and the southbound left-turn movement at the intersections of Eagleson Road at Brophy Drive and McBean Street at Ottawa Street at this horizon to indicate where left-turn lanes should be considered, and are provided in Appendix K. The left turn warrants are met on the southbound approach of Eagleson Road at Brophy Drive and the northbound approach of Eagleson Road at Ottawa Street. Operations will be assessed without these turn lanes to help inform whether they should be provided and a discussion on the appropriateness of providing turn lanes within the study area is provided in Section 12.2.4.



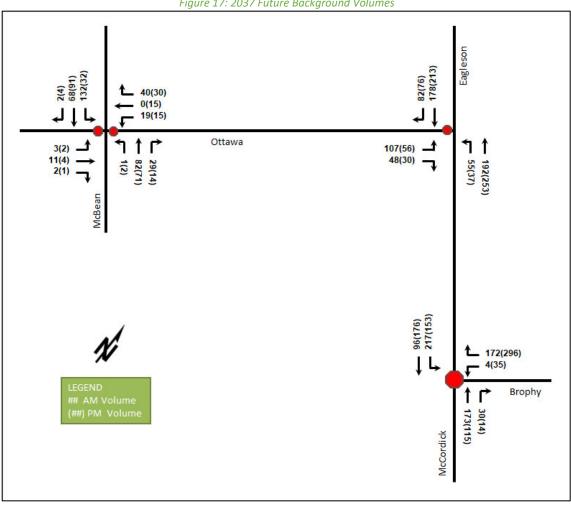


Figure 17: 2037 Future Background Volumes

Table 16: 2037 Future Background Intersection Operations

lutava atiava			AM Pea	ak Hour			PM Pe	ak Hour	
Intersection	Lane	LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Factoria Band 0	EBL/R	В	0.28	13.8	8.3	В	0.16	12.9	4.5
Eagleson Road & Ottawa Street	NBL/T	Α	0.04	7.9	0.8	Α	0.03	7.9	0.8
Unsignalized	SBT/R	-	-	-	-	-	-	-	-
Ulisighulizeu	Overall	Α	-	3.9	-	Α	-	2.1	-
Faglacon Dood 0	WBL/R	Α	0.22	8.9	6.0	В	0.42	10.8	15.8
Eagleson Road &	NBT/R	Α	0.26	9.3	7.5	Α	0.18	9.3	5.3
Brophy Drive <i>Unsignalized</i>	SBT/L	В	0.41	11.0	15.0	В	0.46	12.1	18.0
Olisigilalizea	Overall	Α	-	10.0	-	В	-	11.1	-
	EB	В	0.03	12.5	0.8	В	0.01	10.4	0.0
McBean Street &	WB	В	0.08	10.4	2.3	Α	0.08	9.9	1.5
Ottawa Street Unsignalized	NB	-	-	-	-	-	-	-	-
	SB	Α	0.09	7.7	2.3	Α	0.02	7.4	0.8
	Overall	Α	-	4.7	-	Α	-	3.3	-

Saturation flow rate of 1800 veh/h/lane Notes:

Queue is measured in metres Peak Hour Factor = 1.00

Delay = average vehicle delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity



During both the AM and PM peak hours, the study area intersections operate similarly to 2032 background conditions. No capacity issues are noted, and the intersection of Eagleson Road at Ottawa Street operates well without the inclusion of an auxiliary northbound left-turn lane with negligible delays and queues on the northbound approach. The intersection of Eagleson Road at Brophy Drive on the southbound approach, with the movement operating with LOS B and with queues of less than three vehicles.

10.3 Modal Share Sensitivity

As an increased auto and reduced transit share beyond the typical rural district modal share targets have been applied to the development, the village and rural contexts are maintained, and no capacity constraints are noted within the study area, rationalization for adjusted demand is not required for this TIA.

11 Transit

11.1 Route Capacity

In Section 5.1 the trip generation by mode was estimated, including an estimate of the number of transit trips that will be generated by the proposed development. Table 17 summarizes the transit trip generation.

Table 17: Trip Generation by Transit Mode

Travel Mode	Mada Chara	А	M Peak Ho	ur	P	M Peak Ho	ur
	Mode Share	In	Out	Total	In	Out	Total
Transit	varies	0	38	38	32	0	32

The proposed development is anticipated to generate an additional 38 AM peak hour transit trips and 32 PM peak hour peak direction transit trips. Overall, the forecasted new transit trips would result in the need for up to one additional single bus (55-person capacity) during the AM and PM peak hours. Should transit service increase in the village, the transit share may additionally increase and thus the need for additional buses may result.

11.2 Transit Priority

Site traffic is anticipated to increase delay on existing transit movements within the study area by 0.3 second or less, and thus is anticipated to have negligible impact on transit service in the Village.

12 Intersection Design

12.1 Intersection Control

No change to the existing control is recommended for the network intersections.

The site access road intersections are proposed as having minor stop control with Eagleson Road and McBean Street operating under free-flow conditions based on the projected volumes.

12.2 Intersection Design

12.2.1 2032 Future Total Intersection Operations

The 2032 future total intersection volumes are illustrated in Figure 18 and the intersection operations are summarized below in Table 18. The level of service is based on HCM 2010 delay for unsignalized intersections. The synchro worksheets have been provided in Appendix M. Signal warrants using OTM Justification 7 for the forecasted volumes at the intersections of Eagleson Road at Ottawa Street, Eagleson Road at Brophy Drive, McBean Street at Ottawa Street, Eagleson Road at the new collector, and McBean Street at the new collector are not met at this horizon, and the warrants are provided in Appendix J. Left-turn lane warrants were evaluated for the northbound left-turn movement at the intersection of Eagleson Road at Ottawa Street, and Eagleson Road at



the new collector road, and the southbound left turn at the intersections of Eagleson Road at Brophy Drive and McBean Street at Ottawa Street, and McBean Street at the new collector road at this horizon, and are provided in Appendix K. At this horizon, the left turn warrants are met on the southbound approaches of Eagleson Road at Brophy Drive and McBean Street at Ottawa Street, and on the northbound approaches of Eagleson Road at Ottawa Street and Eagleson Road at the new collector. Operations will be assessed without these turn lanes to help inform whether they should be provided, and a discussion on the appropriateness of providing turn lanes within the study area is provided in Section 12.2.4.

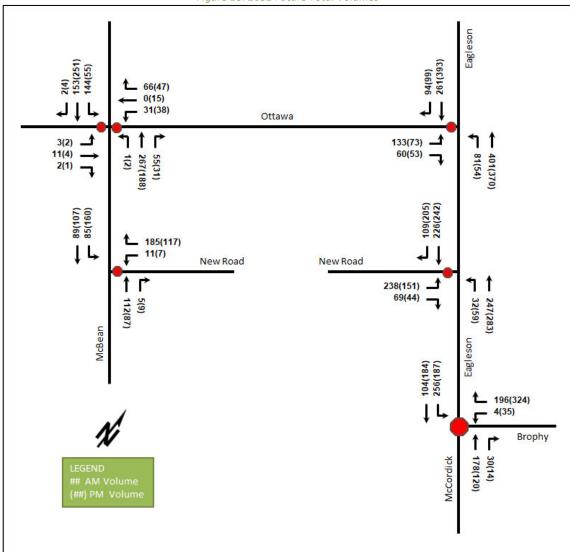


Figure 18: 2032 Future Total Volumes

Table 18: 2032 Future Total Intersection Operations

luta va asti a v	1		AM Pe	ak Hour		PM Peak Hour				
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)	
- 1 - 10	EB	D	0.54	25.9	22.5	С	0.35	20.0	11.3	
Eagleson Road &	NB	Α	0.07	8.2	1.5	Α	0.05	8.5	1.5	
Ottawa Street Unsignalized	SB	-	-	-	-	-	-	-	-	
	Overall	Α	-	5.5	-	Α	-	2.9	-	



Intersection	1	AM Peak Hour				PM Peak Hour			
	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
Eagleson Road & Brophy Drive Unsignalized	WB	Α	0.26	9.4	7.5	В	0.47	11.8	18.8
	NB	Α	0.28	9.6	8.3	Α	0.20	9.6	5.3
	SB	В	0.48	12.2	19.5	В	0.53	13.9	24.0
	Overall	В	-	10.8	-	В	-	12.4	-
	EB	С	0.05	17.2	1.5	В	0.02	13.8	0.8
McBean Street & Ottawa Street <i>Unsignalized</i>	WB	В	0.20	14.1	5.3	В	0.19	13.2	5.3
	NB	Α	0.00	7.5	0.0	Α	0.00	7.8	0.0
	SB	Α	0.12	8.3	3.0	Α	0.04	7.8	0.8
	Overall	Α	-	3.9	-	Α	-	2.9	-
Foologen Dood 9	EB	С	0.61	23.0	30.8	С	0.49	22.2	19.5
Eagleson Road & New Collector	NB	Α	0.03	8.0	0.8	Α	0.05	8.4	1.5
Unsignalized	SB	-	-	-	-	-	-	-	-
	Overall	Α	-	7.9	-	Α	-	4.9	-
McBean Street & New Collector Unsignalized	WB	В	0.22	10.1	6.0	Α	0.14	9.6	3.8
	NB	-	-	-	-	-	-	-	-
	SB	Α	0.06	7.6	1.5	Α	0.11	7.7	3.0
	Overall	Α	-	5.4	-	Α	-	5.0	-

The study intersections at the 2032 future total horizon generally operate similarly to the 2032 future background conditions.

With the addition of site traffic, delays on the eastbound left/right movement at the intersection of Eagleson Road at Ottawa Street are forecast to increase by 12.3 seconds during the AM peak hour and 7.3 seconds during the PM peak hour above the background conditions. Beyond delays, the movement is anticipated to retain a high residual capacity in the total conditions. The intersection is forecast to continue to operate well without the inclusion of an auxiliary northbound left-turn lane with negligible delays and queues.

The intersection of Eagleson Road at Brophy Drive on the southbound approach, with the movement operating with LOS B and with queues of approximately three vehicles.

The site access intersections are anticipated to operate well at the 2032 horizon, with delays of 23.0 seconds or less on the outbound movements of the new collector road, and 8.4 seconds or less on the conflicting left movements on the arterials. The site access operate well without the inclusion of inbound left-turn lanes, with negligible delays and queues on the arterial approaches.

12.2.2 2037 Future Total Intersection Operations

The 2037 future total intersection volumes are illustrated in Figure 19 and the intersection operations are summarized below in Table 19. The level of service is based on HCM 2010 delay for unsignalized intersections. The synchro worksheets have been provided in Appendix N. Signal warrants using OTM Justification 7 for the forecasted volumes at the intersections of Eagleson Road at Ottawa Street, Eagleson Road at Brophy Drive, and McBean Street at Ottawa Street are not met at this horizon, and the warrants are provided in Appendix J. Left-turn lane warrants were evaluated for the northbound left-turn movement at the intersection of Eagleson Road at Ottawa Street, and the southbound left-turn movement at the intersections of Eagleson Road at Brophy Drive and McBean Street at Ottawa Street at this horizon to indicate where left-turn lanes should be considered, and are provided in Appendix K. The left turn warrants are met on the southbound approach of Eagleson Road at Brophy Drive and the northbound approach of Eagleson Road at Ottawa Street. Operations will be assessed



without these turn lanes to help inform whether they should be provided and a discussion on the appropriateness of providing turn lanes within the study area is provided in Section 12.2.4.

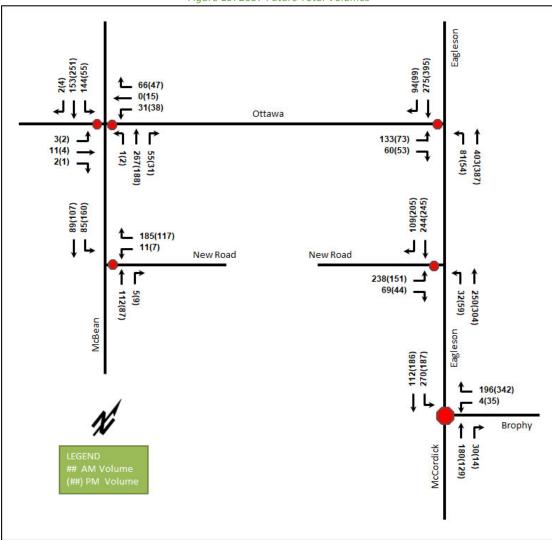


Figure 19: 2037 Future Total Volumes

Table 19: 2037 Future Total Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
Eagleson Road & Ottawa Street Unsignalized	EB	D	0.55	26.8	23.3	С	0.35	20.5	12.0
	NB	Α	0.07	8.2	1.5	Α	0.05	8.5	1.5
	SB	-	-	-	-	-	-	-	-
Onsignanzea	Overall	Α	-	5.6	-	Α	-	2.9	-
Eagleson Road & Brophy Drive Unsignalized	WB	Α	0.27	9.5	7.5	В	0.50	12.5	21.8
	NB	Α	0.28	9.7	8.3	Α	0.22	9.9	6.0
	SB	В	0.51	12.8	21.8	В	0.54	14.3	24.8
	Overall	В	-	11.1	-	В	-	12.8	-



Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	EB	С	0.05	17.2	1.5	В	0.02	13.8	0.8
McBean Street &	WB	В	0.20	14.1	5.3	В	0.19	13.2	5.3
Ottawa Street	NB	Α	0.00	7.5	0.0	Α	0.00	7.8	0.0
Unsignalized	SB	Α	0.12	8.3	3.0	Α	0.04	7.8	0.8
	Overall	Α	-	3.9	-	Α	-	2.9	-
Faciliana Dand 9	EB	С	0.63	24.3	32.3	С	0.50	23.2	20.3
Eagleson Road & New Collector	NB	Α	0.03	8.1	0.8	Α	0.05	8.4	1.5
Unsignalized	SB	-	-	-	-	-	-	-	-
Onsignanzea	Overall	Α	-	8.2	-	Α	-	5.0	-
McBean Street & New Collector Unsignalized	WB	В	0.22	10.1	6.0	Α	0.14	9.6	3.8
	NB	-	-	-	-	-	-	-	-
	SB	Α	0.06	7.6	1.5	Α	0.11	7.7	3.0
	Overall	Α	-	5.4	-	Α	-	5.0	-

The network intersection operations at the 2037 future total horizon operate similarly to the 2032 future total conditions. No capacity or delay issues are noted. The intersection of Eagleson Road at Ottawa Street and at the new collector are forecast to continue to operate well without the inclusion of auxiliary northbound left-turn lane each with negligible delays and queues. The intersection of Eagleson Road at Brophy Drive on the southbound approach, with the movement operating with LOS B and with queues of less than three vehicles.

12.2.3 Intersection MMLOS

As the network and access intersections are not signalized, no intersection MMLOS analysis is required.

12.2.4 Recommended Design Elements

The design elements for the site access intersections will be typical for urban collector road intersections with rural arterial roads, subject to the civil design.

Signal warrants are not met for all network and access intersections. While planning to date has assumed that the intersection of Eagleson Road at Ottawa Street would be signalized in future, it is forecast to operate well with minor stop control and is not recommended to be implemented to support the subject residential development.

Turning lane warrants for evaluated movements are met at locations as of horizons summarized in Table 20.

Table 20: Left-Turn Lane Warrant Results

Intersection	Movement	First Horizon Met		
McBean Street at Ottawa Street	Southbound Left	Future Total 2032		
Eagleson Road at Ottawa Street	Northbound Left	Future Background 2032		
Eagleson Road at Brophy Drive	Southbound Left	Future Background 2032		
Eagleson Road at New Collector Road	Northbound Left	Future Total 2032		
McBean Street at New Collector Road	Southbound Left	Not Warranted		

As summarized in the operational assessment sections, the northbound movement at the intersection of Eagleson Road at Ottawa Street and the southbound movements at the intersections of McBean Street at Ottawa Street and Eagleson Road at Brophy Drive operate well without the inclusion of the warranted turn lanes. Turn lane warrants are only an indicator for the appropriateness of evaluating the inclusion of a turn lane and the subsequent traffic analysis indicated that no turn lane is required to support the operations at the network intersections where turn lanes are warranted. Therefore, no turn lanes are recommended for implementation at these locations.



As discussed in the operational assessments, the northbound Eagleson Road at the new collector road operates well without the inclusion of the inbound left-turn lane into the subject subdivision. Although warranted, this lane is not required to support the site operations and is not recommended for implementation. Should it be provided, the minor delays and negligible queues on the arterial approaches will be shifted to the turn lanes.

The desirability, location, design, and supportability of a northbound left-turn lane on Eagleson Road at the new collector road will ultimately need to be determined through consultation with City staff, given recent construction works on Eagleson Road and the low inbound volumes from the southeast. Should, through this consultation, the northbound left-turn lane on Eagleson Road be deemed required, an RMA would be initiated to satisfy the resulting subdivision conditions. The anticipated storage length for a northbound left-turn lanes on Eagleson Road at the new collector, per equation 9.14.1 of the Geometric Design Guide for Canadian Roads (TAC, 2017) and the TIA Guidelines requirement of using 1.5 times the expected traffic volumes is 21 metres for the 2037 future total horizon. A design length of would be 25 metres would be appropriate for this potential new lane.

13 Summary of Improvements Indicated and Modifications Options

The following summarizes the analysis and results presented in this TIA report:

Proposed Site and Screening

- The proposed site includes up to 1,173 homes, split between 535 single family homes, 144 semi-detached homes, and 494 townhomes
- Accesses will be provided along the internal road network, with a new collector road connecting McBean Street to Eagleson Road
- The development is proposed to be completed through an unconfirmed number of phases by 2032
- The trip generation, location, and safety triggers were met for the TIA Screening
- This report is in support of a draft plan of subdivision application

Existing Conditions

- Eagleson Road, McBean Street, and Brophy Drive are arterial roads, and Ottawa Street is a collector road in the study area
- Sidewalks are provided along the east side of McBean Street to the north of the South Carleton High School and on a few local streets to the north of the study area
- Per geoOttawa, Ottawa Street depicts paved shoulders between McBean Street, however these do not
 exist and the City is studying the need to provide facilities in this location
- One regular and one special transit route service the study area with stops at the northwest extent of the site
- No areas of high collisions are noted within the study area, with SMV other collisions, typical of rural arterials, accounting for a slight majority of collisions within the study area
- The study area intersections operate well during the peak hours

Planned Conditions

- Separated cycling facilities are planned along McBean Street north of Ottawa Street
- Area traffic management measures are planned along Ottawa Street in front of South Carleton High School to commence construction this year



Development Generated Travel Demand

- The proposed residential development is forecasted produce 955 two-way person trips during the AM peak hour and 1033 two-way person trips during the PM peak hour
- Of the forecasted person trips, 771 two-way trips will be vehicle trips during the AM peak hour and 791 two-way trips will be vehicle trips during the PM peak hour based on a 73%-87% auto mode share
- Of the forecasted trips, 55% are anticipated to travel to/from the north, 5% to/from the south, 25% to/from the east, and 15% to/from the west

Development Design

- The Richmond CDP and Secondary Plan show a pathway from the subject lands through the north of the site continuing along the railway line terminating at Ottawa Street and the sidewalks and pathways proposed along the northern extent of the site satisfy this connectivity
- Internal transit stop locations will be subject to OC Transpo requirements
- 18.0-metre local roads with 8.5-metre pavement widths and 16.30-metre local window streets (based on a 14.0 metre standard) with 8.5-metre pavement widths are proposed to be posted as 30 km/h, consistent with Ottawa's 30 km/h Policy framework
- A 24.0-metre collector road with a sidewalk on one side and a MUP on the other is proposed through the subdivision, connecting Eagleson Road and McBean Street and is consistent with the Ottawa's 40 km/h collector road standard
- Traffic calming elements are recommended at the internal intersections, including bulb-outs to narrow each approach to the intersection
- The internal road intersections are recommended to be stop-controlled on the minor approaches of all intersections

Boundary Street Design

- Pedestrian and bicycle LOS targets are not being met along the boundary streets due partly to the operating speeds associated with rural arterial and collector roads and partly to the absence of dedicated active facilities
- Eagleson Road cannot meet pedestrian LOS targets due to traffic volumes and operating speeds and Ottawa Street could meet pedestrian LOS targets with a 1.8-metre sidewalk
- To meet bicycle LOS targets, Eagleson Road would require physically separated facilities due to operating speeds and Ottawa Street would require a bike lane
- The City has indicated that City staff are studying the provision of these facilities along the roadway and no additional facilities are proposed, and active transportation along Eagleson Road will be subject to City plans for the arterial corridor

TDM

- Given the stated intention of not expanding bus service in the Village, limited opportunity to provide supporting TDM measures exists to encourage shifts towards sustainable modes
- The only TDM measure recommended is the provision of a multimodal travel option information package to new residents
- It is recommended that the City revisit its transit strategy in the Village of Richmond given the forecasted population increase and therefore potential ridership base



Background Conditions

- The 2780 Eagleson Road traffic was explicitly included in the background conditions, along with an annual background growth rate derived from the 2011 and 2031 TRANS volume plots applied to the mainline movements of Eagleson Road and the turns to and from Brophy Drive
- No intersections meet signal warrants for the future background conditions
- Left-turn lane warrants are met for the northbound left-turn movement at the intersection of Eagleson Road at Ottawa Street and the southbound left-turn movement at the intersection of Eagleson Road at Brophy Drive for the future background conditions
- The study area intersections at the 2032 and 2037 horizons are forecast to operate similarly to the existing conditions and the left-turn-warranted movements operate well without the inclusion of turn lanes

Transit

- Transit trips at the existing mode share result in 38 outbound AM trips and 32 inbound PM trips, resulting
 in the need for up to one additional peak-direction bus during each peak hour
- Should transit service increase in the village, the transit share may additionally increase and thus the need for additional buses may result
- Site traffic is anticipated to increase delay on existing transit movements within the study area by 0.3 second or less, and thus is anticipated to have negligible impact on transit service in the Village

Network Intersection Design

- The access intersections are recommended to be stop-controlled on the minor approach
- No intersections meet signal warrants for the future total conditions
- In addition to those warranted left-turn lanes for the background conditions, the southbound left-turn lane at the intersection of McBean Street at Eagleson Road and the northbound left-turn lane at the intersection of Eagleson Road at the new collector road are warranted at the future total horizons
- The intersections at the future total horizons generally operate similarly to the future background horizon, with a minor increase in delays on the eastbound approach of the intersection of Eagleson Road at Ottawa Street
- All study area intersections are forecast to operate well without the warranted left-turn lanes
- While planning to date has assumed that the intersection of Eagleson Road at Ottawa Street would be signalized in future, it is not warranted based on forecasted volumes, is forecast to operate well with minor stop control, and is not recommended to support the subject development traffic
- No left-turn lanes are required to address intersection operations at any study horizon, and none are recommended to be provided
- If through consultation with the City, it is deemed that a northbound left-turn lane on Eagleson Road at the new collector road is desirable and feasible, and an RMA would be initiated to satisfy the resulting subdivision conditions
- The anticipated required storage length of a left-turn lane on Eagleson Road at the new collector road is 25 metres



14 Conclusion

It is recommended that, from a transportation perspective, the proposed development applications proceed.

Prepared By:

Reviewed By:



Andrew Harte, P.Eng. Senior Transportation Engineer

John Kingsley
Transportation Engineering-Intern



Appendix A

TIA Screening Form and PM Certification Form





City of Ottawa 2023 Revisions to 2017 TIA Guidelines Step 1 - Screening Form

Date: 06-Dec-24
Project Number: 2023-080
Project Reference: Tamarack Richmond Village

1.1 Description of Proposed Development	
Municipal Address	6038 Ottawa St
Description of Legation	Irregular shaped 67.24 ha parcel south of Ottawa St,
Description of Location	west of Eagleson Road, east of McBean St
Land Use Classification	Rural General Industrial (RG3)
Development Size	1,1773 Residential Dwellings
Accorde	One new collector road connecting McBean St and
Accesses	Eagleson Rd
Phase of Development	Unknown
Buildout Year	2032
TIA Requirement	Full TIA Required

1.2 Trip Generation Trigger	
Land Use Type	Multi-Family (Low-Rise)
Development Size	1173 Units
Trip Generation Trigger	Yes

1.3 Location Triggers	
Does the development propose a new driveway to a boundary street that is designated as part of the Transit Priority Network, Rapid Transit network or	No
Cross-Town Bikeways?	
Is the development in a Hub, a Protected Major Transit Station Area (PMTSA), or a Design Priority Area (DPA)?	No
Location Trigger	No

1.4. Safety Triggers	
Are posted speed limits on a boundary street 80 km/hr or greater?	Yes
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?	No
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?	No
Is the proposed driveway within auxiliary lanes of an intersection?	No
Does the proposed driveway make use of an existing median break that serves an existing site?	No
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?	No
Does the development include a drive-thru facility?	No
Safety Trigger	Yes



Certification Form for TIA Study PM

TIA Plan Reports

CERTIFICATION

On April 14, 2022, the Province's Bill 109 received Royal Assent providing legislative direction to implement the More Homes for Everyone Act, 2022 aiming to increase the supply of a range of housing options to make housing more affordable. Revisions have been made to the TIA guidelines to comply with Bill 109 and streamline the process for applicants and staff.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that they meet the four criteria listed below.

✓ I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines; (Update effective July 2023) ✓ I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review; ✓ I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and ✓ I am either a licensed or registered¹ professional in good standing, whose field of expertise ✓ is either transportation engineering ✓ or transportation planning.

¹ License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

City Of Ottawa Planning, Real Estate and Economic Development 110 Laurier Avenue West, 4th fl. Ottawa, ON K1P 1J1 Tel: 613-580-2424

Tel.: 613-580-2424 Fax: 613-560-6006

Revision Date: June 2023

_{Dated at} Ottawa	this <u>17</u>	_{day of} August	, ₂₀ <u>23</u>
(C	ity)		
Name : Andrew H	arte		
Professional title: So	enior Transportation Er	ngineer / Vice-President Ottawa	a
Juliu Rati	t		
	al certifier that s/he/they m	neet the above criteria	

Office Con	Office Contact Information (Please Print)										
Address:	6 Plaza Court										
City / Postal	Code: Ottawa, K2H 7W1										
Telephone /	Extension: 613-697-3797										
Email Addre	ss: andrew.harte@cghtransportation.com										

Stamp



Revision Date: June 2023

Appendix B

Turning Movement Counts



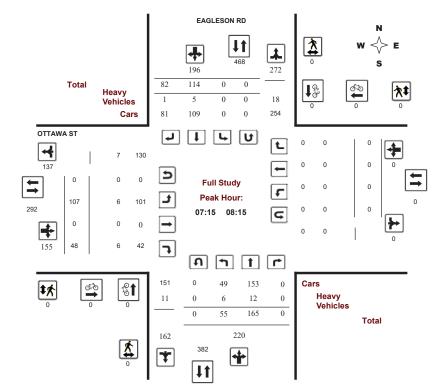


Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

Survey Date: Tuesday, March 07, 2023 WO No: 40864 Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram





Start Time: 07:00

Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

WO No:

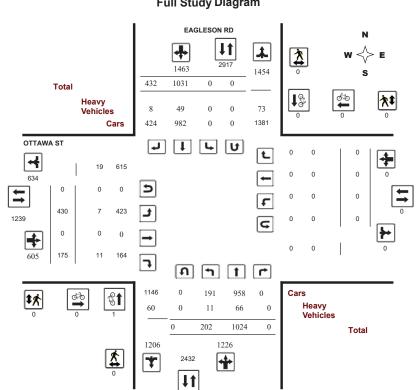
Device:

40864

Miovision

Survey Date: Tuesday, March 07, 2023

Full Study Diagram



September 9, 2024 Page 2 of 11 September 9, 2024 Page 1 of 11



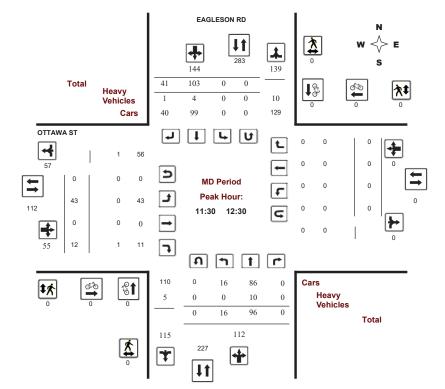
Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

 Survey Date:
 Tuesday, March 07, 2023
 WO No:
 40864

 Start Time:
 07:00
 Device:
 Miovision

MD Period Peak Hour Diagram





Transportation Services - Traffic Services

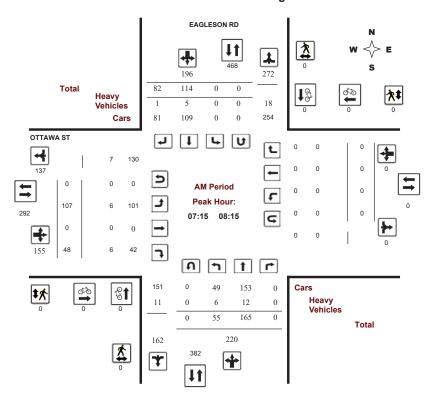
Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

 Survey Date:
 Tuesday, March 07, 2023
 WO No:
 40864

 Start Time:
 07:00
 Device:
 Miovision

AM Period Peak Hour Diagram



 September 9, 2024
 Page 4 of 11
 September 9, 2024
 Page 3 of 11



Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

Survey Date: Tuesday, March 07, 2023 WO No: 40864 Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, March 07, 2023 **Total Observed U-Turns AADT Factor** Northbound: 0 Southbound: 0 1.00

	U		U	
Eastbound:	0	Westbound:	0	

								Eastboul	iu. ()		wesi	Douriu.	U						
			EAG	LESON	N RD							01	TAWA	ST					
	No	rthbou	nd		So	uthbou	und	_	_	Е	astbou	ınd		W	estbou	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	52	160	0	212	0	96	85	181	393	95	0	44	139	0	0	0	0	139	532
08:00 09:00	23	168	0	191	0	115	26	141	332	67	0	19	86	0	0	0	0	86	418
09:00 10:00	19	123	0	142	0	96	31	127	269	36	0	22	58	0	0	0	0	58	327
11:30 12:30	16	96	0	112	0	103	41	144	256	43	0	12	55	0	0	0	0	55	311
12:30 13:30	16	83	0	99	0	94	40	134	233	46	0	16	62	0	0	0	0	62	295
15:00 16:00	19	136	0	155	0	159	67	226	381	47	0	25	72	0	0	0	0	72	453
16:00 17:00	39	141	0	180	0	188	74	262	442	59	0	26	85	0	0	0	0	85	527
17:00 18:00	18	117	0	135	0	180	68	248	383	37	0	11	48	0	0	0	0	48	431
Sub Total	202	1024	0	1226	0	1031	432	1463	2689	430	0	175	605	0	0	0	0	605	3294
U Turns				0				0	0				0				0	0	0
Total	202	1024	0	1226	0	1031	432	1463	2689	430	0	175	605	0	0	0	0	605	3294
EQ 12Hr	281	1423	0	1704	0	1433	600	2034	3738	598	0	243	841	0	0	0	0	841	4579
Note: These	values a	ire calcu	lated by	y multiply	ing the	totals b	y the a	ppropriate	e expans	ion fact	or.			1.39					
AVG 12Hr	281	1423	0	1704	0	1877	787	2034	3738	598	0	243	841	0	0	0	0	841	4579
Note: These	volumes	are cald	culated	by multip	olying t	he Equi	valent 1	2 hr. tota	ls by the	AADT 1	factor.			1.00					
AVG 24Hr	368	1864	0	2232	0	2459	1031	2665	4897	783	0	318	1102	0	0	0	0	1102	5998

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor. 1.31

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

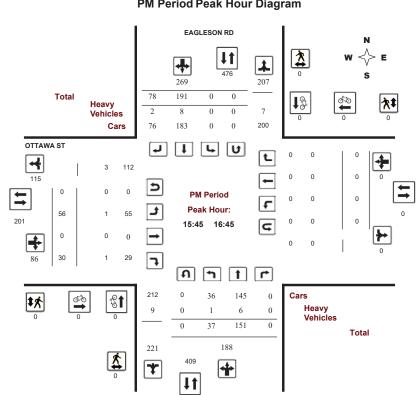
EAGLESON RD @ OTTAWA ST

Survey Date: Tuesday, March 07, 2023 WO No: Start Time: 07:00 Device:

PM Period Peak Hour Diagram

40864

Miovision



September 9, 2024 Page 6 of 11 September 9, 2024 Page 5 of 11



Time Period Northbound

0

0

0

0

Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

 Survey Date:
 Tuesday, March 07, 2023
 WO No:
 40864

 Start Time:
 07:00
 Device:
 Miovision

Street Total

0

EAGLESON RD

Southbound

Full Study Cyclist Volume

Eastbound

OTTAWA ST

Street Total

Grand Total

Westbound

07:00 07:15	1	0	1	0	0	0	1
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45 07:45 08:00	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
08:00 08:15 08:00 08:35 08:30 08:45 08:45 09:00 09:00 09:15 09:15 09:30 09:45 10:00	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
40.00 40.45	0				_		_

0

0

0



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

 Survey Date:
 Tuesday, March 07, 2023
 WO No:
 40864

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute Increments

EAGLESON RD OTTAWA ST

		No	orthbo	und		So	outhbou	ınd			E	astbou	nd		W	estbour	nd			
Time Peri	iod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR	Grand Total
07:00 07	7:15	3	32	0	35	0	17	11	28	63	18	0	5	23	0	0	0	0	23	86
07:15 07	7:30	11	46	0	57	0	26	29	55	112	25	0	10	35	0	0	0	0	35	147
07:30 07	7:45	17	37	0	54	0	31	18	49	103	33	0	14	47	0	0	0	0	47	150
07:45 08	3:00	21	45	0	66	0	22	27	49	115	19	0	15	34	0	0	0	0	34	149
08:00 08	3:15	6	37	0	43	0	35	8	43	86	30	0	9	39	0	0	0	0	39	125
08:15 08	3:30	3	39	0	42	0	28	8	36	78	14	0	4	18	0	0	0	0	18	96
08:30 08	3:45	8	51	0	59	0	31	3	34	93	11	0	4	15	0	0	0	0	15	108
08:45 09	9:00	6	41	0	47	0	21	7	28	75	12	0	2	14	0	0	0	0	14	89
09:00 09	9:15	10	28	0	38	0	22	7	29	67	13	0	5	18	0	0	0	0	18	85
09:15 09	9:30	6	30	0	36	0	26	6	32	68	13	0	7	20	0	0	0	0	20	88
09:30 09	9:45	1	38	0	39	0	20	9	29	68	5	0	6	11	0	0	0	0	11	79
09:45 10	0:00	2	27	0	29	0	28	9	37	66	5	0	4	9	0	0	0	0	9	75
11:30 11	1:45	6	32	0	38	0	31	7	38	76	9	0	7	16	0	0	0	0	16	92
11:45 12	2:00	3	20	0	23	0	23	13	36	59	14	0	1	15	0	0	0	0	15	74
12:00 12	2:15	4	27	0	31	0	19	10	29	60	11	0	3	14	0	0	0	0	14	74
12:15 12	2:30	3	17	0	20	0	30	11	41	61	9	0	1	10	0	0	0	0	10	71
12:30 12	2:45	8	23	0	31	0	31	7	38	69	14	0	3	17	0	0	0	0	17	86
12:45 13	3:00	1	18	0	19	0	20	6	26	45	9	0	3	12	0	0	0	0	12	57
13:00 13	3:15	3	23	0	26	0	20	11	31	57	11	0	8	19	0	0	0	0	19	76
13:15 13	3:30	4	19	0	23	0	23	16	39	62	12	0	2	14	0	0	0	0	14	76
15:00 15	5:15	3	26	0	29	0	36	15	51	80	12	0	6	18	0	0	0	0	18	98
15:15 15	5:30	6	30	0	36	0	28	11	39	75	13	0	8	21	0	0	0	0	21	96
15:30 15	5:45	6	42	0	48	0	44	19	63	111	13	0	4	17	0	0	0	0	17	128
15:45 16	6:00	4	38	0	42	0	51	22	73	115	9	0	7	16	0	0	0	0	16	131
16:00 16	3:15	9	35	0	44	0	46	17	63	107	15	0	5	20	0	0	0	0	20	127
16:15 16	3:30	14	36	0	50	0	41	14	55	105	16	0	6	22	0	0	0	0	22	127
16:30 16	3:45	10	42	0	52	0	53	25	78	130	16	0	12	28	0	0	0	0	28	158
16:45 17	7:00	6	28	0	34	0	48	18	66	100	12	0	3	15	0	0	0	0	15	115
17:00 17	7:15	5	17	0	22	0	39	22	61	83	17	0	2	19	0	0	0	0	19	102
17:15 17	7:30	7	44	0	51	0	50	15	65	116	6	0	1	7	0	0	0	0	7	123
17:30 17	7:45	3	33	0	36	0	58	10	68	104	8	0	5	13	0	0	0	0	13	117
17:45 18	3:00	3	23	0	26	0	33	21	54	80	6	0	3	9	0	0	0	0	9	89
Total:		202	1024	0	1226	0	1031	432	1463	2689	430	0	175	605	0	0	0	0	605	3,294

Note: U-Turns are included in Totals.

 September 9, 2024
 Page 8 of 11
 September 9, 2024
 Page 7 of 11



Northbound

Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

 Survey Date:
 Tuesday, March 07, 2023
 WO No:
 40864

 Start Time:
 07:00
 Device:
 Miovision

Full Study Heavy Vehicles

Eastbound

Westbound

EAGLESON RD OTTAWA ST

Southbound

Time Perio	d LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:1	5 0	3	0	3	0	1	0	1	4	0	0	0	0	0	0	0	0	0	4
07:15 07:3	0 0	4	0	4	0	1	0	1	5	0	0	0	0	0	0	0	0	0	5
07:30 07:4	5 5	2	0	7	0	3	1	4	11	4	0	2	6	0	0	0	0	6	17
07:45 08:0	0 0	2	0	2	0	1	0	1	3	2	0	3	5	0	0	0	0	5	8
08:00 08:1	5 1	4	0	5	0	0	0	0	5	0	0	1	1	0	0	0	0	1	6
08:15 08:3	0 0	3	0	3	0	1	1	2	5	0	0	0	0	0	0	0	0	0	5
08:30 08:4	5 0	2	0	2	0	5	0	5	7	0	0	0	0	0	0	0	0	0	7
08:45 09:0	0 0	1	0	1	0	5	1	6	7	0	0	0	0	0	0	0	0	0	7
09:00 09:1	5 1	0	0	1	0	1	0	1	2	0	0	0	0	0	0	0	0	0	2
09:15 09:3	0 0	1	0	1	0	2	0	2	3	0	0	1	1	0	0	0	0	1	4
09:30 09:4	5 0	6	0	6	0	4	0	4	10	0	0	0	0	0	0	0	0	0	10
09:45 10:0	0 0	4	0	4	0	1	0	1	5	0	0	0	0	0	0	0	0	0	5
11:30 11:4	5 0	3	0	3	0	1	0	1	4	0	0	0	0	0	0	0	0	0	4
11:45 12:0	0 0	3	0	3	0	0	1	1	4	0	0	0	0	0	0	0	0	0	4
12:00 12:1	5 0	2	0	2	0	3	0	3	5	0	0	1	1	0	0	0	0	1	6
12:15 12:3	0 0	2	0	2	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
12:30 12:4	5 0	2	0	2	0	2	0	2	4	0	0	0	0	0	0	0	0	0	4
12:45 13:0	0 0	1	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
13:00 13:1	5 0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	0	0	0	2
13:15 13:3	0 0	2	0	2	0	2	2	4	6	0	0	0	0	0	0	0	0	0	6
15:00 15:1	5 1	3	0	4	0	2	0	2	6	0	0	0	0	0	0	0	0	0	6
15:15 15:3	0 1	2	0	3	0	1	0	1	4	0	0	1	1	0	0	0	0	1	5
15:30 15:4	5 0	1	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
15:45 16:0	0 0	0	0	0	0	2	2	4	4	0	0	0	0	0	0	0	0	0	4
16:00 16:1	5 1	3	0	4	0	2	0	2	6	1	0	1	2	0	0	0	0	2	8
16:15 16:3	0 0	2	0	2	0	2	0	2	4	0	0	0	0	0	0	0	0	0	4
16:30 16:4	5 0	1	0	1	0	2	0	2	3	0	0	0	0	0	0	0	0	0	3
16:45 17:0	0 1	1	0	2	0	3	0	3	5	0	0	1	1	0	0	0	0	1	6
17:00 17:1	5 0	1	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
17:15 17:3	0 0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	0	0	0	2
17:30 17:4	5 0	3	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
17:45 18:0	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total: Non	e 11	66	0	77	0	49	8	57	134	7	0	11	18	0	0	0	0	18	152



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

 Survey Date:
 Tuesday, March 07, 2023
 WO No:
 40864

 Start Time:
 07:00
 Device:
 Miovision

Full Study Pedestrian Volume EAGLESON RD OTTAWA ST

Time Period (NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0

 September 9, 2024
 Page 10 of 11
 September 9, 2024
 Page 9 of 11

17:00 17:15 17:15 17:30 17:30 17:45 17:45 18:00 Total



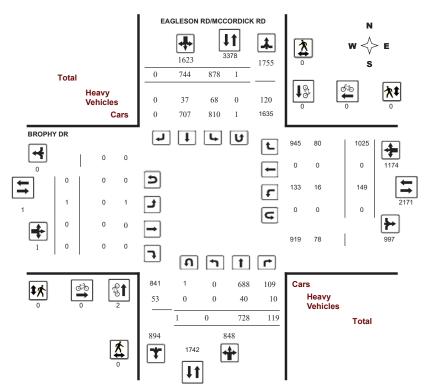
Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

Full Study Diagram





Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD @ OTTAWA ST

 Survey Date:
 Tuesday, March 07, 2023
 WO No:
 40864

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total EAGLESON RD OTTAWA ST

Time	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
T	otal	0	0	0	0	0

 September 9, 2024
 Page 1 of 11
 September 9, 2024
 Page 11 of 11



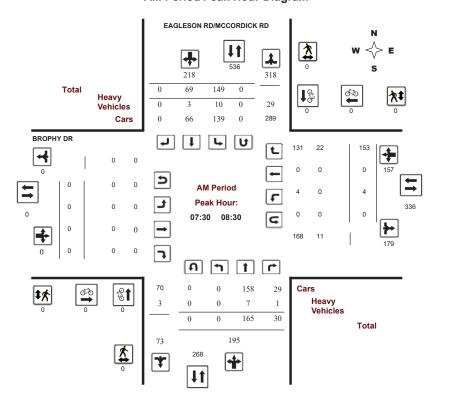
Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

AM Period Peak Hour Diagram





Transportation Services - Traffic Services

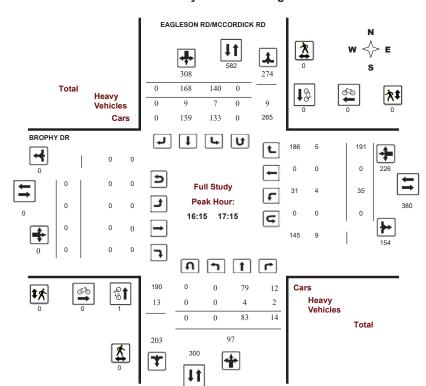
Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

Full Study Peak Hour Diagram



 September 9, 2024
 Page 3 of 11
 September 9, 2024
 Page 2 of 11



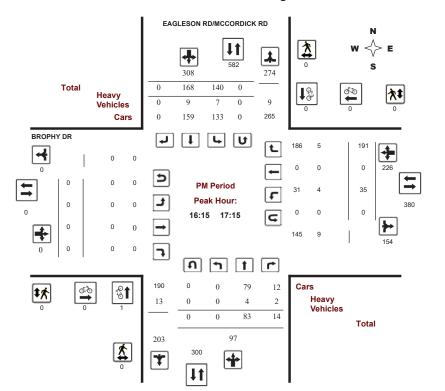
Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

PM Period Peak Hour Diagram





Transportation Services - Traffic Services

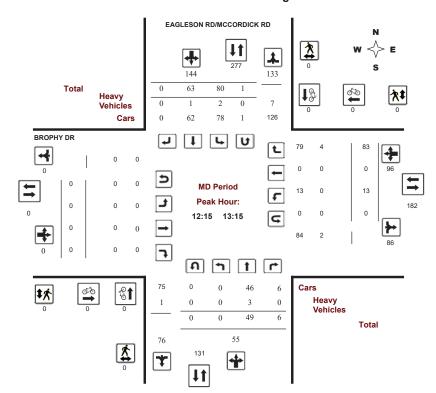
Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

MD Period Peak Hour Diagram



 September 9, 2024
 Page 5 of 11
 September 9, 2024
 Page 4 of 11



Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute Increments

EAGLESON RD/MCCORDICK RD BROPHY DR

	N	orthbou	und		Sc	uthbou	nd			E	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
15:00 15:15	0	25	2	27	24	23	0	47	74	1	0	0	1	5	0	29	34	35	109
15:15 15:30	0	21	2	23	29	35	0	64	87	0	0	0	0	8	0	32	40	40	127
15:30 15:45	0	28	8	36	28	32	0	60	96	0	0	0	0	13	0	33	46	46	142
15:45 16:00	0	27	6	33	31	38	0	69	102	0	0	0	0	7	0	34	41	41	143
16:00 16:15	0	15	4	19	37	34	0	71	90	0	0	0	0	15	0	31	46	46	136
16:15 16:30	0	22	5	27	37	47	0	84	111	0	0	0	0	7	0	56	63	63	174
16:30 16:45	0	21	3	24	35	49	0	84	108	0	0	0	0	14	0	54	68	68	176
16:45 17:00	0	16	2	18	38	30	0	68	86	0	0	0	0	6	0	37	43	43	129
17:00 17:15	0	24	4	28	30	42	0	72	100	0	0	0	0	8	0	44	52	52	152
17:15 17:30	0	18	5	23	32	38	0	70	93	0	0	0	0	5	0	58	63	63	156
17:30 17:45	0	16	2	18	26	41	0	67	85	0	0	0	0	11	0	44	55	55	140
17:45 18:00	0	17	4	22	29	34	0	63	85	0	0	0	0	4	0	45	49	49	134
13:15 13:30	0	15	1	16	22	17	0	39	55	0	0	0	0	3	0	19	22	22	77
11:30 11:45	0	12	2	14	26	13	0	39	53	0	0	0	0	1	0	23	24	24	77
07:00 07:15	0	32	5	37	26	16	0	42	79	0	0	0	0	1	0	39	40	40	119
07:15 07:30	0	49	4	53	28	10	0	38	91	0	0	0	0	0	0	27	27	27	118
07:30 07:45	0	45	13	58	50	15	0	65	123	0	0	0	0	2	0	42	44	44	167
07:45 08:00	0	43	6	49	37	17	0	54	103	0	0	0	0	2	0	44	46	46	149
08:00 08:15	0	33	5	38	30	15	0	45	83	0	0	0	0	0	0	27	27	27	110
08:15 08:30	0	44	6	50	32	22	0	54	104	0	0	0	0	0	0	40	40	40	144
08:30 08:45	0	31	2	33	30	23	0	53	86	0	0	0	0	1	0	36	37	37	123
08:45 09:00	0	22	4	26	18	11	0	29	55	0	0	0	0	1	0	33	34	34	89
09:00 09:15	0	32	3	35	26	10	0	36	71	0	0	0	0	1	0	17	18	18	89
09:15 09:30	0	29	5	34	17	16	0	33	67	0	0	0	0	7	0	20	27	27	94
09:30 09:45	0	16	4	20	20	10	0	30	50	0	0	0	0	4	0	19	23	23	73
09:45 10:00	0	8	2	10	24	18	0	42	52	0	0	0	0	3	0	26	29	29	81
12:00 12:15	0	7	2	9	15	11	0	26	35	0	0	0	0	3	0	18	21	21	56
12:15 12:30	0	10	1	11	21	17	0	38	49	0	0	0	0	4	0	25	29	29	78
12:30 12:45	0	10	2	12	22	12	0	34	46	0	0	0	0	4	0	18	22	22	68
12:45 13:00	0	14	2	16	21	13	0	35	51	0	0	0	0	2	0	21	23	23	74
13:00 13:15	0	15	1	16	16	21	0	37	53	0	0	0	0	3	0	19	22	22	75
11:45 12:00	0	11	2	13	21	14	0	35	48	0	0	0	0	4	0	15	19	19	67
Total:	0	728	119	848	878	744	0	1623	2471	1	0	0	1	149	0	1025	1174	1175	3,646
																		•	

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, April 30, 2019 Total Observed U-Turns AADT Factor

Northbound: 1 Southbound: .90 Westbound: () EAGLESON RD/MCCORDICK RD **BROPHY DR** Northbound Southbound Eastbound Westbound SB STR STR Grand ST RT LT ST RT LT ST RT Period TOT TOT TOT TOT Total 152 07:00 08:00 58 08:00 09:00 97 337 09:00 10:00 11:30 12:30 12:30 13:30 15:00 16:00 128 162 16:00 17:00 17:00 18:00 272 362 191 219 219 581 119 847 744 1622 1174 1175 3644 Sub Total 878 1025 U Turns 0 0 Total 848 1 149 0 1025 1175 3646 728 119 878 744 1623 2471 1174 EQ 12Hr 2256 0 1425 1633 5068 1.39 Note: These values are calculated by multiplying the totals by the appropriate expansion factor 0 1282 1469 1470 4561 Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor .90 244 0 1679 1924 1926 5975

1.31

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

 September 9, 2024
 Page 7 of 11
 September 9, 2024
 Page 6 of 11



Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

Survey Date: Tuesday, April 30, 2019 WO No: 38627 Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

EAGLESON RD/MCCORDICK RD BROPHY DR

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

Survey Date: Tuesday, April 30, 2019 WO No: 38627 Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

	•	•	
EAGLESON RD/MCCORDICK RD			BROPHY DR

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	1	0	1	0	0	0	1
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	1	0	1	0	0	0	1
17:45 18:00	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
Total	2	0	2	0	0	0	2

September 9, 2024 Page 9 of 11 September 9, 2024 Page 8 of 11



Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total EAGLESON RD/MCCORDICK RD BROPHY DR

Time I	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	1	0	0	0	1
13:15	13:30	0	0	0	0	0
11:30	11:45	0	0	0	0	0
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	1	0	0	1
13:00	13:15	0	0	0	0	0
11:45	12:00	0	0	0	0	0



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EAGLESON RD/MCCORDICK RD @ BROPHY DR

 Survey Date:
 Tuesday, April 30, 2019
 WO No:
 38627

 Start Time:
 07:00
 Device:
 Miovision

Full Study Heavy Vehicles

								iluu	у п	avy	vei								
	EA	GLE	SON R	D/MC	COR	DICK	RD					BR	ОРНҮ	DR					
	N	orthbo	ound		Sc	outhbou	ınd	_		Е	astbou	nd	_	W	estbou	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
15:00 15:15	0	2	1	3	2	1	0	3	6	0	0	0	0	2	0	3	5	5	11
15:15 15:30	0	4	0	4	5	0	0	5	9	0	0	0	0	2	0	3	5	5	14
15:30 15:45	0	3	2	5	4	1	0	5	10	0	0	0	0	1	0	2	3	3	13
15:45 16:00	0	1	0	1	4	0	0	4	5	0	0	0	0	0	0	1	1	1	6
16:00 16:15	0	0	0	0	9	2	0	11	11	0	0	0	0	3	0	0	3	3	14
16:15 16:30	0	2	0	2	1	2	0	3	5	0	0	0	0	1	0	1	2	2	7
16:30 16:45	0	1	1	2	4	2	0	6	8	0	0	0	0	3	0	2	5	5	13
16:45 17:00	0	1	0	1	1	0	0	1	2	0	0	0	0	0	0	2	2	2	4
17:00 17:15	0	0	1	1	1	5	0	6	7	0	0	0	0	0	0	0	0	0	7
17:15 17:30	0	0	2	2	1	4	0	5	7	0	0	0	0	1	0	1	2	2	9
17:30 17:45	0	0	0	0	1	1	0	2	2	0	0	0	0	0	0	2	2	2	4
17:45 18:00	0	1	0	1	1	0	0	1	2	0	0	0	0	0	0	5	5	5	7
13:15 13:30	0	3	1	4	2	1	0	3	7	0	0	0	0	0	0	3	3	3	10
11:30 11:45	0	0	0	0	0	2	0	2	2	0	0	0	0	0	0	2	2	2	4
07:00 07:15	0	1	1	2	0	2	0	2	4	0	0	0	0	0	0	5	5	5	9
07:15 07:30	0	3	0	3	4	0	0	4	7	0	0	0	0	0	0	7	7	7	14
07:30 07:45	0	2	0	2	4	2	0	6	8	0	0	0	0	0	0	10	10	10	18
07:45 08:00	0	2	0	2	2	1	0	3	5	0	0	0	0	0	0	3	3	3	8
08:00 08:15	0	2	0	2	3	0	0	3	5	0	0	0	0	0	0	4	4	4	9
08:15 08:30	0	1	1	2	1	0	0	1	3	0	0	0	0	0	0	5	5	5	8
08:30 08:45	0	1	0	1	2	1	0	3	4	0	0	0	0	0	0	3	3	3	7
08:45 09:00	0	1	0	1	1	0	0	1	2	0	0	0	0	0	0	3	3	3	5
09:00 09:15	0	3	0	3	2	2	0	4	7	0	0	0	0	0	0	3	3	3	10
09:15 09:30	0	1	0	1	3	3	0	6	7	0	0	0	0	2	0	2	4	4	11
09:30 09:45	0	1	0	1	3	2	0	5	6	0	0	0	0	1	0	1	2	2	8
09:45 10:00	0	0	0	0	2	1	0	3	3	0	0	0	0	0	0	1	1	1	4
12:00 12:15	0	0	0	0	2	1	0	3	3	0	0	0	0	0	0	1	1	1	4
12:15 12:30	0	2	0	2	1	0	0	1	3	0	0	0	0	0	0	0	0	0	3
12:30 12:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1
12:45 13:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1
13:00 13:15	0	1	0	1	1	1	0	2	3	0	0	0	0	0	0	2	2	2	5
11:45 12:00	0	1	0	1	1	0	0	1	2	0	0	0	0	0	0	1	1	1	3
Total: None	0	40	10	50	68	37	0	105	155	0	0	0	0	16	0	80	96	96	251

 September 9, 2024
 Page 11 of 11
 September 9, 2024
 Page 10 of 11



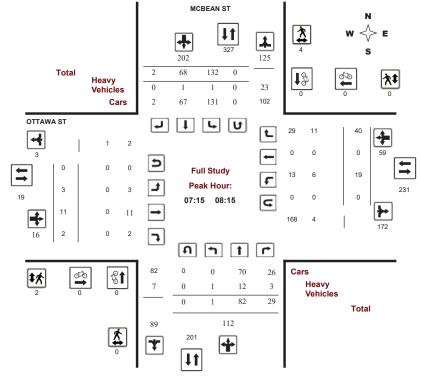
Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

Full Study Peak Hour Diagram



- MAR 12 2020 - 8HRS - KERRY-LYNN MOHR



Transportation Services - Traffic Services

Turning Movement Count - Study Results

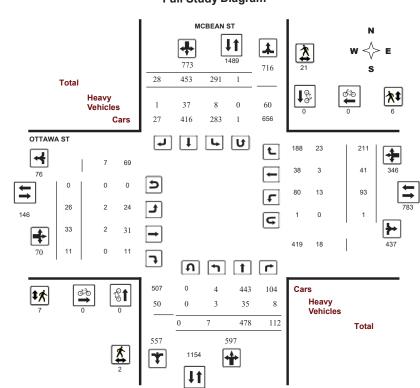
MCBEAN ST @ OTTAWA ST

Survey Date: Thursday, March 12, 2020

WO No: Device: 39618 Miovision

Start Time: 07:00

Full Study Diagram



- MAR 12 2020 - 8HRS - KERRY-LYNN MOHR

 September 9, 2024
 Page 2 of 11
 September 9, 2024
 Page 1 of 11



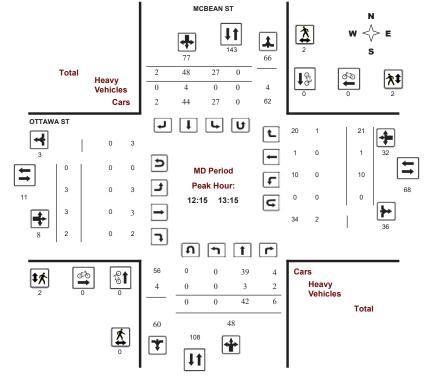
Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

MD Period Peak Hour Diagram



- MAR 12 2020 - 8HRS - KERRY-LYNN MOHR



Transportation Services - Traffic Services

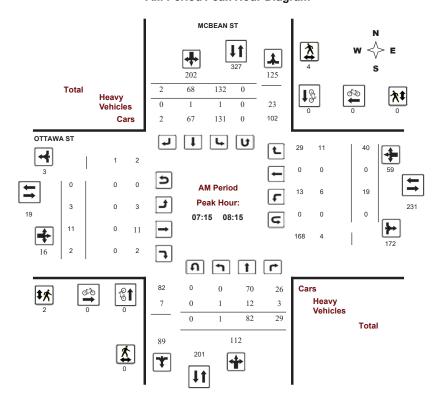
Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

AM Period Peak Hour Diagram



- MAR 12 2020 - 8HRS - KERRY-LYNN MOHR

 September 9, 2024
 Page 4 of 11
 September 9, 2024
 Page 3 of 11



Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, March 12, 2020 Total Observed U-Turns

Northbound: 0 Southbound: 1 1.00

	-		-	
Eastbound:	0	Westbound:	1	

								Eastbour	nd: ()		West	bound:	1						
			MC	BEAN	ST							TO	TAWA	ST					
	No	rthbou	nd		So	uthbou	ınd			E	astbou	nd		W	estbou	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	1	69	31	101	118	53	1	172	273	4	12	2	18	14	0	41	55	73	346
08:00 09:00	1	68	22	91	42	44	4	90	181	3	5	2	10	19	5	13	37	47	228
09:00 10:00	1	65	9	75	27	57	0	84	159	0	4	1	5	7	3	15	25	30	189
11:30 12:30	0	32	15	47	20	40	5	65	112	3	2	1	6	9	3	22	34	40	152
12:30 13:30	0	37	8	45	28	44	1	73	118	3	2	2	7	12	1	21	34	41	159
15:00 16:00	1	70	6	77	26	81	12	119	196	4	1	0	5	11	7	33	51	56	252
16:00 17:00	2	79	15	96	22	74	2	98	194	3	4	1	8	14	18	28	60	68	262
17:00 18:00	1	58	6	65	8	60	3	71	136	6	3	2	11	7	4	38	49	60	196
Sub Total	7	478	112	597	291	453	28	772	1369	26	33	11	70	93	41	211	345	415	1784
U Turns				0				1	1				0				1	1	2
Total	7	478	112	597	291	453	28	773	1370	26	33	11	70	93	41	211	346	416	1786
EQ 12Hr	10	664	156	830	404	630	39	1074	1904	36	46	15	97	129	57	293	481	578	2483
Note: These v	/alues a	re calcu	lated by	y multipl	ying the	totals b	y the a	ppropriate	e expans	ion fact	or.			1.39					
AVG 12Hr	10	664	156	830	404	825	51	1074	1904	36	46	15	97	129	57	293	481	578	2483
Note: These v	olumes/	are cald	culated	by multi	plying tl	ne Equiv	alent 1	2 hr. tota	ls by the	AADT f	actor.			1.00					
AVG 24Hr	13	870	204	1087	529	1081	67	1407	2494	47	60	20	127	169	75	384	630	757	3253

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor. 1.31

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



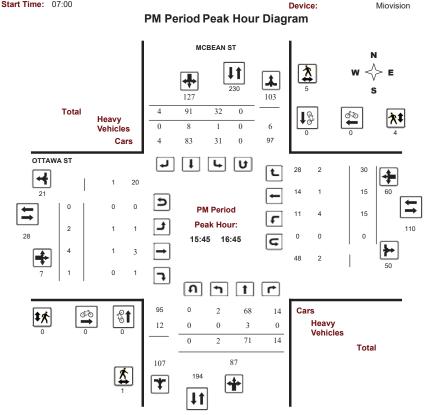
Transportation Services - Traffic Services

Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision



- MAR 12 2020 - 8HRS - KERRY-LYNN MOHR

 September 9, 2024
 Page 6 of 11
 September 9, 2024
 Page 5 of 11



Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

Full Study Cyclist Volume

MCBEAN ST	OTTAWA ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
7:30 07:45	0	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
9:15 09:30	0	0	0	0	0	0	0
9:45 10:00	0	0	0	0	0	0	0
5:30 15:45	0	0	0	0	0	0	0
1:45 12:00	0	0	0	0	0	0	0
2:30 12:45	0	0	0	0	0	0	0
2:45 13:00	0	0	0	0	0	0	0
5:00 15:15	0	0	0	0	0	0	0
5:45 16:00	0	0	0	0	0	0	0
6:00 16:15	0	0	0	0	0	0	0
6:30 16:45	0	0	0	0	0	0	0
6:45 17:00	0	0	0	0	0	0	0
7:00 17:15	0	0	0	0	0	0	0
7:15 17:30	0	0	0	0	0	0	0
8:15 08:30	0	0	0	0	0	0	0
8:45 09:00	0	0	0	0	0	0	0
3:00 13:15	0	0	0	0	0	0	0
9:00 09:15	0	0	0	0	0	0	0
9:30 09:45	0	0	0	0	0	0	0
1:30 11:45	0	0	0	0	0	0	0
6:15 16:30	0	0	0	0	0	0	0
2:00 12:15	0	0	0	0	0	0	0
2:15 12:30	0	0	0	0	0	0	0
3:15 13:30	0	0	0	0	0	0	0
7:30 17:45	0	0	0	0	0	0	0
5:15 15:30	0	0	0	0	0	0	0
Γotal	0	0	0	0	0	0	0



Transportation Services - Traffic Services

Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute Increments

MCBEAN ST OTTAWA ST

		N	orthbo	und		Sc	uthbou	nd			Е	astbour	nd		We	estbour	nd			
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	9	6	15	4	2	0	6	21	1	3	0	4	2	0	4	6	10	31
07:15	07:30	1	17	6	24	22	15	0	37	61	1	4	1	6	2	0	3	5	11	72
07:30	07:45	0	20	7	27	37	13	0	50	77	1	1	1	3	6	0	23	29	32	109
17:45	18:00	0	12	2	14	2	15	1	18	32	1	0	0	1	1	1	13	15	16	48
07:45	08:00	0	23	12	35	55	23	1	79	114	1	4	0	5	4	0	11	15	20	134
08:00	08:15	0	22	4	26	18	17	1	36	62	0	2	0	2	7	0	3	10	12	74
08:30	08:45	0	11	8	19	5	8	1	14	33	0	1	1	2	4	2	1	7	9	42
09:15	09:30	1	17	3	21	11	22	0	33	54	0	1	0	1	2	0	3	6	7	61
09:45	10:00	0	14	3	17	2	10	0	12	29	0	0	0	0	1	1	6	8	8	37
15:30	15:45	0	23	1	24	5	16	4	25	49	0	0	0	0	1	1	8	10	10	59
11:45	12:00	0	6	5	11	7	6	1	14	25	1	1	0	2	4	2	4	10	12	37
12:30	12:45	0	11	0	11	4	10	0	14	25	0	1	0	1	2	0	2	4	5	30
12:45	13:00	0	7	5	12	12	11	0	23	35	1	1	0	2	3	1	9	13	15	50
15:00	15:15	1	15	1	17	4	14	3	21	38	3	0	0	3	1	1	8	10	13	51
15:45	16:00	0	14	2	16	10	35	4	49	65	0	0	0	0	4	3	9	16	16	81
16:00	16:15	0	22	7	29	14	18	0	32	61	0	3	0	3	3	3	9	15	18	79
16:30	16:45	1	22	2	25	3	15	0	18	43	1	1	0	2	5	3	6	14	16	59
16:45	17:00	0	22	3	25	0	18	2	20	45	1	0	0	1	3	6	7	16	17	62
17:00	17:15	0	20	1	21	3	19	2	24	45	0	2	0	2	3	0	7	10	12	57
17:15	17:30	0	15	2	17	1	11	0	12	29	2	0	0	2	2	2	10	14	16	45
08:15	08:30	0	15	8	23	12	10	2	24	47	2	1	0	3	5	2	3	10	13	60
08:45	09:00	1	20	2	23	7	9	0	17	40	1	1	1	3	3	1	6	10	13	53
13:00	13:15	0	11	0	11	5	15	1	21	32	2	0	2	4	3	0	7	10	14	46
09:00	09:15	0	23	2	25	8	12	0	20	45	0	1	1	2	2	2	5	9	11	56
09:30	09:45	0	11	1	12	6	13	0	19	31	0	2	0	2	2	0	1	3	5	36
11:30	11:45	0	6	7	13	3	9	1	13	26	0	0	1	1	3	0	8	11	12	38
16:15	16:30	1	13	3	17	5	23	0	28	45	1	0	1	2	3	6	6	15	17	62
12:00	12:15	0	7	2	9	4	13	2	19	28	2	0	0	2	0	1	7	8	10	38
12:15	12:30	0	13	1	14	6	12	1	19	33	0	1	0	1	2	0	3	5	6	39
13:15	13:30	0	8	3	11	7	8	0	15	26	0	0	0	0	4	0	3	7	7	33
17:30	17:45	1	11	1	13	2	15	0	17	30	3	1	2	6	1	1	8	10	16	46
15:15	15:30	0	18	2	20	7	16	1	24	44	1	1	0	2	5	2	8	15	17	61
Total:		7	478	112	597	291	453	28	773	1370	26	33	11	70	93	41	211	346	416	1,786

Note: U-Turns are included in Totals.

- MAR 12 2020 - 8HRS - KERRY-LYNN MOHR

 September 9, 2024
 Page 8 of 11
 September 9, 2024
 Page 7 of 11



Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

Full Study Heavy Vehicles

MCBEAN ST OTTAWA ST

	No	orthbo	und		Sc	outhbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR	Grand Total
07:00 07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2	2	2
07:15 07:30	1	2	2	5	1	0	0	1	6	0	0	0	0	0	0	0	0	0	6
07:30 07:45	0	8	0	8	0	0	0	0	8	0	0	0	0	3	0	9	12	12	20
17:45 18:00	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
07:45 08:00	0	1	0	1	0	0	0	0	1	0	0	0	0	3	0	2	5	5	6
08:00 08:15	0	1	1	2	0	1	0	1	3	0	0	0	0	0	0	0	0	0	3
08:30 08:45	0	1	0	1	0	1	1	2	3	0	0	0	0	0	1	0	1	1	4
09:15 09:30	1	0	0	1	1	7	0	8	9	0	1	0	1	0	0	0	0	1	10
09:45 10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:30 15:45	0	2	0	2	0	2	0	2	4	0	0	0	0	0	0	0	0	0	4
11:45 12:00	0	0	0	0	1	0	0	1	1	0	0	0	0	0	0	0	0	0	1
12:30 12:45	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
12:45 13:00	0	1	2	3	0	1	0	1	4	0	0	0	0	0	0	0	0	0	4
15:00 15:15	0	2	0	2	0	1	0	1	3	1	0	0	1	0	0	1	1	2	5
15:45 16:00	0	1	0	1	0	1	0	1	2	0	0	0	0	1	0	0	1	1	3
16:00 16:15	0	1	0	1	1	2	0	3	4	0	1	0	1	3	0	0	3	4	8
16:30 16:45	0	1	0	1	0	2	0	2	3	0	0	0	0	0	0	1	1	1	4
16:45 17:00	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
17:00 17:15	0	0	0	0	0	2	0	2	2	0	0	0	0	0	0	0	0	0	2
17:15 17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 08:30	0	1	2	3	1	1	0	2	5	0	0	0	0	2	1	1	4	4	9
08:45 09:00	1	1	0	2	0	1	0	1	3	0	0	0	0	0	0	0	0	0	3
13:00 13:15	0	0	0	0	0	2	0	2	2	0	0	0	0	0	0	1	1	1	3
09:00 09:15	0	3	0	3	1	4	0	5	8	0	0	0	0	0	0	3	3	3	11
09:30 09:45	0	0	0	0	1	2	0	3	3	0	0	0	0	0	0	0	0	0	3
11:30 11:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1
16:15 16:30	0	0	0	0	0	3	0	3	3	1	0	0	1	0	1	1	2	3	6
12:00 12:15	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
12:15 12:30	0	2	0	2	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
13:15 13:30	0	2	1	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
17:30 17:45	0	1	0	1	0	0	0	0	1	0	0	0	0	0	0	1	1	1	2
15:15 15:30	0	4	0	4	1	0	0	1	5	0	0	0	0	0	0	1	1	1	6
Total: None	3	35	8	46	8	37	1	46	92	2	2	0	4	13	3	23	39	43	135



Transportation Services - Traffic Services

Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

Full Study Pedestrian Volume

MCBEAN ST OTTAWA ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	1	1	0	0	0	1
07:15 07:30	0	0	0	2	0	2	2
07:30 07:45	0	2	2	0	0	0	2
17:45 18:00	0	0	0	0	0	0	0
07:45 08:00	0	2	2	0	0	0	2
08:00 08:15	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:30 12:45	0	2	2	2	0	2	4
12:45 13:00	0	0	0	0	0	0	0
15:00 15:15	0	1	1	0	0	0	1
15:45 16:00	1	2	3	0	0	0	3
16:00 16:15	0	0	0	0	1	1	1
16:30 16:45	0	2	2	0	2	2	4
16:45 17:00	0	1	1	1	0	1	2
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	1	0	1	1
08:15 08:30	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	2	2	2
09:00 09:15	0	0	0	0	0	0	0
09:30 09:45	0	2	2	0	0	0	2
11:30 11:45	1	0	1	0	0	0	1
16:15 16:30	0	1	1	0	1	1	2
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
13:15 13:30	0	3	3	0	0	0	3
17:30 17:45	0	0	0	0	0	0	0
15:15 15:30	0	2	2	1	0	1	3
Total	2	21	23	7	6	13	36

- MAR 12 2020 - 8HRS - KERRY-LYNN MOHR

 September 9, 2024
 Page 10 of 11
 September 9, 2024
 Page 9 of 11



Turning Movement Count - Study Results

MCBEAN ST @ OTTAWA ST

 Survey Date:
 Thursday, March 12, 2020
 WO No:
 39618

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total MCBEAN ST OTTAWA ST

Time	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:30	08:45	0	0	0	0	0
09:15	09:30	0	0	0	1	1
09:45	10:00	0	0	0	0	0
15:30	15:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:45	09:00	0	1	0	0	1
13:00	13:15	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:30	09:45	0	0	0	0	0
11:30	11:45	0	0	0	0	0
16:15	16:30	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
13:15	13:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
15:15	15:30	0	0	0	0	0
To	otal	0	1	0	1	2

September 9, 2024 Page 11 of 11

Appendix C

Synchro Intersection Worksheets – Existing Conditions



10-02-2024

HCM 2010 TWSC

1: Eagleson & Ottawa

Intersection						
Intersection Delay, s/veh	9.3					
	9.3 A					
Intersection LOS	А					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1			4
Traffic Vol, veh/h	4	153	165	30	149	69
Future Vol. veh/h	4	153	165	30	149	69
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	4	170	183	33	166	77
Number of Lanes	1	0	103	0	0	1
	-	U		J	-	'
Approach	WB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	NB				WB	
Conflicting Lanes Left	1		0		1	
Conflicting Approach Right	SB		WB			
Conflicting Lanes Right	1		1		0	
HCM Control Delay	8.6		9.2		9.9	
HCM LOS	Α		A		Α.	
HOW LOO	А		А		Λ.	
	Α				٨	
Lane	A	NBLn1	WBLn1	SBLn1	Α	
Lane Vol Left, %	A	0%	WBLn1	68%	^	
Lane Vol Left, % Vol Thru, %		0% 85%	WBLn1 3% 0%	68% 32%	^	
Lane Vol Left, % Vol Thru, % Vol Right, %		0%	WBLn1	68%		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	Α	0% 85%	WBLn1 3% 0%	68% 32%		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	A	0% 85% 15%	WBLn1 3% 0% 97%	68% 32% 0%		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane	7	0% 85% 15% Stop	WBLn1 3% 0% 97% Stop	68% 32% 0% Stop		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol	7	0% 85% 15% Stop 195	WBLn1 3% 0% 97% Stop 157	68% 32% 0% Stop 218		
Lane Vol Left, % Vol Tryn, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 85% 15% Stop 195	WBLn1 3% 0% 97% Stop 157 4	68% 32% 0% Stop 218 149		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		0% 85% 15% Stop 195 0	WBLn1 3% 0% 97% Stop 157 4 0	68% 32% 0% Stop 218 149 69	^	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		0% 85% 15% Stop 195 0 165 30	WBLn1 3% 0% 97% Stop 157 4 0 153	68% 32% 0% Stop 218 149 69	^	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		0% 85% 15% Stop 195 0 165 30 217	WBLn1 3% 0% 97% Stop 157 4 0 153 174	68% 32% 0% Stop 218 149 69 0 242		
Lane Vol Left, % Vol Tipt, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 85% 15% Stop 195 0 165 30 217	WBLn1 3% 0% 97% Stop 157 4 0 153 174	68% 32% 0% Stop 218 149 69 0 242		
Lane Vol Left, % Vol Tipht, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 85% 15% Stop 195 0 165 30 217 1	WBLn1 3% 0% 97% Stop 157 4 0 153 174 1 0.213	68% 32% 0% Stop 218 149 69 0 242 1 0.317		
Lane Vol Left, % Vol Tinru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0% 85% 15% Stop 195 0 165 30 217 1 0.272 4.517 Yes	WBLn1 3% 0% 97% Stop 157 4 0 153 174 1 0.213 4.402 Yes	68% 32% 0% Stop 218 149 69 0 242 1 0.317 4.706 Yes		
Lane Vol Left, % Vol Try, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0% 85% 15% Stop 195 0 165 30 217 1 0.272 4.517 Yes 793	WBLn1 3% 0% 97% Stop 157 4 0 153 174 1 0.213 4.402 Yes 813	68% 32% 0% Stop 218 149 69 0 242 1 0.317 4.706 Yes 763		
Lane Vol Left, % Vol Tipt, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 85% 15% Stop 195 0 165 30 217 1 0.272 4.517 Yes 793 2.554	WBLn1 3% 0% 97% Stop 157 4 0 153 174 1 0.213 4.402 Yes 813 2.441	68% 32% 0% Stop 218 149 69 0 242 1 0.317 4.706 Yes 763 2.743		
Lane Vol Left, % Vol Tipht, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 85% 15% Stop 195 0 165 30 217 1 0.272 4.517 Yes 793 2.554 0.274	WBLn1 3% 0% 97% Stop 157 4 0 153 174 1 0.213 4.402 Yes 813 2.441 0.214	68% 32% 0% Stop 218 149 69 0 242 1 0.317 4.706 Yes 763 2.743		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		0% 85% 15% Stop 195 0 165 30 217 1 0.272 4.517 Yes 793 2.554 0.274 9.2	WBLn1 3% 0% 97% Stop 157 4 0 153 174 1 0.213 4.402 Yes 813 2.441 0.214 8.6	68% 32% 0% Stop 218 149 69 0 242 1 0.317 4.706 Yes 763 2.743 0.317 9.9		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay HCM 45th-tile O HCM 95th-tile O		0% 85% 15% Stop 195 0 165 30 217 1 0.272 4.517 Yes 793 2.554 0.274	WBLn1 3% 0% 97% Stop 157 4 0 153 174 1 0.213 4.402 Yes 813 2.441 0.214	68% 32% 0% Stop 218 149 69 0 242 1 0.317 4.706 Yes 763 2.743		

Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ર્ન	ß	
Traffic Vol, veh/h	107	48	55	165	114	82
Future Vol, veh/h	107	48	55	165	114	82
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	119	53	61	183	127	91
Major/Minor	Minor2		Major1	N	Major2	
Conflicting Flow All	478	173	218	0	-	0
Stage 1	173	-	-	-	-	-
Stage 2	305	-		-		-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-		-
Critical Hdwy Stg 2	5.42	-		_		_
Follow-up Hdwy	3.518	3.318	2.218	-		-
Pot Cap-1 Maneuver	546	871	1352	_		_
Stage 1	857	- 071	1002			
Stage 2	748					- :
Platoon blocked, %	740				- 1	
	519	074	1252	-	-	-
Mov Cap-1 Maneuver		871	1352		-	-
Mov Cap-2 Maneuver	519	-	-	-	-	-
Stage 1	814	-	-	-	-	-
Stage 2	748	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	13.5		1.9		0	
HCM LOS	В					
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR
	IL				ועט	SDIX
Capacity (veh/h)		1352	-	593 0.29	-	-
HCM Cantral Dalay (a)		0.045	-		-	-
HCM Control Delay (s)		7.8	0	13.5	-	-
HCM Lane LOS	١	A	Α	В	-	-
HCM 95th %tile Q(veh)	0.1	-	1.2	-	-

HCM 2010 TWSC 3: McBean & Ottawa

Intersection						
Int Delay, s/veh	2.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	TOL.	EDR	INDL	IND I	3B1 ♣	SDR
Traffic Vol., veh/h	'T' 56	30	37	심 151	191	76
Future Vol. veh/h	56	30	37	151	191	76
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-		-		-
Veh in Median Storage	-	-		0	0	-
Grade. %	0		-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	62	33	41	168	212	84
WWITETIOW	02	00	71	100	212	07
	Minor2		Major1		Major2	
Conflicting Flow All	504	254	296	0	-	0
Stage 1	254	-	-	-	-	-
Stage 2	250	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318		-	-	-
Pot Cap-1 Maneuver	528	785	1265	-	-	-
Stage 1	788	-	-	-	-	-
Stage 2	792	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	509	785	1265	-	-	-
Mov Cap-2 Maneuver	509	-	-	-	-	-
Stage 1	760	-	-	-	-	-
Stage 2	792	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	12.4		1.6		0.0	
HCM LOS	12.4 B		1.0		U	
I IOW LOG	٥					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1265	-	580	-	-
HCM Lane V/C Ratio		0.032	-	0.165	-	-
HCM Control Delay (s)		7.9	0	12.4	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q(veh)	0.1	-	0.6	-	-
,						

Intersection												
Int Delay, s/veh	4.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	11	2	19	0	40	1	82	29	132	68	2
Future Vol. veh/h	3	11	2	19	0	40	1	82	29	132	68	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e.# -	0	_	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	3	12	2	21	0	44	1	91	32	147	76	2
								- 01	- 02			
Major/Minor	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	502	496	77	487	481	107	78	0	0	123	0	0
Stage 1	371	371	-	109	109	-	-	-	-	-	-	-
Stage 2	131	125		378	372		-		-		-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518		3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	480	475	984	491	485	947	1520	-	-	1464	-	-
Stage 1	649	620	-	896	805	-	-	-	-	-	-	
Stage 2	873	792	-	644	619	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	420	425	984	440	434	947	1520	-	-	1464	-	-
Mov Cap-2 Maneuver	420	425	-	440	434	-	-	-	-	-	-	-
Stage 1	648	555	-	895	804	-	-	-	-	-	-	-
Stage 2	831	791	-	562	554	-	-	-	-	-	-	-
Ů.												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.2			10.8			0.1			5.1		
HCM LOS	В			В								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1520	-	-	456	691	1464	-	-			
HCM Lane V/C Ratio		0.001	-	-	0.039	0.095	0.1	-	-			
HCM Control Delay (s)		7.4	0	-	13.2	10.8	7.7	0	-			
HCM Lane LOS		Α	Α	-	В	В	Α	Α	-			
HCM 95th %tile Q(veh)	0	-	-	0.1	0.3	0.3	-	-			
	,											

Intersection
Int Delay, s/veh 3.3
Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
Lane Configurations 💠 🗘
Traffic Vol, veh/h 2 4 1 15 15 30 2 71 14 32 91 4
Future Vol, veh/h 2 4 1 15 15 30 2 71 14 32 91 4
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0
Sign Control Stop Stop Stop Stop Stop Stop Free Free Free Free Free Free
RT Channelized None None None
Storage Length
Veh in Median Storage, # - 0 0 0 -
Grade, % - 0 0 0 -
Peak Hour Factor 90 90 90 90 90 90 90 90 90 90 90 90
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2
Mvmt Flow 2 4 1 17 17 33 2 79 16 36 101 4
Major/Minor Minor2 Minor1 Major1 Major2
Conflicting Flow All 291 274 103 269 268 87 105 0 0 95 0 0
Stage 1 175 175 - 91 91

Major/Minor	Minor2			Minor1			Major1		N	/lajor2			
Conflicting Flow All	291	274	103	269	268	87	105	0	0	95	0	0	
Stage 1	175	175	-	91	91	-	-	-	-	-	-	-	
Stage 2	116	99	-	178	177	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	661	633	952	684	638	971	1486	-	-	1499	-	-	
Stage 1	827	754	-	916	820	-	-	-	-	-	-	-	
Stage 2	889	813	-	824	753	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	613	616	952	666	621	971	1486	-	-	1499	-	-	
Mov Cap-2 Maneuver	613	616	-	666	621	-	-	-	-	-	-	-	
Stage 1	826	734	-	915	819	-	-	-	-	-	-	-	
Stage 2	840	812	-	797	733	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	10.6			10.1			0.2			1.9			
HCM LOS	В			В									

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1486	-	-	648	773	1499	-	-	
HCM Lane V/C Ratio	0.001	-	-	0.012	0.086	0.024	-	-	
HCM Control Delay (s)	7.4	0	-	10.6	10.1	7.5	0	-	
HCM Lane LOS	Α	Α	-	В	В	Α	Α	-	
HCM 95th %tile Q(veh)	0	-	-	0	0.3	0.1	-	-	

Intersection						
Intersection Delay, s/veh	10.5					
Intersection LOS	В					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1>			4
Traffic Vol, veh/h	35	191	83	14	140	168
Future Vol, veh/h	35	191	83	14	140	168
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	212	92	16	156	187
Number of Lanes	1	0	1	0	0	1
Approach	WB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	NB				WB	
Conflicting Lanes Left	1		0		1	
Conflicting Approach Right	SB		WB			
Conflicting Lanes Right	1		1		0	
HCM Control Delay	9.7		8.7		11.6	
HCM LOS	Α		Α		В	
Lane		NBLn1	WBLn1	SBLn1		
Vol Left, %		0%	15%	45%		
Vol Thru, %		86%	0%	55%		
Vol Right, %		14%	85%	0%		
Sign Control		Stop	Stop	Stop		
Traffic Vol by Lane		97	226	308		
LT Vol		0	35	140		
Through Vol		83	0	168		
RT Vol		14	191	0		
Lane Flow Rate		108	251	342		
Geometry Grp		1	1	1		
Degree of Util (X)		0.145	0.316	0.45		
Departure Headway (Hd)		4.841	4.527	4.737		
Convergence, Y/N		Yes	Yes	Yes		
Сар		735	791	756		
Service Time		2.905	2.571	2.789		
HOME VIO D-E-		0 4 47	0.047	0.450		

0.147 0.317 0.452

9.7 11.6

8.7

0.5 1.4 2.3

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS

HCM 95th-tile Q

Appendix D

Collision Data



Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Traffic Control Condition	Classification Of Accident	Initial Impact Type	Road Surface Condition	# Vehicles	# Motorcycles	# Bicycles	# Pedestrians
2018-08-08	2018	13:01	MCBEAN ST @ OTTAWA ST (0000133)	01 - Clear	01 - Daylight	02 - Stop sign	0	02 - Non-fatal injury	02 - Angle	01 - Dry	0	1	0	0
2021-11-24	2021	7:44	KING ST @ OTTAWA ST (0000369)	01 - Clear	01 - Daylight	02 - Stop sign	0	03 - P.D. only	02 - Angle	01 - Dry	0	0	0	0
2018-09-15	2018	14:19	EAGLESON RD/MCCORDICK RD @ BROPHY DR (0000160)	01 - Clear	01 - Daylight	02 - Stop sign	0	02 - Non-fatal injury	07 - SMV other	01 - Dry	0	1	0	0
2018-09-29	2018	20:30	EAGLESON RD/MCCORDICK RD @ BROPHY DR (0000160)	01 - Clear	07 - Dark	02 - Stop sign	0	03 - P.D. only	07 - SMV other	01 - Dry	0	0	0	0
2020-07-09	2020	9:40	EAGLESON RD btwn BROPHY DR & OTTAWA ST (3ZA4ZN)	01 - Clear	01 - Daylight	10 - No control	0	02 - Non-fatal injury	03 - Rear end	01 - Dry	0	0	0	0
2021-07-30	2021	20:32	EAGLESON RD btwn BROPHY DR & OTTAWA ST (3ZA4ZN)	01 - Clear	05 - Dusk	10 - No control	0	02 - Non-fatal injury	07 - SMV other	01 - Dry	0	0	0	0
2022-01-06	2022	17:45	EAGLESON RD btwn BROPHY DR & OTTAWA ST (3ZA4ZN)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	07 - SMV other	01 - Dry	0	0	0	0

Appendix E

MMLOS Analysis



Multi-Modal Level of Service - Segments Form

Consultant
Scenario
Comments

GH Transportation Inc.	Project
kisting/Future	Date

2023-080	
2024-12-06	

SEGMENTS		Street A	Eagleson	Ottawa	
	Sidewalk Width		Rd no sidewalk	St no sidewalk	
	Sidewaik Width Boulevard Width		no sidewaik n/a	no sidewaik n/a	
	Avg Daily Curb Lane Traffic Volume		> 3000	≤ 3000	
⊑	Operating Speed		> 60 km/h	> 50 to 60 km/h	
Pedestrian	On-Street Parking		no	no	
est	Exposure to Traffic PLoS		F	F	
pa	Effective Sidewalk Width				
ď	Pedestrian Volume				
	Crowding PLoS		-	-	
	Level of Service		-	-	
	Type of Cycling Facility		Mixed Traffic	Mixed Traffic	
	Number of Travel Lanes		2-3 lanes total	2-3 lanes total	
	Operating Speed		≥ 60 km/h	≥ 50 to 60 km/h	
	# of Lanes & Operating Speed LoS		F	E	
Bicycle	Bike Lane (+ Parking Lane) Width				
Š	Bike Lane Width LoS		-	-	
Ξ	Bike Lane Blockages				
	Blockage LoS		-	-	
	Median Refuge Width (no median = < 1.8 m)		< 1.8 m refuge	< 1.8 m refuge	
	No. of Lanes at Unsignalized Crossing Sidestreet Operating Speed		≤ 3 lanes ≤ 40 km/h	≤ 3 lanes ≤ 40 km/h	
	Unsignalized Crossing - Lowest LoS		A	≥ 40 KIII/II	
	Level of Service		F	E	
	Level of Service		'	-	
#	Facility Type				
Transit	Friction or Ratio Transit:Posted Speed				
Tra	Level of Service		-	-	
	Truck Lane Width		> 3.7 m	> 3.7 m	
2	Travel Lanes per Direction		1	1	
Truck	Level of Service		В	В	
Auto	Level of Service	Not Applicable			

Appendix F

TDM Checklist



TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

	Legend
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance
*	The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions	
	1.	TDM PROGRAM MANAGEMENT		
	1.1	Program coordinator		
BASIC *	1.1.1	Designate an internal coordinator, or contract with an external coordinator		
	1.2	Travel surveys		
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress		
	2.	WALKING AND CYCLING		
	2.1	Information on walking/cycling routes & destinations		
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)		
	2.2	Bicycle skills training		
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses		

	TDM	measures: Residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	
BETTER	3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
	3.2	Transit fare incentives	
BASIC ★	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER	3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
	3.3	Enhanced public transit service	
BETTER ★	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	
	3.4	Private transit service	
BETTER	3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
	4.	CARSHARING & BIKESHARING	
	4.1	Bikeshare stations & memberships	
BETTER	4.1.1	Contract with provider to install on-site bikeshare station (multi-family)	
BETTER	4.1.2	Provide residents with bikeshare memberships, either free or subsidized (multi-family)	
	4.2	Carshare vehicles & memberships	
BETTER	4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	
BETTER	4.2.2	Provide residents with carshare memberships, either free or subsidized	
	5.	PARKING	
	5.1	Priced parking	
BASIC *	5.1.1	Unbundle parking cost from purchase price (condominium)	
BASIC ★	5.1.2	Unbundle parking cost from monthly rent (multi-family)	

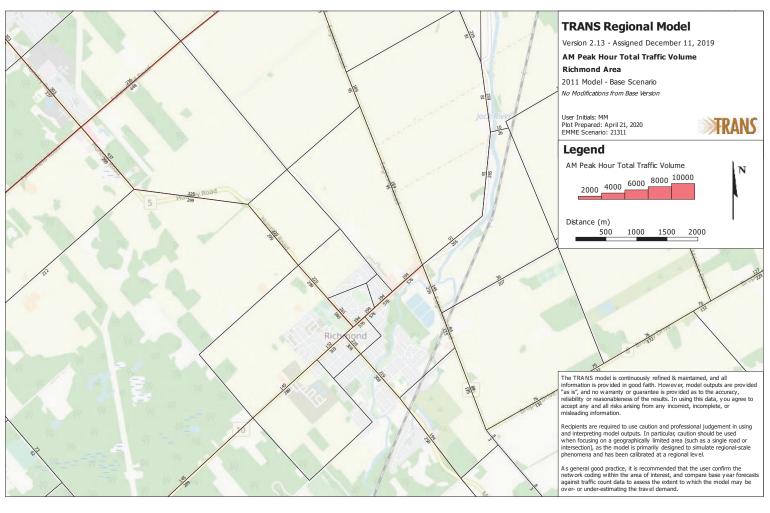
12

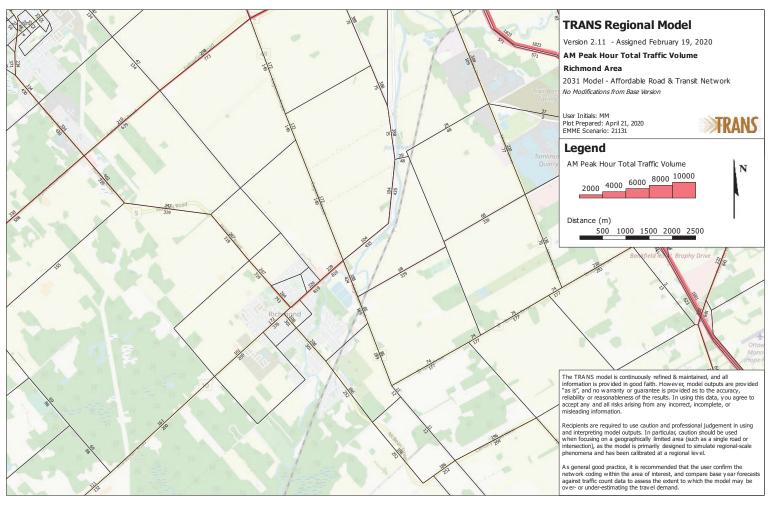
TDM measures: Residential developments		Check if proposed & add descriptions
6.	TDM MARKETING & COMMUNICATIONS	
6.1	Multimodal travel information	
BASIC ★ 6.1.1	Provide a multimodal travel option information package to new residents	
6.2	Personalized trip planning	
BETTER ★ 6.2.1	Offer personalized trip planning to new residents	

Appendix G

TRANS Model Plots



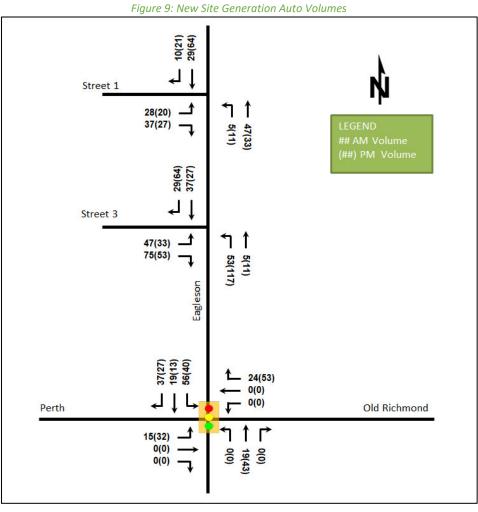




Appendix H

Background Development Volumes





Exemption Review

Table 13 summarizes the exemptions for this TIA.



Appendix I

Synchro Intersection Worksheets – 2032 Future Background Conditions



HCM 95th-tile Q

Intersection						
Intersection	9.7					
Intersection Delay, s/veh Intersection LOS	9.7 A					
intersection LOS	А					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1≽			ની
Traffic Vol, veh/h	4	172	171	30	203	88
Future Vol, veh/h	4	172	171	30	203	88
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	172	171	30	203	88
Number of Lanes	1	0	1	0	0	1
Annuagah	WB		NB		SB	
Approach	WD					
Opposing Approach	0		SB		NB 1	
Opposing Lanes	0		1			
Conflicting Approach Left	NB		•		WB	
Conflicting Lanes Left	1		0		1	
Conflicting Approach Right	SB		WB			
Conflicting Lanes Right	1		1		0	
HCM Control Delay	8.8		9.2		10.6	
HCM LOS	Α		Α		В	
Lane		NBLn1	WBLn1	SBLn1		
Vol Left, %		0%	2%	70%		
Vol Thru, %		85%	0%	30%		
Vol Right, %		15%	98%	0%		
Sign Control		Stop	Stop	Stop		
Traffic Vol by Lane		201	176	291		
LT Vol		0	4	203		
Through Vol		171	0	88		
RT Vol		30	172	0		
Lane Flow Rate		201	176	291		
Geometry Grp		1	1	1		
Degree of Util (X)		0.256	0.219	0.38		
Departure Headway (Hd)		4.584	4.481	4.702		
Convergence, Y/N		Yes	Yes	Yes		
Сар		782	799	762		
Service Time		2.628	2.522	2.744		
HCM Lane V/C Ratio		0.257	0.22	0.382		
HCM Control Delay		9.2	8.8	10.6		
HCM Lane LOS		Α	Α	В		

Intersection						
Int Delay, s/veh	3.9					
		EDD	ND	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	₩	40		4	- ↑	00
Traffic Vol, veh/h	107	48	55	190	164	82
Future Vol, veh/h	107	48	55	190	164	82
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	107	48	55	190	164	82
Major/Minor	Minor2		Major1	N	//ajor2	
Conflicting Flow All	505	205	246	0	-	0
Stage 1	205	200	240	-		-
Stage 2	300					
Critical Hdwy	6.42	6.22	4.12			
Critical Hdwy Stg 1	5.42	0.22	7.12			
Critical Hdwy Stg 2	5.42					
Follow-up Hdwy	3.518	3.318				
Pot Cap-1 Maneuver	527	836	1320			
Stage 1	829	000	1320		- 0	- 1
Stage 2	752	-		-		_
	152	-	-			
Platoon blocked, %	500	000	4200	-	-	-
Mov Cap-1 Maneuver	502 502	836	1320		-	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	790	-	-	-	-	-
Stage 2	752	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	13.6		1.8		0	
HCM LOS	В				-	
Minor Long/Mais - Mar		ND	NDT	EDI m4	CDT	CDD
Minor Lane/Major Mvr	III	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1320	-	573	-	-
HCM Lane V/C Ratio		0.042		0.271	-	-
HCM Control Delay (s)	7.8	0	13.6	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q(veh	1)	0.1	-	1.1	-	-

0.8 1.8

HCM LOS

Minor Lane/Major Mvmt

Capacity (veh/h)

HCM Lane LOS

HCM Lane V/C Ratio

HCM Control Delay (s)

HCM 95th %tile Q(veh)

10-03-2024

Intersection						
Int Delay, s/veh	2.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ની	ĥ	
Traffic Vol, veh/h	56	30	37	236	211	76
Future Vol, veh/h	56	30	37	236	211	76
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	56	30	37	236	211	76
Major/Minor	Minor2		Major1	N	Major2	
Conflicting Flow All	559	249				
Stage 1		243	287	0	-	0
Olaye I	249	249	287	0	-	0
Stage 2	249 310					
Stage 2		-	-	-	-	-
	310	-	-	-	-	-
Stage 2 Critical Hdwy	310 6.42	6.22	-	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1	310 6.42 5.42 5.42	6.22	4.12	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2	310 6.42 5.42 5.42	6.22	4.12	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy	310 6.42 5.42 5.42 3.518	6.22	- 4.12 - - 2.218	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver	310 6.42 5.42 5.42 3.518 490	6.22 - - 3.318 790	- 4.12 - - 2.218	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1	310 6.42 5.42 5.42 3.518 490 792	6.22 - 3.318 790	4.12 - 2.218 1275	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2	310 6.42 5.42 5.42 3.518 490 792	6.22 - 3.318 790	4.12 - 2.218 1275	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, %	310 6.42 5.42 5.42 3.518 490 792 744	6.22 - - 3.318 790	4.12 - 2.218 1275		-	
Stage 2 Critical Hdwy Stg 1 Critical Hdwy Stg 2 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver	310 6.42 5.42 5.42 3.518 490 792 744	6.22 - 3.318 790 - 790	4.12 - 2.218 1275 - 1275		-	
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver	310 6.42 5.42 5.42 3.518 490 792 744 474 474	6.22 - 3.318 790 - 790	4.12 - 2.218 1275 - 1275		-	
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Mov Cap-2 Maneuver Stage 1	310 6.42 5.42 5.42 3.518 490 792 744 474 474 766	6.22 - 3.318 790 - - 790	4.12 - 2.218 1275 - 1275	-	-	-
Stage 2 Critical Hdwy Stg 1 Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2	310 6.42 5.42 5.42 3.518 490 792 744 474 474 766 744	6.22 - 3.318 790 - - 790	4.12 - 2.218 1275 - - 1275	-	-	-
Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Mov Cap-2 Maneuver Stage 1	310 6.42 5.42 5.42 3.518 490 792 744 474 474 766	6.22 - 3.318 790 - - 790	4.12 - 2.218 1275 - 1275	-	-	-

NBL NBT EBLn1 SBT SBR - 551

7.9 0 12.7 - -

0.1 - 0.6 - -

A A B - -

- 0.156

1275

0.029

Intersection												
Int Delay, s/veh	4.7											
	EDI	EDT	EDD	WDI	MOT	WIDD	NDI	NDT	NDD	CDI	ODT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	11	2	19	0	40	1	82	29	132	68	2
Future Vol, veh/h	3	11	2	19	0	40	1	82	29	132	68	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	11	2	19	0	40	1	82	29	132	68	2
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	452	446	69	439	433	97	70	0	0	111	0	0
Stage 1	333	333	-	99	99	-		-	-		-	-
Stage 2	119	113		340	334							
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12		
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	4.12			7.12		
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52				_			
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218			2.218		
Pot Cap-1 Maneuver	518	507	994	528	516	959	1531			1479		
Stage 1	681	644	- 004	907	813	-	-			- 1410		
Stage 2	885	802	_	675	643		_		_		_	
Platoon blocked, %	000	002		010	010							
Mov Cap-1 Maneuver	461	459	994	480	467	959	1531			1479		
Mov Cap-1 Maneuver	461	459	334	480	467	333	1001			1413	- 1	
Stage 1	680	584		906	812			_				
Stage 2	847	801	- 1	599	583			- 1				
Olago Z	0+1	001		555	505							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.5			10.4			0.1			5		
HCM LOS	В			В								
Minor Lane/Major Mvr	nt	NBL	NBT	NBR	EBLn1\	NBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1531	-	-	493	726	1479	-	-			
HCM Lane V/C Ratio		0.001	-	-	0.032	0.081	0.089	-	-			
HCM Control Delay (s)	7.4	0	-	12.5	10.4	7.7	0	-			
HCM Lane LOS		Α	Α	-	В	В	Α	A	-			
HCM 95th %tile Q(veh	1)	0	-	-	0.1	0.3	0.3	_	-			
	,											

Intersection												
Int Delay, s/veh	3.3											
•				14/01		14/00		LIDT		0.01	007	000
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			- 43→			ቆ	
Traffic Vol, veh/h	2	4	1	15	15	30	2	71	14	32	91	4
Future Vol, veh/h	2	4	1	15	15	30	2	71	14	32	91	4
Conflicting Peds, #/hi	r 0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storag	ge,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	4	1	15	15	30	2	71	14	32	91	4
Major/Minor	Minor2			Minor1		N	/lajor1		ı	Major2		
Conflicting Flow All	262	246	93	242	241	78	95	0	0	85	0	0
Stage 1	157	157	-	82	82	-	-	-	-	-	-	-
3												

Major/Minor	Minor2			Minor1		- 1	Major1		N	lajor2			
Conflicting Flow All	262	246	93	242	241	78	95	0	0	85	0	0	
Stage 1	157	157	-	82	82	-	-	-	-	-	-	-	
Stage 2	105	89	-	160	159	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	691	656	964	712	660	983	1499	-	-	1512	-	-	
Stage 1	845	768	-	926	827	-	-	-	-	-	-	-	
Stage 2	901	821	-	842	766	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	647	641	964	696	645	983	1499	-	-	1512	-	-	
Mov Cap-2 Maneuver	647	641	-	696	645	-	-	-	-	-	-	-	
Stage 1	844	751	-	925	826	-	-	-	-	-	-	-	
Stage 2	857	820	-	818	749	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	10.4			9.9			0.2			1.9			
HCM LOS	В			A									

Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1V	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1499	-	-	675	797	1512	-	-	
HCM Lane V/C Ratio	0.001	-	-	0.01	0.075	0.021	-	-	
HCM Control Delay (s)	7.4	0	-	10.4	9.9	7.4	0	-	
HCM Lane LOS	Α	Α	-	В	Α	Α	Α	-	
HCM 95th %tile Q(veh)	0	-	-	0	0.2	0.1	-	-	

Intersection	_				_	_
	10.9					
Intersection Delay, s/veh	10.9 B					
Intersection LOS	В					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		12			4
Traffic Vol, veh/h	35	278	106	14	153	174
Future Vol., veh/h	35	278	106	14	153	174
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	35	278	106	14	153	174
Number of Lanes	1	0	1	0	0	1
						•
Approach	WB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	NB				WB	
Conflicting Lanes Left	1		0		1	
Conflicting Approach Right	SB		WB			
Conflicting Lanes Right	1		1		0	
HCM Control Delay	10.5		9.1		11.9	
HCM LOS	В		Α		В	
Lane		NBLn1	WBLn1	SBLn1		
Vol Left, %		0%	11%	47%		
Vol Thru. %		88%	0%	53%		
Vol Right, %		12%	89%	0%		
Sign Control		Stop	Stop	Stop		
Traffic Vol by Lane		120	313	327		
LT Vol		0	35	153		
Through Vol		106	0	174		
RT Vol		14	278	0		
Lane Flow Rate		120	313	327		
		120	1	321		
Geometry Grp			0.392	0.445		
Degree of Util (X)		0.167 4.999	4.509	4.899		
Departure Headway (Hd)				4.099 Yes		
Convergence, Y/N		Yes 710	Yes 795	729		
Cap		3.082	2.563	2.969		
Service Time						
HCM Lane V/C Ratio		0.169	0.394	0.449		
HCM Control Delay		9.1	10.5	11.9		
HCM Lane LOS		A	В	В		
HCM 95th-tile Q		0.6	1.9	2.3		

Appendix J

Signalization Warrants



Eagleson Road @ Ottawa Street Future Background 2032

Justification #7

	Latter than	Minimum R	lequirement	Minimum R	equirement		Compliance			
Justification	Description	1 Lane	Highway	2 or Mo	re Lanes	Sect	ional	Entire %	Signal	
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	EIILII e 76		
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	323	67%	67%	No	
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	90	75%	07%	INO	
	A. Vehicle volumes, major street (average hour)	480	720	600	900	263	55%			
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	41	82%	55%	No	

Eagleson Road @ Ottawa Street Future Background 2037

		Minimum R	equirement	Minimum R	equirement				
Justification	Description	1 Lane	Highway	2 or More Lanes		Sectional		Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Entire %	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	332	69%	69%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	90	75%	09%	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	272	57%		
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	41	82%	57%	No

Notes

1. Refer to OTM Book 12, pg 92, Mar 2012

2. Lowest section percentage governs justification

3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors

4. T-intersection factor corrected, applies only to 18

Notes

1. Refer to OTM Book 12, pg 92, Mar 2012

2. Lowest section percentage govern subtification

3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors

4. T-intersection factor corrected, applies only to 18

Eagleson Road @ Ottawa Street Future Total 2032

Justification #7

1		Minimum R	lequirement	Minimum R	equirement		Compliance			
Justification	Description	1 Lane	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal	
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Entire %		
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	518	108%	100%	No	
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	120	100%	100%	NO	
	A. Vehicle volumes, major street (average hour)	480	720	600	900	438	91%			
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	52	103%	91%	No	

Eagleson Road @ Ottawa Street Future Total 2037

		Minimum R	lequirement	Minimum R	equirement					
Justification	Description	1 Lane I	Highway	2 or More Lanes		Sectional		Entire %	Signal	
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Entire %		
Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	527	110%	100%	No	
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	120	100%	100%	INU	
	A. Vehicle volumes, major street (average hour)	480	720	600	900	447	93%			
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	52	103%	93%	No	

Notes

1. Refer to OTM Book 12, pg 92, Mar 2012

2. Lowest section percentage governs justification

3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors

4. T-intersection factor corrected, applies only to 18

Notes

1. Refer to OTM Book 12, pg 92, Mar 2012

2. Lowest section percentage govern subtification

3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors

4. T-intersection factor corrected, applies only to 18

Eagleson Road @ Brophy Drive Future Total 2037

Justification #7

	·	Minimum R	lequirement	Minimum F	Requirement		Compliance		
Justification	Description	1 Lane I	Highway	2 or Mo	re Lanes	Sect	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	EIILII e 76	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	421	88%	88%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	216	180%	00%	NO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	277	58%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	10	20%	20%	No
lotes . Refer to OTM Book 12, pg 92, . Lowest section percentage go . Average hourly volumes estim . T-intersection factor correcte	Mar 2012 verns justification nated from peak hour volumes, AHV = PM/2 or (AM	+ PM) / 4, includii	ng amplifcation fa	ctors					

McBean Street @ Ottawa Street Future Total 2037

		Minimum R	Requirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane I	Highway	2 or Mo	re Lanes	Secti	onal	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	EIILII e 76	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	343	72%	46%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	55	46%	40%	INO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	288	60%		
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	22	45%	45%	No

- Notes

 1. Refer to OTM Book 12, pg 92, Mar 2012

 2. Lowest section percentage governs justification

 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplifcation factors

 4. T-intersection factor corrected, applies only to 18

Eagleson Road @ New Collector Future Total 2032

Justification #7

		Minimum R	lequirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	EIILII e 76	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	476	99%	99%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	188	157%	3370	140
	A. Vehicle volumes, major street (average hour)	480	720	600	900	351	73%		
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	97	195%	73%	No

Eagleson Road @ New Collector Future Total 2037

		Minimum R	lequirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane I	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	EIILII e 76	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	488	102%	102%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	188	157%	102%	INO
	A. Vehicle volumes, major street (average hour)	480	720	600	900	362	75%		
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	97	195%	75%	No

Notes

1. Refer to OTM Book 12, pg 92, Mar 2012

2. Lowest section percentage governs justification

3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors

4. T-intersection factor corrected, applies only to 18

Notes

1. Refer to OTM Book 12, pg 92, Mar 2012

2. Lowest section percentage governs justification

3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors

4. T-intersection factor corrected, applies only to 18

McBean Street @ New Collector Future Total 2032/Future Total 2037

		Minimum R	lequirement	Minimum R	equirement		Compliance		
Justification	Description	1 Lane	Highway	2 or Mo	re Lanes	Secti	ional	Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	EIILII e 76	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	244	51%	51%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	120	100%	31/6	140
	A. Vehicle volumes, major street (average hour)	480	720	600	900	164	34%		
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	5	9%	9%	No

Notes

1. Refer to OTM Book 12, pg 92, Mar 2012

2. Lowest section percentage governs justification

3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplifcation factors

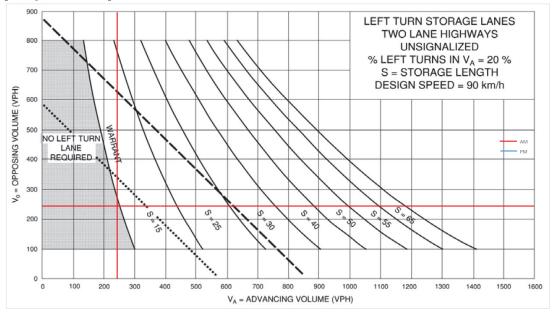
4. T-intersection factor corrected, applies only to 18

Appendix K

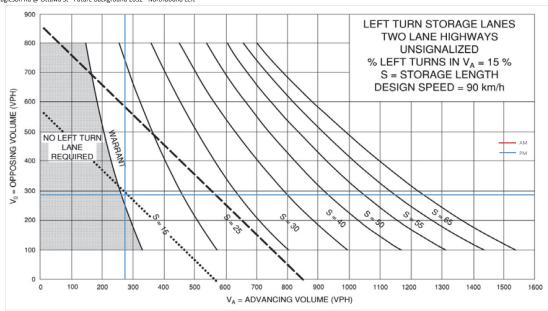
Left-Turn Lane Warrants



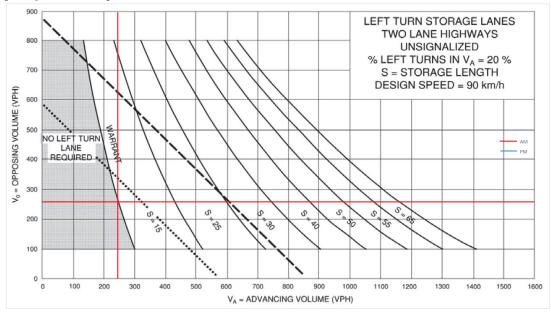
Eagleson Rd @ Ottawa St - Future Background 2032 - Northbound Left



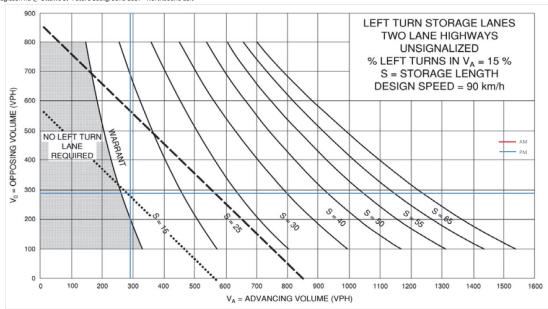
Eagleson Rd @ Ottawa St - Future Background 2032 - Northbound Left



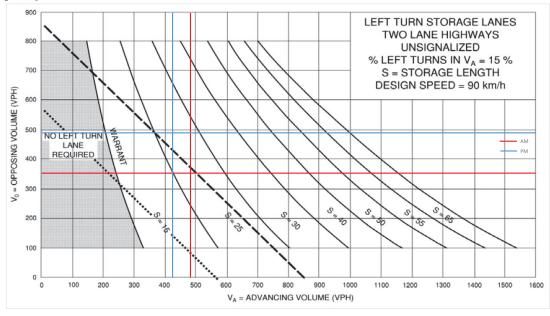
Eagleson Rd @ Ottawa St -Future Background 2037 - Northbound Left



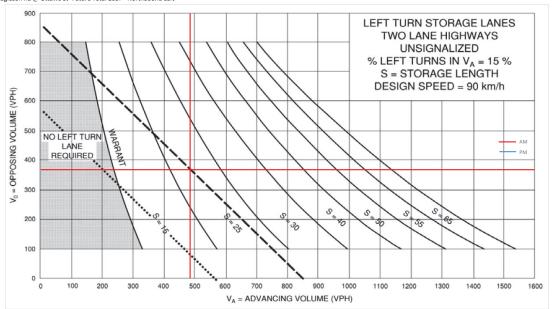




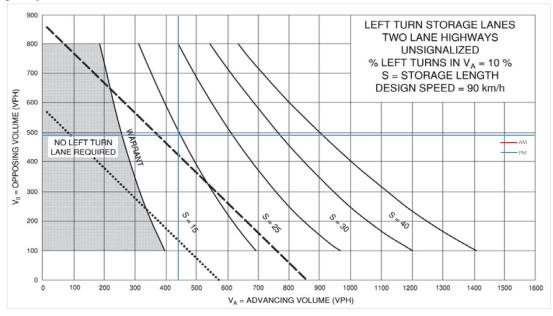
Eagleson Rd @ Ottawa St -Future Total 2032 - Northbound Left

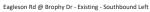


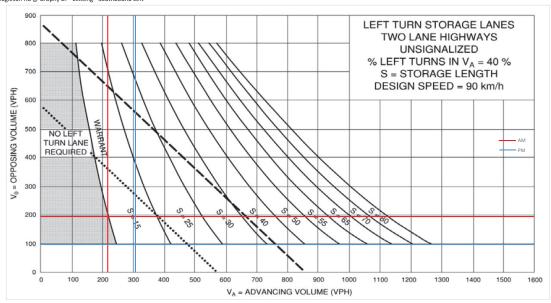




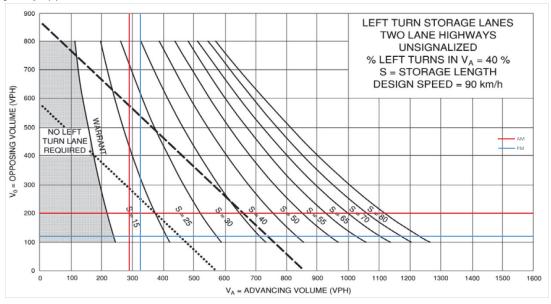
Eagleson Rd @ Ottawa St -Future Total 2037 - Northbound Left

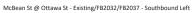


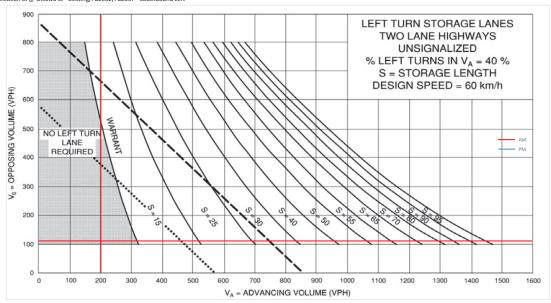




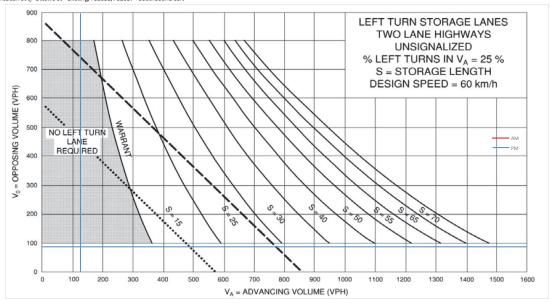
Eagleson Rd @ Brophy Dr - FB 2032 - Southbound Left

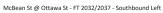


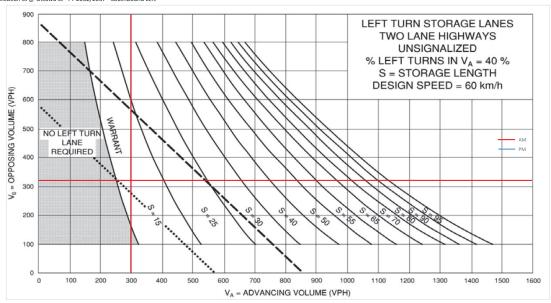


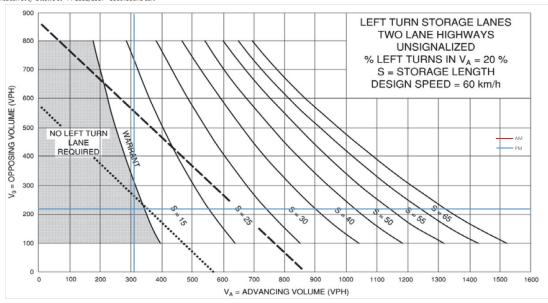


McBean St @ Ottawa St - Existing/FB2032/FB2037 - Southbound Left

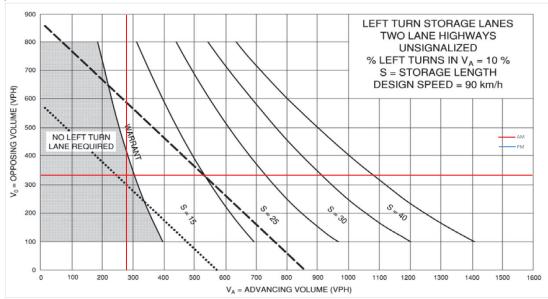




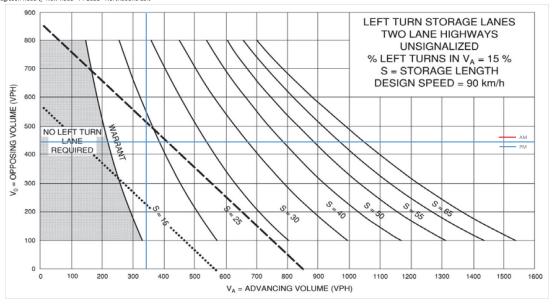




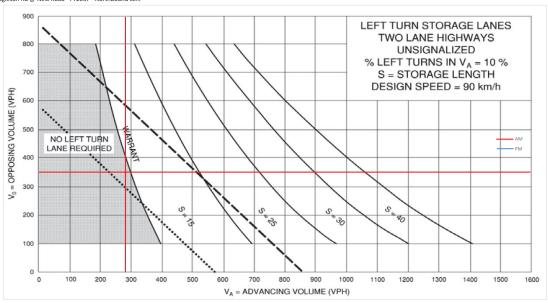




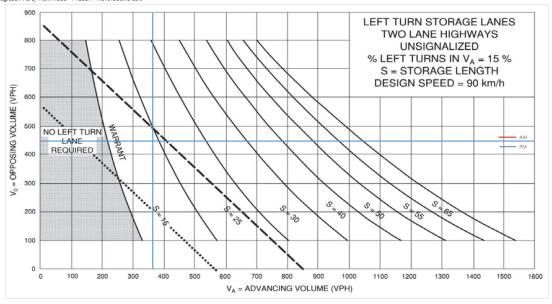
Eagleson Road @ New Road - FT 2032 - Northbound Left



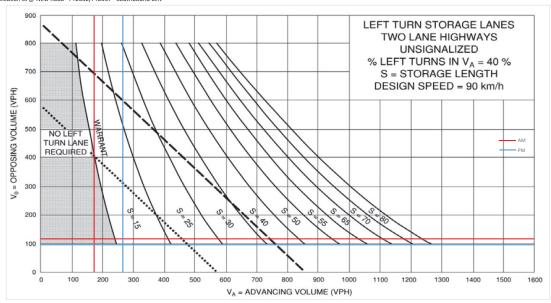
Eagleson Rd @ New Road - FT2037 - Northbound Left



Eagleson Rd @ New Road - FT2037 - Northbound Left







Appendix L

Synchro Intersection Worksheets – 2037 Future Background Conditions



HCM 2010 TWSC

1: Eagleson & Ottawa

Movement	Intersection						
Movement	Intersection Delay, s/veh	10					
Lane Configurations	Intersection LOS	Α					
Conflicting Approach Left							
Conflicting Approach Left	Movement	WBI	WBR	NRT	NBR	SBI	SBT
Traffic Vol, veh/h Future Vol, veh/h Future Vol, veh/h Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 Peak Hour Factor 1.00 1.00 1.00 1.00 Pepa Mmt Flow NB NB NB NB NB Opposing Aproach NB NB Opposing Aproach NB NB Conflicting Approach Left NB WB Conflicting Approach Right SB			*****		пын	052	
Future Vol, veh/h Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0			172		30	217	
Peak Hour Factor 1.00							
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2							
Mvmt Flow 4 172 173 30 217 96 Number of Lanes 1 0 1 0 0 1 Approach WB NB SB NB Opposing Approach SB NB NB Opposing Lanes 0 1 1 1 Conflicting Approach Left NB WB WB NB Conflicting Approach Right SB WB WB Conflicting Approach Right 1 0 1 Conflicting Approach Right 1 1 0 1 Conflicting Approach Right 8 WB WB Conflicting Approach Right 1 1 0 1 Conflicting Approach Right 1 1 0 1 Conflicting Approach Right 8 WB WB Description Approach Right 1 1 0 4 A B WB Lane Flow Rate 8 WB WB WB Lane Flow Rate 20% 69% WB WB WB Lane F							
Number of Lanes					_		
Approach							
Opposing Approach SB NB Opposing Lanes 0 1 1 Conflicting Approach Left NB WB WB Conflicting Lanes Left 1 0 1 Conflicting Lanes Left 1 0 1 Conflicting Lanes Right 1 1 0 HCM Control Delay 8.9 9.3 11 HCM Control Delay 8.9 9.3 11 HCM LOS A A A B Lane MBLnt WBLnt WB	Number of Lanes	1	0	1	0	U	1
Opposing Lanes 0 1 1 Conflicting Approach Left NB WB Conflicting Approach Left 1 0 1 Conflicting Lanes Left 1 0 1 Conflicting Approach Right SB WB Conflicting Lanes Right 1 1 0 HCM Control Delay 8.9 9.3 11 HCM LOS A A B Lane NBLn1 WBLn1 SBLn1 Vol Left, % 0% 2% 69% Vol Trun, % 85% 0% 31% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1	Approach	WB	_	NB		SB	_
Opposing Lanes 0 1 1 Conflicting Approach Left NB WB Conflicting Lanes Left 1 0 1 Conflicting Approach Right SB WB Conflicting Lanes Right 1 1 0 HCM Control Delay 8.9 9.3 11 HCM Control Delay 8.9 9.3 11 HCM LOS A A A B Lane NBLn1 WBLn1 SBLn1 Vol Left, % 0% 2% 69% Vol Trut, % 85% 0% 31% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 1 Through Vol 173 0 96 8 RT Vol 30 172 0 1 Lane Flow Rate 203 176 313 <td>Opposing Approach</td> <td></td> <td></td> <td>SB</td> <td></td> <td>NB</td> <td></td>	Opposing Approach			SB		NB	
Conflicting Approach Left NB WB Conflicting Lanes Left 1 0 1 Conflicting Approach Right SB WB Conflicting Approach Right 1 1 0 HCM Control Delay 8.9 9.3 11 HCM Control Delay 8.9 9.3 11 HCM LOS A A B Lane NBLn1 WBLn1 SBLn1 Vol Left, % 0% 2% 69% Vol Thru, % 85% 0% 31% Vol Right, % 15% 98% 0% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 0 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1		0					
Conflicting Lanes Left 1 0 1 Conflicting Approach Right SB WB Conflicting Lanes Right 1 1 0 HCM Control Delay 8.9 9.3 11 HCM LOS A A B Conflicting Lanes Right 1 1 0 HCM LOS A A A B Conflicting Lanes Right 1 1 0 HCM LOS A A A B Conflicting Lanes Right 1 1 0 0 Wol Thru,				'			
Conflicting Approach Right SB WB Conflicting Lanes Right 1 1 0 HCM Control Delay 8.9 9.3 11 HCM Cos A A A B Lane NBLn1 WBLn1 SBLn1 Vol Left, % 0% 2% 69% Vol Thru, % 85% 0% 31% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degrature Headway (Hd) 4,615 4,536 4,708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time<				0			
Conflicting Lanes Right 1 1 0 HCM Control Delay 8.9 9.3 11 HCM LOS A A B Lane NBLn1 WBLn1 SBLn1 Vol Left, % 0% 2% 69% Vol Trynu, % 85% 0% 31% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degrature Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Rati							
HCM Control Delay 8.9 9.3 11						Λ	
HCM LOS							
Lane							
Vol Left, % 0% 2% 69% Vol Thru, % 85% 0% 31% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Cantrol Delay 9.3 8.9 11 HCM Cantrol COS A A B	TIOW LOO	A		A		D	
Vol Left, % 0% 2% 69% Vol Thru, % 85% 0% 31% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Cantrol Delay 9.3 8.9 11 HCM Cantrol COS A A B							
Vol Thru, % 85% 0% 31% Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Cantel LOS A A B	Lane		NBLn1	WRI n1	SBLn1		
Vol Right, % 15% 98% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Cante LOS A A B							
Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Cante LOS A A B							
Sign Control Stop Stop Stop Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Cante LOS A A B	Vol Left, %		0%	2%	69%		
Traffic Vol by Lane 203 176 313 LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, %		0% 85%	2% 0%	69% 31%		
LT Vol 0 4 217 Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HHCM Control Delay 9.3 8.9 11 HCM Cantrol OS A B	Vol Left, % Vol Thru, % Vol Right, %		0% 85% 15%	2% 0% 98%	69% 31% 0%		
Through Vol 173 0 96 RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control		0% 85% 15% Stop	2% 0% 98% Stop	69% 31% 0% Stop		
RT Vol 30 172 0 Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		0% 85% 15% Stop 203	2% 0% 98% Stop 176	69% 31% 0% Stop 313		
Lane Flow Rate 203 176 313 Geometry Grp 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		0% 85% 15% Stop 203 0	2% 0% 98% Stop 176 4	69% 31% 0% Stop 313 217		
Geometry Grp 1 1 1 1 Degree of Util (X) 0.26 0.222 0.409 Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 85% 15% Stop 203 0 173	2% 0% 98% Stop 176 4	69% 31% 0% Stop 313 217 96		
Degree of Util (X)	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Trrough Vol RT Vol		0% 85% 15% Stop 203 0 173 30	2% 0% 98% Stop 176 4 0	69% 31% 0% Stop 313 217 96		
Departure Headway (Hd) 4.615 4.536 4.708 Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		0% 85% 15% Stop 203 0 173 30 203	2% 0% 98% Stop 176 4 0 172 176	69% 31% 0% Stop 313 217 96 0		
Convergence, Y/N Yes Yes Yes Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		0% 85% 15% Stop 203 0 173 30 203	2% 0% 98% Stop 176 4 0 172 176	69% 31% 0% Stop 313 217 96 0 313		
Cap 775 788 761 Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.441 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 85% 15% Stop 203 0 173 30 203 1	2% 0% 98% Stop 176 4 0 172 176 1	69% 31% 0% Stop 313 217 96 0 313 1 0.409		
Service Time 2.662 2.58 2.753 HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 85% 15% Stop 203 0 173 30 203 1 0.26 4.615	2% 0% 98% Stop 176 4 0 172 176 1 0.222 4.536	69% 31% 0% Stop 313 217 96 0 313 1 0.409 4.708		
HCM Lane V/C Ratio 0.262 0.223 0.411 HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0% 85% 15% Stop 203 0 173 30 203 1 0.26 4.615 Yes	2% 0% 98% Stop 176 4 0 172 176 1 0.222 4.536 Yes	69% 31% 0% Stop 313 217 96 0 313 1 0.409 4.708 Yes		
HCM Control Delay 9.3 8.9 11 HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0% 85% 15% Stop 203 0 173 30 203 1 0.26 4.615 Yes 775	2% 0% 98% Stop 176 4 0 172 176 1 0.222 4.536 Yes 788	69% 31% 0% Stop 313 217 96 0 313 1 0.409 4.708 Yes 761		
HCM Lane LOS A A B	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 85% 15% Stop 203 0 173 30 203 1 0.26 4.615 Yes 775 2.662	2% 0% 98% Stop 176 4 0 172 176 1 0.222 4.536 Yes 788 2.58	69% 31% 0% Stop 313 217 96 0 313 1 0.409 4.708 Yes 761 2.753		
	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 85% 15% Stop 203 0 173 30 203 1 0.26 4.615 Yes 775 2.662 0.262	2% 0% 98% Stop 176 4 0 172 176 1 0.222 4.536 Yes 788 2.58	69% 31% 0% Stop 313 217 96 0 313 1 0.409 4.708 Yes 761 2.753 0.411		
HCM 95th-tile Q 1 0.8 2	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		0% 85% 15% Stop 203 0 173 30 203 1 0.26 4.615 Yes 775 2.662 0.262 9.3	2% 0% 98% Stop 176 4 0 172 176 1 0.222 4.536 Yes 788 2.58 0.223 8.9	69% 31% 0% Stop 313 217 96 0 313 1 0.409 4.708 Yes 761 2.753 0.411		
	Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Lane LOS		0% 85% 15% Stop 203 0 173 30 203 1 0.26 4.615 Yes 775 2.662 0.262 9.3	2% 0% 98% Stop 176 4 0 172 176 1 0.222 4.536 Yes 788 2.58 0.223 8.9	69% 31% 0% Stop 313 217 96 0 313 1 0.409 4.708 Yes 761 2.753 0.411 B		

ane Configurations raffic Vol, veh/h 107 48 55 192 178 82 100 100 48 55 192 178 82 100 100 100 100 100 100 100 100 100 10							
Movement EBL EBR NBL NBT SBT SBR	lata						
Approach		2 0					
Anniestation	**						
Traffic Vol, veh/h Traffic Veh Veh/h Traffic Vol, veh/h Traffic Veh Veh/h Traffic Veh Veh/h Traffic Veh Veh/h Traffic Veh Veh/h Traffic Ve	Movement		EBR	NBL			SBR
Tuture Vol, veh/h Tuture Vol,							
Conflicting Peds, #/hr							
Sign Control Stop Stop Free None							
None None None None None None None		-	_	-	-	-	•
Storage Length							
Veh in Median Storage, # 0				-	None	-	None
Grade, %		-					
Neak Hour Factor			-	-	_	_	-
Reavy Vehicles, % 2 2 2 2 2 2 2 2 2	Grade, %	•					
Major/Minor	Peak Hour Factor						
Major/Minor Minor2 Major1 Major2	Heavy Vehicles, %						
Stage 1	Mvmt Flow	107	48	55	192	178	82
Stage 1							
Stage 1 219 -	Major/Minor	Minor2		Major1	1	Major2	
Stage 1 219 -							0
Stage 2 302		219			-	-	-
Critical Hdwy 6.42 6.22 4.12					-		-
Critical Hdwy Stg 1 5.42			6.22	4.12	-	-	-
Critical Hdwy Stg 2					-		-
Follow-up Hdwy 3.518 3.318 2.218		5.42	-	-	-	-	-
Stage 1		3.518	3.318	2.218	-		-
Stage 1		516	821	1304	-	-	-
Stage 2 750 - - - -				-	-		-
Platoon blocked, %			_	-	-		-
Mov Cap-1 Maneuver					-		-
Mov Cap-2 Maneuver		492	821	1304	_		-
Stage 1 779 - Stage 2 750 Approach EB NB SB HCM Control Delay, s 13.8 1.8 0 HCM LOS B Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) 1304 - 562 HCM Lane V/C Ratio 0.042 - 0.276 HCM Control Delay (s) 7.9 0 13.8 HCM Lane LOS A A B							
Stage 2 750 -				_		_	_
Approach							
Amort Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) 1304 - 562 CM Control Delay (s) 7.9 0 13.8 CM Lane LOS A A B	Olugo 2	700					
Amort Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) 1304 - 562 CM Control Delay (s) 7.9 0 13.8 CM Lane LOS A A B	Anneach	ED		ND		CD	
Alinor Lane/Major Mvmt							
### April				1.8		0	
Capacity (veh/h) 1304 - 562 - 1CM Lane V/C Ratio 0.042 - 0.276 - - 1CM Control Delay (s) 7.9 0 13.8 - - 1CM Lane LOS A A B - -	HCM LOS	В					
Capacity (veh/h) 1304 - 562 - 1CM Lane V/C Ratio 0.042 - 0.276 - - 1CM Control Delay (s) 7.9 0 13.8 - - 1CM Lane LOS A A B - -							
ICM Lane V/C Ratio 0.042 - 0.276 ICM Control Delay (s) 7.9 0 13.8 ICM Lane LOS A A B	Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1	SBT	SBR
HCM Control Delay (s) 7.9 0 13.8 HCM Lane LOS A A B	Capacity (veh/h)		1304	-	562	-	-
HCM Lane LOS A A B	HCM Lane V/C Ratio		0.042	-	0.276	-	-
	HCM Control Delay (s)	7.9	0	13.8	-	-
HCM 95th %tile Q(veh) 0.1 - 1.1	HCM Lane LOS		Α	Α	В	-	-
	HCM 95th %tile Q(veh	1)	0.1	-	1.1	-	-

3: McBean & Ottawa 10-03-2024

HCM 2010 TWSC

Intersection						
Int Delay, s/veh	2.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ની	ĥ	
Traffic Vol, veh/h	56	30	37	253	213	76
Future Vol, veh/h	56	30	37	253	213	76
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	56	30	37	253	213	76

Major/Minor	Minor2		Major1	Ma	ajor2	
Conflicting Flow All	578	251	289	0	-	0
Stage 1	251	-	-	-	-	-
Stage 2	327	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	478	788	1273	-	-	-
Stage 1	791	-	-	-	-	-
Stage 2	731	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver		788	1273	-	-	-
Mov Cap-2 Maneuver	462	-	-	-	-	-
Stage 1	764	-	-	-	-	-
Stage 2	731	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s			1		0	
HCM LOS	В				•	
110111 200	D					

Minor Lane/Major Mvmt	NBL	NBT E	BLn1	SBT	SBR	
Capacity (veh/h)	1273	-	540	-	-	
HCM Lane V/C Ratio	0.029	- (0.159	-	-	
HCM Control Delay (s)	7.9	0	12.9	-	-	
HCM Lane LOS	Α	Α	В	-	-	
HCM 95th %tile Q(veh)	0.1	-	0.6	-	-	

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	11	2	19	0	40	1	82	29	132	68	2
Future Vol, veh/h	3	11	2	19	0	40	1	82	29	132	68	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	11	2	19	0	40	1	82	29	132	68	2
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	452	446	69	439	433	97	70	0	0	111	0	0
Stage 1	333	333	-	99	99	-	-	-	-	-	-	-
Stage 2	119	113	-	340	334	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	518	507	994	528	516	959	1531	-	-	1479	-	-
Stage 1	681	644	-	907	813	-	-	-	-	-	-	-
Ctogo 2	905	000		675	612							

Olage I	001	077	_	301	010	_	_	_	_	_	_	_	
Stage 2	885	802	-	675	643	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	461	459	994	480	467	959	1531	-	-	1479	-	-	
Mov Cap-2 Maneuver	461	459	-	480	467	-	-	-	-	-	-	-	
Stage 1	680	584	-	906	812	-	-	-	-	-	-	-	
Stage 2	847	801	-	599	583	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	12.5			10.4			0.1			5			
HCM LOS	В			В									
Minor Lane/Major Mymt		NIRI	NIRT	MRDE	RI n1W	RI n1	CRI	CRT	CRD				

Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1\	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1531	-	-	493	726	1479	-	-	
HCM Lane V/C Ratio	0.001	-	-	0.032	0.081	0.089	-	-	
HCM Control Delay (s)	7.4	0	-	12.5	10.4	7.7	0	-	
HCM Lane LOS	Α	Α	-	В	В	Α	Α	-	
HCM 95th %tile Q(veh)	0	-	-	0.1	0.3	0.3	-	-	

Intersection												
Int Delay, s/veh	3.3											
**												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	2	4	1	15	15	30	2	71	14	32	91	4
Future Vol, veh/h	2	4	1	15	15	30	2	71	14	32	91	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	4	1	15	15	30	2	71	14	32	91	4
Majar/Minor	Minaro			Minart			Majart			Majano		
	Minor2	0.40		Minor1	044		Major1			Major2		
Conflicting Flow All	262	246	93	242	241	78	95	0	0	85	0	0
Stage 1	157	157	-	82	82	-	-	-	-	-	-	-
Stage 2	105	89	- 0.00	160	159	- 0.00	- 4.40	-	-	- 4.40	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-		4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018		3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	691	656	964	712	660	983	1499	-	-	1512	-	-
Stage 1	845	768	-	926	827	-	-	-	-	-	-	-
Stage 2	901	821	-	842	766	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	647	641	964	696	645	983	1499	-	-	1512	-	-
Mov Cap-2 Maneuver	647	641	-	696	645	-	-	-	-	-	-	-
Stage 1	844	751	-	925	826	-	-	-	-	-	-	-
Stage 2	857	820	-	818	749	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.4			9.9			0.2			1.9		
HCM LOS	В			Α.			0.2					
200	٥			А								
			N.D.	LIBE			0.01	0.00	005			
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1\		SBL	SBT	SBR			
Capacity (veh/h)		1499	-	-	675	797	1512	-	-			
HCM Lane V/C Ratio		0.001	-	-	0.01	0.075	0.021	-	-			
HCM Control Delay (s)		7.4	0	-	10.4	9.9	7.4	0	-			
HCM Lane LOS		Α	Α	-	В	Α	Α	Α	-			
HCM 95th %tile Q(veh)	0	-	-	0	0.2	0.1	-	-			

Intersection						
Intersection Intersection Delay, s/veh	11.1					
Intersection LOS	11.1 B					
IIILEISECLIOII LOS	В					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.			ર્ન
Traffic Vol, veh/h	35	296	115	14	153	176
Future Vol, veh/h	35	296	115	14	153	176
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	35	296	115	14	153	176
Number of Lanes	1	0	1	0	0	1
Approach	WB		NB		SB	
	VVD		SB		NB	
Opposing Approach Opposing Lanes	0		5B 1		NB 1	
	NB		1		WB	
Conflicting Approach Left	NB 1		0		WB 1	
Conflicting Lanes Left	SB		0 WB		1	
Conflicting Approach Right	SB 1		WB 1		0	
Conflicting Lanes Right					-	
HCM Control Delay	10.8		9.3		12.1	
HCM LOS	В		Α		В	
Lane		NBLn1	WBLn1	SBLn1		
Vol Left. %				ODEIII		
VOI Leit, 76		0%	11%	47%		
Vol Thru, %		0% 89%	11% 0%			
		- , -		47%		
Vol Thru, %		89%	0%	47% 53%		
Vol Thru, % Vol Right, %		89% 11%	0% 89%	47% 53% 0%		
Vol Thru, % Vol Right, % Sign Control		89% 11% Stop	0% 89% Stop	47% 53% 0% Stop		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		89% 11% Stop 129	0% 89% Stop 331	47% 53% 0% Stop 329		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		89% 11% Stop 129	0% 89% Stop 331 35	47% 53% 0% Stop 329 153		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		89% 11% Stop 129 0 115	0% 89% Stop 331 35	47% 53% 0% Stop 329 153 176		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		89% 11% Stop 129 0 115	0% 89% Stop 331 35 0	47% 53% 0% Stop 329 153 176 0		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		89% 11% Stop 129 0 115 14 129	0% 89% Stop 331 35 0 296 331	47% 53% 0% Stop 329 153 176 0 329		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		89% 11% Stop 129 0 115 14 129	0% 89% Stop 331 35 0 296 331	47% 53% 0% Stop 329 153 176 0 329		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		89% 11% Stop 129 0 115 14 129 1 0.181	0% 89% Stop 331 35 0 296 331 1 0.417	47% 53% 0% Stop 329 153 176 0 329 1 0.453		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		89% 11% Stop 129 0 115 14 129 1 0.181 5.057	0% 89% Stop 331 35 0 296 331 1 0.417 4.536	47% 53% 0% Stop 329 153 176 0 329 1 0.453 4.956		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		89% 11% Stop 129 0 115 14 129 1 0.181 5.057 Yes	0% 89% Stop 331 35 0 296 331 1 0.417 4.536 Yes	47% 53% 0% Stop 329 153 176 0 329 1 0.453 4.956 Yes		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		89% 11% Stop 129 0 115 14 129 1 0.181 5.057 Yes 700	0% 89% Stop 331 35 0 296 331 1 0.417 4.536 Yes 788	47% 53% 0% Stop 329 153 176 0 329 1 0.453 4.956 Yes 721		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		89% 11% Stop 129 0 115 14 129 1 0.181 5.057 Yes 700 3.149	0% 89% Stop 331 35 0 296 331 1 0.417 4.536 Yes 788 2.596	47% 53% 0% Stop 329 153 176 0 329 1 0.453 4.956 Yes 721 3.034		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		89% 11% Stop 129 0 115 14 129 1 0.181 5.057 Yes 700 3.149 0.184	0% 89% Stop 331 35 0 296 331 1 0.417 4.536 Yes 788 2.596 0.42	47% 53% 0% Stop 329 153 176 0 329 1 0.453 4.956 Yes 721 3.034 0.456		
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		89% 11% Stop 129 0 115 14 129 1 0.181 5.057 Yes 700 3.149 0.184 9.3	0% 89% Stop 331 35 0 296 331 1 0.417 4.536 Yes 788 2.596 0.42 10.8	47% 53% 0% Stop 329 153 176 0 329 1 0.453 4.956 Yes 721 3.034 0.456 12.1		

Appendix M

Synchro Intersection Worksheets – 2032 Future Total Conditions



	Intersection	
terrestian LOC	Intersection Delay, s/veh	10.8
lersection LOS	Intersection LOS	В

Movement	WDL	WDR	INDI	NDK	ODL	901
Lane Configurations	W		î,			4
Traffic Vol, veh/h	4	196	178	30	256	104
Future Vol, veh/h	4	196	178	30	256	104
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	196	178	30	256	104
Number of Lanes	1	0	1	0	0	1
Approach	WB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	NB				WB	
Conflicting Lanes Left	1		0		1	
Conflicting Approach Right	SB		WB			
Conflicting Lanes Right	1		1		0	
HCM Control Delay	9.4		9.6		12.2	
HCM LOS	Α		Α		B	

Lane	NBLn1	WBLn1	SBLn1
Vol Left, %	0%	2%	71%
Vol Thru, %	86%	0%	29%
Vol Right, %	14%	98%	0%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	208	200	360
LT Vol	0	4	256
Through Vol	178	0	104
RT Vol	30	196	0
Lane Flow Rate	208	200	360
Geometry Grp	1	1	1
Degree of Util (X)	0.274	0.259	0.479
Departure Headway (Hd)	4.746	4.667	4.789
Convergence, Y/N	Yes	Yes	Yes
Cap	751	766	749
Service Time	2.809	2.724	2.847
HCM Lane V/C Ratio	0.277	0.261	0.481
HCM Control Delay	9.6	9.4	12.2
HCM Lane LOS	Α	Α	В
HCM 95th-tile Q	1.1	1	2.6

Interpolition						
Intersection Int Delay, s/veh	5.5					
iiii Delay, S/veri	5.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ર્ની	ĵ.	
Traffic Vol, veh/h	133	60	81	401	261	94
Future Vol, veh/h	133	60	81	401	261	94
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	133	60	81	401	261	94
IVIVIIIL I IUW	100	00	01	401	201	34
	Minor2		Major1		Major2	
Conflicting Flow All	871	308	355	0	-	0
Stage 1	308	-	-	-	-	-
Stage 2	563	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	_	-	_		-
Follow-up Hdwy		3.318	2 218	-		-
Pot Cap-1 Maneuver	322	732	1204	-		-
Stage 1	745	102	1204			
Stage 2	570					
Platoon blocked. %	310	-			- 0	- :
	004	720	4004	-	_	-
Mov Cap-1 Maneuver	294	732	1204	-	-	-
Mov Cap-2 Maneuver	294	-	-	-	-	-
Stage 1	680	-	-	-	-	-
Stage 2	570	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	25.9		1.4		0.0	
HCM LOS	23.9 D		1.4		0	
I IOIVI LUO	U					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1204	-	361	-	-
HCM Lane V/C Ratio		0.067	-	0.535	-	
HCM Control Delay (s)		8.2	0	25.9	-	-
HCM Lane LOS		A	A	D		-
			, ,			
HCM 95th %tile Q(veh	١	0.2	_	3	-	-

Intersection						
Int Delay, s/veh	7.9					
		EDE	NID:	NDT	007	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			↑	↑	
Traffic Vol, veh/h	238	69	32	247	226	109
Future Vol, veh/h	238	69	32	247	226	109
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	238	69	32	247	226	109
Major/Minor	Minor2		Major1	N	Major2	
Conflicting Flow All	592	281	335	0	-	0
Stage 1	281	201	-	U	_	U
Stage 2	311					
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	0.22	4.12			
Critical Hdwy Stg 1	5.42	-	-		-	
		3.318	2 210	- 1	- 1	
Follow-up Hdwy	469		1224	-	-	
Pot Cap-1 Maneuver			1224	-	-	-
Stage 1	767	-	-		-	-
Stage 2	743	-	-	-	-	-
Platoon blocked, %		==0	1001	-	-	-
Mov Cap-1 Maneuver	455	758	1224	-	-	-
Mov Cap-2 Maneuver	455	-	-	-	-	-
Stage 1	744	-	-	-	-	-
Stage 2	743	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	23		0.9		0	
HCM LOS	C		0.3		U	
TICWI LOG	C					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1224	-	500	-	-
HCM Lane V/C Ratio		0.026	-	0.614	-	-
HCM Control Delay (s))	8	-	23	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh)	0.1	-	4.1	-	-
7000 7000 34(1011	/	0.1				

Intersection												
Int Delay, s/veh	3.9											
•		EDT	EDD	WDI	WDT	WDD	NIDI	NDT	NDD	ODI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		0.4	4			4			4	
Traffic Vol, veh/h	3	11	2	31	0	66	1	267	55	144	153	2
Future Vol, veh/h	3	11	2	31	0	66	1	267	55	144	153	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage		0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	11	2	31	0	66	1	267	55	144	153	2
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	772	766	154	746	740	295	155	0	0	322	0	0
Stage 1	442	442	-	297	297	-	-	-	-	-	-	-
Stage 2	330	324		449	443	-	_					-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12			4.12		
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-			-		
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	-	_		_	-		_
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218			2.218		
Pot Cap-1 Maneuver	317	333	892	330	345	744	1425		-	1238		_
Stage 1	594	576	-	712	668	144	1420			1200		
Stage 2	683	650		589	576	-	_		_	-		_
Platoon blocked, %	-000	-000		-000	0.0							
Mov Cap-1 Maneuver	261	290	892	288	301	744	1425	_	_	1238		_
Mov Cap-1 Maneuver	261	290	- 002	288	301		1-12-0			-		
Stage 1	593	503	-	711	667		-	_	_	-	_	_
Stage 2	622	649		502	503							
Oluge Z	JLL	373		302	300							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	17.2			14.1			0			4		
HCM LOS	С			В								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1425	-	-	310	494	1238	-	-			
HCM Lane V/C Ratio		0.001					0.116					
HCM Control Delay (s)		7.5	0	-	17.2	14.1	8.3	0	-			
HCM Lane LOS		A	A		C	В	A	A				
HCM 95th %tile Q(veh)	0	-	-	0.2	0.7	0.4	-	-			
	,	-			0.2	0.1	0.1					

HCM 95th %tile Q(veh)

1: Eagleson & Ottawa 10-03-2024 7: McBean & New Road 10-03-2024

HCM 2010 TWSC

Intersection							
Int Delay, s/veh	2.9						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			ની	ĵ.		
Traffic Vol, veh/h	73	53	54	370	393	99	
Future Vol, veh/h	73	53	54	370	393	99	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	e, # 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	73	53	54	370	393	99	
Major/Minor	Minor2	- 1	Major1	N	Major2		
Conflicting Flow All	921	443	492	0	-	0	
Stage 1	443	440	432	-		-	
Stage 2	478						
Critical Hdwy	6.42	6.22	4.12				
Critical Hdwy Stg 1	5.42	0.22	7.12				
Critical Hdwy Stg 2	5.42			_	_	_	
Follow-up Hdwy		3.318	2.218		-		
Pot Cap-1 Maneuver	300	615	1071	-	-	-	
Stage 1	647	-	-				
Stage 2	624	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	281	615	1071	-	-	-	
Mov Cap-2 Maneuver	281	-	-	-	-	-	
Stage 1	606	-	-	-	-	-	
Stage 2	624	-	-	-	-	-	
ŭ							
Approach	EB		NB		SB		
HCM Control Delay, s	20		1.1		0		
HCM LOS	20 C		1.1		U		
I IOWI LOG	U						
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR	
Capacity (veh/h)	it.	1071	INDII	364	- 100	JDK -	
HCM Lane V/C Ratio		0.05		0.346		- 1	
		8.5	0	20	-	-	
HCM Control Delay (s) HCM Lane LOS		0.5 A	A	20 C	- 1		
HOM FAME FOS		Α	Α	U	-	-	

0.2 - 1.5 - -

Intersection									
Int Delay, s/veh	5.4						 		
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations	WDL.	TTDIN	ND1	HOIN	ODL	<u>361</u>			
Traffic Vol, veh/h	11	185	112	5	85	89			
Future Vol. veh/h	11	185	112	5	85	89			
Conflicting Peds, #/hr	0	100	0	0	00	09			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	otop -	None	-	None	-	None			
Storage Length	0	NOHE -		NOHE -		NONE -			
Veh in Median Storage	-		0			0			
Grade. %	0		0			0			
Peak Hour Factor	100	100	100	100	100	100			
Heavy Vehicles, %	2	2	2	2	2	2			
Mymt Flow	11	185	112	5	85	89			
INIVITIC I IOW	- 11	100	112	0	00	09			
	Minor1		Major1		Major2				
Conflicting Flow All	374	115	0	0	117	0			
Stage 1	115	-	-	-	-	-			
Stage 2	259	-	-	-	-	-			
Critical Hdwy	6.42	6.22	-	-	4.12	-			
Critical Hdwy Stg 1	5.42	-	-	-	-	-			
Critical Hdwy Stg 2	5.42	-	-	-	-	-			
Follow-up Hdwy	3.518		-	-	2.218	-			
Pot Cap-1 Maneuver	627	937	-	-	1471	-			
Stage 1	910	-	-	-	-	-			
Stage 2	784	-	-	-	-	-			
Platoon blocked, %			-	-		-			
Mov Cap-1 Maneuver	589	937	-	-	1471	-			
Mov Cap-2 Maneuver	589	-	-	-	-	-			
Stage 1	910	-	-	-	-	-			
Stage 2	736	-	-	-	-	-			
Approach	WB		NB		SB			_	
HCM Control Delay, s	10.1		0		3.7				
HCM LOS	10.1		U		5.1				
I IOWI LOG	0								
Minor Lane/Major Mvn	nt	NBT	NBR\	VBLn1	SBL	SBT			
Capacity (veh/h)		-	-	907	1471	-			
HCM Lane V/C Ratio		-	-	0.216		-			
HCM Control Delay (s))	-	-	10.1	7.6	-			
HCM Lane LOS		-	-	В	Α	-			
HCM 95th %tile Q(veh)	-	-	0.8	0.2	-			

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations		4			4			4			44	
Traffic Vol, veh/h	2	4	1	38	15	47	2	188	31	55	251	
Future Vol, veh/h	2	4	1	38	15	47	2	188	31	55	251	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	- 1
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	2	4	1	38	15	47	2	188	31	55	251	
Major/Minor N	/linor2			Minor1		N	/lajor1		- 1	Major2		

Major/Minor I	Minor2			Minor1			Major1		N	Λá	ajor2	ajor2
Conflicting Flow All	602	586	253	574	573	204	255	0	0		219	219 0
Stage 1	363	363	-	208	208	-	-	-	-		-	
Stage 2	239	223	-	366	365	-	-	-	-	-		-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12		-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-		-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-		-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218		-
Pot Cap-1 Maneuver	412	422	786	430	430	837	1310	-	-	1350		-
Stage 1	656	625	-	794	730	-	-	-	-	-		-
Stage 2	764	719	-	653	623	-	-	-	-	-		-
Platoon blocked, %								-	-			-
Mov Cap-1 Maneuver	364	401	786	410	409	837	1310	-	-	1350		-
Mov Cap-2 Maneuver	364	401	-	410	409	-	-	-	-	-		-
Stage 1	655	596	-	792	729	-	-	-	-	-		-
Stage 2	705	718	-	617	594	-	-	-	-	-		-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.8			13.2			0.1			1.4		
HCM LOS	В			В								

Mineral Land (Marian Marian)	NDI	NDT	NDD	EDI 41	MDI 4	ODI	CDT	CDD
Minor Lane/Major Mvmt	NBL	NBT	NRK	EBLn1\	MRFUJ	SBL	SBT	SBR
Capacity (veh/h)	1310	-	-	418	539	1350	-	-
HCM Lane V/C Ratio	0.002	-	-	0.017	0.186	0.041	-	-
HCM Control Delay (s)	7.8	0	-	13.8	13.2	7.8	0	-
HCM Lane LOS	Α	Α	-	В	В	Α	Α	-
HCM 95th %tile Q(veh)	0	_	-	0.1	0.7	0.1	-	-

Intersection						
	10.4					
Intersection Delay, s/veh Intersection LOS	12.4					
intersection LOS	В					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ.			ર્લ
Traffic Vol, veh/h	35	324	120	14	187	184
Future Vol, veh/h	35	324	120	14	187	184
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	35	324	120	14	187	184
Number of Lanes	1	0	1	0	0	1
Anneach	MP		ND		CD	_
Approach	WB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	NB				WB	
Conflicting Lanes Left	1		0		1	
Conflicting Approach Right	SB		WB		C	
Conflicting Lanes Right	. 1		1		0	
HCM Control Delay	11.8		9.6		13.9	
HCM LOS	В		Α		В	
Lane		NBLn1	WBLn1	SBLn1	_	_
Vol Left, %		0%	10%	50%		
Vol Thru, %		90%	0%	50%		
Vol Right, %		10%	90%	0%		
Sign Control		Stop	Stop	Stop		
Traffic Vol by Lane		134	359	371		
LT Vol		0	35	187		
Through Vol		120	0	184		
RT Vol		14	324	0		
Lane Flow Rate		134	359	371		
Geometry Grp		1	1	1		
Degree of Util (X)		0.198	0.465	0.532		
Departure Headway (Hd)		5.325	4.76	5.158	_	
Convergence, Y/N		Yes	Yes	Yes		
Cap		677	761	705		
Service Time		3.336	2.76	3.158		
HCM Lane V/C Ratio		0.198	0.472	0.526		
HCM Control Delay		9.6	11.8	13.9		
HCM Lane LOS		Α	В	В	_	
HCM 95th-tile Q		0.7	2.5	3.2		

HCM Control Delay (s)
HCM Lane LOS
HCM 95th %tile Q(veh)

HCM 2010 TWSC 6: Fagleson & New Road

Intersection						
Int Delay, s/veh	5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	TIBIT	A	HOIT	052	A
Traffic Vol, veh/h	7	117	87	9	160	107
Future Vol. veh/h	7	117	87	9	160	107
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Olop	None	-	None	-	None
Storage Length	0	TVOTIC		-	_	TVOITE
Veh in Median Storage	-		0			0
Grade. %	0, # 0		0			0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	7	117	87	9	160	107
WWIIIL FIOW	- 1	117	01	9	100	107
	Minor1		Major1		Major2	
Conflicting Flow All	519	92	0	0	96	0
Stage 1	92	-	-	-	-	-
Stage 2	427	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	517	965	-	-	1498	-
Stage 1	932	-	-	-	-	-
Stage 2	658	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	458	965	-	_	1498	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	932	-	-	-	-	-
Stage 2	583		-	-	-	-
Olago 2	000					
	MA		ND		0.0	
Approach	WB		NB		SB	
HCM Control Delay, s			0		4.6	
HCM LOS	Α					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)				908	1498	-
HCM Lane V/C Ratio				0.137		
HCM Control Delay (s	.\				7.7	

- - 0.137 0.107 -- - 9.6 7.7 -- A A -- 0.5 0.4 -

Interception						
Intersection	4.0					
Int Delay, s/veh	4.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	•	
Traffic Vol, veh/h	151	44	59	283	242	205
Future Vol. veh/h	151	44	59	283	242	205
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	_	0	0	_
Grade, %	0			0	0	
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	151	44	59	283	242	205
WWITELLIOW	131	44	33	200	242	200
Major/Minor	Minor2		Major1	N	/lajor2	
Conflicting Flow All	746	345	447	0	-	0
Stage 1	345	-	-	-	-	-
Stage 2	401	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	2.218	-	-	-
Pot Cap-1 Maneuver	381	698	1113	-	-	-
Stage 1	717	-	-		-	
Stage 2	676	-	-	-	-	-
Platoon blocked. %	310					
Mov Cap-1 Maneuver	357	698	1113			
Mov Cap-1 Maneuver	357	090	1113			
Stage 1	672			_		
		-		-		-
Stage 2	676	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	22.2		1.5		0	
HCM LOS	С					
		N.D.	LIBE			005
Minor Lane/Major Mvn	nt	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1113	-	401	-	-
HCM Lane V/C Ratio		0.053	-	0.486	-	-
HCM Control Delay (s)		8.4	-	22.2	-	-
HCM Lane LOS		Α	-	С	-	-

0.2 - 2.6 - -

HCM 95th %tile Q(veh)

Appendix N

Synchro Intersection Worksheets – 2037 Future Total Conditions



10-03-2024

illersection Delay, siven	Intersection	
ntersection LOS B	Intersection Delay, s/veh	
	Intersection LOS	В

Movement	WBL	WBR	NRI	NBK	SBL	SBT
Lane Configurations	¥		î,			ર્ન
Traffic Vol, veh/h	4	196	180	30	270	112
Future Vol, veh/h	4	196	180	30	270	112
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	196	180	30	270	112
Number of Lanes	1	0	1	0	0	1
Approach	WB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	NB				WB	
Conflicting Lanes Left	1		0		1	
Conflicting Approach Right	SB		WB			
Conflicting Lanes Right	1		1		0	
HCM Control Delay	9.5		9.7		12.8	
HCM LOS	Α		Α		В	

Lane	NBLn1	WBLn1	SBLn1
Vol Left, %	0%	2%	71%
Vol Thru, %	86%	0%	29%
Vol Right, %	14%	98%	0%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	210	200	382
LT Vol	0	4	270
Through Vol	180	0	112
RT Vol	30	196	0
Lane Flow Rate	210	200	382
Geometry Grp	1	1	1
Degree of Util (X)	0.279	0.262	0.509
Departure Headway (Hd)	4.78	4.723	4.798
Convergence, Y/N	Yes	Yes	Yes
Cap	746	756	749
Service Time	2.845	2.785	2.857
HCM Lane V/C Ratio	0.282	0.265	0.51
HCM Control Delay	9.7	9.5	12.8
HCM Lane LOS	Α	Α	В
HCM 95th-tile Q	1.1	1	2.9

Intersection	F.0					
Int Delay, s/veh	5.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			લી	ĵ.	
Traffic Vol, veh/h	133	60	81	403	275	94
Future Vol, veh/h	133	60	81	403	275	94
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e.# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	133	60	81	403	275	94
IVIVIIILIIOW	100	00	01	400	213	34
	Minor2		Major1		//ajor2	
Conflicting Flow All	887	322	369	0	-	0
Stage 1	322	-	-	-	-	-
Stage 2	565	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	315	719	1190	-	-	-
Stage 1	735	-	-	-	-	
Stage 2	569		-		-	
Platoon blocked, %						
Mov Cap-1 Maneuver	287	719	1190		_	_
Mov Cap-1 Maneuver	287	- 113	-		-	
Stage 1	670					
Stage 2	569					
Slaye 2	509	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	26.8		1.4		0	
HCM LOS	D					
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR
	ıı					
Capacity (veh/h)		1190	-	353	-	-
HCM Lane V/C Ratio		0.068		0.547	-	-
HCM Control Delay (s)	8.2	0	26.8	-	-
HCM Lane LOS	,	A	Α	D	-	-
LICALOGADO O/ Alla O/ vale		0.0				

0.2 - 3.1 - -

HCM 95th %tile Q(veh)

HCM 2010 TWSC 3: McBean & Ottawa

Intersection						
Int Delay, s/veh	8.2					
-						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y					
Traffic Vol, veh/h	238	69	32	250	244	109
Future Vol, veh/h	238	69	32	250	244	109
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	238	69	32	250	244	109
		-				
	Minor2		Major1		/lajor2	
Conflicting Flow All	613	299	353	0	-	0
Stage 1	299	-	-	-	-	-
Stage 2	314	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	456	741	1206	-	-	-
Stage 1	752	-	-	-	-	-
Stage 2	741	_	-	-	-	-
Platoon blocked, %				-		-
Mov Cap-1 Maneuver	442	741	1206	_		_
Mov Cap-2 Maneuver	442		-			
Stage 1	729			_		_
Stage 2	741			_	_	
Stage 2	741					
Approach	EB		NB		SB	
HCM Control Delay, s	24.3		0.9		0	
HCM LOS	С					
Mineral and Maine Man	-4	NDI	NDT	EDL -4	ODT	CDD
Minor Lane/Major Mvn	nt	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1206	-	486	-	-
HCM Lane V/C Ratio		0.027		0.632	-	-
HCM Control Delay (s))	8.1	-	24.3	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh	1)	0.1	-	4.3	-	-

Intersection												
Int Delay, s/veh	3.9											
•	EDI	EDT	EDD	WDI	WDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	2	4	0	31	- ♣	66	4	↔		144	450	2
Traffic Vol, veh/h	3	11	2	31	0	66	1	267 267	55 55		153 153	2
Future Vol, veh/h		11	0	0	0	00			0	144		0
Conflicting Peds, #/hr	0	0	-	_		_	0	0		0	0	-
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized		-	None	-	-	None	-		None	-		None
Storage Length	- ш		-	-	0			0	_		0	
Veh in Median Storage		0	-	-	0	-	-	_		-	0	-
Grade, % Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	11	2	31	2	2 66	2	267	55 55	144	153	2
Mvmt Flow	3	- 11	2	31	0	рb	1	20/	55	144	153	2
	Minor2			Minor1			Major1		- 1	Major2		
Conflicting Flow All	772	766	154	746	740	295	155	0	0	322	0	0
Stage 1	442	442	-	297	297	-	-	-	-	-	-	-
Stage 2	330	324	-	449	443	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	317	333	892	330	345	744	1425	-	-	1238	-	-
Stage 1	594	576	-	712	668	-	-	-	-	-	-	-
Stage 2	683	650	-	589	576	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	261	290	892	288	301	744	1425	-	-	1238	-	-
Mov Cap-2 Maneuver	261	290	-	288	301	-	-	-	-	-	-	-
Stage 1	593	503	-	711	667	-	-	-	-	-	-	-
Stage 2	622	649	-	502	503	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	17.2			14.1			0			4		
HCM LOS	17.2			14.1 B			U			4		
I IOWI LOG	U			ь								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1\		SBL	SBT	SBR			
Capacity (veh/h)		1425	-	-	310	494	1238	-	-			
HCM Lane V/C Ratio		0.001	-	-	0.052	0.196	0.116	-	-			
HCM Control Delay (s)		7.5	0	-	17.2	14.1	8.3	0	-			
HCM Lane LOS		Α	Α	-	С	В	Α	Α	-			
HCM 95th %tile Q(veh)	0	-	-	0.2	0.7	0.4	-	-			

HCM 2010 TWSC 1: Eagleson & Ottawa 10-03-2024 7: McBean & New Road 10-03-2024

Intersection						
Int Delay, s/veh	2.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	LDI	HUL	4	1	ODIN
Traffic Vol. veh/h	73	53	54	387	395	99
Future Vol, veh/h	73	53	54	387	395	99
	0	0		0		0
Conflicting Peds, #/hr	_	_	0	_	0	Free
Sign Control	Stop	Stop	Free	Free	Free	
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	73	53	54	387	395	99
Major/Minor I	Minor2	- 1	Major1	N	Major2	
Conflicting Flow All	940	445	494	0	-	0
Stage 1	445	-	-707	-		-
Stage 2	495				- 1	
Critical Hdwy	6.42	6.22	4.12			
Critical Hdwy Stg 1	5.42	0.22	4.12			
Critical Hdwy Stg 1	5.42		_		_	
		3.318	2 210		-	
Follow-up Hdwy	293	613		-	-	-
Pot Cap-1 Maneuver		013	1070	-	-	-
Stage 1	646	-	-		-	-
Stage 2	613	-	-		-	
Platoon blocked, %	07.4	0.10	4000	-	-	-
Mov Cap-1 Maneuver	274	613	1070	-	-	-
Mov Cap-2 Maneuver	274	-	-	-	-	-
Stage 1	605	-	-	-	-	-
Stage 2	613	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	20.5		1		0	
HCM LOS	20.5 C				0	
I IOWI LOG	U					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1070	-	357	-	-
HCM Lane V/C Ratio		0.05	-	0.353	-	-
HCM Control Delay (s)		8.5	0	20.5	-	-
HCM Lane LOS		Α	Α	С	-	-
HCM 95th %tile Q(veh)	0.2	-	1.6	-	-

Movement WBL WBR NBT NBR SBL SBT American Configurations Traffic Vol, veh/h 11 185 112 5 85 89 Strutre Vol, veh/h 11 185 112 5 85 89 Strutre Vol, veh/h 11 185 112 5 85 89 Strutre Vol, veh/h 11 185 112 5 85 89 Strutre Vol, veh/h 11 185 112 5 85 89 Strutre Vol, veh/h 11 185 112 5 85 89 Strutre Vol, veh/h 10 00 0 0 0 0 0 0 0							
Movement	Intersection						
ane Configurations	Int Delay, s/veh	5.4					
ane Configurations	Movement	WBL	WBR	NBT	NBR	SBL	SBT
Traffic Vol, veh/h Traff							
Future Vol, veh/h Future Vol, v			185		5	85	
Conflicting Peds, #/hr 0							
Rign Control Stop Stop Free None - None							
None			Stop	Free	Free	Free	Free
Control Delay Control Dela			P				
// Veh in Median Storage, # 0							
Brade, % 0 - 0 - - 0 Peak Hour Factor 100 100 100 100 100 100 Ideavy Vehicles, % 2		-					
Peak Hour Factor 100				_			_
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		-		-			-
Algor/Minor							
Major/Minor Minor1 Major1 Major2							
Nonflicting Flow All 374 115 0 0 117 0	WWIIIL FIOW	- 11	100	112	5	00	09
Stage 1							
Stage 1							
Stage 2 259 - - - - -	Conflicting Flow All	374	115	0	0	117	0
Critical Howy 6.42 6.22 - 4.12 - Critical Howy Stg 1 5.42	Stage 1	115	-	-	-	-	-
Difficial Hdwy Stg 1	Stage 2	259	-	-	-	-	-
Cortical Hdwy Stg 2	Critical Hdwy	6.42	6.22	-	-	4.12	-
Tollow-up Hdwy	Critical Hdwy Stg 1	5.42	-	-	-	-	-
Tollow-up Hdwy	Critical Hdwy Stg 2	5.42	-	-	-	_	-
Pot Cap-1 Maneuver 627 937 - 1471 -	Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Stage 1 910 -		627	937	-	-	1471	-
Stage 2 784 -					-		-
Alaborn blocked, %				-	-	-	-
Mov Cap-1 Maneuver		101					
Mov Cap-2 Maneuver 589 - Stage 1 910 - Stage 2 736 - Approach WB NB SB ICM Control Delay, s 10.1 0 3.7 ICM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - 907 1471 - ICM Lane V/C Ratio - 0.216 0.058 - ICM Control Delay (s) - 10.1 7.6 - ICM Lane LOS - B A -		580	037			1/171	
Stage 1 910 - Stage 2 736 Approach WB NB SB HCM Control Delay, s 10.1 0 3.7 HCM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - 907 1471 - HCM Lane V/C Ratio - 0.216 0.058 - HCM Control Delay (s) - 10.1 7.6 - HCM Lane LOS - B A -						1471	
Stage 2 736 -							
Approach					-		-
CM Control Delay, s 10.1 0 3.7	Stage 2	136	-	-	-	-	-
CM Control Delay, s 10.1 0 3.7							
Alinor Lane/Major Mvmt	Approach	WB		NB		SB	
### A PRINT ABRUBLIN SBL SBT	HCM Control Delay, s	10.1		0		3.7	
Capacity (veh/h) - - 907 1471 - ICM Lane V/C Ratio - - 0.216 0.058 - ICM Control Delay (s) - - 10.1 7.6 - ICM Lane LOS - - B A -	HCM LOS	В					
Capacity (veh/h) - - 907 1471 - ICM Lane V/C Ratio - - 0.216 0.058 - ICM Control Delay (s) - - 10.1 7.6 - ICM Lane LOS - - B A -							
Capacity (veh/h) - - 907 1471 - ICM Lane V/C Ratio - - 0.216 0.058 - ICM Control Delay (s) - - 10.1 7.6 - ICM Lane LOS - - B A -	Minor Lane/Major Myn	nt	NRT	NRRV	WRI n1	SRI	SRT
ICM Lane V/C Ratio - - 0.216 0.058 - ICM Control Delay (s) - - 10.1 7.6 - ICM Lane LOS - B A -		nt.		אוטוא			
HCM Control Delay (s) 10.1 7.6 - HCM Lane LOS - B A -				-			
HCM Lane LOS B A -		`	-	-			-
	7 ())	-	-			-
HCM 95th %tile Q(veh) 0.8 0.2 -			-	-			
	HCM 95th %tile Q(veh	1)	-	-	0.8	0.2	-

HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)

10-03-2024

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	2	4	1	38	15	47	2	188	31	55	251	4
Future Vol, veh/h	2	4	1	38	15	47	2	188	31	55	251	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	4	1	38	15	47	2	188	31	55	251	4
Major/Minor I	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	602	586	253	574	573	204	255	0	0	219	0	0
Stage 1	363	363	-	208	208	-	-	-	-	-	-	-
Stage 2	239	223	-	366	365	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	412	422	786	430	430	837	1310	-	-	1350	-	-
Stage 1	656	625	-	794	730	-	-	-	-	-	-	-
Stage 2	764	719	-	653	623	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	364	401	786	410	409	837	1310	-	-	1350	-	-
Mov Cap-2 Maneuver	364	401	-	410	409	-	-	-	-	-	-	-
Stage 1	655	596	-	792	729	-	-	-	-	-	-	-
Stage 2	705	718	-	617	594	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.8			13.2			0.1			1.4		
HCM LOS	В			В								
Minor Lane/Major Mvm	ıt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1310	-	-	418	539	1350	-	-			
HCM Lane V/C Ratio		0.002	-	-	0.017	0.186	0.041	-	-			
HCM Control Doloy (a)		70	٥		12.0	12.2	70	٥				

7.8 0 - 13.8 13.2 7.8 0 - A A - B B A A - O - 0.1 0.7 0.1 - -

Movement	Intersection						
Movement		12.8					
Movement							
Lane Configurations Y Image: Configuration of the property of the pr							
Lane Configurations Traffic Vol, veh/h 35 342 129 14 187 186 Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mmrt Flow 35 342 129 14 187 186 Number of Lanes 1 0 1 0 0 0 1 Approach WB NB SB Opposing Approach SB NB Opposing Approach SB NB Opposing Lanes 0 1 1 1 Conflicting Approach Right SB WB Conflicting Lanes Left 1 0 1 1 Conflicting Approach Right SB WB Conflicting Lanes Right 1 1 0 0 1 Conflicting Lanes Right 1 1 0 0 1 Conflicting Lanes Right 1 1 0 0 1 Conflicting Lanes Right 1 1 1 0 0 HCM Control Delay 12.5 9.9 14.3 HCM LOS B A B A B Ban A B Lane NBLn1 WBLn1 SBLn1 Vol Left, % 99% 9% 50% Vol Thru, % 90% 9% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, V/N Yes Yes Yes Service Ima Vol. 241 0.502 0.541 Departure Headway (Hd) 0.215 0.497 0.541 HCM Lone VIC Ratio 0.215 0.497 0.541	Movement	WBL	WBR	NBT	NBR	SBL	SBT
Traffic Vol, veh/h 35 342 129 14 187 186 Future Vol, veh/h 35 342 129 14 187 186 Peak Hour Factor 1.00							
Future Vol, veh/h Peak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0			342		14	187	
Peak Hour Factor 1.00							
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2							
Mvmt Flow 35 342 129 14 187 186 Number of Lanes 1 0 1 0 0 1 Approach WB NB SB NB Opposing Approach SB NB NB Opposing Lanes 0 1 1 Conflicting Approach Left NB WB WB Conflicting Lanes Right 1 0 1 Conflicting Lanes Right 1 1 0 1 HCM Control Delay 12.5 9.9 14.3 HCM HCM LOS B A B B Lane NBLn1 WBLn1 SBLn1 WBL Vol Left, % 0% 9% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Number of Lanes 1 0 1 0 0 1 Approach WB NB SB NB Opposing Approach SB NB NB Opposing Lanes 0 1 1 Conflicting Approach Left NB WB WB Conflicting Lanes Left 1 0 1 Conflicting Approach Right SB WB WB Conflicting Lanes Right 1 1 0 1 HCM Control Delay 12.5 9.9 14.3 3 HCM Control Delay 12.5 9.9 14.3 B Lane NBLn1 WBLn1 WBLn1 WBLn1 Vol Left, % 0% 9% 50% Vol Thru, % 90% 0% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol		35	342	129	14	187	186
Opposing Approach SB NB Opposing Lanes 0 1 1 Conflicting Approach Left NB WB WB Conflicting Lanes Left 1 0 1 Conflicting Lanes Right 1 1 0 1 Conflicting Lanes Right 1 1 0 0 1 HCM Control Delay 12.5 9.9 14.3 1 0 HCM LOS B A B	Number of Lanes	1	0	1	0		1
Opposing Approach SB NB Opposing Lanes 0 1 1 Conflicting Approach Left NB WB WB Conflicting Lanes Left 1 0 1 Conflicting Lanes Right 1 1 0 H Conflicting Lanes Right 1 1 0 H HCM Control Delay 12.5 9.9 14.3 H HCM LOS B A B B **PARTICLE METERS AND STONGS A	Annroach	WD		ND		CD	•
Opposing Lanes 0 1 1 Conflicting Approach Left NB WB Conflicting Lanes Left 1 0 1 Conflicting Approach Right SB WB Conflicting Approach Right 1 1 0 HCM Control Delay 12.5 9.9 14.3 HCM LOS B A B Lane NBLn1 WBLn1 SBLn1		WB					
Conflicting Approach Left NB WB Conflicting Lanes Left 1 0 1 Conflicting Approach Right SB WB WB Conflicting Lanes Right 1 1 0 HCM Control Delay 12.5 9.9 14.3 HCM LOS B A B Lane NBLnt WBLnt SBLnt		0					_
Conflicting Lanes Left				1			
Conflicting Approach Right		–		^			_
Conflicting Lanes Right 1 1 0 HCM Control Delay 12.5 9.9 14.3 HCM LOS B A B Lane NBLn1 WBLn1 SBLn1 Vol Left, % 0% 9% 50% Vol Thru, % 90% 0% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C						1	
HCM Control Delay						^	
Lane							
Lane NBLn1 WBLn1 SBLn1 Vol Left, % 0% 9% 50% Vol Thru, % 90% 0% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, YIN Yes Yes Yes Gap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Lane V/C Ratio 9.9 12.5 14.3 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Vol Left, % 0% 9% 50% Vol Thru, % 90% 0% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Lane V/C Ratio 0.215 0.497 0.541	HCM LOS	В		А		В	
Vol Left, % 0% 9% 50% Vol Thru, % 90% 0% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Lane V/C Ratio 0.215 0.497 0.541							
Vol Thru, % 90% 0% 50% Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM LontrOl Delay 9.9 12.5 14.3							
Vol Right, % 10% 91% 0% Sign Control Stop Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degarture Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Lane V/C Ratio 9.9 12.5 14.3			- , -				
Sign Control Stop Stop Stop Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Lontol Delay 9.9 12.5 14.3							
Traffic Vol by Lane 143 377 373 LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3	Vol Right, %		10%	91%	0%		
LT Vol 0 35 187 Through Vol 129 0 186 RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3			Stop	Stop	Stop		
Through Vol 129 0 186 RT Vol 14 342 0			143				
RT Vol 14 342 0 Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3							
Lane Flow Rate 143 377 373 Geometry Grp 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3				-			
Geometry Grp 1 1 1 1 Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3							
Degree of Util (X) 0.214 0.502 0.541 Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3	Lane Flow Rate						
Departure Headway (Hd) 5.396 4.798 5.222 Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3							
Convergence, Y/N Yes Yes Yes Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3							
Cap 665 758 690 Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3	Departure Headway (Hd)		5.396	4.798	5.222		
Service Time 3.431 2.798 3.25 HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3	Convergence, Y/N		Yes	Yes			
HCM Lane V/C Ratio 0.215 0.497 0.541 HCM Control Delay 9.9 12.5 14.3	Cap			758	690		
HCM Control Delay 9.9 12.5 14.3	Service Time						
	HCM Lane V/C Ratio		0.215		0.541		
HCM Lane LOS A B B				12.5	14.3		
	HCM Lane LOS						
HCM 95th-tile Q 0.8 2.9 3.3	HCM 95th-tile Q		0.8	2.9	3.3		

HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh) HCM 2010 TWSC 6: Eagleson & New Road

Intersection						
Int Delay, s/veh	5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	וטוו	<u>NDI</u>	ווטוו	ODL	<u>361</u>
Traffic Vol, veh/h	7	117	87	9	160	107
Future Vol. veh/h	7	117	87	9	160	107
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-		-		-
Veh in Median Storage	-		0			0
Grade. %	0		0			0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	7	117	87	9	160	107
WWW.CT IOW		- 117	01	0	100	101
	Minor1		Major1		Major2	
Conflicting Flow All	519	92	0	0	96	0
Stage 1	92	-	-	-	-	-
Stage 2	427	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		-	-		-
Pot Cap-1 Maneuver	517	965	-	-	1498	-
Stage 1	932	-	-	-	-	-
Stage 2	658	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	458	965	-	-	1498	-
Mov Cap-2 Maneuver	458	-	-	-	-	-
Stage 1	932	-	-	-	-	-
Stage 2	583	-	-	-	-	-
·						
Approach	WB		NB		SB	
	9.6		0		4.6	
HCM Control Delay, s HCM LOS			U		4.0	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	908	1498	-
HCM Lane V/C Ratio		-	-	0.137	0.107	-
LIOM Control Dolon (a)				0.0	7.7	

- 9.6 7.7 -- A A -- 0.5 0.4 -

Intersection						
Int Delay, s/veh	5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y				•	
Traffic Vol, veh/h	151	44	59	304	245	205
Future Vol, veh/h	151	44	59	304	245	205
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	151	44	59	304	245	205
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	770	348	450	0	viajoiz -	0
	348					-
Stage 1	422	-	-	-	-	
Stage 2	6.42	6.22	4.12	-	-	-
Critical Hdwy	5.42		4.12	-	-	-
Critical Hdwy Stg 1		-	-	-	-	-
Critical Hdwy Stg 2	5.42	3.318	0.040	-	-	-
Follow-up Hdwy				-	-	-
Pot Cap-1 Maneuver	369	695	1110	-	-	-
Stage 1	715	-	-	-	-	-
Stage 2	662	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	345	695	1110	-	-	-
Mov Cap-2 Maneuver	345	-	-	-	-	-
Stage 1	669	-	-	-	-	-
Stage 2	662	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	23.2		1.4		0	
HCM LOS	23.2 C		1.4		0	
HOM FOS	U					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1110	-	389	-	-
HCM Lane V/C Ratio		0.053	-	0.501		-
HCM Control Delay (s)		8.4	-	23.2	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh)	0.2	-	2.7	-	-
	,					