

October 3, 2025
File: PG4216-LET.02 Revision 2



**PATERSON
GROUP**

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Noise and Vibration Studies

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3187 Albion Road South
Ottawa, Ontario
K1V 8Y3

Attention: **Mike Green**

Subject: **Sump Pump Feasibility Report
Proposed Residential Development
5970 and 6038 Ottawa Street - Ottawa**

Further to your request, Paterson Group (Paterson) completed a sump pump feasibility study as per City guidelines to determine the use of sump pumps for the proposed residential development to be located at 5970 and 6038 Ottawa Street in the City of Ottawa.

This report provides a discussion on the criteria to design the underside of footing elevations with respect to the existing groundwater table. The current report should be read in conjunction with Paterson Report PG4216-1 Revision 6 dated October 3, 2025.

1.0 Background Information

The field investigation programs completed at the subject site were carried out by Paterson between December 2018 and September 2025. The investigations consisted of 47 boreholes, 25 test pits and 7 hand auger holes extending to a maximum depth of 9.8 m, 3.8 m and 1.1 m below ground surface (bgs), respectively. A bedrock delineation program was also carried out in December 2020, consisting of a total of 87 probe holes to the bedrock surface.

The test holes were distributed in a manner to provide general coverage of the subject site and has been presented in Drawing PG4216-2 - Test Hole Location Plan attached to this letter report.





Field Survey

The test hole locations were selected by Paterson to provide general coverage of the proposed development, taking into consideration the existing site features and underground utilities. The location and ground surface elevation at each borehole and test pit location were provided by Stantec Geomatics Ltd during the initial investigation, and by Paterson for the supplemental investigations.

The test hole locations and ground surface elevations at each test hole location are referenced to a geodetic datum. The location of the test holes and ground surface elevations at each test hole location are presented on Drawing PG4216-2 - Test Hole Location Plan in Appendix 2.

Subsurface Profile

Generally, the subsurface profile encountered within the subject site consists of topsoil underlain by a compact to dense glacial till deposit, and a firm to very stiff silty clay deposit followed by glacial till throughout the eastern portion of the subject site. The glacial till deposit is typically comprised of silty sand and/or silty clay with varying amounts of gravel, cobbles and boulders. The above noted layers have been underlain by a fair to excellent quality limestone bedrock.

Practical refusal to drill and excavation was encountered at multiple locations throughout the subject site. Bedrock surface elevations are presented in Drawing PG4216-4 - Bedrock Contour Plan attached to the current report.

Based on available geological mapping, the bedrock consists of dolostone of the Oxford formation with an overburden drift thickness ranging between 5 and 10 m.

For the purpose of this study, reference should be made to the area deemed acceptable for sump pumps presented on Drawing PG4216-5 – Designated Silty Clay Areas attached to the current report. Specific details of the soil profiles at the test hole locations relevant to the sump pump study area are presented on the Soil Profile and Test Data sheets also attached to the current report.





Monitoring Well Installation

PVC groundwater monitoring wells within the sump pump study area were installed in select borehole locations by Paterson to permit monitoring of the groundwater levels subsequent to the completion of the sampling programs. Monitoring well construction details are described below:

- ☐ Slotted PVC 1.5 m screen at the base of each borehole.
- ☐ 51 mm diameter PVC riser pipe from the top of the screen to the ground surface.
- ☐ No.3 silica sand backfill within annular space around screen.
- ☐ Bentonite hole plug placed directly above PVC slotted screen.
- ☐ Clean backfill from top of bentonite plug to the ground surface.

Specific details of the installation of each monitoring well are further included in the Soil Profile and Test Data sheets and attached to the current letter report.

2.0 Groundwater Monitoring Program

The monitoring wells within the sump pump study area were equipped with a Van Essen Instrument Mini-Diver Water Level Logger to monitor fluctuations in the groundwater levels during the spring melt as part of a review for the use of sump pumps within the study area of the proposed development. The Mini-Divers were programmed to continuously measure and record groundwater levels throughout the study area during the pre-construction stages of the development at a rate of 1 reading every 12 hours. The results of the groundwater fluctuations and correlated precipitation events for each monitoring well location between April 8, 2021, and April 21, 2023, have been summarized in Figure 1 through Figure 11 attached to the current report.

Groundwater Monitoring Results

The data presented in Figure 1 through Figure 11 illustrate the collected groundwater elevations. The readings measured within the monitoring wells varied from below the tip of the data logger (>3.6 m and below 90.3 m) to above the original ground surface (Approximately 94.45 m). The low and high groundwater elevation measurements at each well location between April 8, 2021, and April 21, 2023, are summarized in Table 1 below.





Table 1 – Recorded Groundwater Elevations				
Test Hole ID	Ground Surface Elevation (2021) (m)	Low Groundwater Elevation (m)	High Groundwater Elevation (m)	Difference in Groundwater Depth (m)
BH 1D-21	94.24	91.00	94.25	3.25
BH 1S-21	94.24	92.10	94.00	1.90
BH 2-21	93.91	91.00	93.30	2.30
BH 3-21	93.91	91.55	93.65	2.10
BH 4D-21	94.51	91.70	94.45	2.75
BH 4S-21	94.51	92.65	94.40	1.75
BH 5-21	94.21	91.00	94.15	3.15
BH 6-21	94.04	90.60	93.95	3.35
BH 7D-21	93.62	91.80	94.05	2.25
BH 7S-21	93.62	91.75	94.00	2.25
BH 8-21	94.05	90.65	94.00	3.35
BH 9-21	94.21	91.55	94.00	2.45
BH 10D-21	94.08	91.35	93.70	2.35
BH 10S-21	94.08	92.10	93.55	1.45
BH 11-21	93.92	90.30	92.95	2.65

Based on our analysis of the data retrieved from the data loggers between April 8, 2021 and April 21, 2023 groundwater fluctuations can be observed with a difference in elevation between the low and high readings ranging from 90.3 to 94.45 m. The lowest water level readings were found to range between an elevation of 90.3 (or lower) and 92.7 m throughout the monitoring period. During the spring melt season and heavy rain events, the highest water level readings were found to range between an elevation of 92.95 m and 94.45 m, with an average fluctuation of approximately 2.5 m.

Upon analysing the data collected from the monitoring wells and the subsequent rain events, groundwater reading spikes occurred within short periods of time after each rain event followed by a rapid lowering of the groundwater readings over the following days. It should be noted that the fluctuating groundwater readings are expected to be the result of the accumulation of rainwater within the fissures of the clay layer, as well as surficial permeation through the sand layers, which provides the collected water access to the shallow monitoring well screens. This is an indication that the groundwater level readings have been influenced by temporarily ‘perched’ groundwater, which is typical of monitoring well data within low permeability clay deposits, where water from rain events or spring melt tend to be maintained within the upper portion of the subsoil profile until either sheet drainage or evapotranspiration can eliminate the collected water, as well as surficial rainwater infiltration through the sand layers.

Furthermore, the usage of silty clay backfill material around the foundation and capped at surface will mitigate the potential for higher groundwater elevations and reduce infiltration as discussed in Section 4 of this report.



3.0 Hydraulic Conductivity Testing

Based on discussions with David Schaeffer Engineering Ltd. (DSEL), it is understood the proposed underside of footing depths will be approximately 1.8 m below the proposed center line of the roadways. As such, the underside of footings were estimated to be located slightly below to slightly above existing ground surface. Therefore, monitoring wells could not be positioned to straddle both above and below the expected USF elevation to facilitate hydraulic testing. As such, Atterberg testing and hydrometers in accordance with Unified Soil Classification System (USCS) identified on-site materials as clay soils and is in agreement with the ISTB-2018-04. As such, the hydraulic conductivity values for the silty clay within the sump pump study area were conservatively estimated based on experience at similar sites and published values. Hydraulic conductivity for silty clay generally ranges from 1×10^{-7} to 1×10^{-12} m/sec and is dependent on the moisture level and consistency of the material.

Hydraulic conductivity (slug) testing was conducted on the glacial till underlying the silty clay deposit. The test data was analyzed as per the method set out by Hvorslev (1951). Assumptions inherent in the Hvorslev method include a homogeneous and isotropic aquifer of infinite extent with zero-storage assumption, and a screen length significantly greater than the monitoring well diameter. The assumption regarding aquifer storage is considered to be appropriate for groundwater flow through the overburden aquifer. The assumption regarding screen length and well diameter is considered to be met based on a screen length of 1.5 m and a diameter of 0.05 m. While the idealized assumptions regarding aquifer extent, homogeneity, and isotropy are not strictly met in this case (or in any real-world situation), it has been our experience that the Hvorslev method produces effective point estimates of hydraulic conductivity in conditions similar to those encountered at the subject site.

Hvorslev analysis is based on the line of best fit through the field data (hydraulic head recovery vs. time), plotted on a semi-logarithmic scale. In cases where the initial hydraulic head displacement is known with relative certainty, such as in this case where a physical slug has been introduced, the line of best fit is considered to pass through the origin.

Based on the above test methods, the monitoring wells screened in the glacial till displayed hydraulic conductivity values ranging between 1.27×10^{-7} to 1.00×10^{-5} m/sec. The values measured within the monitoring wells are generally consistent with similar material Paterson has encountered on other sites and typical published values for glacial till. The results of the hydraulic conductivity testing completed by Paterson and others have been attached to the current letter report.





4.0 Summary and Recommendations

It is understood that the proposed residential buildings within the subject site will include a basement level. It is also understood that sump pump systems are proposed for the buildings within the sump pump study area of the subject development to provide an outlet for stormwater and spring melt water collected from the perimeter foundation drainage system. Specific locations are to be determined by the Civil engineer during detailed design.

Groundwater Table Versus USF Elevations

Based on the above discussion, the pre-development seasonal high groundwater table within the sump pump study area is anticipated to be at elevations ranging between 92.95 and 94.45 m. Paterson reviewed the following grading plans prepared by DSEL as part of our assessment:

DSEL – Tamarack Richmond, Grading Plan, Project No. 1042, Drawing No. 04D, dated September 2025.

While detailed grading has not been provided for the proposed development, DSEL has noted that the proposed underside of footing (USF) will be placed at an approximate depth of 1.8 m below the center line elevation of the proposed roads identified in the above noted grading plan.

Seasonal High Groundwater Level

As indicated above, the pre-development seasonal high water was measured between 92.95 to 94.45 m. Based on our review of the latest conceptual plan provided by DSEL, the proposed USF elevations across the study area are generally below the measured seasonal high groundwater table. It is important to note that the groundwater levels recorded for the site are considered pre-development groundwater levels. From a geotechnical perspective, the pre-development groundwater levels are usually higher than post-development groundwater levels and highly dependent on the inverts of the proposed site servicing pipes.

Sump Pumps (Study Area)

It should be noted that based on the Technical Bulletin ISTB-2018-04 and ISTB-2019-02 issued by the City of Ottawa regarding installation of sump pumps, for typical sites, a minimum 300 mm vertical separation is recommended between the design underside of footing elevation and the seasonal high groundwater level.





As previously noted, the proposed USF elevations across the study area are generally below the pre-development seasonal high groundwater table. As such, assessment of the backfill permeability and estimates of the rate of groundwater ingress are required to rationalize the use of sump pumps at the site, as per Appendix 8 of ISTB-2018-04.

Clay Continuity

The boreholes completed within the sump pump study area of the subject site are in conformance with the City of Ottawa borehole spacing guidelines. The native silty clay soils within the study area are considered to be laterally vertically continuous. The boreholes within the study area of the subject site identify a silty clay deposit at the borehole locations at similar elevations throughout. Therefore, the silty clay deposit is considered to be laterally continuous across the sump pump study area of the proposed residential development.

Backfill Recommendations

It is recommended that silty clay backfill material be used around the foundation and capped at surface to mitigate the potential for higher groundwater elevations and reduce infiltration. Backfill material comprised of silty clay with a hydraulic conductivity $< 1 \times 10^{-7}$ m/sec will be used and consist of site excavated or imported silty clay fill. As per Section 8.4 of the ISTB, characterization of the imported fill will be completed and consist of Falling Head Permeability laboratory testing completed in accordance with ASTM D5084 - Option B. Alternatively, hydrometer/Atterberg testing may be completed to ensure the backfill material is similar or contains higher fines than the site specific material. Furthermore, visual inspections of the placement of the silty clay backfill at the foundation locations and the clay seals at the property line within lateral servicing trenches will be carried out by the geotechnical consultant.

All surfaces adjacent to the proposed buildings should be shaped to shed water away from the building's foundation. All the sump pump installations should be inspected and approved by Paterson at the time of installation.

Permeability of Soils and Groundwater Ingress Rate

Based on Atterberg Limits testing completed on the silty clay within the sump pump study area, the material has been classified as CL-CH - Inorganic Clay of Low to High Plasticity per the (USCS). Atterberg Limits testing results have been attached to the current report. Silty clay properties in the assessment are based on the backfill specification noted in the previous section.





The ingress rate has been calculated using the Dupuit Forchheimer method:

$$Q = \pi k((h_0^2 - h_p^2) / \ln(R/r))$$

- ☐ k = hydraulic conductivity (m/sec)
- ☐ h_0 = thickness of the aquifer (m)
- ☐ h_p = thickness of the aquifer from the base of the excavation to the base of the aquifer (m)
- ☐ R = effective drawdown radius for the excavation (m)
- ☐ r = equivalent radius of the excavation (m)

For the purpose of completing the calculation, the following values were conservatively used in the analysis:

- ☐ $k = 1 \times 10^{-7}$ m/sec, silty clay
- ☐ $h_0 = 10$ m
- ☐ $h_p = 9$ m
- ☐ $R = < 5$ m
- ☐ $r = 8$ m

Based on the silty clay profile below the proposed footings and used as backfill material, the groundwater ingress rate was calculated to be less than 0.06 L/s (<5,000 L/day) and significantly less than the minimum pump capacity of 0.9 L/s (77,760 L/day) at 3.6 m head as per the above noted sump pump design Bulletin. Also, due to the characteristics of the underlying silty clay, the groundwater levels will not impact the performance of a sump pump due to the low permeability of the soils.

Based on the above, the sump pumps are not expected to be overloaded and/or continuously running. **As such, the minimum design requirements for the main sump pump system and the backup pump battery will be achieved for the estimated groundwater rate of ingress under worst case scenarios.**



5.0 Conclusion

Based on our observations and available design details, the subject area identified in Drawing PG4216-5 – Designated Silty Clay Areas is considered to have met the requirements detailed in the Technical Bulletin ISTB-2018-04 and ISTB-2019-02 issued by the City of Ottawa regarding installation of sump pumps.

We trust that the current submission meets your immediate requirements.

Best Regards,

Paterson Group Inc.

Nicholas Zulinski, P.Geo., géo.

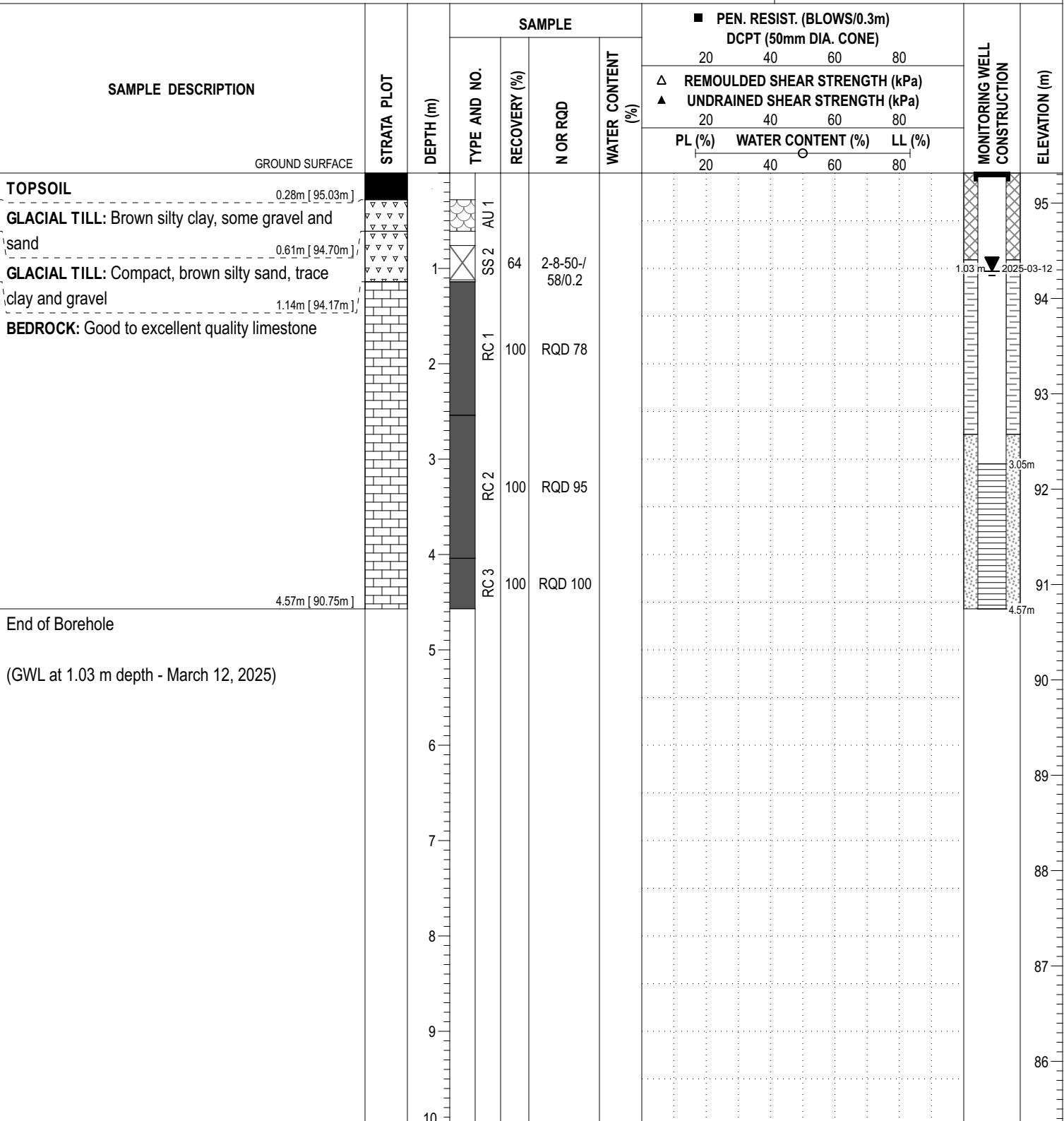
Michael Killam, P.Eng.

Attachments:

- ☐ Soil Profile and Test Data Sheets
- ☐ Symbol and Terms
- ☐ Drawing PG4216-2 – Test Hole Location Plan
- ☐ Drawing PG4216-4 – Bedrock Contour Plan
- ☐ Drawing PG4216-5 – Designated Silty Clay Areas
- ☐ Figures 1 through 11 - Groundwater Monitoring Levels
- ☐ Atterberg Testing Results
- ☐ Hydraulic Conductivity Testing
- ☐ DSEL – Grading Plan

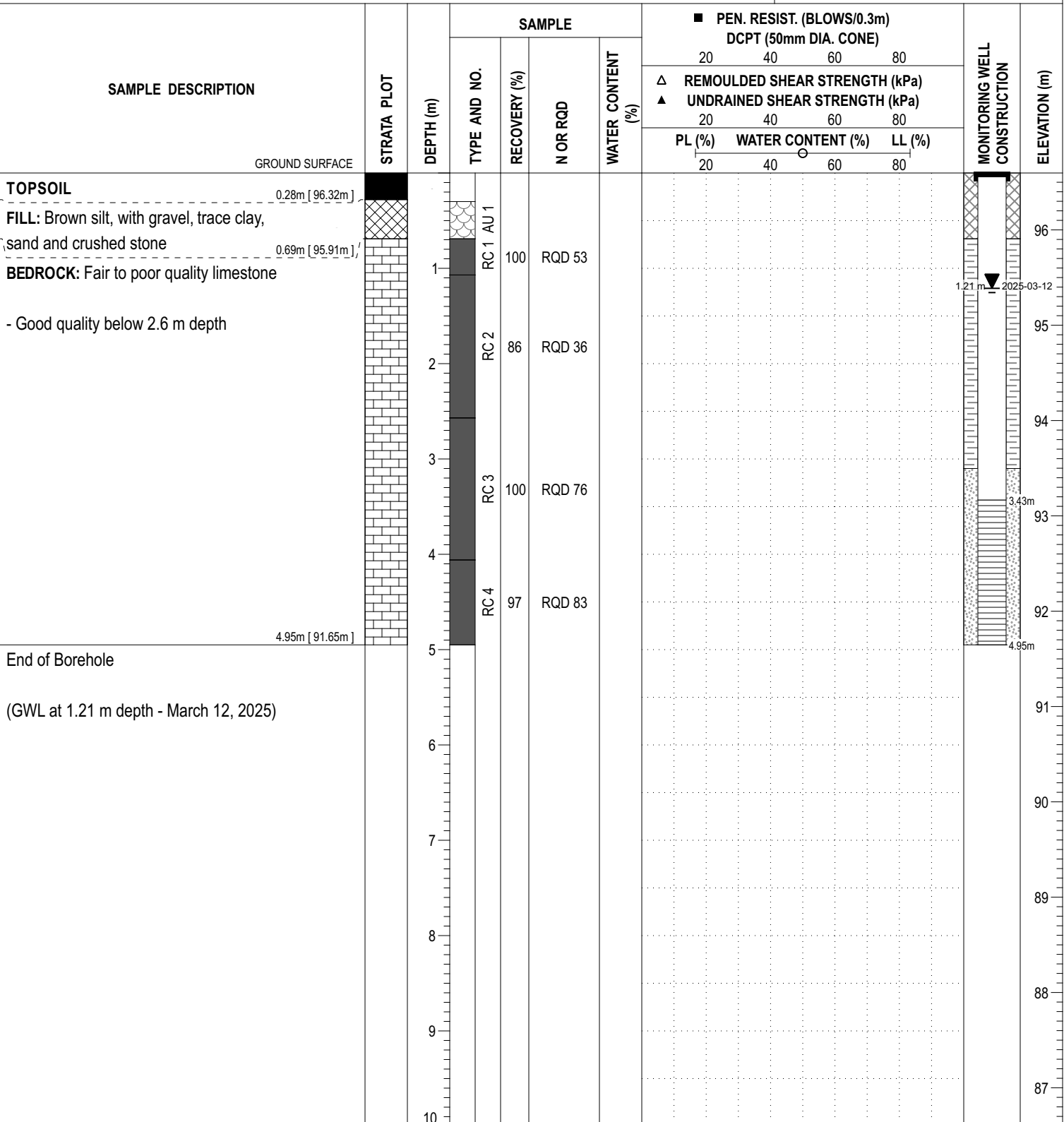


COORD. SYS.: UTM ZONE 18	EASTING: 435369.03	NORTHING: 5003654.34	ELEVATION: 95.31
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 1-25
REMARKS:			
DATE: March 6, 2025			



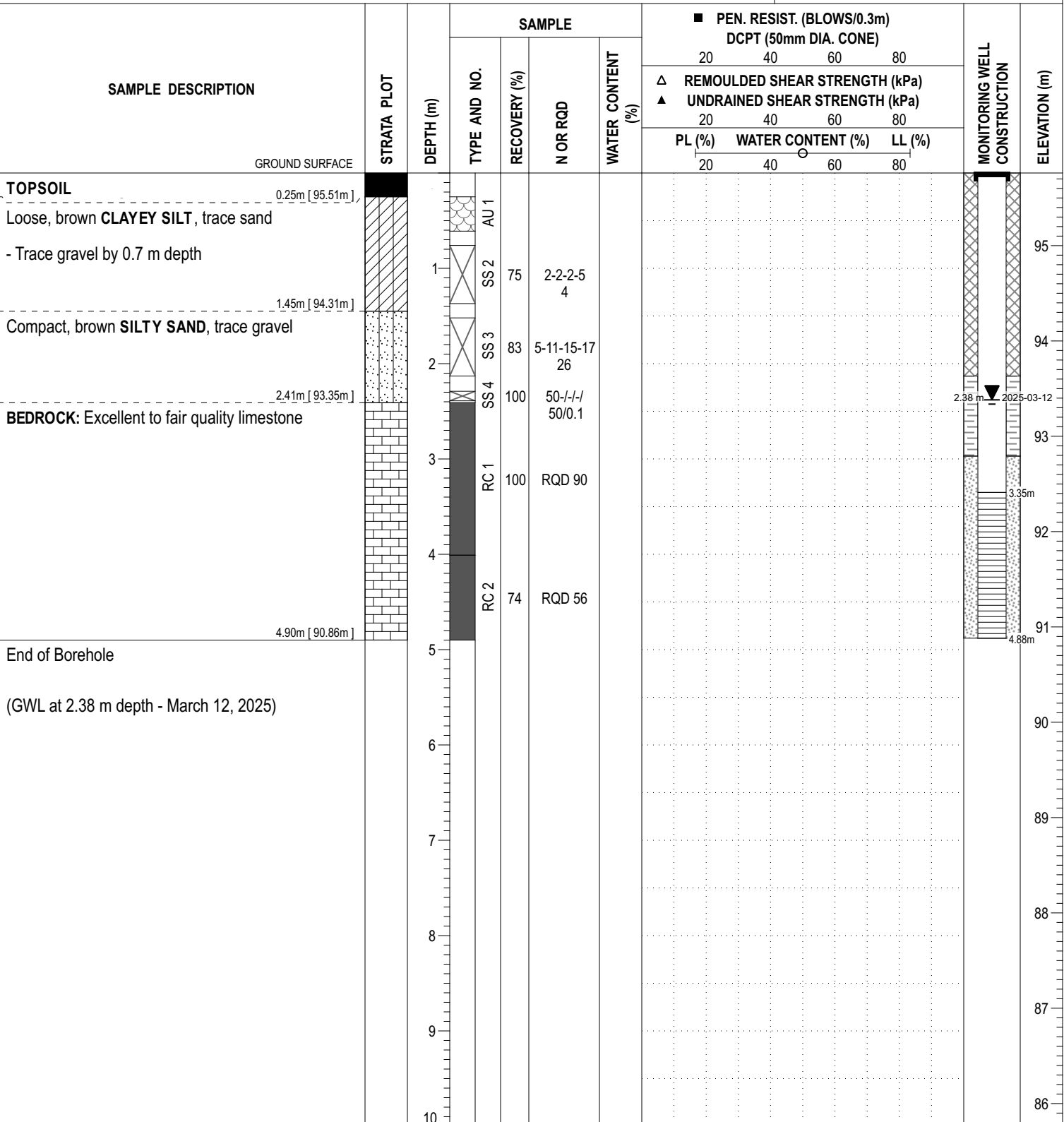
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COORD. SYS.: UTM ZONE 18	EASTING: 435568.93	NORTHING: 5003478.60	ELEVATION: 96.60
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 2-25
REMARKS:			
DATE: March 6, 2025			



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COORD. SYS.: UTM ZONE 18	EASTING: 435522.74	NORTHING: 5003885.37	ELEVATION: 95.76
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 3-25
REMARKS:			
DATE: March 6, 2025			



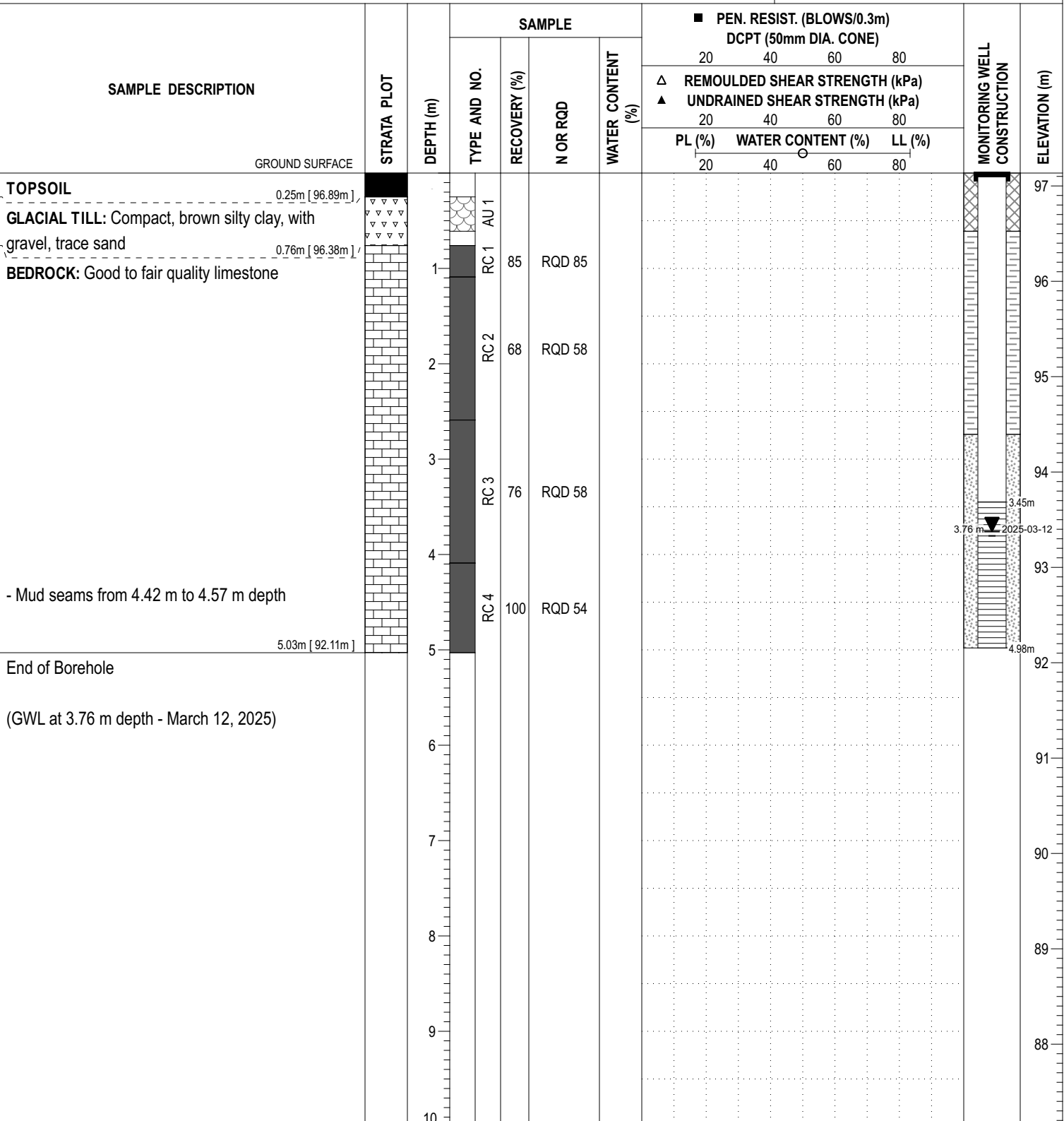
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COORD. SYS.: UTM ZONE 18 EASTING: 435522.74 NORTHING: 5003885.37 ELEVATION: 95.76	
PROJECT: Proposed Mixed-Use Development ADVANCED BY: Track Mounted Drill Rig REMARKS:	FILE NO. : PG4216 HOLE NO. : BH 3A-25
DATE: March 6, 2025	

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			MONITORING WELL CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%)	WATER CONTENT (%)				LL (%)
										</		

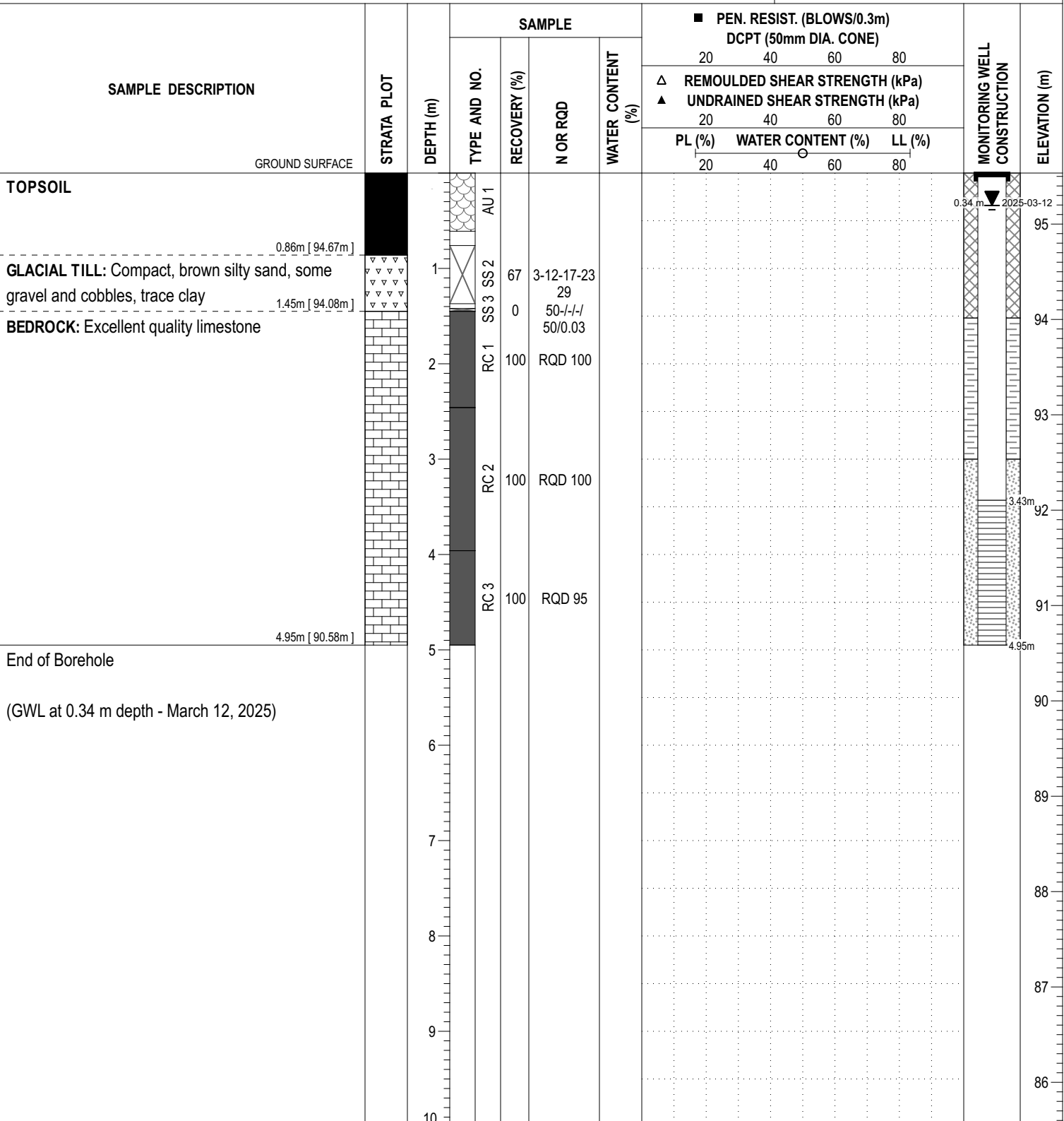
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COORD. SYS.: UTM ZONE 18	EASTING: 435611.53	NORTHING: 5003727.26	ELEVATION: 97.14
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 4-25
REMARKS:			
DATE: March 6, 2025			



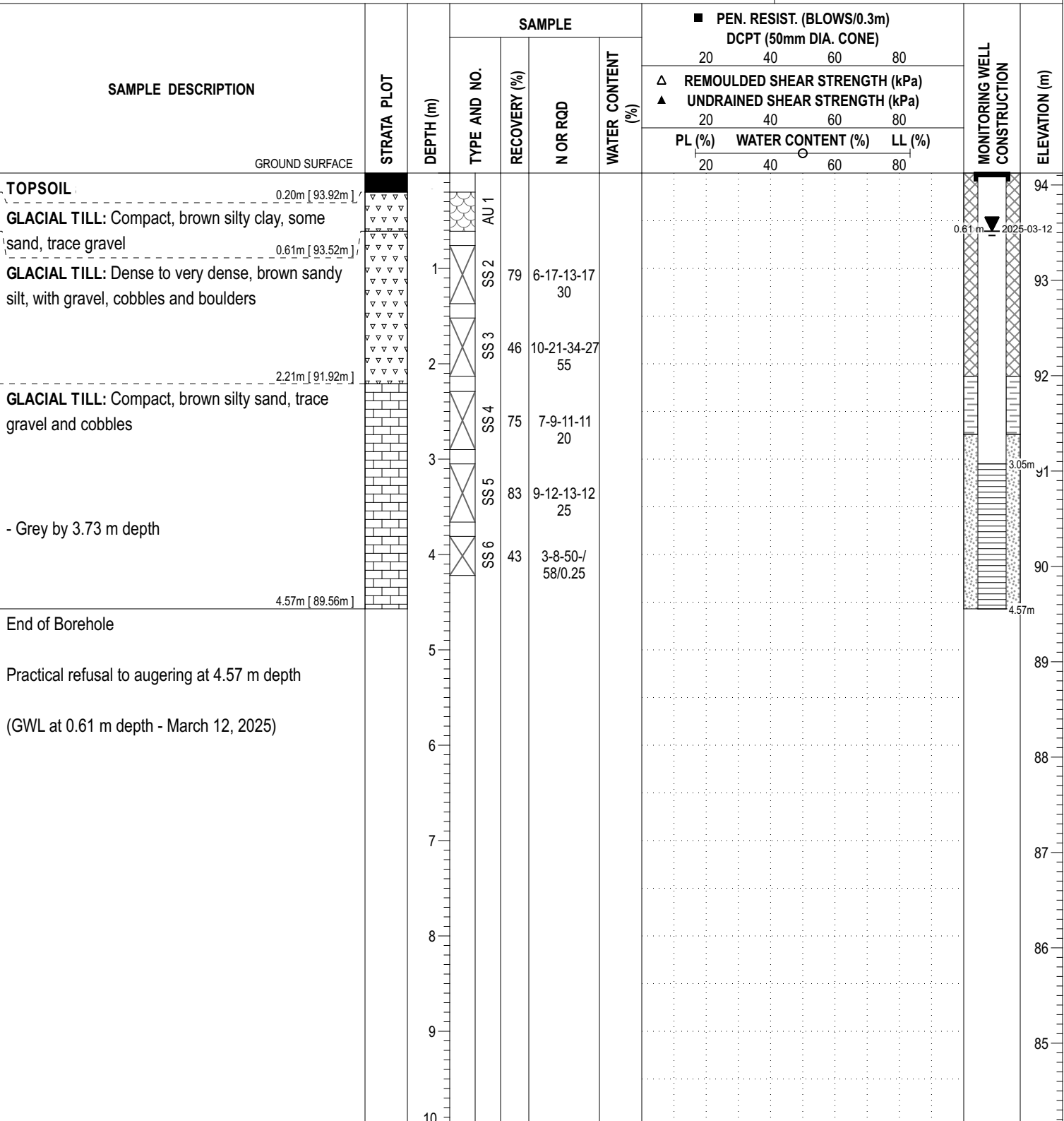
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COORD. SYS.: UTM ZONE 18	EASTING: 435856.20	NORTHING: 5003665.21	ELEVATION: 95.53
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 5-25
REMARKS:			
DATE: March 7, 2025			



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COORD. SYS.: UTM ZONE 18	EASTING: 435817.85	NORTHING: 5004471.80	ELEVATION: 94.12
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 6-25
REMARKS:			
DATE: March 7, 2025			



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COORD. SYS.: UTM ZONE 18	EASTING: 435895.27	NORTHING: 5004286.27	ELEVATION: 94.05
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Hang Auger			HOLE NO. : HA 1-25
REMARKS:			
DATE: August 21, 2025			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60	80		
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
PL (%)	WATER CONTENT (%)		LL (%)									
20	40	60	80									
GROUND SURFACE												
TOPSOIL												94
0.25m [93.80m]												
Brown SILTY CLAY												
0.90m [93.15m]												
End of Hand Auger Hole												
		1										93

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SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
		20	40	60	80							
		PL (%)	WATER CONTENT (%)		LL (%)							
		20	40	60	80							
GROUND SURFACE												
TOPSOIL												
0.30m [93.54m]												
Brown SILTY CLAY												
0.85m [92.99m]												
End of Hand Auger Hole												
		1										
		2										
		3										
		4										
		5										

PAGE: 1 / 1

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)	PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20 40 60 80		
							Δ REMOULDED SHEAR STRENGTH (kPa)		
							▲ UNDRAINED SHEAR STRENGTH (kPa)		
							PL (%) WATER CONTENT (%) LL (%)		
							20 40 60 80		
GROUND SURFACE									
TOPSOIL									
Brown SILTY CLAY									
End of Hand Auger Hole									

PAGE: 1 / 1

COORD. SYS.: UTM ZONE 18	EASTING: 435856.94	NORTHING: 5004047.50	ELEVATION: 93.88
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Hang Auger			HOLE NO. : HA 4-25
REMARKS:			
DATE: August 21, 2025			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60	80		
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							20	40	60	80		
PL (%)	WATER CONTENT (%)		LL (%)									
20	40	60	80									
GROUND SURFACE												
TOPSOIL												
0.30m [93.58m]												
Brown SILTY CLAY												
0.65m [93.22m]												
End of Hand Auger Hole												
		1										93
		2										92
		3										91
		4										90
		5										89

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Supplemental Geotechnical Investigation

5970 and 6038 Ottawa Street, Ottawa, Ontario

ELEVATION: 93.79

FILE NO. : PG4216

HOLE NO. : HA 5-25

DATE: August 21, 2025

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SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)	PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20 40 60 80		
							Δ REMOULDED SHEAR STRENGTH (kPa)		
							▲ UNDRAINED SHEAR STRENGTH (kPa)		
							PL (%) WATER CONTENT (%) LL (%)		
							20 40 60 80		
GROUND SURFACE									
TOPSOIL	[Solid Black]								
Brown SILTY CLAY	[Diagonal Hatching]								
End of Hand Auger Hole			G 1						
		1						93-	
		2						92-	
		3						91-	
		4						90-	
		5						89-	

PAGE: 1 / 1



Supplemental Geotechnical Investigation

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18

EASTING: 436123.11

NORTHING: 5003874.87

ELEVATION: 94.36

PROJECT: Proposed Mixed-Use Development

FILE NO. : PG4216

ADVANCED BY: Hang Auger

DATE: August 21, 2025

HOLE NO.: HA 7-25

REMARKS:

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ELEVATION: 94.61

FILE NO. : PG4216

HOLE NO. : BH 1-24

DATE: December 16, 2024

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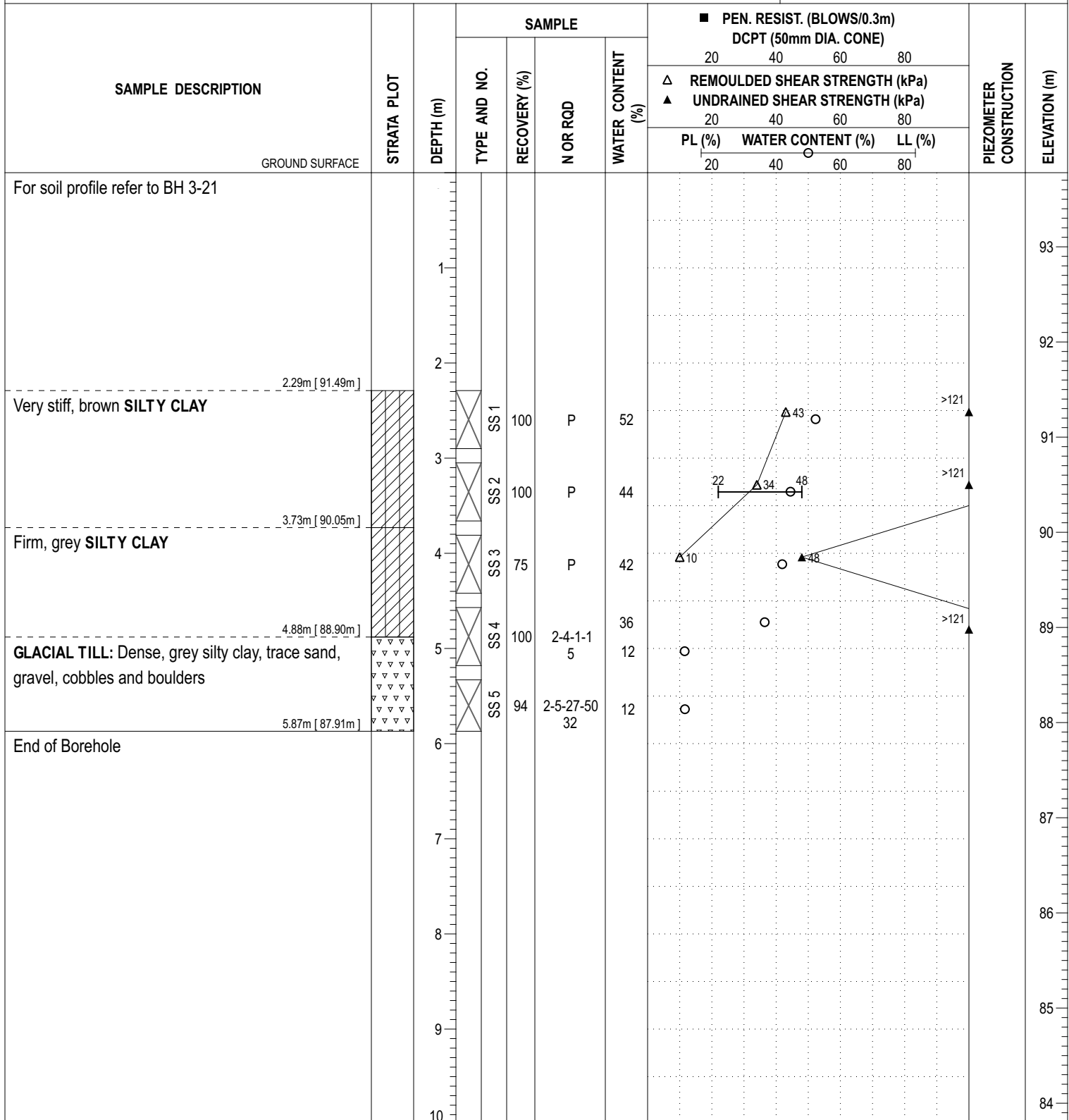
COORD. SYS.: UTM ZONE 18		EASTING: 435899.31	NORTHING: 5004264.51	ELEVATION: 94.06
PROJECT: Proposed Mixed-Use Development			FILE NO. :	PG4216
ADVANCED BY: Track Mounted Drill Rig				
REMARKS:			DATE: December 16, 2024	HOLE NO. : BH 3-24

[illegible]

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PAGE: 1 / 1

COORD. SYS.: UTM ZONE 18	EASTING: 435891.01	NORTHING: 5004133.10	ELEVATION: 93.78
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 3A-21
REMARKS:			DATE: December 19, 2024



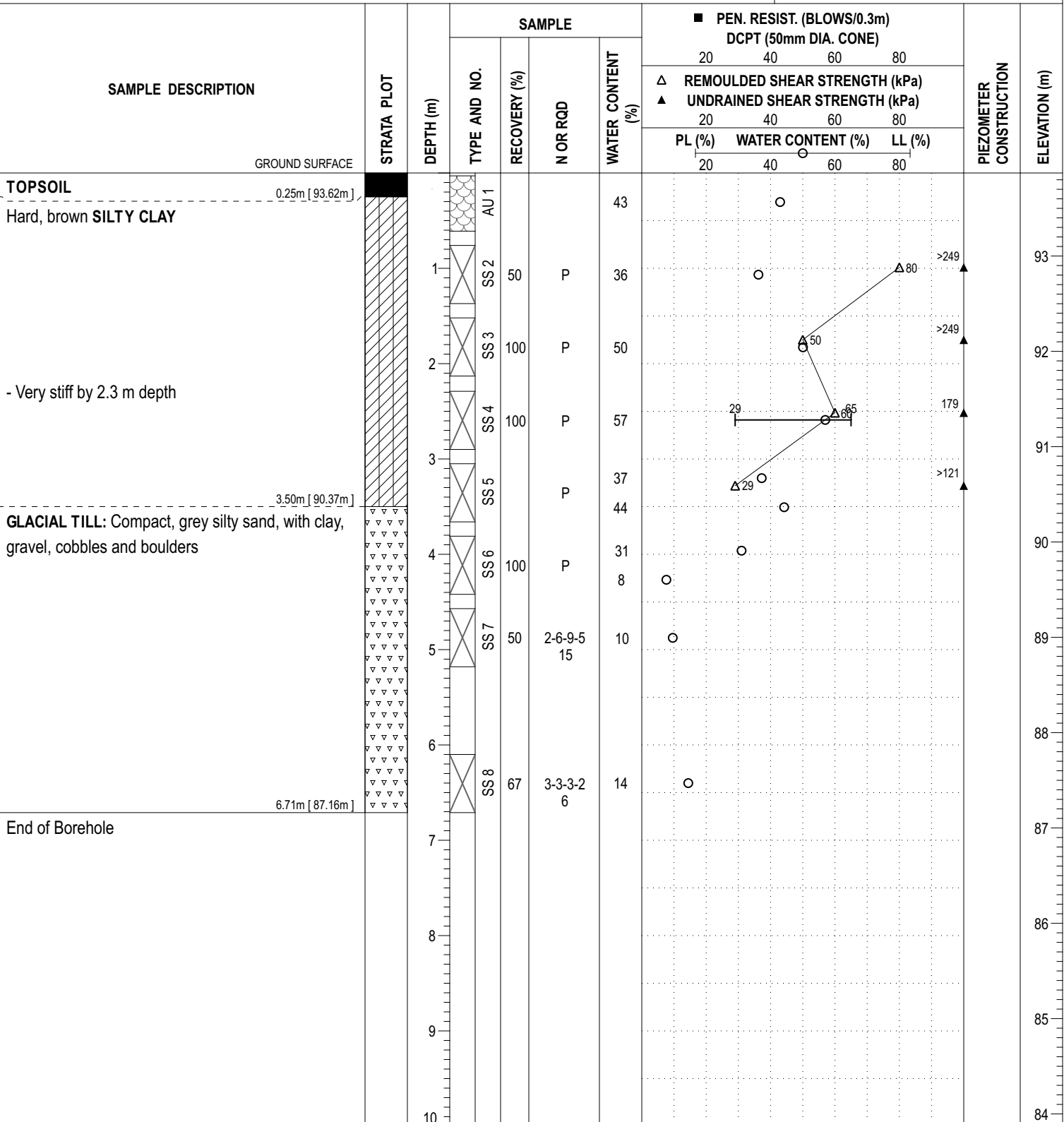
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COORD. SYS.: UTM ZONE 18	EASTING: 435891.01	NORTHING: 5004133.10	ELEVATION: 93.78
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 3B-21
REMARKS:			
DATE: December 19, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60	80			
							Δ	REMOULDED SHEAR STRENGTH (kPa)					
							▲	UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%)	WATER CONTENT (%)		LL (%)			
						20	40	60	80				
GROUND SURFACE													
For soil profile refer to BH 3-21													
		1									93		
		2									92		
		3									91		
Brown SILTY CLAY			TW 1	100									
		4									90		
Grey SILTY CLAY			TW 2	100									
		5									89		
End of Borehole		6									88		
		7									87		
		8									86		
		9									85		
		10									84		

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COORD. SYS.: UTM ZONE 18	EASTING: 435936.41	NORTHING: 5004099.07	ELEVATION: 93.87
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 4-24
REMARKS:			
DATE: December 16, 2024			



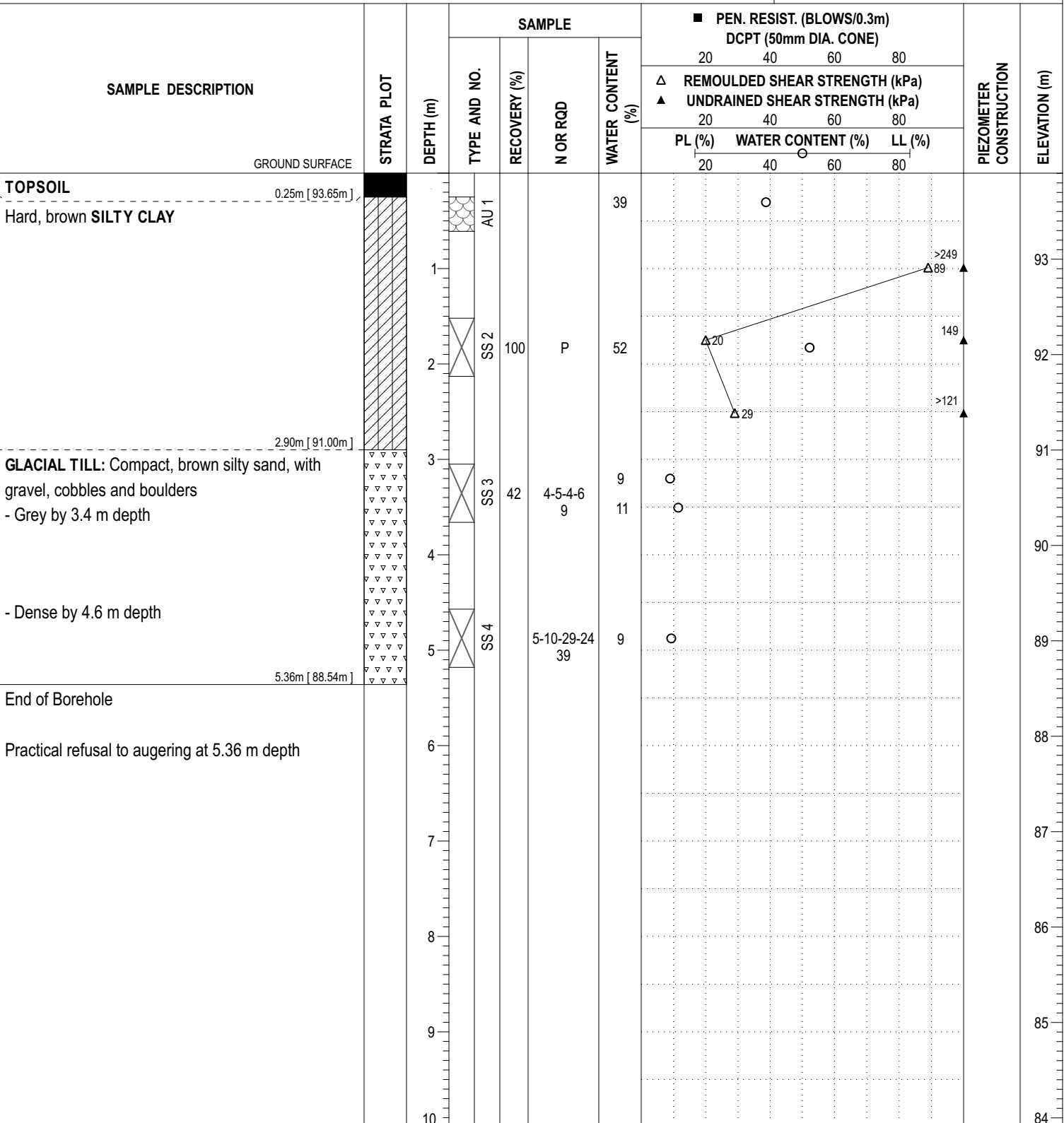
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COORD. SYS.: UTM ZONE 18	EASTING: 435936.41	NORTHING: 5004099.07	ELEVATION: 93.87
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 4A-24
REMARKS:			
DATE: December 19, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%)	WATER CONTENT (%)	LL (%)			
20	40	60	80									
GROUND SURFACE												
For soil profile refer to BH 4-24												
		1								93		
		2								92		
		3								91		
2.90m [90.97m] Brown SILTY CLAY			TW 1	100								
3.51m [90.36m] End of Borehole		4								90		
		5								89		
		6								88		
		7								87		
		8								86		
		9								85		
		10								84		

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COORD. SYS.: UTM ZONE 18	EASTING: 435994.61	NORTHING: 5003956.27	ELEVATION: 93.90
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 5-24
REMARKS:			
DATE: December 16, 2024			



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COORD. SYS.: UTM ZONE 18	EASTING: 435422.68	NORTHING: 5003879.79	ELEVATION: 94.24
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 6-24
REMARKS:			DATE: December 17, 2024

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20 40 60 80					
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							20 40 60 80					
GROUND SURFACE						PL (%)	WATER CONTENT (%)		LL (%)			
						20 40 60 80						
TOPSOIL, trace clay			AU 1			39					94	
GLACIAL TILL: Compact, brown sandy clay, with silt, gravel, cobbles and boulders		1	SS 2	100	P	30					93	
		2	SS 3	50	3-3-3-2 6	14					92	
		3	SS 4	92	6-8-3-2 11	13					91	
		4									90	
GLACIAL TILL: Compact grey silty sand, with gravel, cobbles and boulders		5	SS 5	83	8-13-18-19 31	9					89	
		6									88	
GLACIAL TILL: Compact, grey sand, with cobbles and boulders			SS 6	100	2-3-16-43 19	19					88	
End of Borehole		7									87	
		8									86	
		9									85	
		10										

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COORD. SYS.: UTM ZONE 18	EASTING: 435600.21	NORTHING: 5004031.42	ELEVATION: 93.94
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH 7-24
REMARKS:			
DATE: December 17, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20 40 60 80				
							△ REMOULDED SHEAR STRENGTH (kPa)	▲ UNDRAINED SHEAR STRENGTH (kPa)	20 40 60 80		
GROUND SURFACE											
TOPSOIL											
GLACIAL TILL: Brown silty sand, with gravel, cobbles and boulders											
- Increasing clay content by 1.5 m depth											
- Increasing sand content by 3.0 m depth											
- Grey by 3.5 m depth											
End of Borehole											
Practical refusal to augering at 3.71 m depth											

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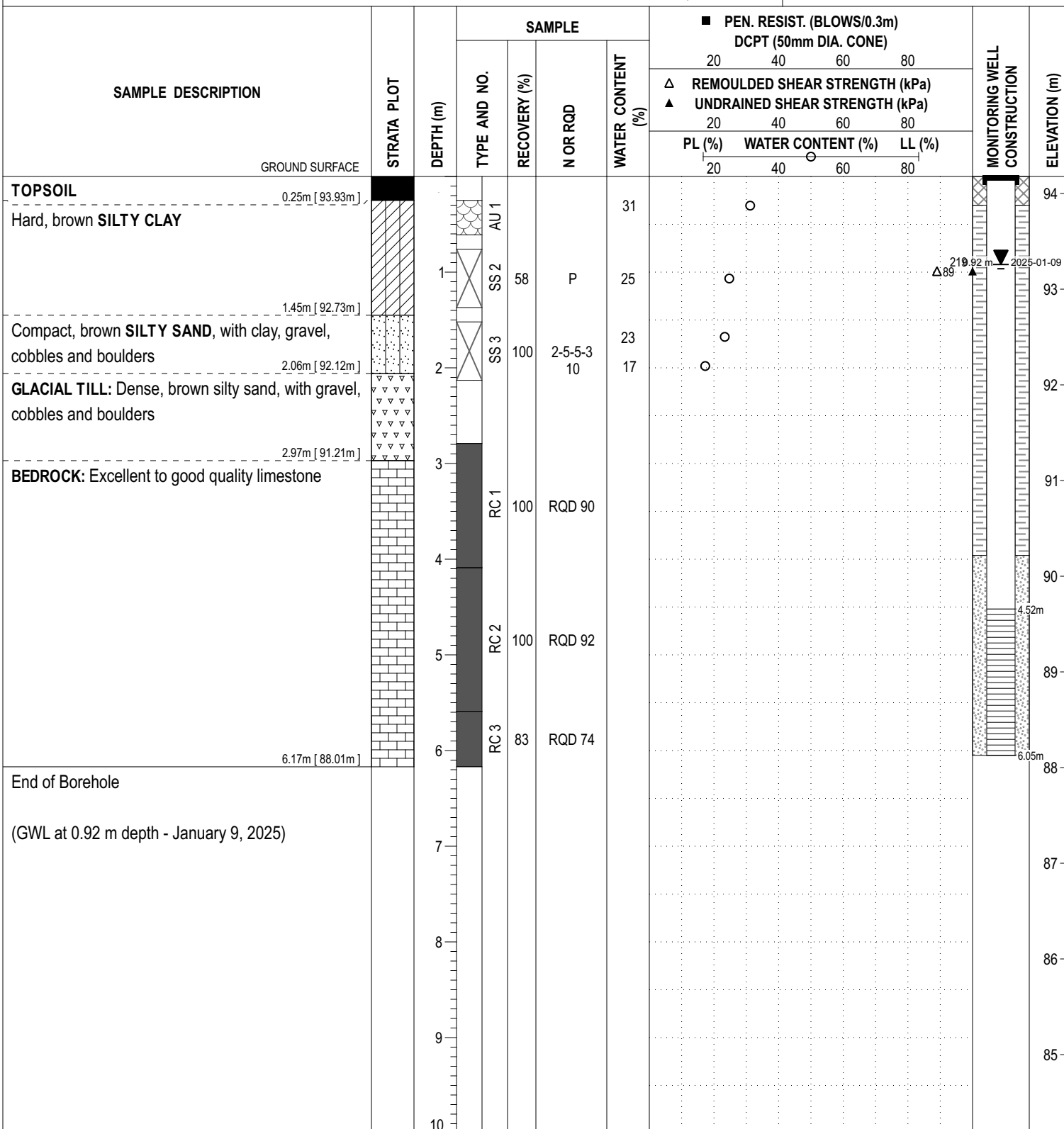


COORD. SYS.: UTM ZONE 18		EASTING: 435672.21	NORTHING: 5003946.75	ELEVATION: 94.52
PROJECT: Proposed Mixed-Use Development				FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig				HOLE NO. : BH 8-24
REMARKS:		DATE: December 17, 2024		

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE			■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)				
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40			60	80		
							△ REMOULDED SHEAR STRENGTH (kPa)	▲ UNDRAINED SHEAR STRENGTH (kPa)			20	40	60	80
							PL (%)	WATER CONTENT (%)			LL (%)	20	40	60
GROUND SURFACE														
TOPSOIL														
GLACIAL TILL: Compact, brown silty sand, with clay, gravel, cobbles and boulders			AU 1			22				94				
		1	SS 2	33	4-9-5-3 14	25								
		2	SS 3	67	6-5-4-6 9	14				93				
										92				
- Increasing sand content by 3.0 m depth		3	SS 4		12-50-/-/ 50/0.08	11				91				
End of Borehole		4								90				
Practical refusal to augering at 3.53 m depth		5								89				
		6								88				
		7								87				
		8								86				
		9												
		10								85				

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COORD. SYS.: UTM ZONE 18		EASTING: 435719.68	NORTHING: 5004015.59	ELEVATION: 94.18
PROJECT: Proposed Mixed-Use Development				FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig				HOLE NO. : BH 9-24
REMARKS:		DATE: December 17, 2024		



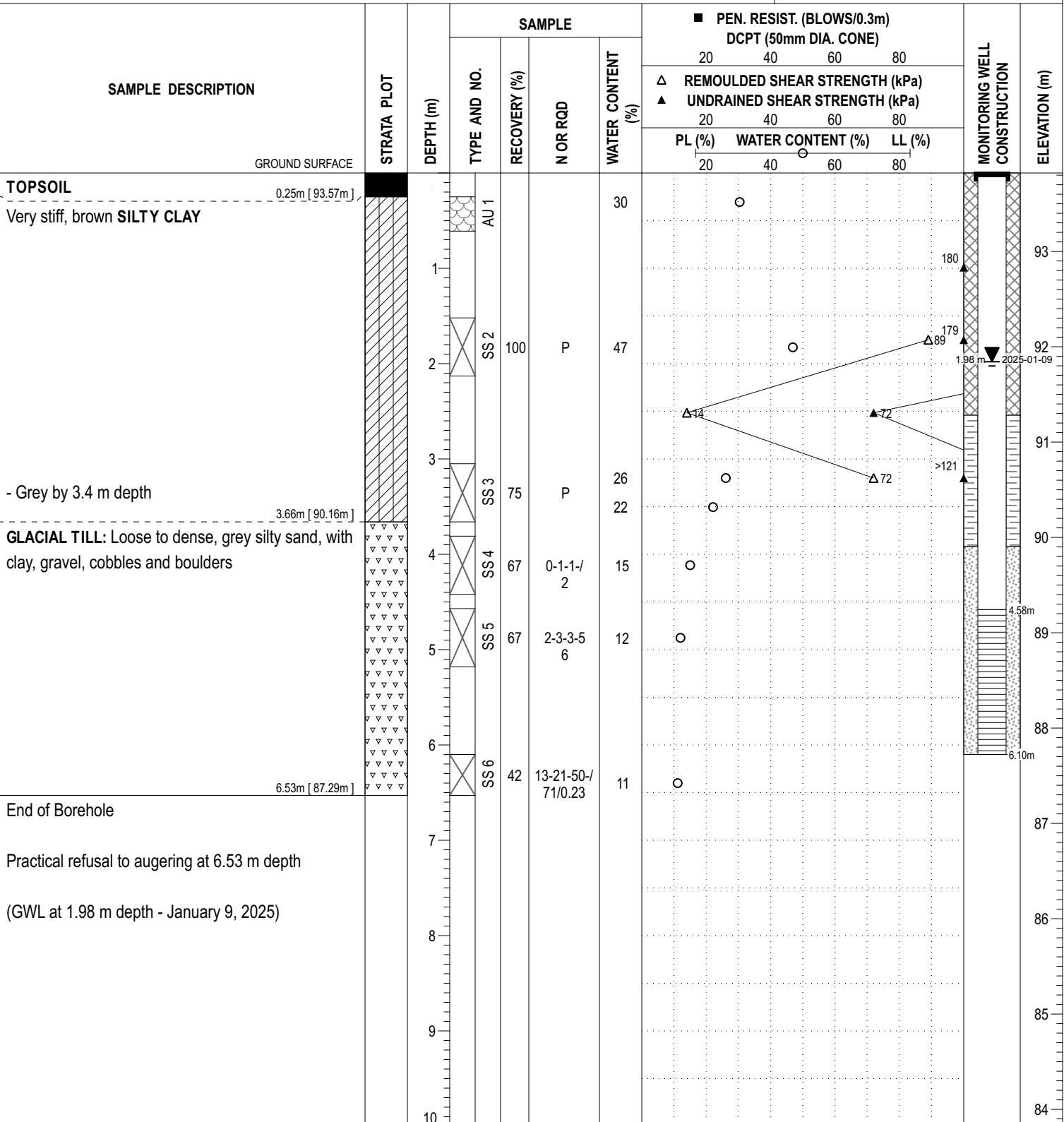
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COORD. SYS.: UTM ZONE 18	EASTING: 435719.68	NORTHING: 5004015.59	ELEVATION: 94.18
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH10-24
REMARKS:			
DATE: December 18, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			MONITORING WELL CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%) WATER CONTENT (%) LL (%)					
						20	40	60	80			
GROUND SURFACE												
For soil profile refer to BH 9-24												
		1								0.94 m	94	
		2									93	
		3									92	
2.97m [91.21m]		4									91	
End of Borehole												
Practical refusal to augering at 2.97 m depth		5									90	
(GWL at 0.94 m depth - January 9, 2025)		6									89	
		7									88	
		8									87	
		9									86	
		10									85	

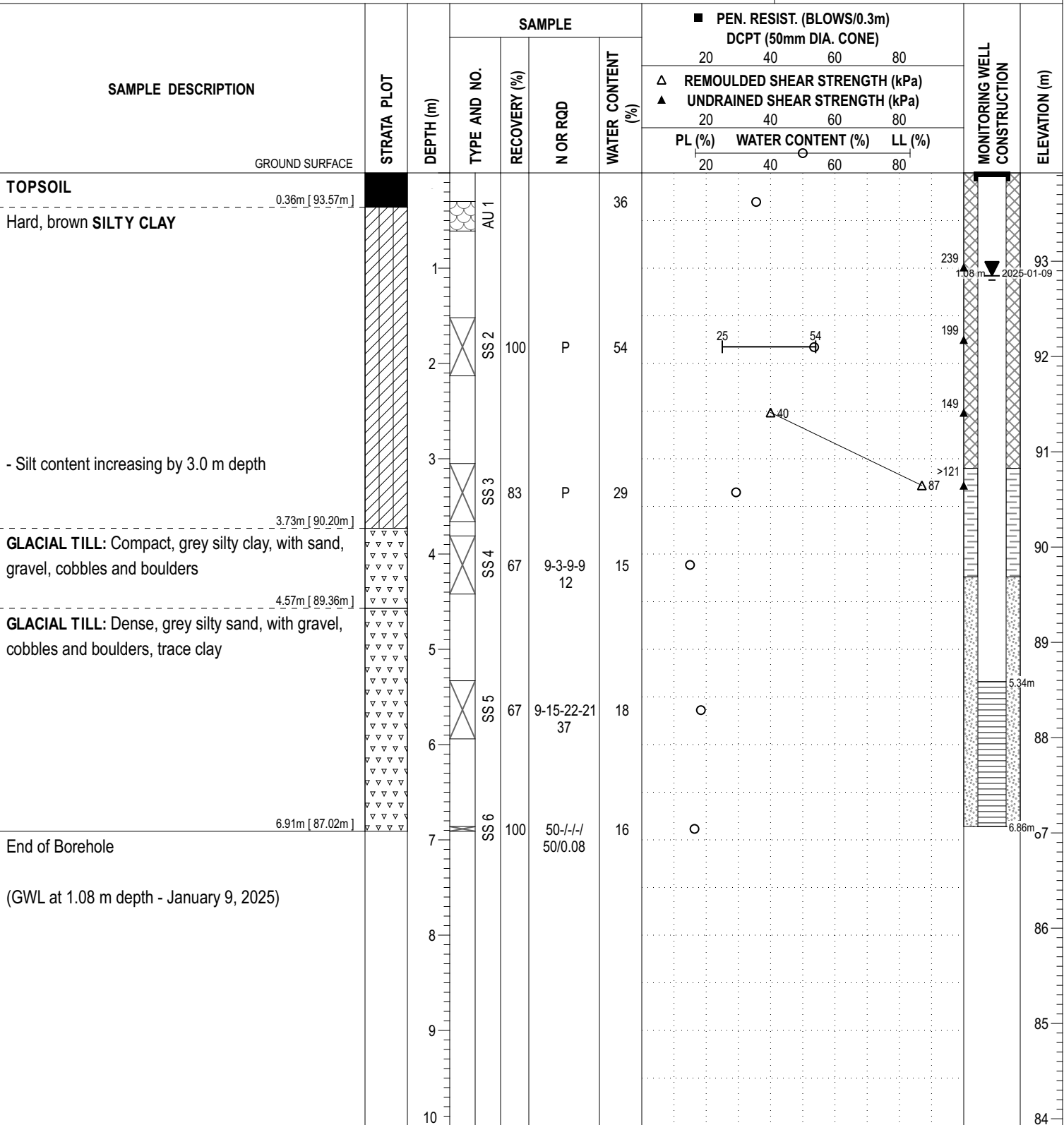
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COORD. SYS.: UTM ZONE 18	EASTING: 435692.30	NORTHING: 5004138.10	ELEVATION: 93.82
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH11-24
REMARKS:			
DATE: December 18, 2024			



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COORD. SYS.: UTM ZONE 18	EASTING: 435768.33	NORTHING: 5004247.28	ELEVATION: 93.93
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH12-24
REMARKS:			DATE: December 18, 2024



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COORD. SYS.: UTM ZONE 18	EASTING: 435783.20	NORTHING: 5004220.10	ELEVATION: 93.50
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH12A-24
REMARKS:			
DATE: December 19, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60	80		
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%)	WATER CONTENT (%)		LL (%)		

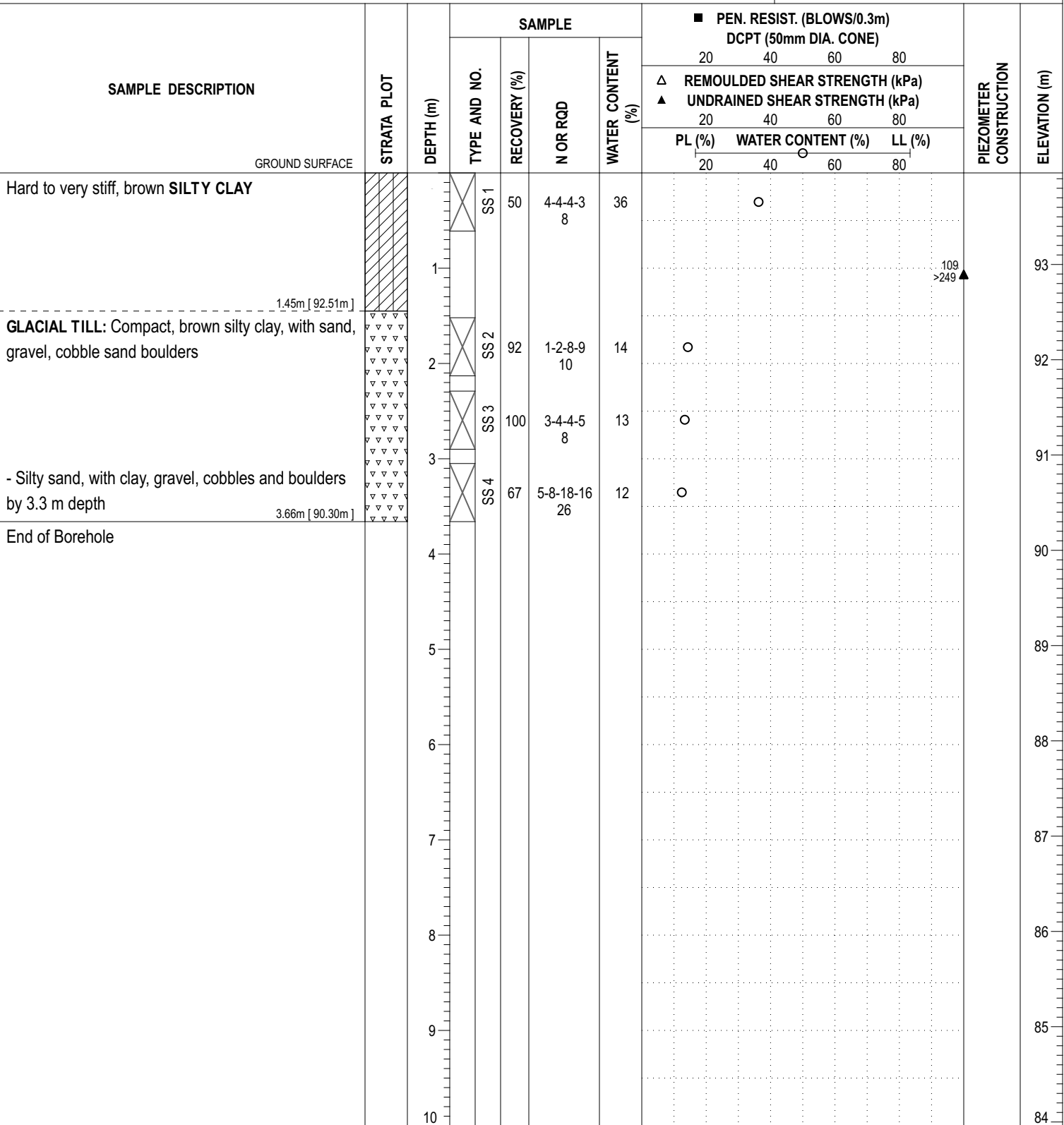
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COORD. SYS.: UTM ZONE 18	EASTING: 435783.20	NORTHING: 5004220.10	ELEVATION: 93.50
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH12B-24
REMARKS:			
DATE: December 19, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60	80		
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%) WATER CONTENT (%) LL (%)					
20 40 60 80												
20 40 60 80												
GROUND SURFACE												
For soil profile refer to BH 12-24												

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COORD. SYS.: UTM ZONE 18	EASTING: 435792.21	NORTHING: 5004358.09	ELEVATION: 93.96
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH13-24
REMARKS:			
DATE: December 19, 2024			

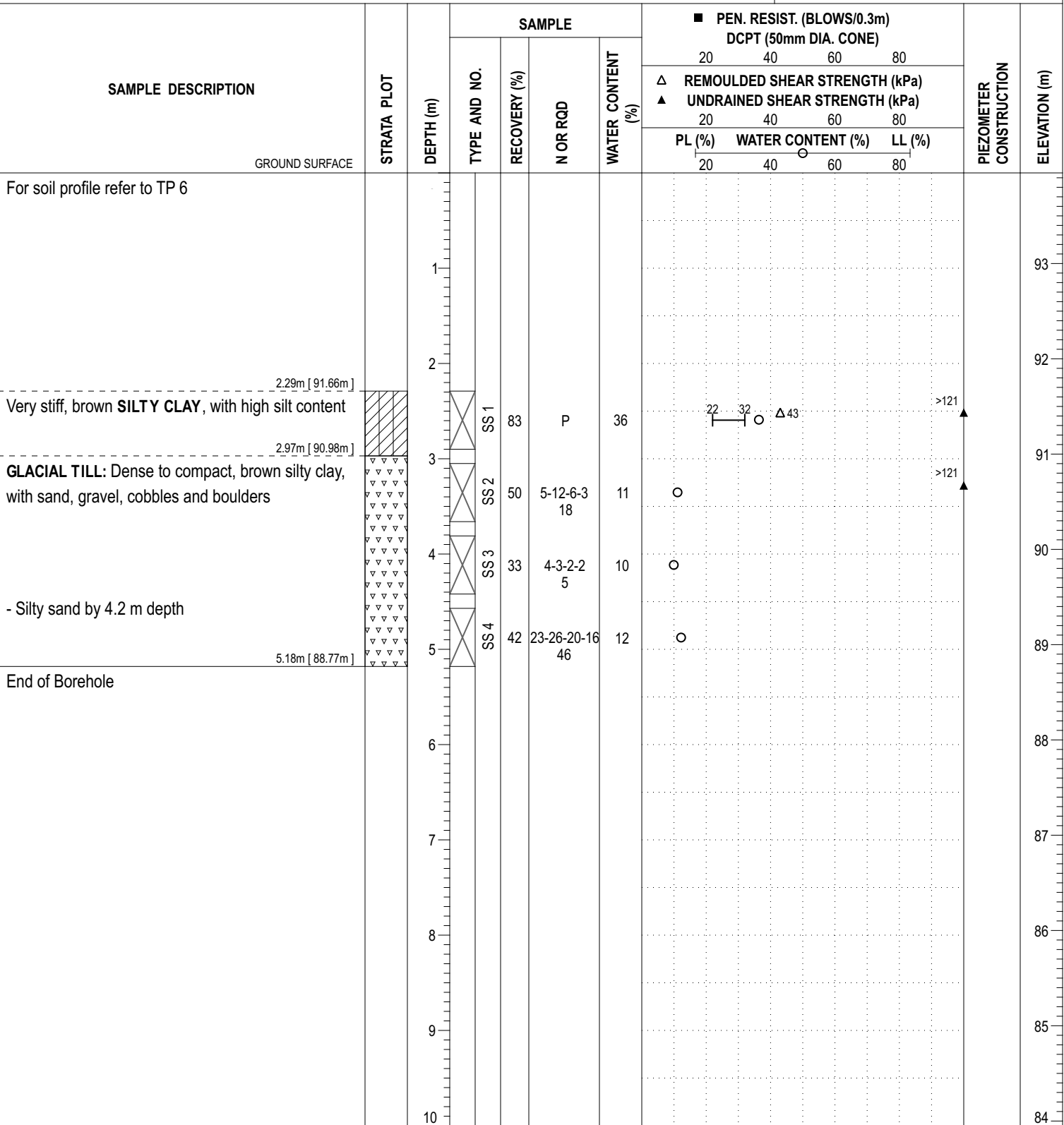


COORD. SYS.: UTM ZONE 18	EASTING: 435792.21	NORTHING: 5004358.09	ELEVATION: 93.96
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH13A-24
REMARKS:			
DATE: December 19, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%)	WATER CONTENT (%)				LL (%)
						20	40	60	80			
GROUND SURFACE												
For soil profile refer to BH 13-24												
Brown SILTY CLAY			TW 1	62								
End of Borehole		1									93	
		2									92	
		3									91	
		4									90	
		5									89	
		6									88	
		7									87	
		8									86	
		9									85	
		10									84	

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COORD. SYS.: UTM ZONE 18	EASTING: 436034.93	NORTHING: 5004037.45	ELEVATION: 93.95
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH14-24
REMARKS:			
DATE: December 19, 2024			



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COORD. SYS.: UTM ZONE 18	EASTING: 436034.93	NORTHING: 5004037.45	ELEVATION: 93.95
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Track Mounted Drill Rig			HOLE NO. : BH14A-24
REMARKS:			
DATE: December 19, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							PL (%)	WATER CONTENT (%)				LL (%)
											</	

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COORD. SYS.: UTM ZONE 18		EASTING: 436079.41	NORTHING: 5003734.86	ELEVATION: 94.53
PROJECT: Proposed Mixed-Use Development ADVANCED BY: Excavator				FILE NO. : PG4216
REMARKS:				HOLE NO. : TP 1-24
DATE: December 16, 2024				

[illegible]

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COORD. SYS.: UTM ZONE 18	EASTING: 435766.77	NORTHING: 5003865.03	ELEVATION: 95.89
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Excavator			HOLE NO. : TP 2-24
REMARKS:			DATE: December 16, 2024

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)	△ UNDRAINED SHEAR STRENGTH (kPa)				
							20	40	60			80
							PL (%)	WATER CONTENT (%)	LL (%)			
20	40	60	80									
GROUND SURFACE												
TOPSOIL												
GLACIAL TILL: Compact to very dense, brown silty sand, some gravel, cobbles and boulders												

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COORD. SYS.: UTM ZONE 18		EASTING: 435639.53	NORTHING: 5003762.56	ELEVATION: 97.15
PROJECT: Proposed Mixed-Use Development				FILE NO. : PG4216
ADVANCED BY: Excavator				HOLE NO. : TP 3-24
REMARKS:				
DATE: December 16, 2024				

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)	PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20 40 60 80		
							△ REMOULDED SHEAR STRENGTH (kPa)		
							▲ UNDRAINED SHEAR STRENGTH (kPa)		
							PL (%) WATER CONTENT (%) LL (%)		
GROUND SURFACE						20 40 60 80			
TOPSOIL									97 -
0.20m [96.95m]									
GLACIAL TILL: Compact, brown silty sand, some gravel and cobbles, occasional boulders									
		</							

DISCLAIMER: THE DATA PRESENTED IN THIS SHEET IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHOM IT WAS PRODUCED. THIS SHEET SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.



COORD. SYS.: UTM ZONE 18		EASTING: 435453.45	NORTHING: 5003767.76	ELEVATION: 94.94
PROJECT: Proposed Mixed-Use Development				FILE NO. : PG4216
ADVANCED BY: Excavator				HOLE NO. : TP 4-24
REMARKS:				
DATE: December 16, 2024				

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)	PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20 40 60 80		
							Δ REMOULDED SHEAR STRENGTH (kPa)		
							▲ UNDRAINED SHEAR STRENGTH (kPa)		
							PL (%) WATER CONTENT (%) LL (%)		
							20 40 60 80		
GROUND SURFACE									
TOPSOIL									
0.25m [94.69m]									
Compact, brown SILT , trace clay and sand			G 1			33			94-
1.00m [93.94m]		1							
GLACIAL TILL: Loose, brown silt, some clay and gravel									
2.05m [92.89m]		2	G 2			26			93-
End of Test Pit									
Test pit terminated on bedrock surface									
Groundwater infiltration was observed at 1.30 m depth		3							92-
		4							91-
		5							90-

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COORD. SYS.: UTM ZONE 18		EASTING: 435320.86	NORTHING: 5003704.85	ELEVATION: 94.65
PROJECT: Proposed Mixed-Use Development				FILE NO. : PG4216
ADVANCED BY: Excavator				HOLE NO. : TP 5-24
REMARKS:				
DATE: December 16, 2024				

[illegible]

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COORD. SYS.: UTM ZONE 18	EASTING: 435449.30	NORTHING: 5003642.99	ELEVATION: 95.32
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Excavator			HOLE NO. : TP 6-24
REMARKS:			
DATE: December 16, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
				20	40	60	80	PL (%) WATER CONTENT (%) LL (%)				
				20	40	60	80					
GROUND SURFACE												
TOPSOIL												
GLACIAL TILL: Compact, brown silty sand, with gravel, cobbles and boulders												
End of Test Pit												
Test pit terminated on bedrock surface												
No groundwater infiltration was observed upon completion of the test pit												
	</											

DISCLAIMER: THE DATA PRESENTED IN THIS SHEET IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHOM IT WAS PRODUCED. THIS SHEET SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.

COORD. SYS.: UTM ZONE 18	EASTING: 435565.10	NORTHING: 5003574.64	ELEVATION: 96.93
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Excavator			HOLE NO. : TP 7-24
REMARKS:			
DATE: December 16, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)		
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80	
							△ REMOULDED SHEAR STRENGTH (kPa)	20	40			60	80
							▲ UNDRAINED SHEAR STRENGTH (kPa)	20	40			60	80
							PL (%)	WATER CONTENT (%)	LL (%)			20	40
GROUND SURFACE													
TOPSOIL													
GLACIAL TILL: Compact, brown silty sand, some gravel and cobbles, occasional boulders													

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COORD. SYS.: UTM ZONE 18		EASTING: 435618.80	NORTHING: 5003684.74	ELEVATION: 97.72
PROJECT: Proposed Mixed-Use Development				FILE NO. : PG4216
ADVANCED BY: Excavator				HOLE NO. : TP 8-24
REMARKS:				
DATE: December 16, 2024				

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
PL (%)	WATER CONTENT (%)		LL (%)									
20	40	60	80									
GROUND SURFACE												
TOPSOIL												
0.25m [97.47m]												
GLACIAL TILL: Compact, brown silty sand, some gravel, cobbles and boulders												
								</				

DISCLAIMER: THE DATA PRESENTED IN THIS SHEET IS THE PROPERTY OF PATERSON GROUP AND THE CLIENT FOR WHOM IT WAS PRODUCED. THIS SHEET SHOULD BE READ IN CONJUNCTION WITH ITS CORRESPONDING REPORT. PATERSON GROUP IS NOT RESPONSIBLE FOR THE UNAUTHORIZED USE OF THIS DATA.

COORD. SYS.: UTM ZONE 18	EASTING: 435764.56	NORTHING: 5003694.37	ELEVATION: 95.94
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Excavator			HOLE NO. : TP 9-24
REMARKS:			DATE: December 16, 2024

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							20	40	60	80		
							PL (%)	WATER CONTENT (%)		LL (%)		
							20	40	60	80		
GROUND SURFACE												
TOPSOIL												
0.15m [95.79m]												
GLACIAL TILL: Compact, brown silty sand, some gravel, cobbles and boulders												

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COORD. SYS.: UTM ZONE 18	EASTING: 435773.40	NORTHING: 5003504.87	ELEVATION: 97.38
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Excavator			HOLE NO. : TP10-24
REMARKS:			DATE: December 16, 2024

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60	80		
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							20	40	60	80		
PL (%)		WATER CONTENT (%)		LL (%)								
20		40		60		80						

GROUND SURFACE												
TOPSOIL												
0.20m [97.18m]												
GLACIAL TILL: Compact, brown silty sand, some gravel, cobbles and boulders												

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COORD. SYS.: UTM ZONE 18	EASTING: 435928.90	NORTHING: 5003701.30	ELEVATION: 94.87
PROJECT: Proposed Mixed-Use Development	FILE NO. : PG4216		
ADVANCED BY: Excavator	HOLE NO. : TP11-24		
REMARKS:	DATE: December 16, 2024		

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)				PIEZOMETER CONSTRUCTION	ELEVATION (m)
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60	80		
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							20	40	60	80		
PL (%)	WATER CONTENT (%)		LL (%)									
20	40	60	80									
GROUND SURFACE												
TOPSOIL												
0.25m [94.62m]												
GLACIAL TILL: Compact, brown silty sand, clay, with gravel, some cobbles, boulders and clay			G 1			13						94
		1										
			G 2			14						
1.80m [93.07m]												
End of Test Pit		2										93
Test pit terminated on bedrock surface												
No groundwater infiltration was observed upon completion of the test pit		3										92
		4										91
		5										90

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Supplemental Geotechnical Investigation

5970 and 6038 Ottawa Street, Ottawa, Ontario

ELEVATION: 95.92

FILE NO. : PG4216

HOLE NO.: TP12-24

DATE: December 16, 2024

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COORD. SYS.: UTM ZONE 18	EASTING: 435577.21	NORTHING: 5003410.85	ELEVATION: 96.69
PROJECT: Proposed Mixed-Use Development			FILE NO. : PG4216
ADVANCED BY: Excavator			HOLE NO. : TP13-24
REMARKS:			
DATE: December 16, 2024			

SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	SAMPLE				PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)			PIEZOMETER CONSTRUCTION	ELEVATION (m)	
			TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20	40	60			80
							△ REMOULDED SHEAR STRENGTH (kPa)					
							▲ UNDRAINED SHEAR STRENGTH (kPa)					
							20	40	60			80
PL (%)		WATER CONTENT (%)		LL (%)								
20	40	60	80									
GROUND SURFACE												
TOPSOIL												
0.25m [96.44m]												
GLACIAL TILL: Compact, brown silty sand, some gravel, occasional cobbles and boulders			G 1			21						
0.40m [96.29m]												
Weathered BEDROCK										96		
0.60m [96.09m]												
End of Test Pit												
Test pit terminated on bedrock surface		1										
No groundwater infiltration was observed upon completion of the test pit		2								95		
		3								94		
		4								93		
		5								92		

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SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Residential Development - Eagleson Road
Ottawa, Ontario**

PG4216

HOLE NO.

BH 1D-21

DATE: 2021 March 19

[illegible]

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows / 0.3m ● 50 mm Dia. Cone				MONITORING WELL CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
GROUND SURFACE								20	40	60	80	
TOPSOIL	0.30					0	94.24					
Very stiff, brown SILTY CLAY						1	93.24					
1.83												
GLACIAL TILL: Compact silt, some sand and clay, trace gravel	2.13					2	92.24					
End of Borehole												
(GWL @ 0.16m - March 31, 2021)												

20406080100

Shear Strength (kPa)

▲ Undisturbed △ Remoulded

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Residential Development - Eagleson Road
Ottawa, Ontario**

EASTING:

NORTHING:

ELEVATION: 93.908

FILE NO.

PG4216

DATUM: Geodetic

REMARKS:

HOLE NO.

BORINGS BY: CME 75 Power Auger

DATE: 2021 March 19

BH 2-21

[illegible]

EASTING:

NORTHING:

ELEVATION: 92.787

FILE NO.

PG4216

DATUM: Geodetic

REMARKS:

HOLE NO.

BH 3-21

BORINGS BY: CME 75 Power Auger

DATE: 2021 March 19

[illegible]

EASTING: NORTHING: ELEVATION: 94.514

DATUM: Geodetic

REMARKS:

BORINGS BY: CME 55 Power Auger

DATE: 2021 March 22

FILE NO. **PG4216**

HOLE NO. **BH 4D-21**

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows / 0.3m ● 50 mm Dia. Cone				MONITORING WELL CONSTRUCTION	
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %					
								20	40	60	80		
GROUND SURFACE						0	94.51						
TOPSOIL	0.18												
Very stiff to stiff, brown SILTY CLAY , trace sand		AU	1										
		SS	2	58	5	1	93.51						
		SS	3	50	11	2	92.51						
	2.36												
GLACIAL TILL: Brown silty clay with sand, gravel, cobbles and boulders		SS	4	58	20								
		SS	5	8	+50	3	91.51						
	3.38												
End of Borehole													
Practical refusal to augering at 3.38m depth													
(GWL @ 0.29m - March 31, 2021)													

EASTING:

NORTHING:

ELEVATION: 94.514

FILE NO.

PG4216

DATUM: Geodetic

REMARKS:

HOLE NO.

BH 4S-21

BORINGS BY: CME 55 Power Auger

DATE: 2021 March 22

[illegible]

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Residential Development - Eagleson Road
Ottawa, Ontario**

PG4216

HOLE NO.

BH 5-21

DATE: 2021 March 22

[illegible]

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Residential Development - Eagleson Road
Ottawa, Ontario**

PG4216

HOLE NO.

BH 6-21

DATE: 2021 March 22

[illegible]

EASTING:

NORTHING:

ELEVATION: 93.623

FILE NO.

PG4216

DATUM: Geodetic

REMARKS:

HOLE NO.

BH 7D-21

BORINGS BY: CME 75 Power Auger

DATE: 2021 March 23

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows / 0.3m ● 50 mm Dia. Cone				MONITORING WELL CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
GROUND SURFACE						0	93.62					
Very stiff to stiff, brown SILTY CLAY		AU	1									
		SS	2	33	9	1	92.62					
		SS	3	17	4							
		SS	4	50	6	2	91.62					
		SS	5	67	6	3	90.62					
GLACIAL TILL: Brown silty clay with sand, gravel, cobbles and boulders												
End of Borehole												

3.35

3.66

20

40

60

80

100

Shear Strength (kPa)

▲ Undisturbed △ Remoulded

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Residential Development - Eagleson Road
Ottawa, Ontario**

EASTING:

NORTHING:

ELEVATION: 93.623

FILE NO.

PG4216

DATUM: Geodetic

REMARKS:

HOLE NO.

BH 7S-21

BORINGS BY: CME 55 Power Auger

DATE: 2021 March 23

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows / 0.3m ● 50 mm Dia. Cone				MONITORING WELL CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
GROUND SURFACE								20	40	60	80	
Very stiff to stiff, brown SILTY CLAY						0	93.62					
						1	92.62					
						2	91.62					
End of Borehole	2.13											

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Residential Development - Eagleson Road
Ottawa, Ontario**

PG4216

HOLE NO.

BH 8-21

DATE: 2021 March 23

[illegible]

EASTING: NORTHING: ELEVATION: 94.209

DATUM: Geodetic

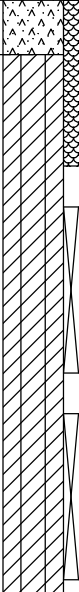
REMARKS:

BORINGS BY: CME 55 Power Auger

DATE: 2021 March 23

FILE NO. **PG4216**

HOLE NO. **BH 9-21**

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows / 0.3m ● 50 mm Dia. Cone				MONITORING WELL CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
GROUND SURFACE												
TOPSOIL	0.20					0	94.21					
Very stiff to stiff, brown SILTY CLAY		AU	1									
		SS	2	100	2	1	93.21					
		SS	3	75	2	2	92.21					
GLACIAL TILL: Compact, brown silty sand with gravel, clay, cobbles and boulders		SS	4	50	23							
		SS	5	8	+50	3	91.21					
End of Borehole	3.10											
(GWL @ 0.68m - March 31, 2021)												
								20	40	60	80	100
								Shear Strength (kPa)				
								▲ Undisturbed △ Remoulded				

[illegible]

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Residential Development - Eagleson Road
Ottawa, Ontario**

EASTING:

NORTHING:

ELEVATION: 92.938

FILE NO.

PG4216

DATUM: Geodetic

REMARKS:

HOLE NO.

BH11-21

BORINGS BY: CME 55 Power Auger

DATE: 2021 March 23

[illegible]

[illegible]

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.42
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: Hydraulic Shovel DATE: 2019 February 27

FILE NO. PG4216
HOLE NO. TP 2

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface						0	94.42					
TOPSOIL												
	0.30											
Very stiff, grey-brown SILTY CLAY - some rootlets at upper 0.2m depth - grey by 1.0m depth		G	1			1	93.42					>130
		G	2									>130
		G	3			2	92.42					
		G	4									
GLACIAL TILL: Grey silty clay with sand, gravel, cobbles and boulders		G	5			3	91.42					
		G	6									
		G	7									
		G	7									
End of Test Pit (TP dry upon completion)	3.70											
								20	40	60	80	100
								Shear Strength (kPa)				
								▲ Undisturbed △ Remoulded				

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario**

PG4216

TP 3

DATE: 2019 February 27

TP 3

[illegible]

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 93.63
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: Hydraulic Shovel DATE: 2019 February 27

FILE NO. **PG4216**
HOLE NO. **TP 4**

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
Ground Surface						0	93.63	20	40	60	80	
TOPSOIL												
	0.30											
Very stiff, brown SILTY CLAY		G	1									>130
		G	2			1	92.63					>130
		G	3									>130
		G	4			2	91.63					
		G	5									
		G	6			3	90.63					
		G	7									
- grey by 2.8m depth												
	3.70											
End of Test Pit												
(Groundwater infiltration at 1.0m depth)												
								20	40	60	80	100
								Shear Strength (kPa)				
								▲ Undisturbed △ Remoulded				

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario

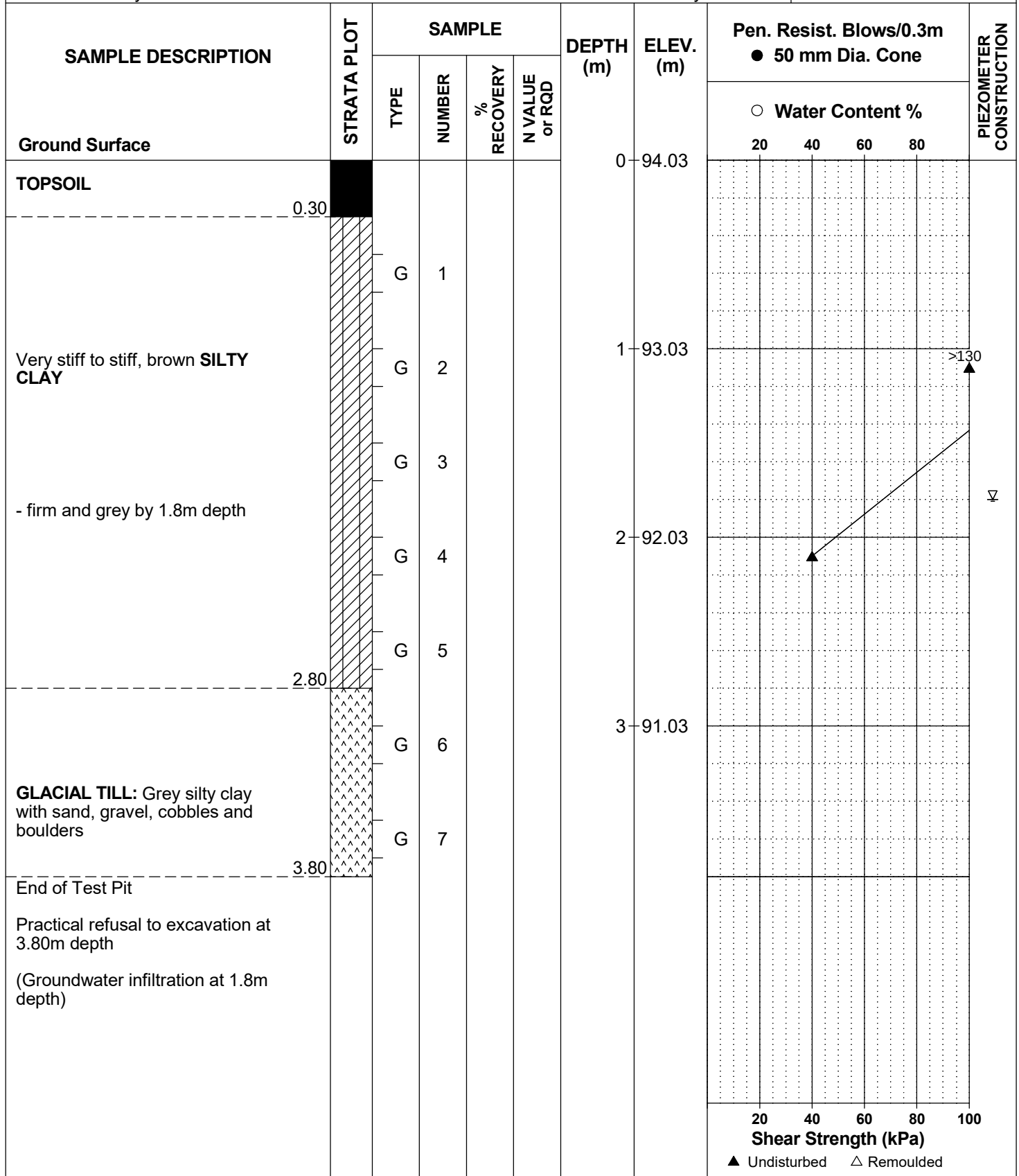
EASTING: NORTHING: ELEVATION: 94.28
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: Hydraulic Shovel DATE: 2019 February 27

FILE NO. PG4216
HOLE NO. TP 5

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface						0	94.28					
TOPSOIL												
	0.30											
Very stiff, brown SILTY CLAY - grey-brown by 1.0m depth - grey by 1.8m depth		G	1									>130
		G	2			1	93.28					>130
		G	3									▽
		G	4			2	92.28					
		G	5									
		G	6			3	91.28					
		G	7									
	3.50											
End of Test Pit												
Practical refusal to excavation at 3.50m depth												
(Groundwater infiltration at 1.2m depth)												
								20	40	60	80	100
								Shear Strength (kPa)				
								▲ Undisturbed △ Remoulded				

EASTING: NORTHING: ELEVATION: 94.03
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: Hydraulic Shovel DATE: 2019 February 27

FILE NO. **PG4216**
HOLE NO. **TP 6**



EASTING: NORTHING: ELEVATION: 94.46
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: Hydraulic Shovel DATE: 2019 February 27

FILE NO. **PG4216**
HOLE NO. **TP 7**

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface						0	94.46					
TOPSOIL												
	0.30											
Very stiff, brown SILTY CLAY		G	1									
		G	2			1	93.46					
		G	3									
		G	4				2	92.46				
		G	5									
- grey and with sand by 1.8m depth												
	3.00					3	91.46					
Grey CLAYEY SILT	3.20	G	6									
GLACIAL TILL: Grey silty clay with sand, gravel, cobbles and boulders												
	3.70	G	7									
End of Test Pit												
Practical refusal to excavation at 3.70m depth												
(Groundwater infiltration at 1.8m depth)												

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.53
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: Hydraulic Shovel DATE: 2019 February 27

FILE NO. **PG4216**
HOLE NO. **TP 8**

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface						0	94.53					
TOPSOIL												
	0.30											
Very stiff, brown SILTY CLAY - grey-brown by 0.8m depth - grey by 1.4m depth		G	1									
		G	2			1	93.53					
		G	3									
		G	4			2	92.53					
		G	5									
	2.80											
GLACIAL TILL: Grey silty clay with sand, gravel, cobbles and boulders		G	6			3	91.53					
		G	7									
	3.80											
End of Test Pit												
Practical refusal to excavation at 3.80m depth												
(Groundwater infiltration at 1.9m depth)												
								20	40	60	80	100
								Shear Strength (kPa)				
								▲ Undisturbed △ Remoulded				

PG4216

TP 9

DATE: 2019 February 27

TP 9

[illegible]

SOIL PROFILE AND TEST DATA

FILE NO. PG4216

HOLE NO. **TP 9A**

DATE: 2019 February 27

[illegible]

FILE NO.	PG4216
HOLE NO.	TP10

[illegible]

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.45
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: Hydraulic Shovel DATE: 2019 February 27

FILE NO. PG4216
HOLE NO. TP11

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface						0	94.45					
FILL: Sand with cobbles	0.25											
GLACIAL TILL: Loose to compact, grey-brown silty sand with clay, gravel, cobbles and boulders		G	1			1	93.45					
		G	2									
		G	3			2	92.45					
		G	4									
End of Test Pit	2.80											
Practical refusal to excavation at 2.80m depth												
(TP dry upon completion)												
								20	40	60	80	100
								Shear Strength (kPa)				
								▲ Undisturbed △ Remoulded				

EASTING:

NORTHING:

ELEVATION: 93.96

FILE NO.

PG4216

DATUM: Ground surface elevations were referenced to a geodetic datum.

REMARKS:

HOLE NO.

BH 1

BORINGS BY: CME 55 Power Auger

DATE: 2018 December 13

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface						0	93.96					
TOPSOIL	0.20	AU	1									
Loose, brown SANDY SILT, some clay	1.22	SS	2	54	5	1	92.96					
Dense to compact, brown SILTY SAND, trace gravel		SS	3	67	39	2	91.96					
		SS	4	71	23							
		SS	5	58	14	3	90.96					
		SS	6	71	32	4	89.96					
		SS	7		24	5	88.96					
		SS	8	100	3							
		SS	9	67	42	6	87.96					
GLACIAL TILL: Dense to very dense, grey silty sand with gravel and clay	5.94					7	86.96					
		SS	10	91	82	8	85.96					
		SS	11		7	9	84.96					
End of Borehole	9.75											
(GWL @ 0.61m - Dec. 28, 2018)												

20 40 60 80 100

Shear Strength (kPa)

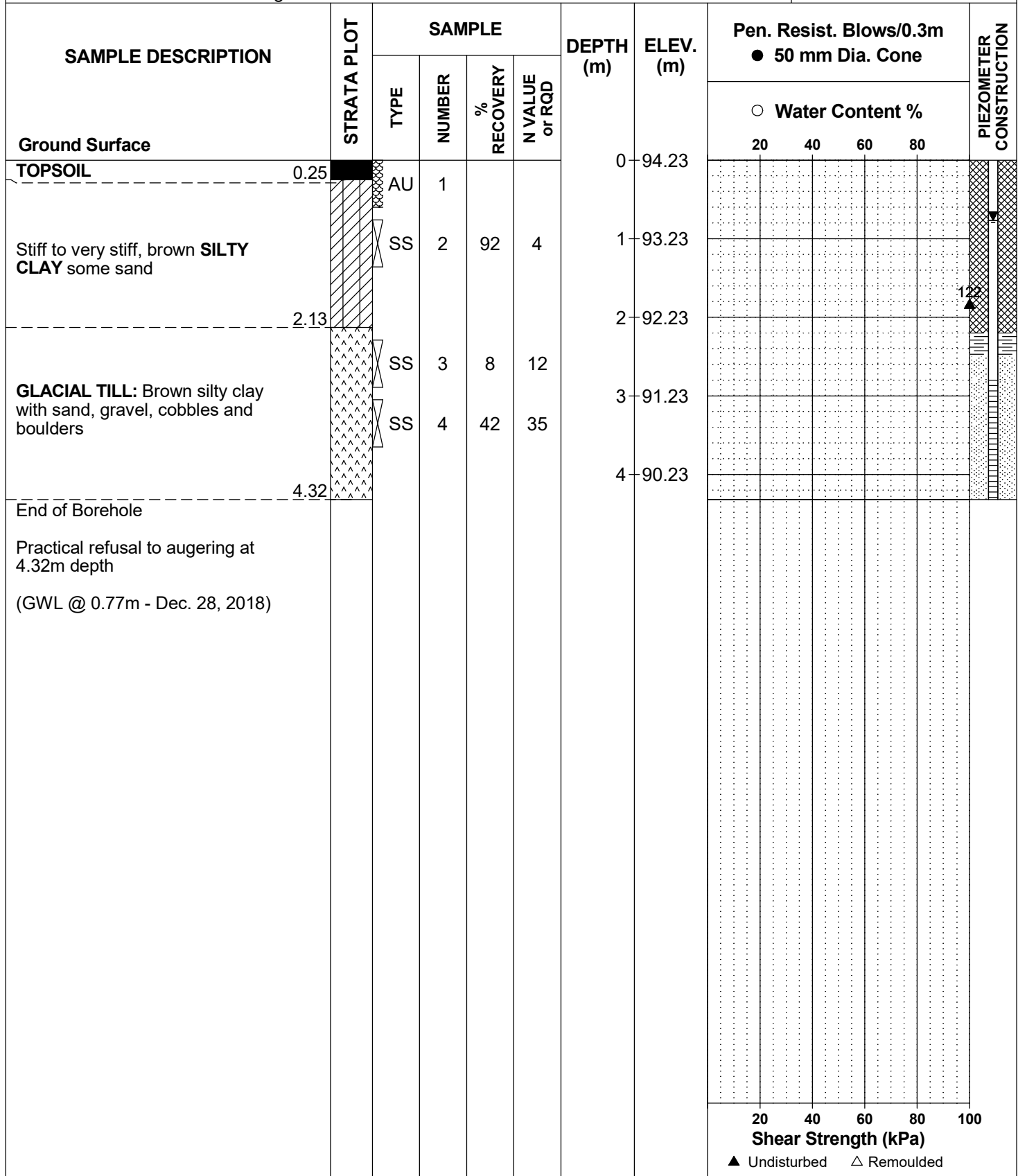
▲ Undisturbed △ Remoulded

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.23
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: CME 55 Power Auger DATE: 2018 December 13

FILE NO. PG4216
HOLE NO. BH 2



EASTING:

NORTHING:

ELEVATION: 94.76

FILE NO.

PG4216

DATUM: Ground surface elevations were referenced to a geodetic datum.

REMARKS:

HOLE NO.

BH 3

BORINGS BY: CME 55 Power Auger

DATE: 2018 December 13

SAMPLE DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				PIEZOMETER CONSTRUCTION
		TYPE	NUMBER	% RECOVERY	N VALUE or RQD			○ Water Content %				
								20	40	60	80	
Ground Surface												
TOPSOIL	0.15	AU	1			0	94.76					
Stiff to firm, brown SILTY CLAY		SS	2	92	6	1	93.76					
		SS	3	92	6	2	92.76					
	2.13	SS	4	0	50+							
GLACIAL TILL: Brown sandy silt with clay, gravel, cobbles and boulders		SS	5	67	23	3	91.76					
	4.42					4	90.76					
End of Borehole												
Practical refusal to augering at 4.42m depth												
(GWL @ 0.62m - Dec. 28, 2018)												

20406080100

Shear Strength (kPa)

▲ Undisturbed △ Remoulded

[illegible]

SOIL PROFILE AND TEST DATA

**Geotechnical Investigation
Proposed Development - Eagleson Road at Ottawa St.
Ottawa, Ontario**

FILE NO. **PG4216**

REMARKS:

HOLE NO. **BH 5**

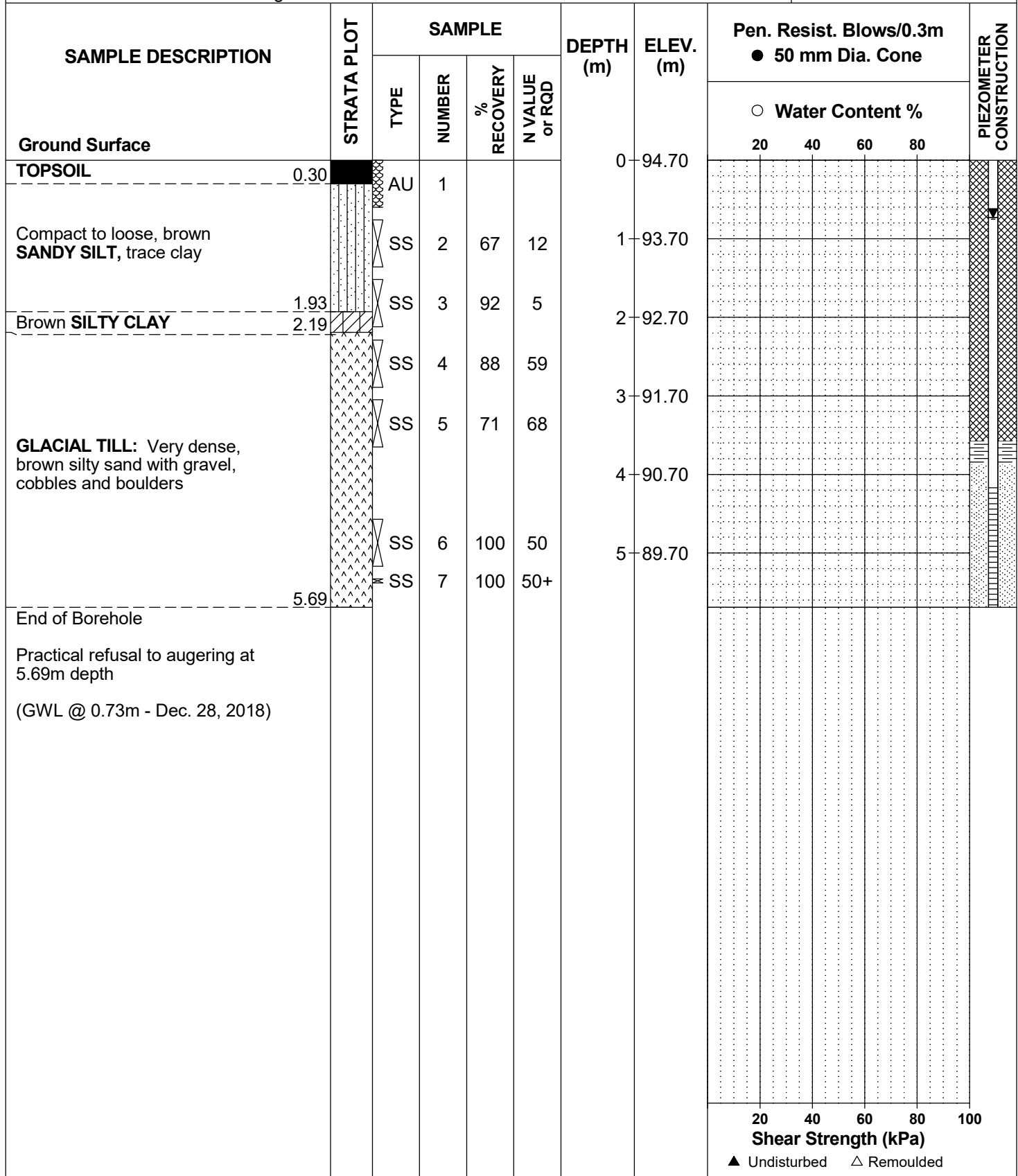
BORINGS BY: CME 55 Power Auger

DATE: 2018 December 13

[illegible]

EASTING: NORTHING: ELEVATION: 94.70
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: CME 55 Power Auger DATE: 2018 December 13

FILE NO. PG4216
HOLE NO. BH 6

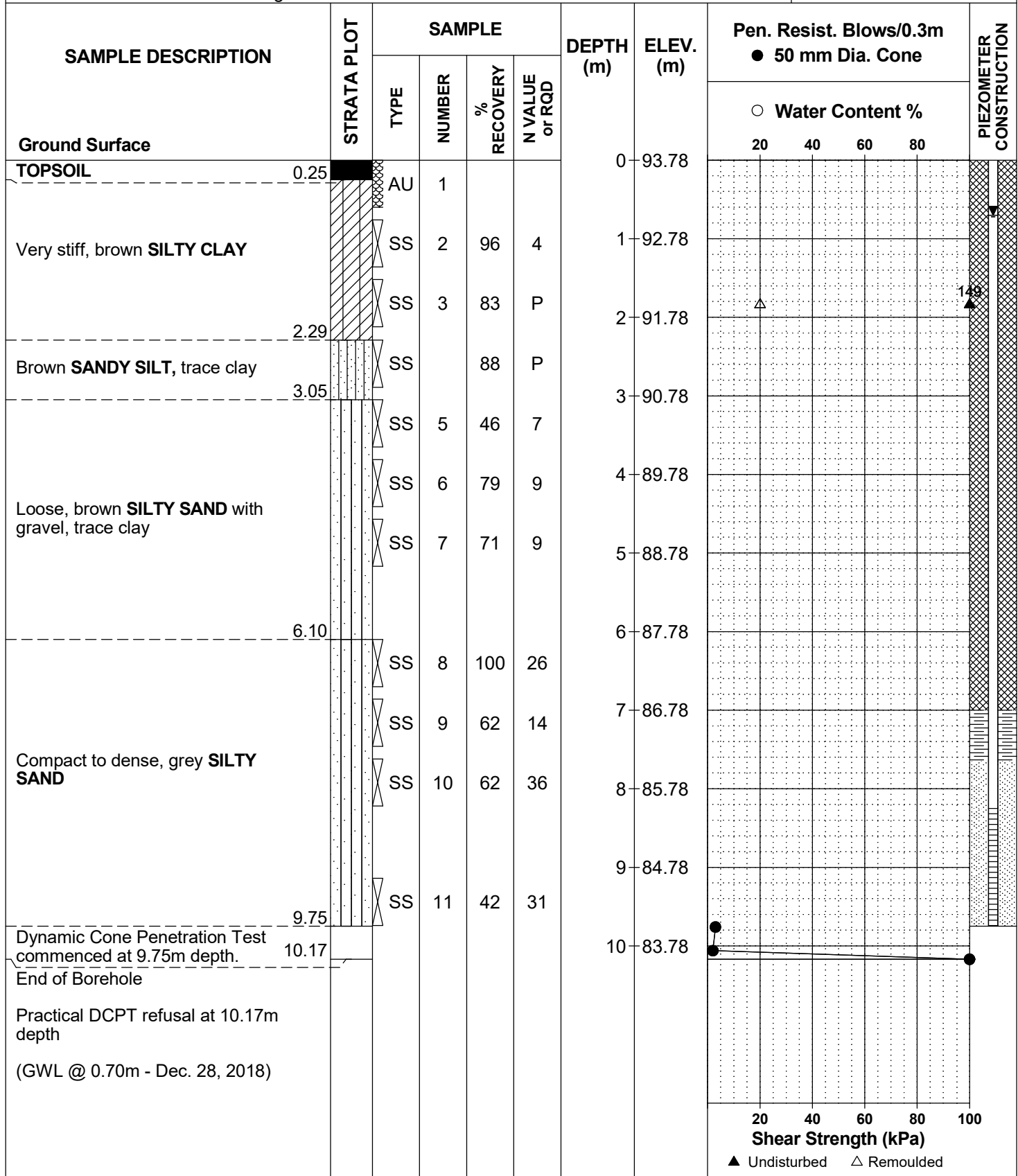


[illegible]

[illegible]

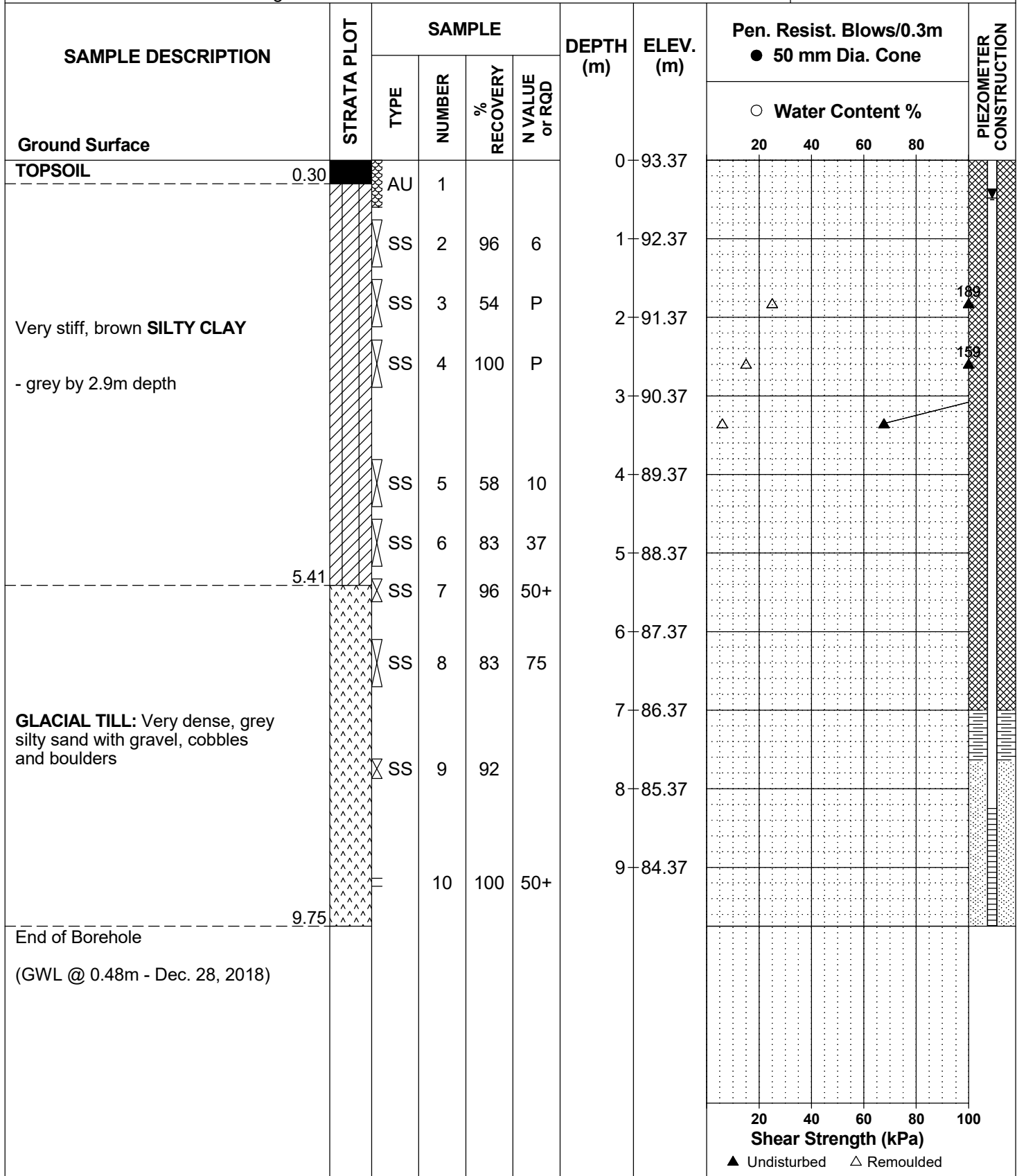
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DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: CME 55 Power Auger DATE: 2018 December 13

FILE NO. **PG4216**
HOLE NO. **BH 9**



EASTING: NORTHING: ELEVATION: 93.37
DATUM: Ground surface elevations were referenced to a geodetic datum.
REMARKS:
BORINGS BY: CME 55 Power Auger DATE: 2018 December 13

FILE NO. PG4216
HOLE NO. BH10



SYMBOLS AND TERMS

SOIL DESCRIPTION

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the strength of cohesionless soils is the relative density, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm.

Relative Density	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory vane tests, penetrometer tests, unconfined compression tests, or occasionally by Standard Penetration Tests.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

SYMBOLS AND TERMS (continued)

SOIL DESCRIPTION (continued)

Cohesive soils can also be classified according to their “sensitivity”. The sensitivity is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil.

Terminology used for describing soil strata based upon texture, or the proportion of individual particle sizes present is provided on the Textural Soil Classification Chart at the end of this information package.

ROCK DESCRIPTION

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NXL size core. However, it can be used on smaller core sizes, such as BX, if the bulk of the fractures caused by drilling stresses (called “mechanical breaks”) are easily distinguishable from the normal in situ fractures.

RQD %	ROCK QUALITY
90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard Penetration Test (SPT))
TW	-	Thin wall tube or Shelby tube
PS	-	Piston sample
AU	-	Auger sample or bulk sample
WS	-	Wash sample
RC	-	Rock core sample (Core bit size AXT, BXL, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.
P	-	Split-spoon pushed through sampling interval which was also tested using a vane apparatus and resulted in obtaining a sample of disturbed material (i.e., blow-counts not reflective of undisturbed, in-situ soils and not considered relevant)

SYMBOLS AND TERMS (continued)

GRAIN SIZE DISTRIBUTION

MC%	-	Natural moisture content or water content of sample, %
LL	-	Liquid Limit, % (water content above which soil behaves as a liquid)
PL	-	Plastic limit, % (water content above which soil behaves plastically)
PI	-	Plasticity index, % (difference between LL and PL)
Dxx	-	Grain size which xx% of the soil, by weight, is of finer grain sizes These grain size descriptions are not used below 0.075 mm grain size
D10	-	Grain size at which 10% of the soil is finer (effective grain size)
D60	-	Grain size at which 60% of the soil is finer
Cc	-	Concavity coefficient = $(D_{30})^2 / (D_{10} \times D_{60})$
Cu	-	Uniformity coefficient = D_{60} / D_{10}

Cc and Cu are used to assess the grading of sands and gravels:

Well-graded gravels have: $1 < Cc < 3$ and $Cu > 4$

Well-graded sands have: $1 < Cc < 3$ and $Cu > 6$

Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded.

Cc and Cu are not applicable for the description of soils with more than 10% silt and clay
(more than 10% finer than 0.075 mm or the #200 sieve)

CONSOLIDATION TEST

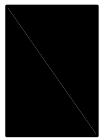
p'_o	-	Present effective overburden pressure at sample depth
p'_c	-	Preconsolidation pressure of (maximum past pressure on) sample
Ccr	-	Recompression index (in effect at pressures below p'_c)
Cc	-	Compression index (in effect at pressures above p'_c)
OC Ratio		Overconsolidation ratio = p'_c / p'_o
Void Ratio		Initial sample void ratio = volume of voids / volume of solids
Wo	-	Initial water content (at start of consolidation test)

PERMEABILITY TEST

k	-	Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.
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SYMBOLS AND TERMS (continued)

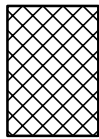
STRATA PLOT



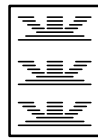
Topsoil



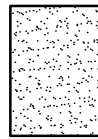
Asphalt



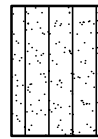
Fill



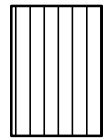
Peat



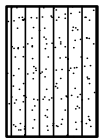
Sand



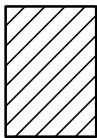
Silty Sand



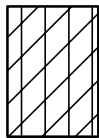
Silt



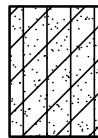
Sandy Silt



Clay



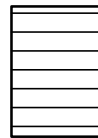
Silty Clay



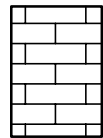
Clayey Silty Sand



Glacial Till



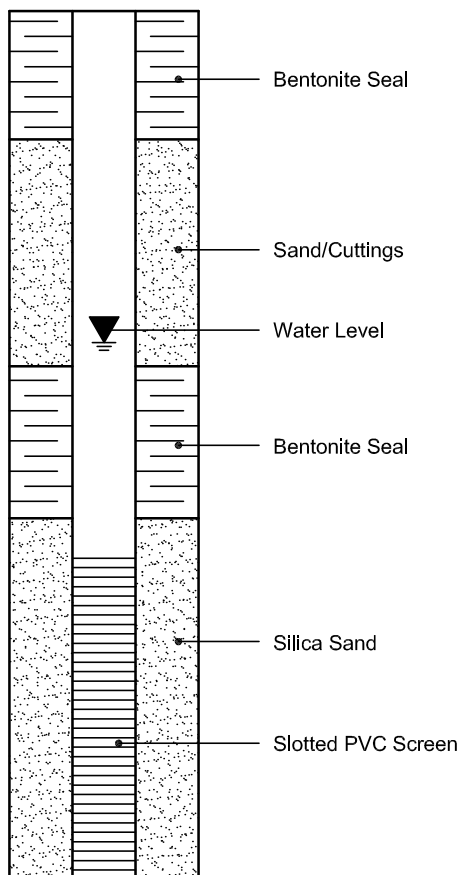
Shale



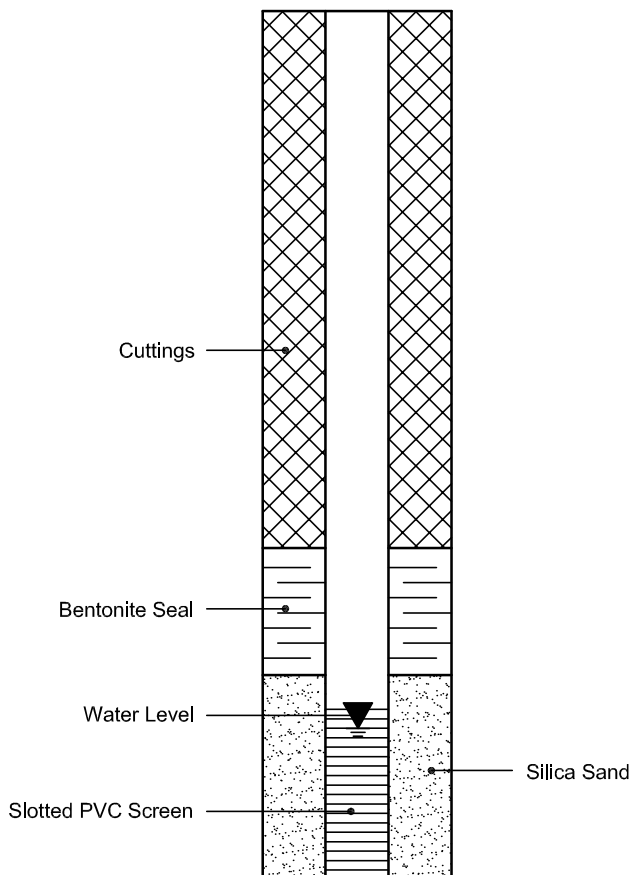
Bedrock

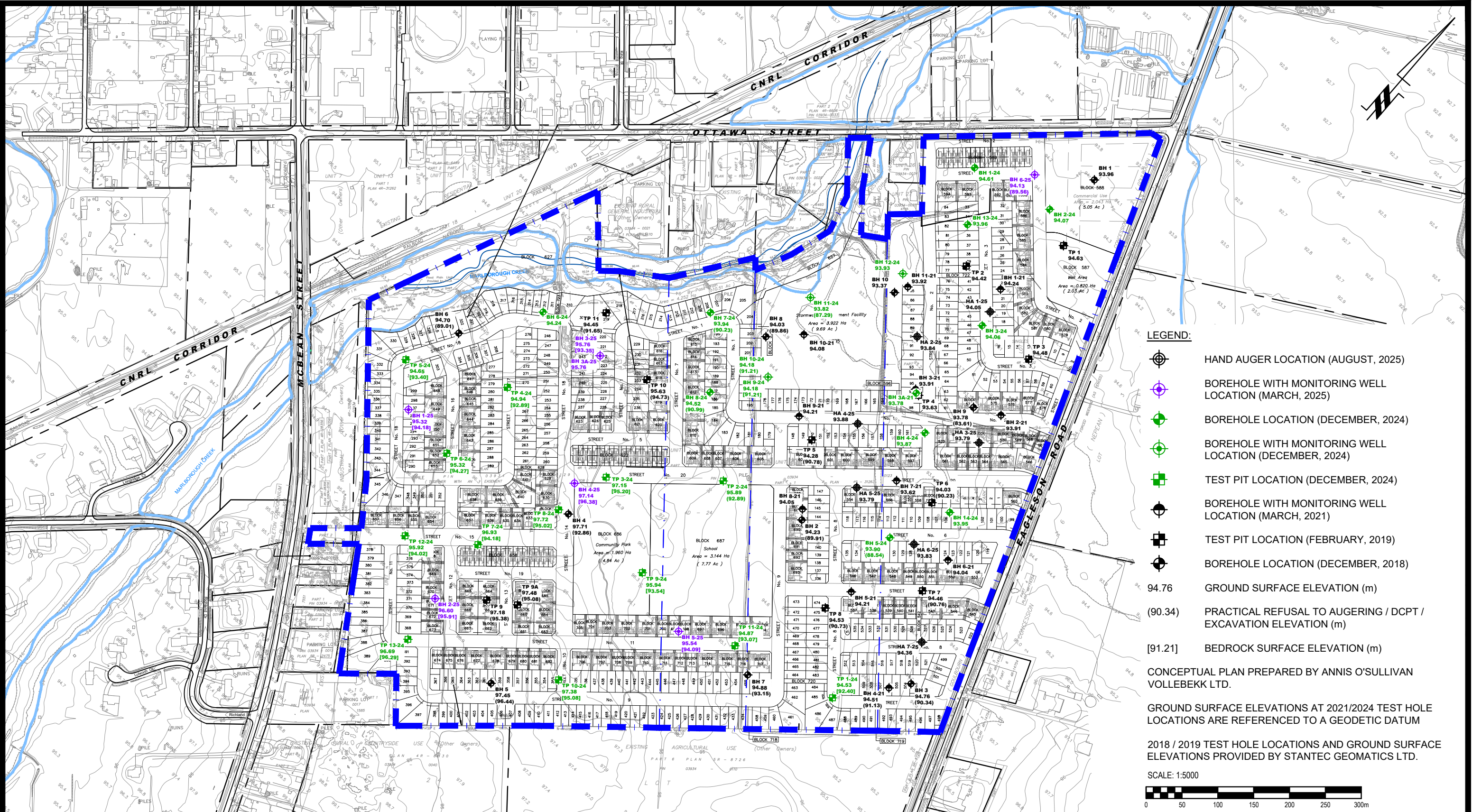
MONITORING WELL AND PIEZOMETER CONSTRUCTION

MONITORING WELL CONSTRUCTION



PIEZOMETER CONSTRUCTION







PATERSON
GROUP

9 AURIGA DRIVE
OTTAWA, ON
K2E 7T9
TEL: (613) 226-7381

6	UPDATED CONCEPTUAL PLAN	10/02/2025	NZ
5	ADDED 2025 HAND AUGERS	09/11/2025	NZ
4	ADDED 2024/2025 BOREHOLES AND 2024 TEST PITS	03/14/2025	FC
3	ADDED CONCEPTUAL PLAN	05/12/2024	FC
2	BH 1-21 TO BH 11-21 ADDED	05/04/2021	OC
NO.	REVISIONS	MM/DD/YYYY	INITIAL

OTTAWA,
Title:

TAMARACK (RICHMOND) CORPORATION
SUPPLEMENTAL GEOTECHNICAL INVESTIGATION
PROPOSED MIXED-USE DEVELOPMENT
5970 AND 6038 OTTAWA STREET
ONTARIO

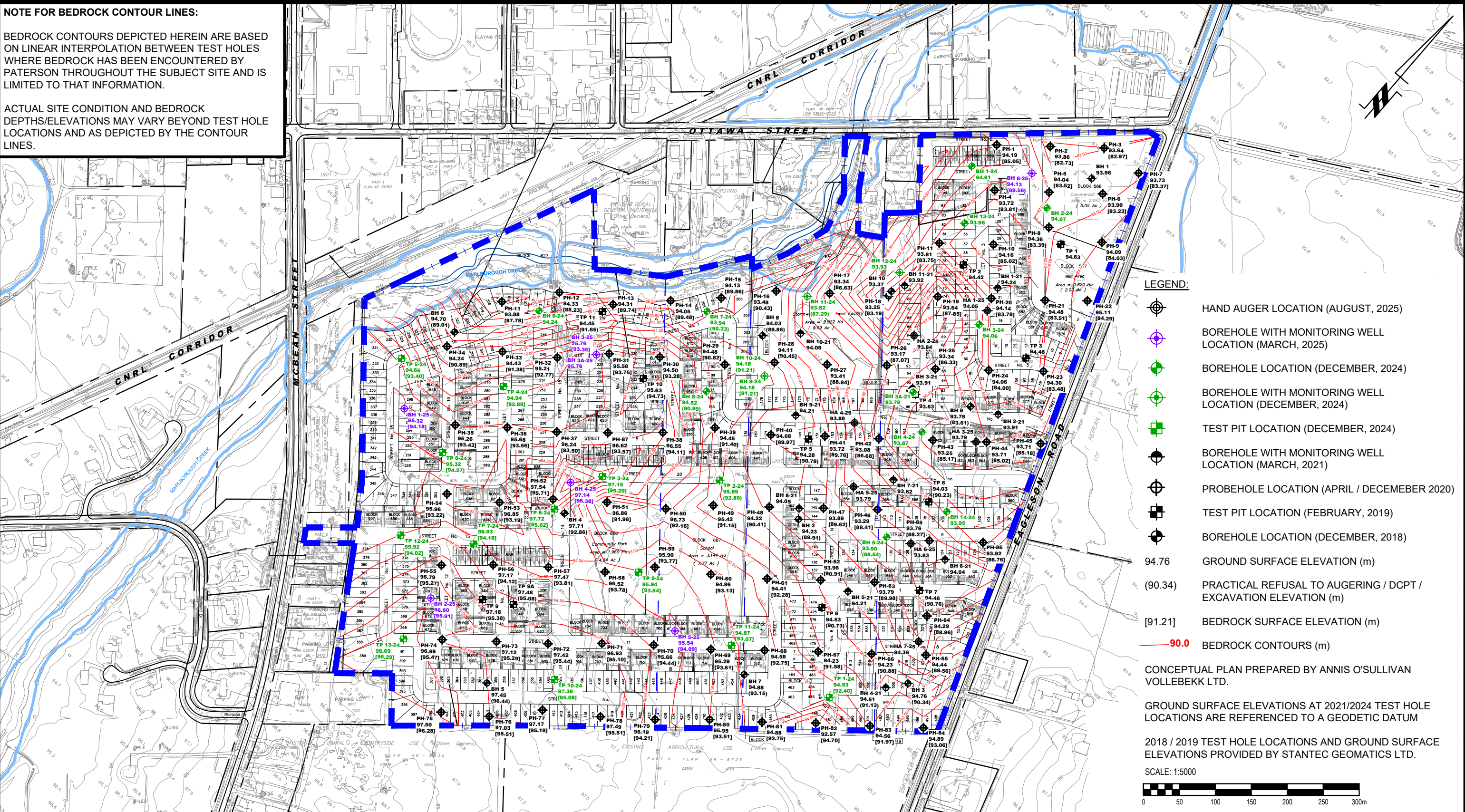
TEST HOLE LOCATION PLAN

Scale:	1:5000	Date:	01/2019
Drawn by:	MPG	Report No.:	PG4216-1
Checked by:	KP	Dwg. No.:	PG4216-2
Approved by:	FA	Revision No.:	6

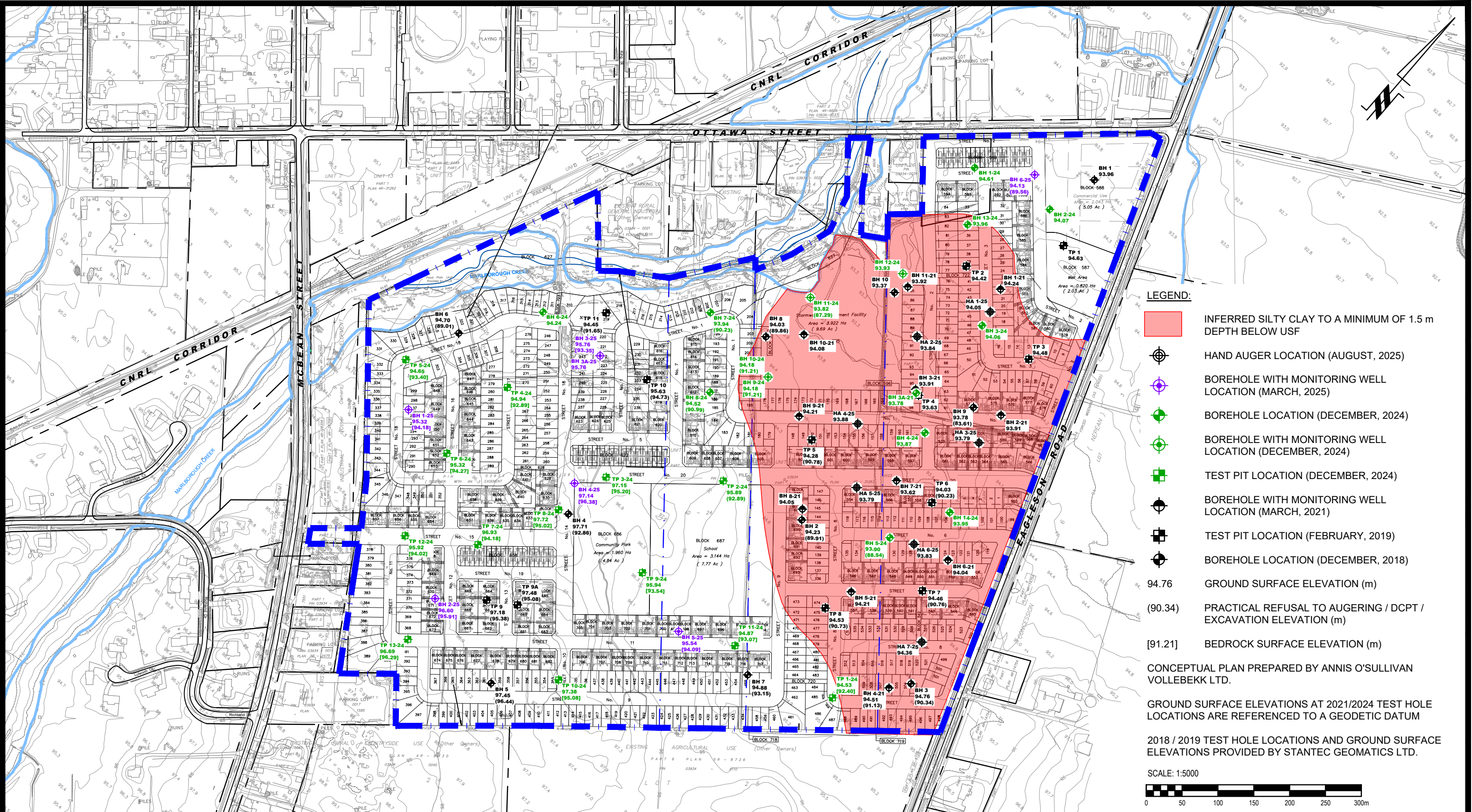
NOTE FOR BEDROCK CONTOUR LINES:

BEDROCK CONTOURS DEPICTED HEREIN ARE BASED ON LINEAR INTERPOLATION BETWEEN TEST HOLES WHERE BEDROCK HAS BEEN ENCOUNTERED BY PATERSON THROUGHOUT THE SUBJECT SITE AND IS LIMITED TO THAT INFORMATION.

ACTUAL SITE CONDITION AND BEDROCK DEPTHS/ELEVATIONS MAY VARY BEYOND TEST HOLE LOCATIONS AND AS DEPICTED BY THE CONTOUR LINES.



PATERSON GROUP 9 AURIGA DRIVE OTTAWA, ON K2E 7T9 TEL: (613) 226-7381	5	UPDATED CONCEPTUAL PLAN	10/02/2025	NZ	OTTAWA, Title: TAMARACK (RICHMOND) CORPORATION SUPPLEMENTAL GEOTECHNICAL INVESTIGATION PROPOSED MIXED-USE DEVELOPMENT 5970 AND 6038 OTTAWA STREET ONTARIO BEDROCK CONTOUR PLAN	Scale:	1:5000	Date:	02/2020
	4	ADDED 2025 HAND AUGERS	09/11/2025	NZ		Drawn by:	RCG	Report No.:	PG4216
	3	ADDED 2024/2025 BOREHOLES AND 2024 TEST PITS UPDATED BEDROCK CONTOURS	03/14/2025	FC		Checked by:	KP	Dwg. No.:	PG4216-4
	2	ADDED CONCEPTUAL PLAN	05/12/2024	FC		Approved by:	FA	Revision No.:	5
	1	BH 1-21 TO BH 11-21 ADDED	05/04/2021	OC					
	NO.	REVISIONS	MM/DD/YYYY	INITIAL					





5

UPDATED CONCEPTUAL PLAN

10/02/2025

NZ

4

ADDED 2025 HAND AUGERS

09/11/2025

NZ

3

ADDED 2024/2025 BOREHOLES AND 2024 TEST PITS

03/14/2025

FC

2

ADDED CONCEPTUAL PLAN

05/12/2024

FC

1

BH 1-21 TO BH 11-21 ADDED

05/04/2021

OC

NO.

REVISIONS

MM/DD/YYYY

INITIAL

OTTAWA,
Title:

TAMARACK (RICHMOND) CORPORATION
SUPPLEMENTAL GEOTECHNICAL INVESTIGATION
PROPOSED MIXED-USE DEVELOPMENT
5970 AND 6038 OTTAWA STREET
DESIGNATED SILTY CLAY AREAS

ONTARIO

Scale:
1:5000

Drawn by:
RCG

Checked by:
KP

Approved by:
FA

Date:
02/2020

Report No.:
PG4216-1

Dwg. No.:
PG4216-5

Revision No.:
5

Figure 1: BH 1-21 - Groundwater Monitoring Levels vs Precipitation Data

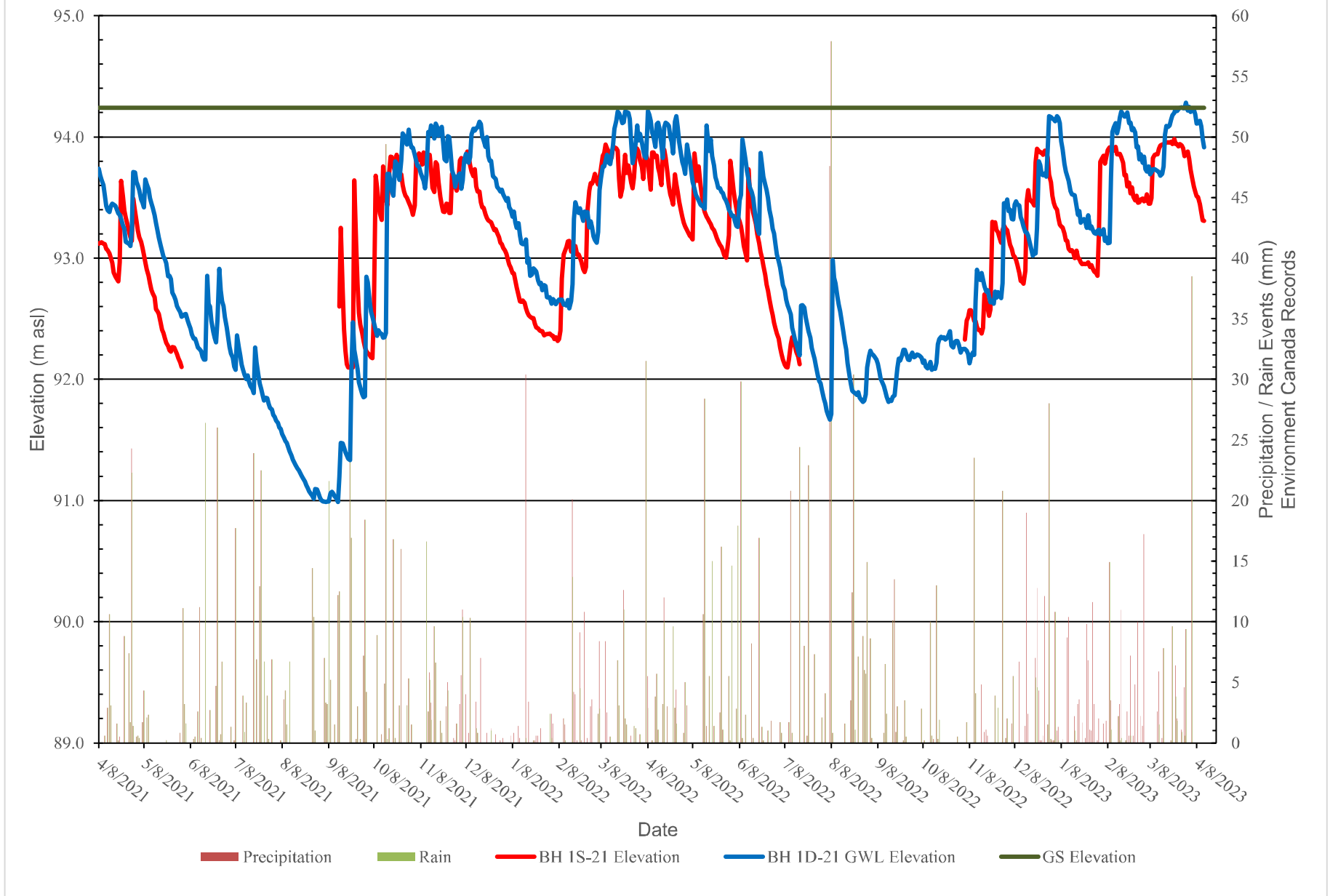


Figure 2: BH 2-21 - Groundwater Monitoring Levels vs Precipitation Data

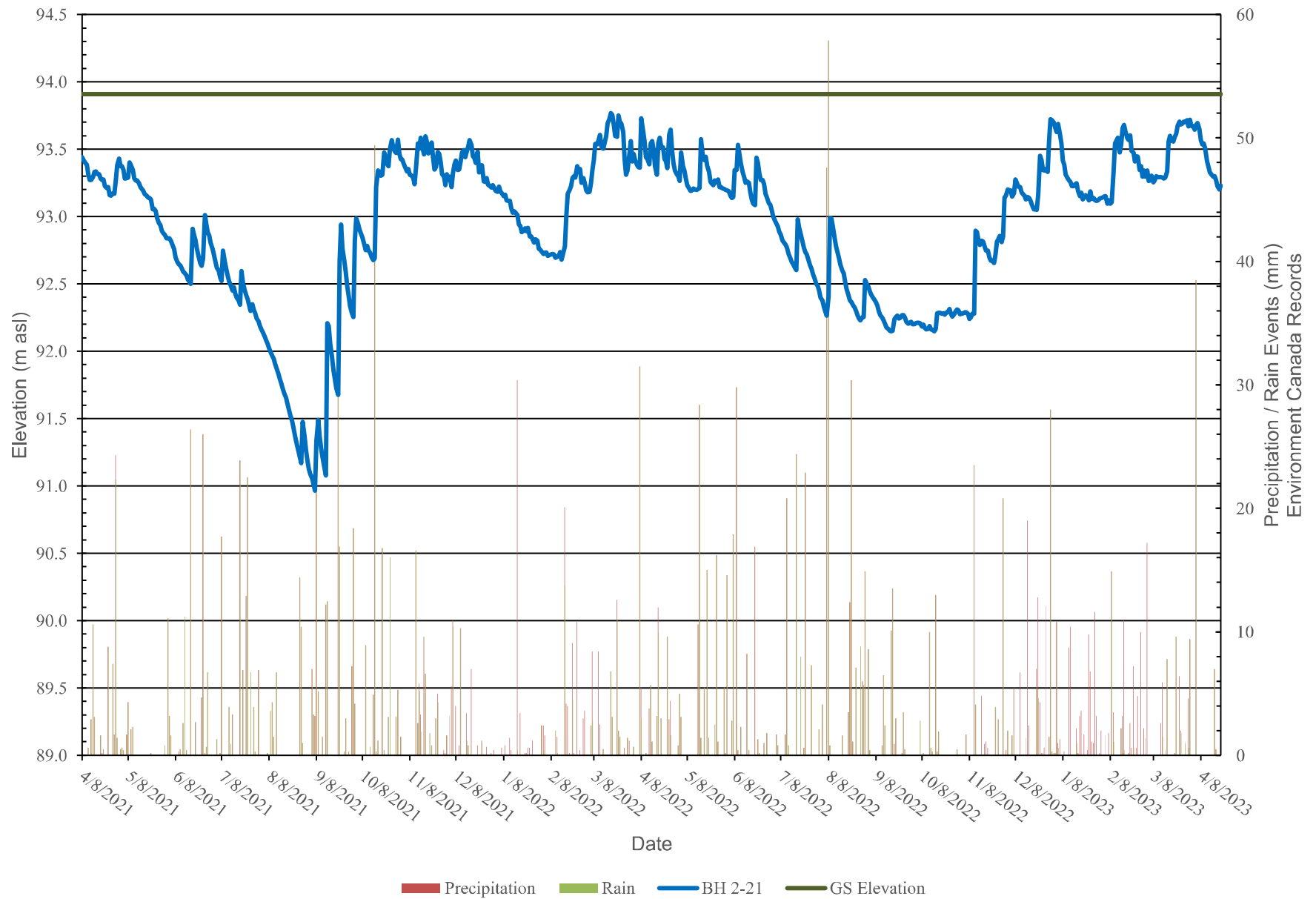


Figure 3: BH 3-21 - Groundwater Monitoring Levels vs Precipitation Data

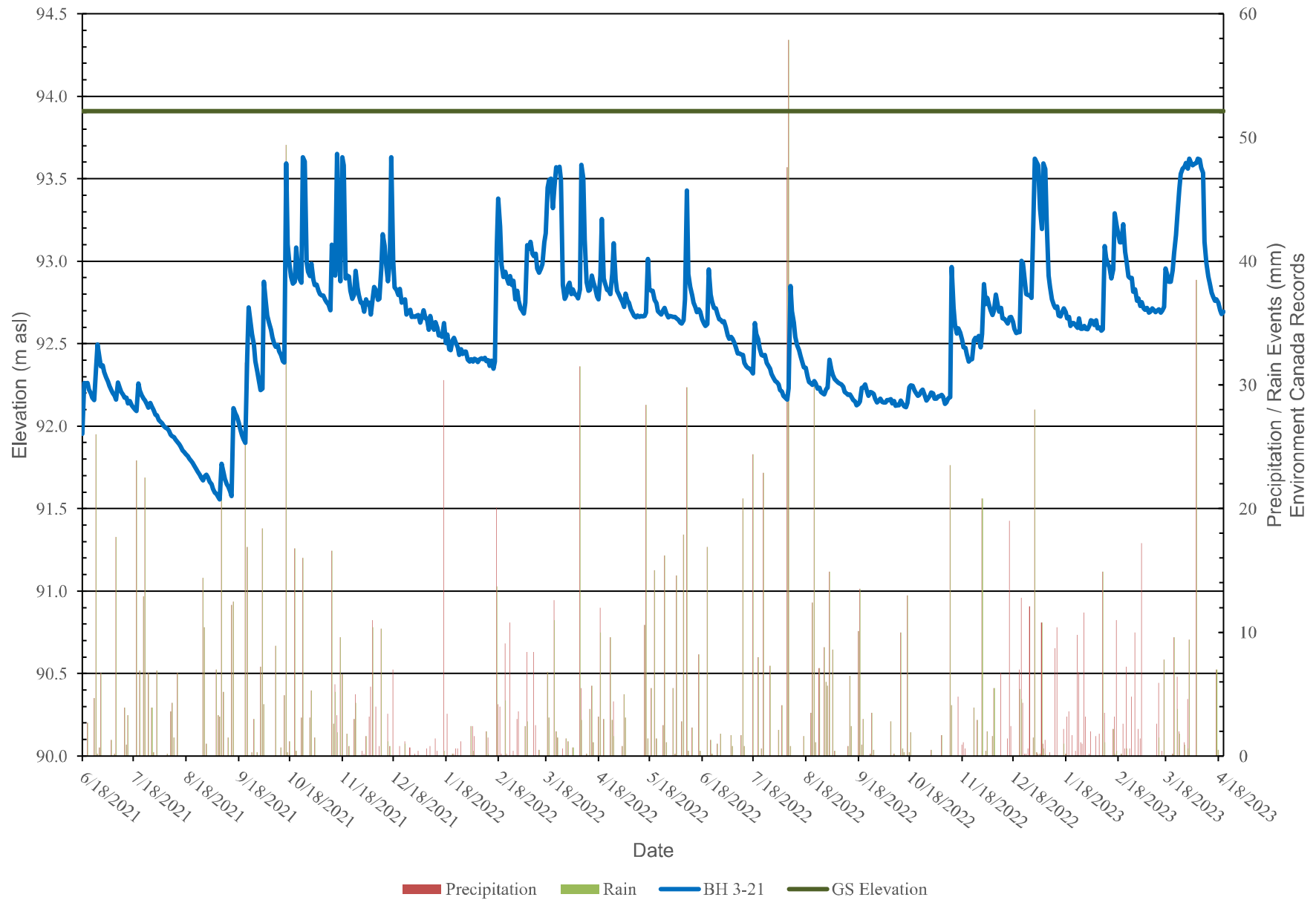


Figure 4: BH 4-21 - Groundwater Monitoring Levels vs Precipitation Data

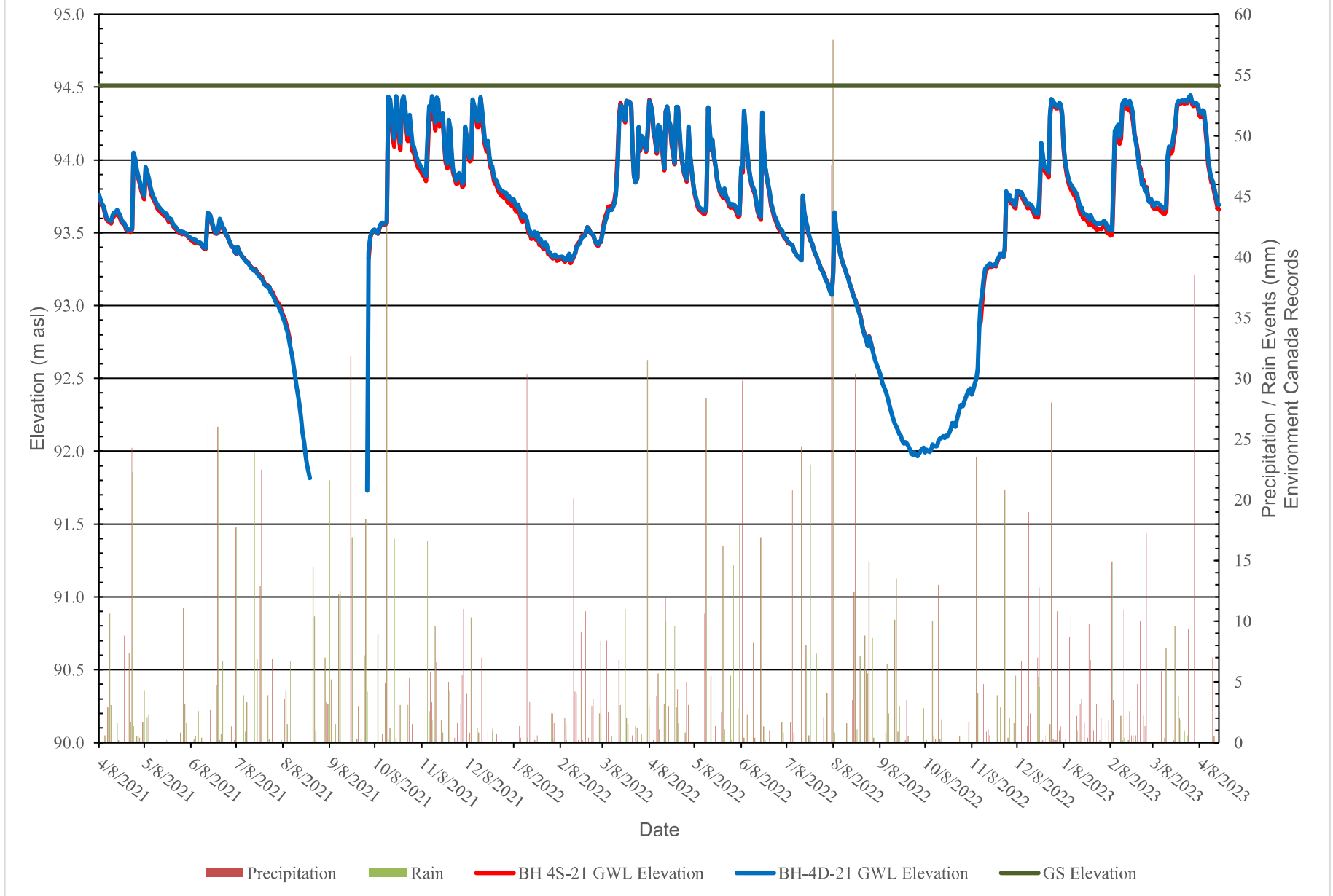


Figure 5: BH 5-21 - Groundwater Monitoring Levels vs Precipitation Data

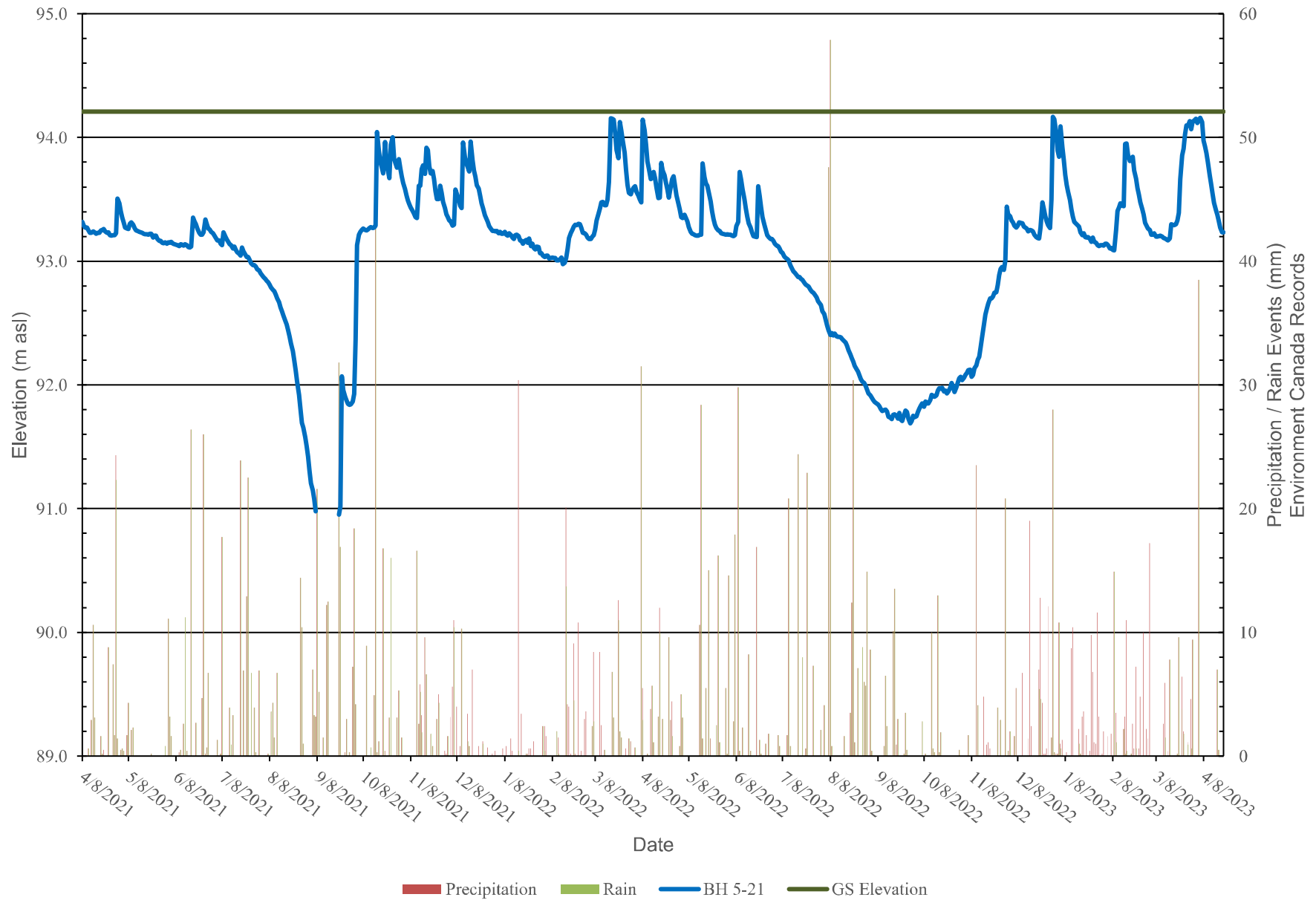


Figure 6: BH 6-21 - Groundwater Monitoring Levels vs Precipitation Data

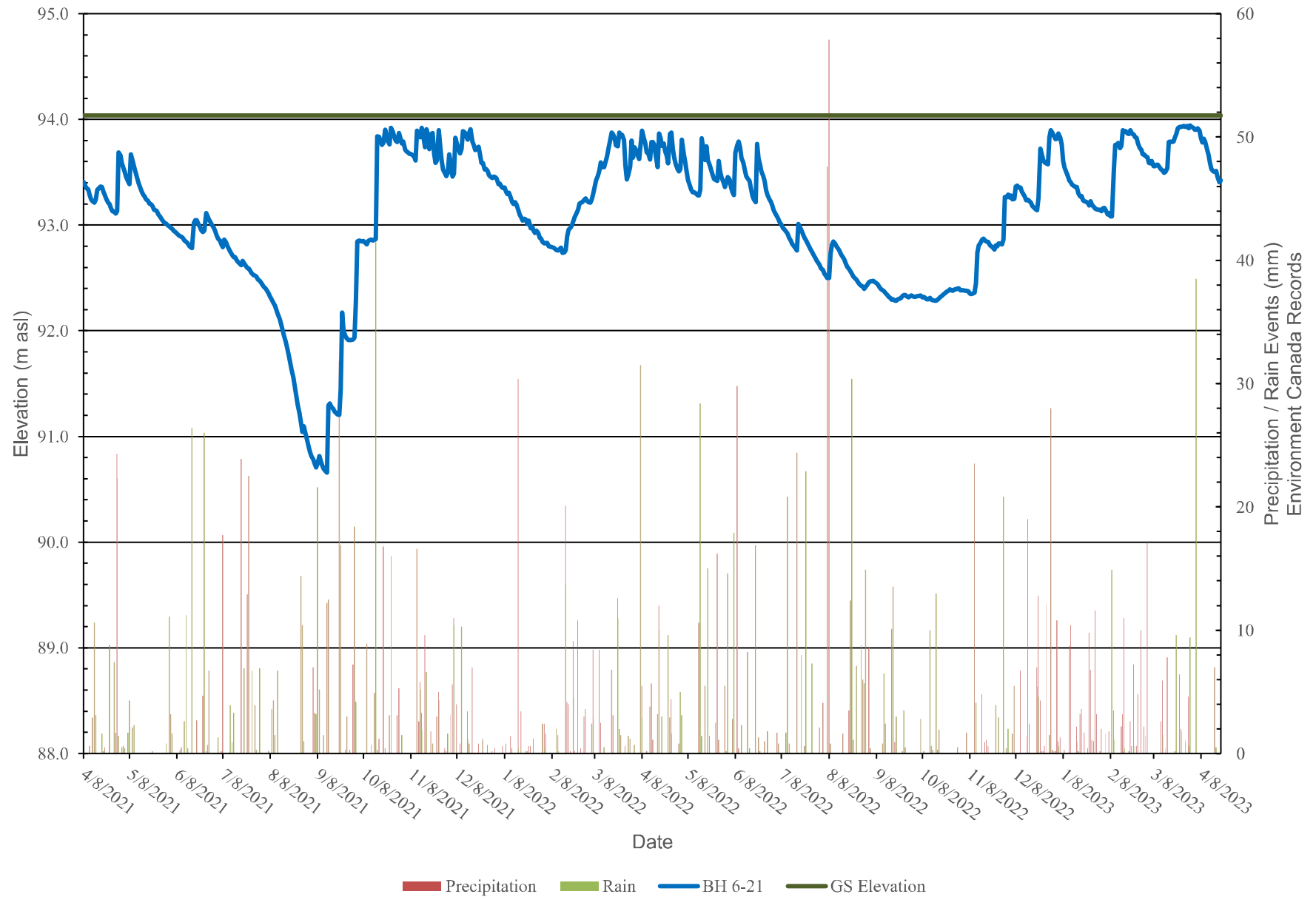


Figure 7: BH 7-21 - Groundwater Monitoring Levels vs Precipitation Data

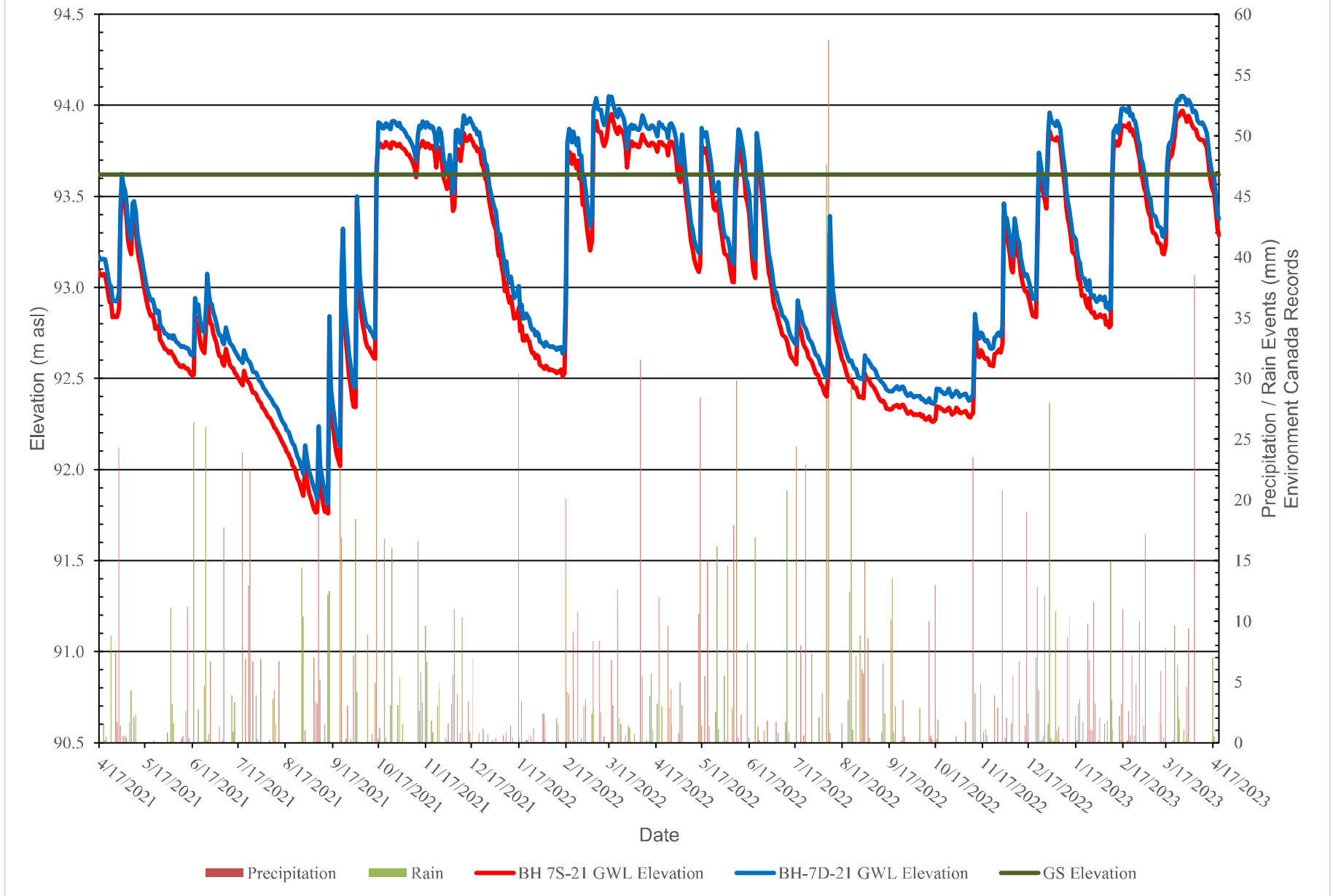


Figure 8: BH 8-21 - Groundwater Monitoring Levels vs Precipitation Data

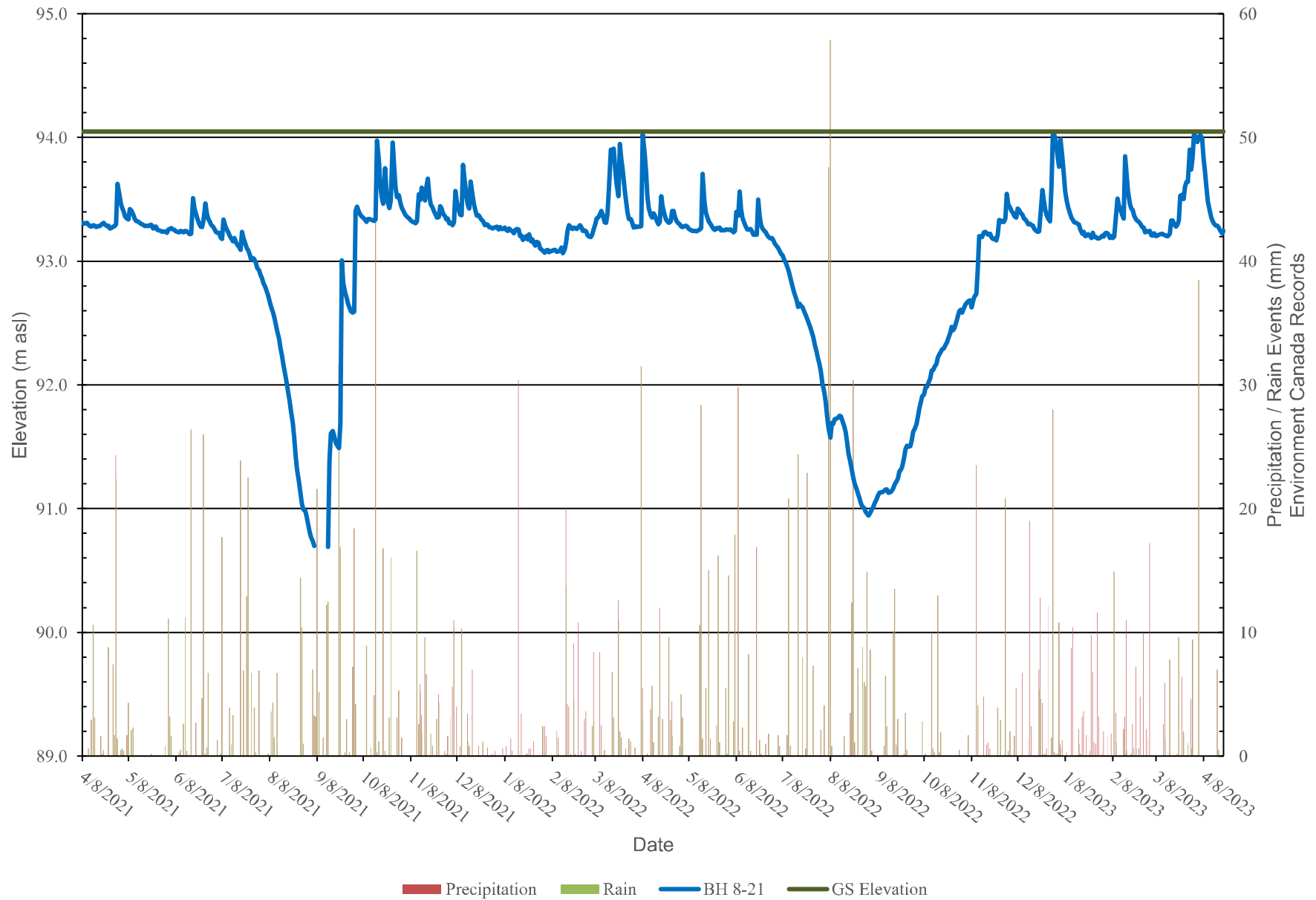


Figure 9: BH 9-21 - Groundwater Monitoring Levels vs Precipitation Data

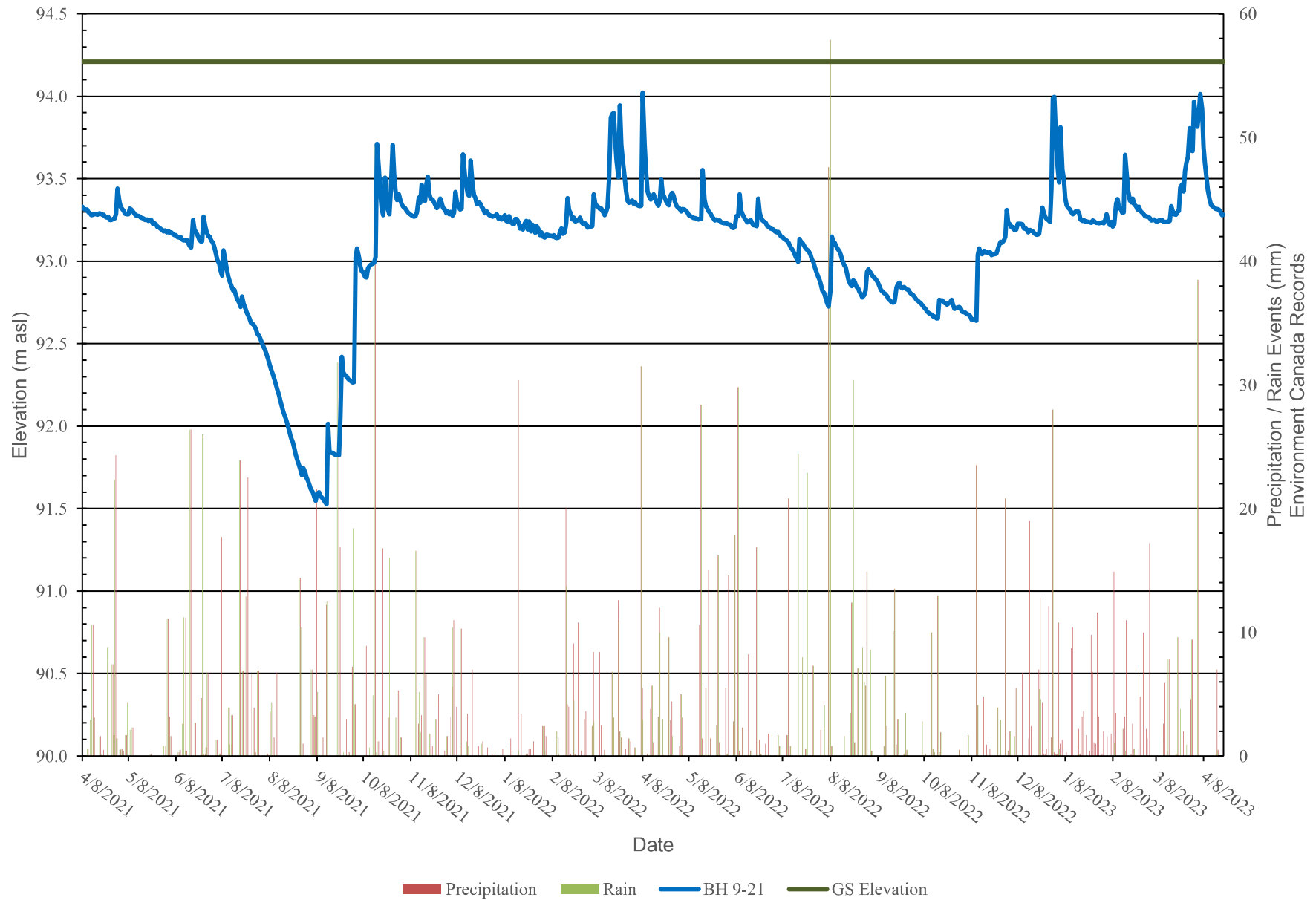


Figure 10: BH 10-21 - Groundwater Monitoring Levels vs Precipitation Data

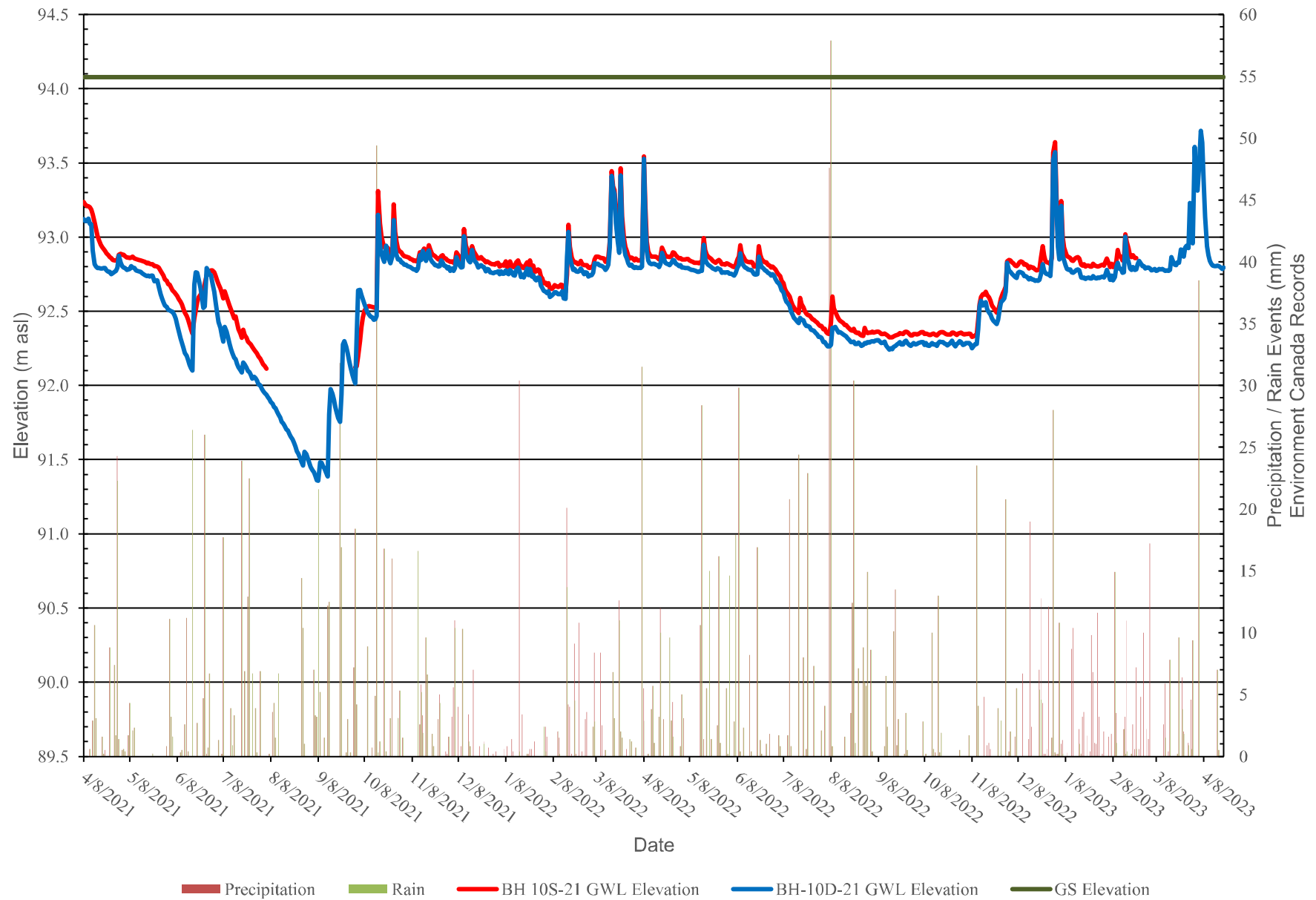
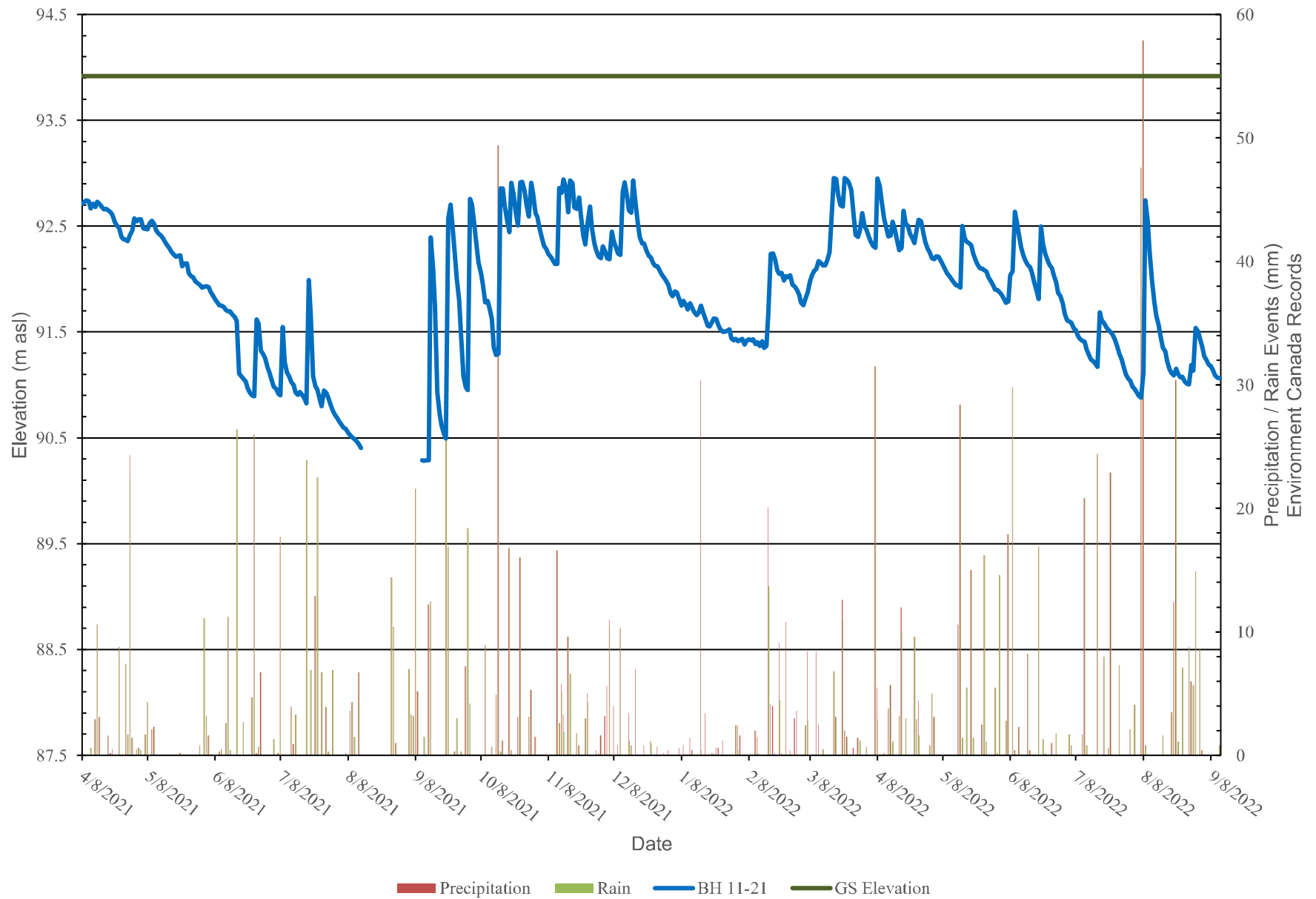
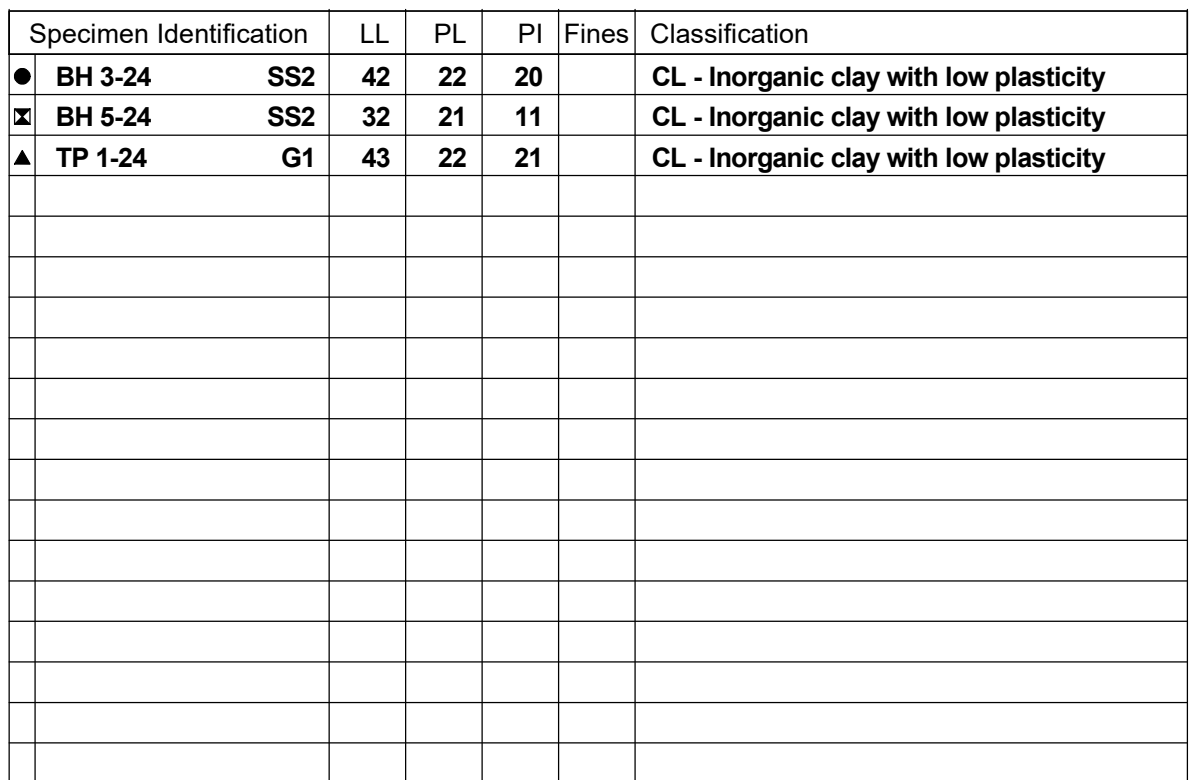
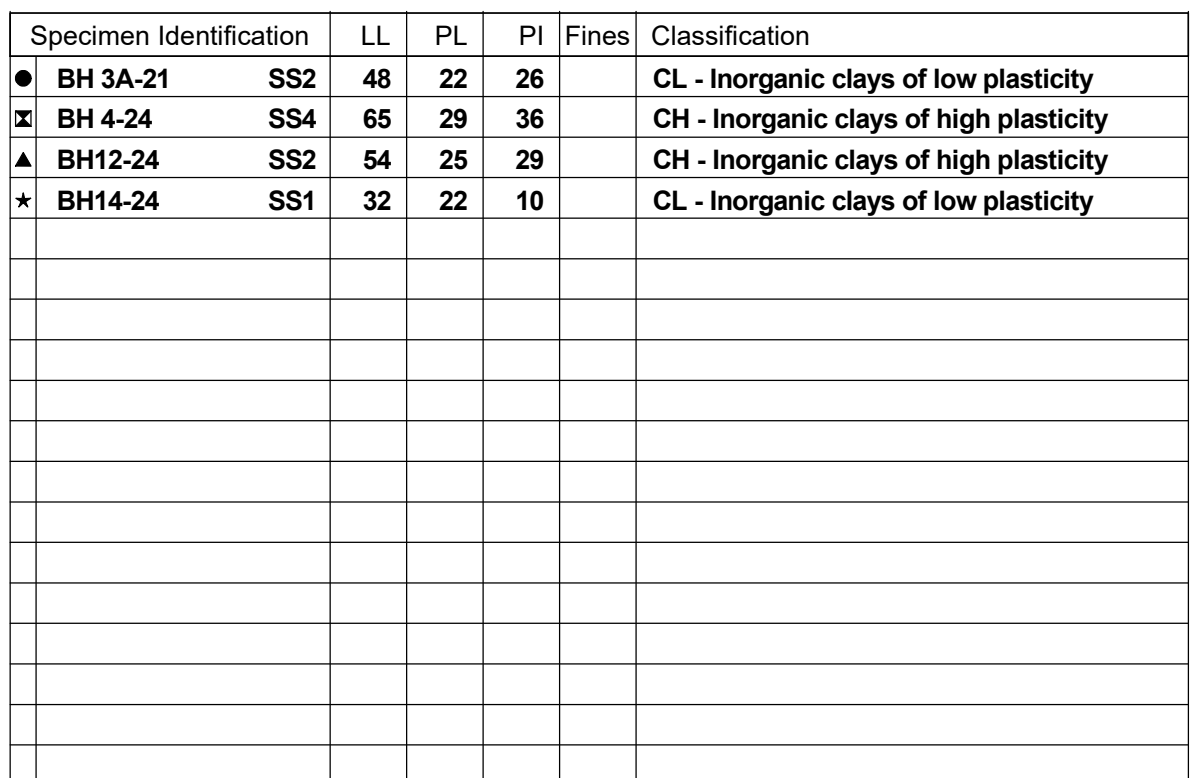


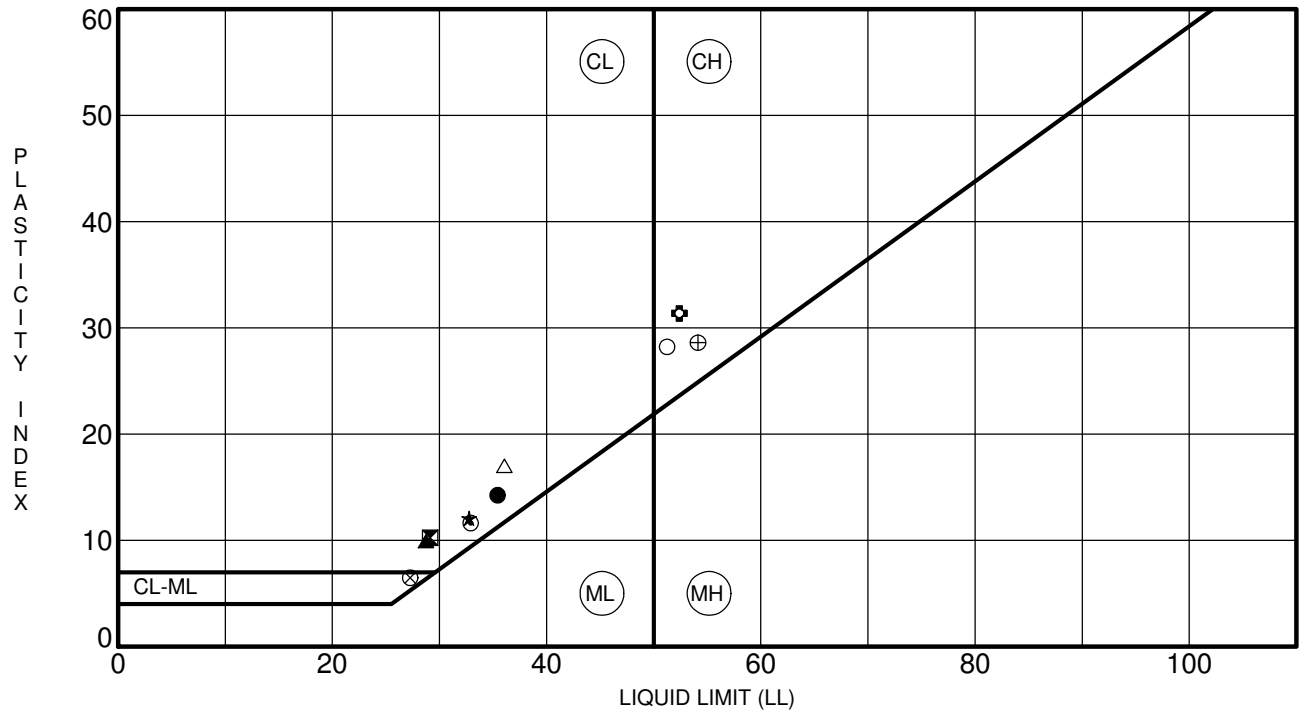
Figure 11: BH 11-21 - Groundwater Monitoring Levels vs Precipitation Data







ATTERBERG LIMITS'



Specimen Identification	LL	PL	PI	Fines	Classification
● BH 2 SS 2	35	21	14		CL - Inorganic clay with low plasticity
⊠ BH 3 SS 3	29	19	10		CL - Inorganic clay with low plasticity
▲ BH 8 SS 2	29	19	10		CL - Inorganic clay with low plasticity
★ TP 2 G 3	33	21	12		CL - Inorganic clay with low plasticity
⊙ TP 3 G 4	33	21	12		CL - Inorganic clay with low plasticity
⊕ TP 4 G 6	52	21	31		CH - Inorganic clays of high plasticity
○ TP 5 G 6	51	23	28		CH - Inorganic clays of high plasticity
△ TP 6 G 5	36	19	17		CL - Inorganic clay with low plasticity
⊗ TP 7 G 6	27	21	6		CL-ML - Inorganic silt with some clay with low plasticity
⊕ TP 8 G 5	54	26	29		CH - Inorganic clays of high plasticity

CLIENT Taggart Construction

PROJECT Geotechnical Investigation - Proposed

Development - Eagleson Road at Ottawa St.

FILE NO. PG4216

DATE 27 Feb 19

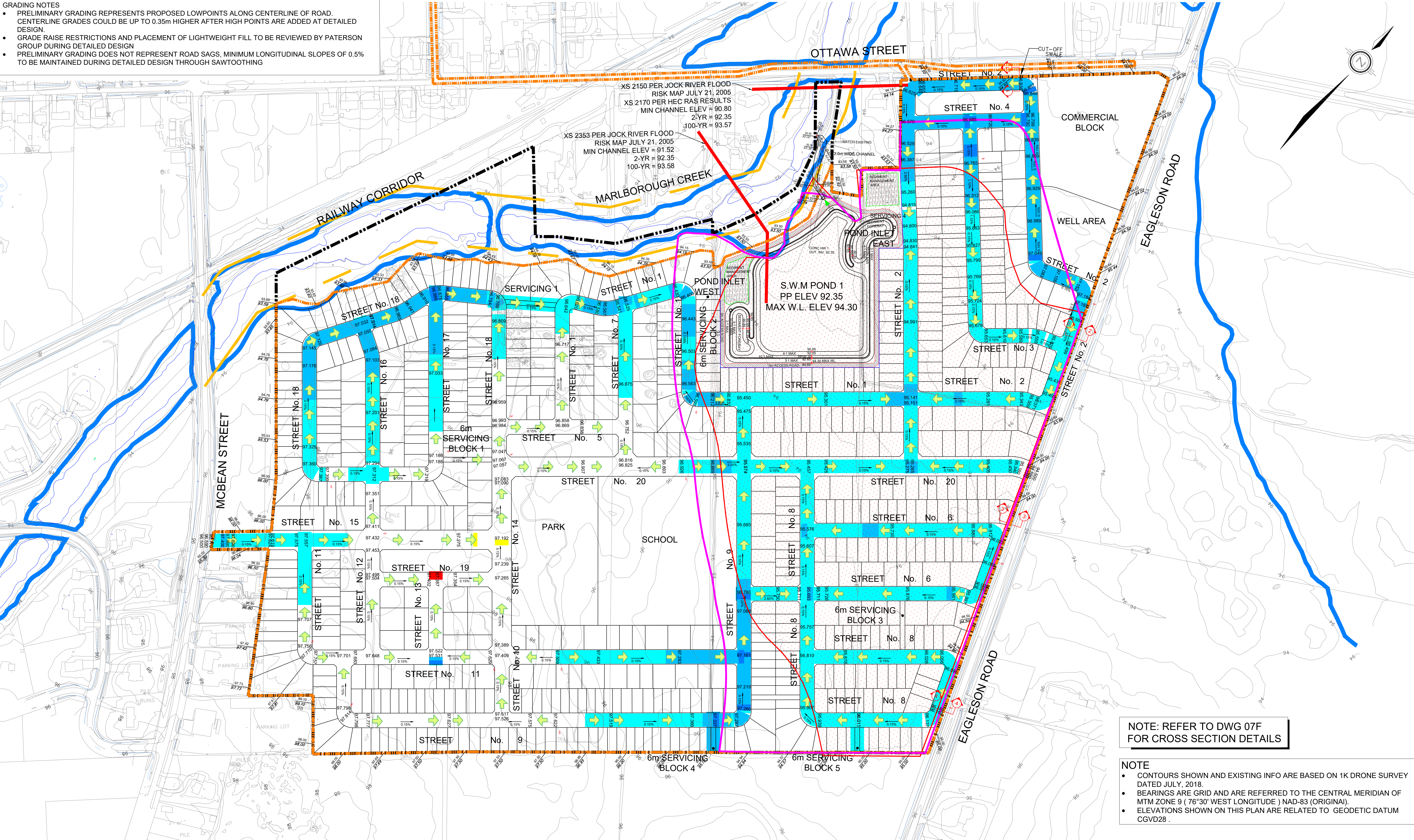
patersongroup Consulting Engineers

9 Auriga Drive, Ottawa, Ontario K2E 7T9

ATTERBERG LIMITS' RESULTS

GRADING NOTES

- PRELIMINARY GRADING REPRESENTS PROPOSED LOWPOINTS ALONG CENTERLINE OF ROAD. CENTERLINE GRADES COULD BE UP TO 0.35m HIGHER AFTER HIGH POINTS ARE ADDED AT DETAILED DESIGN.
- GRADE RAISE RESTRICTIONS AND PLACEMENT OF LIGHTWEIGHT FILL TO BE REVIEWED BY PATERSON GROUP DURING DETAILED DESIGN
- PRELIMINARY GRADING DOES NOT REPRESENT ROAD SAGS. MINIMUM LONGITUDINAL SLOPES OF 0.5% TO BE MAINTAINED DURING DETAILED DESIGN THROUGH SAWTOOTHING



XS 2150 PER JOCK RIVER FLOOD
RISK MAP JULY 21, 2005
XS 2170 PER HEC RAS RESULTS
MIN CHANNEL ELEV = 90.80
2-YR = 92.35
100-YR = 93.57

XS 2353 PER JOCK RIVER FLOOD
RISK MAP JULY 21, 2005
MIN CHANNEL ELEV = 91.52
2-YR = 92.35
100-YR = 93.58

NOTE: REFER TO DWG 07F
FOR CROSS SECTION DETAILS

NOTE

- CONTOURS SHOWN AND EXISTING INFO ARE BASED ON 1K DRONE SURVEY DATED JULY, 2018.
- BEARINGS ARE GRID AND ARE REFERRED TO THE CENTRAL MERIDIAN OF MTM ZONE 9 (76°30' WEST LONGITUDE) NAD-83 (ORIGINAL).
- ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM CGVD28 .

LEGEND

- SUBJECT LANDS
- STORM OVERLAND FLOW ARROW
- PROPOSED CENTERLINE ELEVATION
- FUTURE GRADES (BY OTHERS)
- FLOODPLAIN

CUT-FILL DEPTH TO EXISTING GROUND

CUT DEPTH (m)	FILL DEPTH (m)
0 - 1.0	0 - 1.0
1.0 - 2.0	1.0 - 2.0
2.0 - 3.0	2.0 - 3.0
>3.0	>3.0

MEANDER BELT LIMIT

- SUMP PUMP LIMIT per PATERSON PG4216-1 REV.5 DATED SEPT 17, 2025
- PERMISSIBLE GRADE RAISE LIMIT per PATERSON PG4216-1 REV.5 DATED SEPT 17, 2025

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TAMARACK RICHMOND

CITY OF OTTAWA

GRADING PLAN

SCALE:	1:2000	PROJECT No.:	1042
DATE:	SEPTEMBER 2025	DRAWING:	04D