

October 3, 2025

K1V 8Y3

File: PG4216-LET.02 Revision 2

**Tamarack (Richmond) Corporation** 3187 Albion Road South Ottawa, Ontario

Attention: Mike Green

Subject: Sump Pump Feasibility Report

Proposed Residential Development 5970 and 6038 Ottawa Street - Ottawa

**Consulting Engineers** 

9 Auriga Drive Ottawa, Ontario K2E 7T9 Tel: (613) 226-7381

Geotechnical Engineering
Environmental Engineering
Hydrogeology
Materials Testing
Building Science
Rural Development Design
Temporary Shoring Design
Retaining Wall Design
Noise and Vibration Studies

patersongroup.ca

Further to your request, Paterson Group (Paterson) completed a sump pump feasibility study as per City guidelines to determine the use of sump pumps for the proposed residential development to be located at 5970 and 6038 Ottawa Street in the City of Ottawa.

This report provides a discussion on the criteria to design the underside of footing elevations with respect to the existing groundwater table. The current report should be read in conjunction with Paterson Report PG4216-1 Revision 6 dated October 3, 2025.

#### 1.0 Background Information

The field investigation programs completed at the subject site were carried out by Paterson between December 2018 and September 2025. The investigations consisted of 47 boreholes, 25 test pits and 7 hand auger holes extending to a maximum depth of 9.8 m, 3.8 m and 1.1 m below ground surface (bgs), respectively. A bedrock delineation program was also carried out in December 2020, consisting of a total of 87 probe holes to the bedrock surface.

The test holes were distributed in a manner to provide general coverage of the subject site and has been presented in Drawing PG4216-2 - Test Hole Location Plan attached to this letter report.

Toronto Ottawa North Bay



#### Field Survey

The test hole locations were selected by Paterson to provide general coverage of the proposed development, taking into consideration the existing site features and underground utilities. The location and ground surface elevation at each borehole and test pit location were provided by Stantec Geomatics Ltd during the initial investigation, and by Paterson for the supplemental investigations.

The test hole locations and ground surface elevations at each test hole location are referenced to a geodetic datum. The location of the test holes and ground surface elevations at each test hole location are presented on Drawing PG4216-2 - Test Hole Location Plan in Appendix 2.

#### Subsurface Profile

Generally, the subsurface profile encountered within the subject site consists of topsoil underlain by a compact to dense glacial till deposit, and a firm to very stiff silty clay deposit followed by glacial till throughout the eastern portion of the subject site. The glacial till deposit is typically comprised of silty sand and/or silty clay with varying amounts of gravel, cobbles and boulders. The above noted layers have been underlain by a fair to excellent quality limestone bedrock.

Practical refusal to drill and excavation was encountered at multiple locations throughout the subject site. Bedrock surface elevations are presented in Drawing PG4216-4 - Bedrock Contour Plan attached to the current report.

Based on available geological mapping, the bedrock consists of dolostone of the Oxford formation with an overburden drift thickness ranging between 5 and 10 m.

For the purpose of this study, reference should be made to the area deemed acceptable for sump pumps presented on Drawing PG4216-5 – Designated Silty Clay Areas attached to the current report. Specific details of the soil profiles at the test hole locations relevant to the sump pump study area are presented on the Soil Profile and Test Data sheets also attached to the current report.



#### **Monitoring Well Installation**

PVC groundwater monitoring wells within the sump pump study area were installed in select borehole locations by Paterson to permit monitoring of the groundwater levels subsequent to the completion of the sampling programs. Monitoring well construction details are described below:

Slotted PVC 1.5 m screen at the base of each borehole.
51 mm diameter PVC riser pipe from the top of the screen to the ground
surface.
No.3 silica sand backfill within annular space around screen.
Bentonite hole plug placed directly above PVC slotted screen.
Clean backfill from top of bentonite plug to the ground surface.

Specific details of the installation of each monitoring well are further included in the Soil Profile and Test Data sheets and attached to the current letter report.

# 2.0 Groundwater Monitoring Program

The monitoring wells within the sump pump study area were equipped with a Van Essen Instrument Mini-Diver Water Level Logger to monitor fluctuations in the groundwater levels during the spring melt as part of a review for the use of sump pumps within the study area of the proposed development. The Mini-Divers were programmed to continuously measure and record groundwater levels throughout the study area during the pre-construction stages of the development at a rate of 1 reading every 12 hours. The results of the groundwater fluctuations and correlated precipitation events for each monitoring well location between April 8, 2021, and April 21, 2023, have been summarized in Figure 1 through Figure 11 attached to the current report.

#### **Groundwater Monitoring Results**

The data presented in Figure 1 through Figure 11 illustrate the collected groundwater elevations. The readings measured within the monitoring wells varied from below the tip of the data logger (>3.6 m and below 90.3 m) to above the original ground surface (Approximately 94.45 m). The low and high groundwater elevation measurements at each well location between April 8, 2021, and April 21, 2023, are summarized in Table 1 below.

Table 1 – Rec	orded Groundwater	Elevations		
Test Hole ID	Ground Surface Elevation (2021) (m)	Low Groundwater Elevation (m)	High Groundwater Elevation (m)	Difference in Groundwater Depth (m)
BH 1D-21	94.24	91.00	94.25	3.25
BH 1S-21	94.24	92.10	94.00	1.90
BH 2-21	93.91	91.00	93.30	2.30
BH 3-21	93.91	91.55	93.65	2.10
BH 4D-21	94.51	91.70	94.45	2.75
BH 4S-21	94.51	92.65	94.40	1.75
BH 5-21	94.21	91.00	94.15	3.15
BH 6-21	94.04	90.60	93.95	3.35
BH 7D-21	93.62	91.80	94.05	2.25
BH 7S-21	93.62	91.75	94.00	2.25
BH 8-21	94.05	90.65	94.00	3.35
BH 9-21	94.21	91.55	94.00	2.45
BH 10D-21	94.08	91.35	93.70	2.35
BH 10S-21	94.08	92.10	93.55	1.45
BH 11-21	93.92	90.30	92.95	2.65

Based on our analysis of the data retrieved from the data loggers between April 8, 2021 and April 21, 2023 groundwater fluctuations can be observed with a difference in elevation between the low and high readings ranging from 90.3 to 94.45 m. The lowest water level readings were found to range between an elevation of 90.3 (or lower) and 92.7 m throughout the monitoring period. During the spring melt season and heavy rain events, the highest water level readings were found to range between an elevation of 92.95 m and 94.45 m, with an average fluctuation of approximately 2.5 m.

Upon analysing the data collected from the monitoring wells and the subsequent rain events, groundwater reading spikes occurred within short periods of time after each rain event followed by a rapid lowering of the groundwater readings over the following days. It should be noted that the fluctuating groundwater readings are expected to be the result of the accumulation of rainwater within the fissures of the clay layer, as well as surficial permeation through the sand layers, which provides the collected water access to the shallow monitoring well screens. This is an indication that the groundwater level readings have been influenced by temporarily 'perched' groundwater, which is typical of monitoring well data within low permeability clay deposits, where water from rain events or spring melt tend to be maintained within the upper portion of the subsoil profile until either sheet drainage or evapotranspiration can eliminate the collected water, as well as surficial rainwater infiltration through the sand layers.

Furthermore, the usage of silty clay backfill material around the foundation and capped at surface will mitigate the potential for higher groundwater elevations and reduce infiltration as discussed in Section 4 of this report.



# 3.0 Hydraulic Conductivity Testing

Based on discussions with David Schaeffer Engineering Ltd. (DSEL), it is understood the proposed underside of footing depths will be approximately 1.8 m below the proposed center line of the roadways. As such, the underside of footings were estimated to be located slightly below to slightly above existing ground surface. Therefore, monitoring wells could not be positioned to straddle both above and below the expected USF elevation to facilitate hydraulic testing. As such, Atterberg testing and hydrometers in accordance with Unified Soil Classification System (USCS) identified on-site materials as clay soils and is in agreement with the ISTB-2018-04. As such, the hydraulic conductivity values for the silty clay within the sump pump study area were conservatively estimated based on experience at similar sites and published values. Hydraulic conductivity for silty clay generally ranges from 1 x 10-7 to 1 x 10-12 m/sec and is dependent on the moisture level and consistency of the material.

Hydraulic conductivity (slug) testing was conducted on the glacial till underlying the silty clay deposit. The test data was analyzed as per the method set out by Hvorslev (1951). Assumptions inherent in the Hvorslev method include a homogeneous and isotropic aquifer of infinite extent with zero-storage assumption, and a screen length significantly greater than the monitoring well diameter. The assumption regarding aquifer storage is considered to be appropriate for groundwater flow through the overburden aquifer. The assumption regarding screen length and well diameter is considered to be met based on a screen length of 1.5 m and a diameter of 0.05 m. While the idealized assumptions regarding aquifer extent, homogeneity, and isotropy are not strictly met in this case (or in any real-world situation), it has been our experience that the Hvorslev method produces effective point estimates of hydraulic conductivity in conditions similar to those encountered at the subject site.

Hvorslev analysis is based on the line of best fit through the field data (hydraulic head recovery vs. time), plotted on a semi-logarithmic scale. In cases where the initial hydraulic head displacement is known with relative certainty, such as in this case where a physical slug has been introduced, the line of best fit is considered to pass through the origin.

Based on the above test methods, the monitoring wells screened in the glacial till displayed hydraulic conductivity values ranging between  $1.27 \times 10^{-7}$  to  $1.00 \times 10^{-5}$  m/sec. The values measured within the monitoring wells are generally consistent with similar material Paterson has encountered on other sites and typical published values for glacial till. The results of the hydraulic conductivity testing completed by Paterson and others have been attached to the current letter report.



# 4.0 Summary and Recommendations

It is understood that the proposed residential buildings within the subject site will include a basement level. It is also understood that sump pump systems are proposed for the buildings within the sump pump study area of the subject development to provide an outlet for stormwater and spring melt water collected from the perimeter foundation drainage system. Specific locations are to be determined by the Civil engineer during detailed design.

#### **Groundwater Table Versus USF Elevations**

Based on the above discussion, the pre-development seasonal high groundwater table within the sump pump study area is anticipated to be at elevations ranging between 92.95 and 94.45 m. Paterson reviewed the following grading plans prepared by DSEL as part of our assessment:

DSEL – Tamarack Richmond, Grading Plan, Project No. 1042, Drawing No. 04D, dated September 2025.

While detailed grading has not been provided for the proposed development, DSEL has noted that the proposed underside of footing (USF) will be placed at an approximate depth of 1.8 m below the center line elevation of the proposed roads identified in the above noted grading plan.

#### **Seasonal High Groundwater Level**

As indicated above, the pre-development seasonal high water was measured between 92.95 to 94.45 m. Based on our review of the latest conceptual plan provided by DSEL, the proposed USF elevations across the study area are generally below the measured seasonal high groundwater table. It is important to note that the groundwater levels recorded for the site are considered pre-development groundwater levels. From a geotechnical perspective, the pre-development groundwater levels are usually higher than post-development groundwater levels and highly dependent on the inverts of the proposed site servicing pipes.

#### **Sump Pumps (Study Area)**

It should be noted that based on the Technical Bulletin ISTB-2018-04 and ISTB-2019-02 issued by the City of Ottawa regarding installation of sump pumps, for typical sites, a minimum 300 mm vertical separation is recommended between the design underside of footing elevation and the seasonal high groundwater level.



As previously noted, the proposed USF elevations across the study area are generally below the pre-development seasonal high groundwater table. As such, assessment of the backfill permeability and estimates of the rate of groundwater ingress are required to rationalize the use of sump pumps at the site, as per Appendix 8 of ISTB-2018-04.

#### **Clay Continuity**

The boreholes completed within the sump pump study area of the subject site are in conformance with the City of Ottawa borehole spacing guidelines. The native silty clay soils within the study area are considered to be laterally vertically continuous. The boreholes within the study area of the subject site identify a silty clay deposit at the borehole locations at similar elevations throughout. Therefore, the silty clay deposit is considered to be laterally continuous across the sump pump study area of the proposed residential development.

#### **Backfill Recommendations**

It is recommended that silty clay backfill material be used around the foundation and capped at surface to mitigate the potential for higher groundwater elevations and reduce infiltration. Backfill material comprised of silty clay with a hydraulic conductivity < 1 x 10<sup>-7</sup> m/sec will be used and consist of site excavated or imported silty clay fill. As per Section 8.4 of the ISTB, characterization of the imported fill will be completed and consist of Falling Head Permeability laboratory testing completed in accordance with ASTM D5084 - Option B. Alternatively, hydrometer/Atterberg testing may be completed to ensure the backfill material is similar or contains higher fines than the site specific material. Furthermore, visual inspections of the placement of the silty clay backfill at the foundation locations and the clay seals at the property line within lateral servicing trenches will be carried out by the geotechnical consultant.

All surfaces adjacent to the proposed buildings should be shaped to shed water away from the building's foundation. All the sump pump installations should be inspected and approved by Paterson at the time of installation.

#### Permeability of Soils and Groundwater Ingress Rate

Based on Atterberg Limits testing completed on the silty clay within the sump pump study area, the material has been classified as CL-CH - Inorganic Clay of Low to High Plasticity per the (USCS). Atterberg Limits testing results have been attached to the current report. Silty clay properties in the assessment are based on the backfill specification noted in the previous section.

The ingress rate has been calculated using the Dupuit Forchheimer method:

For the purpose of completing the calculation, the following values were conservatively used in the analysis:

□  $k = 1 \times 10^{-7}$  m/sec, silty clay □  $h_0 = 10$  m □  $h_p = 9$  m □ R = < 5 m □ r = 8 m

Based on the silty clay profile below the proposed footings and used as backfill material, the groundwater ingress rate was calculated to be less than 0.06 L/s (<5,000 L/day) and significantly less than the minimum pump capacity of 0.9 L/s (77,760 L/day) at 3.6 m head as per the above noted sump pump design Bulletin. Also, due to the characteristics of the underlying silty clay, the groundwater levels will not impact the performance of a sump pump due to the low permeability of the soils.

Based on the above, the sump pumps are not expected to be overloaded and/or continuously running. As such, the minimum design requirements for the main sump pump system and the backup pump battery will be achieved for the estimated groundwater rate of ingress under worst case scenarios.



#### 5.0 Conclusion

Based on our observations and available design details, the subject area identified in Drawing PG4216-5 – Designated Silty Clay Areas is considered to have met the requirements detailed in the Technical Bulletin ISTB-2018-04 and ISTB-2019-02 issued by the City of Ottawa regarding installation of sump pumps.

We trust that the current submission meets your immediate requirements.

Best Regards,

Paterson Group Inc.

Nicholas Zulinski, P.Geo., géo.

#### Attachments:

- Soil Profile and Test Data Sheets
- Symbol and Terms
- ☐ Drawing PG4216-2 Test Hole Location Plan
- ☐ Drawing PG4216-4 Bedrock Contour Plan
- ☐ Drawing PG4216-5 Designated Silty Clay Areas
- ☐ Figures 1 through 11 Groundwater Monitoring Levels
- Atterberg Testing Results
- Hydraulic Conductivity Testing
- DSEL Grading Plan

Michael Killam, P.Eng.





## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **EASTING:** 435369.03 **NORTHING:** 5003654.34 ELEVATION: 95.31

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

지 100 RQD 95 원 100 RQD 100 원 100 유QD 100 원 100	HOLE NO.: BH 1-25	202	/larch 6	DATE: N					REMARKS:
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TOPSOIL  Q2-3m [96,05m]  QLACIAL TILL: Brown silty clay, some gravel and sand  QLACIAL TILL: Compact, brown silty sand, trace	PL (%) WATER CONTENT (%) LL (%)		ATE	8	ECO	YPE	Ē	TR	
GLACIAL TILL: Brown silty clay, some gravel and sand 0.6 Im [94.70m] 1.14m [94.17m] 2.2 64 2.9-50-7 58/0.2 clay and gravel BEDROCK: Good to excellent quality limestone 2.2 100 RQD 78 2.3 100 RQD 95 2.3	20 40 60 80		<	z	∞	<b>⊢</b>		ω	
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **EASTING:** 435568.93 **NORTHING:** 5003478.60 ELEVATION: 96.60

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

ELEVATION: 95.76 COORD. SYS.: UTM ZONE 18 **EASTING:** 435522.74 **NORTHING:** 5003885.37

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:					DATE: M	larch 6	, 202	5		HOLI	E NO. :	BH 3-25		
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development FILE NO. : PG4216

ADVANCED BY: Track Mounted Drill Rig

DATE: March 6, 2025

HOLE NO.: BH 3A-25

REMARKS:					DATE:	March 6	, 202	25		HOL	.E NO. :	BH 3A-	25	
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End of Borehole									:			: :		93
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(GWL at 1.88 m depth - March 12, 2025)		3							:					-
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435611.53 **NORTHING:** 5003727.26 **ELEVATION:** 97.14

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Track Mounted Drill Rig

REMARKS: DATE: March 6, 2025 HOLE NO.: BH 4-25

REMARKS:					DATE: N	1arch 6	, 202	5		HOLE	NO. :	BH 4-	25		
				S	AMPLE				DCPT (	SIST. (BLO 50mm DI		)			
SAMPLE DESCRIPTION	STRATA PLOT	н (ш)	TYPE AND NO.	RECOVERY (%)	RQD	R CONTENT (%)	Δ.		OULDED RAINED S	SHEAR S SHEAR S	TRENGT 60			MONITORING WELL CONSTRUCTION	ELEVATION (m)
GROUND SURFACE	STRA	DEPTH (m)	TYPE	RECO	N OR RQD	WATER (%)		PL (%) 20		R CONTE	ENT (%) 60	LL (%)		MONICONS	ELEW
TOPSOIL 0.25m [96.89m]	~ ~ ~ <i>\</i>		<del>,</del> -												97 -
GLACIAL TILL: Compact, brown silty clay, with	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	-	<b>☆</b> ₹										k		
gravel, trace sand <sub>0.76m[96.38m]</sub> , <b>BEDROCK:</b> Good to fair quality limestone		1-	2 2 1	85	RQD 85					: :					
SEDICOR. Good to fall quality liftlestone	莊	=						:							96 -
		=	2												
		2	RC 2	68	RQD 58					i					
		=						:	-						95-
		=													
		3													0.4
		=	RC 3	76	RQD 58			:	•						94 -
		=	"										3.7	3.4 6 m 2025	5m 5-03-12
		4													
Mud seams from 4.42 m to 4.57 m depth		-	_					:							93-
Mud Seams nom 4.42 m to 4.57 m depth		=	RC 4	100	RQD 54										
5.03m [ 92.11m ]		5												4.9	8m
End of Borehole		=						•	-						92-
GWL at 3.76 m depth - March 12, 2025)		=								y					
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Track Mounted Drill Rig

PEMARKS:

DATE: March 7, 2025

HOLE NO.: BH 5-25

REMARKS:					DATE: M	larch 7	, 202					BH 5-2	<b>)</b>	
				S	AMPLE			<b>■</b> P		IST. (BLO 0mm DIA				
SAMPLE DESCRIPTION	STRATA PLOT	(m)	TYPE AND NO.	RECOVERY (%)	άĐ	R CONTENT (%)	<b>△</b>		40 ULDED S	HEAR ST	30 RENGT	80 Г <b>Н (kPa)</b>	MONITORING WELL	ELEVATION (m)
	TRAT	DEPTH (m)	YPE ,	EC0	N OR RQD	WATER (%		PL (%)	WATER	R CONTE	NT (%)	LL (%)	ONST	LEVA:
GROUND SURFACE TOPSOIL	ί		<del>V</del>	~	Z	<	:	20	40	) 6	60	80	X X	-
TOFSOIL		. =	¥ ₹										0.34 m ¥ 202	
0.86m [ 94.67m ]		-												95
GLACIAL TILL: Compact, brown silty sand, some	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1-	SS 2	67	3-12-17-23									
gravel and cobbles, trace clay 1.45m 94.08m	\(\sigma\) \(\sigma\) \(\sigma\) \(\sigma\) \(\sigma\)	=	SS 3 8	0	29 50-/-/-/		:							
BEDROCK: Excellent quality limestone		=			50/0.03									94
		2	RC 1	100	RQD 100						<u>.</u>			
		=					:							
		=												93
		3						<u>.</u>						
		=	RC 2	100	RQD 100								2	43m
		=												43m 92
		4												
			RC 3	400	DOD OF		:			:				
		=	2	100	RQD 95									91
4.95m [ 90.58m ]  End of Borehole		5 _											4.9	95m
End of Borenole		=					:							
(GWL at 0.34 m depth - March 12, 2025)												· · · · · · · · · · · · · · · · · · ·	• •	90
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **ELEVATION: 94.12 EASTING:** 435817.85 **NORTHING:** 5004471.80

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:					DATE: M	larch 7	, 202	5		HOLE	NO. :	BH 6-25		
				s	SAMPLE				DCPT (5	IST. (BLC 0mm DIA	. CONE	)	4	
SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	ER CONTENT (%)	Δ.	UNDRA 20	AINED S 40	HEAR ST	<b>RENGT</b> 60	<b>H (kPa)</b> 80	MONITORING WELL	ELEVATION (m)
GROUND SURFACE	STR/	DEP1	₹	RECO	N OR	WATER (%		PL (%)	WATER 40	R CONTE	<b>NT (%)</b> 60	LL (%)	MON	ELEV
TOPSOIL 0.20m [93.92m]	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	· =	AU 7										0.61 m <u>▼</u> 202	94 - 5-03-12
sand, trace gravel 0.61m[93.52m],  GLACIAL TILL: Dense to very dense, brown sandy silt, with gravel, cobbles and boulders	A A A A A A A A A A A A A A A A	1-	SS 2	79	6-17-13-17 30									93-
2.21m [ 91.92m ]	A A A A A A A A A A A A A A A A A A A A	2	SS 83	46	10-21-34-27 55									92-
GLACIAL TILL: Compact, brown silty sand, trace gravel and cobbles			SS 4	75	7-9-11-11 20									
		3-	SS 5	83	9-12-13-12 25								3.0	<sup>(5m</sup> ษ1-
- Grey by 3.73 m depth		4-	SS 6	43	3-8-50-/ 58/0.25									90-
4.57m [89.56m] End of Borehole		-											4.5	7m
Practical refusal to augering at 4.57 m depth		5-												89
(GWL at 0.61 m depth - March 12, 2025)		6-												88-
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Hang Auger

MARKS:						DATE: /	August 2	21, 202				E NO. :		1-23		_
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							WATER CONTENT (%)		20	40	)	60	80		Z	
SAMPLE DESCRIPTION		STRATA PLOT		TYPE AND NO.	RECOVERY (%)	_	ONT					STRENG STRENG			PIEZOMETER CONSTRUCTION	
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own SILTY CLAY			_					:	:		-		:			
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PAGE: 1/1

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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**EASTING:** 435840.63 **NORTHING:** 5004193.74 **ELEVATION:** 93.84

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Hang Auger

P:/AutoCAD Drawings/Test Hole Data Files/PG42xx/PG4216/data.sqlite 2025-09-11, 11:18 Paterson\_Template

COORD. SYS.: UTM ZONE 18

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SAMPLE DESCRIPTION		STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	Sab	WATER CONTENT (%)		DED SHEAR STE IED SHEAR STE 40 60	RENGTH (kPa)	PIEZOMETER CONSTRUCTION	
		TRAI	EPT	ΥPE	ECO	N OR RQD	WATEI	PL (%) V	VATER CONTEN	T (%) LL (%)	IEZO CONS.	ĺ
OPSOIL (	GROUND SURFACE	S			<u> </u>		>	20	40 60	0 80		+
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **ELEVATION**: 93.78 **EASTING:** 436002.41 **NORTHING:** 5004137.63

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Hang Auger

EMARKS:							August 2	21, 2025	EN DEC			HA 3-25		
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c	ROUND SURFACE	STRAT	DEPTH (m)	TYPE	RECO	N OR RQD	WATER	PL (%)		CONT	ENT (%)	LL (%)	PIEZOI	\ \ \ \ \ \ \
OPSOIL	SKOOND SOKFACE							20	40	:	60	00		
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rown SILTY CLAY			-							-				
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435856.94 **NORTHING:** 5004047.50 **ELEVATION:** 93.88

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Hang Auger

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REMARKS:						DATE: A	August 2		HOLE NO. :			
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SAMPLE DESCRIPTION		STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	▲ UNDRAINED	SHEAR STRENG 40 60	<b>TH (kPa)</b> 80	PIEZOMETER CONSTRUCTION	FI EVATION (m)
(	GROUND SURFACE	STR/	DEP.	¥	REC	N O	WAT	PL (%) WAT	TER CONTENT (%) 40 60	LL (%)	PIEZ	<u>[</u>
OPSOIL												
	0.30m[93.58m]		-									
rown SILTY CLAY			-									
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435913.96 **NORTHING:** 5003979.17 **ELEVATION:** 93.79

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Hang Auger

REMARKS:			I		DATE: A	ugust 2			IO.: HA 5-25		_
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	TRATA	DEPTH (m)	YPE /	ECOV	N OR RQD	WATER	PL (%) V	VATER CONTEN	IT (%) LL <sub>.</sub> (%)	IEZON	EVA
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 436026.01 **NORTHING:** 5003971.20 **ELEVATION:** 93.83

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Hang Auger

REMARKS:						DATE: /	august 2	۷۱, ۷		PEN. RES		BLOWS/0.3	HA 6-25		
					S	AMPLE	¥		20	DCPT (5	0mm	DIA. CONE	E) 80		
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C	GROUND SURFACE	STRAT	DEPTH (m)	TYPE	RECO	N OR RQD	WATE		PL (%)		RCON	<b>ITENT (%)</b> 60		PIEZO	H
TOPSOIL			• -						20	40		- 00			
Brown SILTY CLAY	0.25m [ 93.58m ]		-												
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nd of Hand Auger Hole	0.85m [ 92.98m ]		-	0							-		· · · · · · · · · · · · · · · · · · ·		9
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 436123.11 **NORTHING:** 5003874.87 **ELEVATION:** 94.36

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Hang Auger

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REMARKS:						DATE: /	August 2	۷۱, ۷		PEN. RFS		BLOWS/0.3	HA 7-2	.5		—
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SAMPLE DESCRIPTION		STRATA PLOT	(m) +	TYPE AND NO.	RECOVERY (%)	go	WATER CONTENT (%)	<b>△</b>			HEAR	STRENG STRENGT 60			PIEZOMETER CONSTRUCTION	FI EVATION (m)
G	GROUND SURFACE	STRAT	DEPTH (m)	TYPE	RECO	N OR RQD	WATE		PL (%)		CON	<b>TENT (%)</b> 60			PIEZO CONS	
TOPSOIL									20				- 00			
Brown <b>SILTY CLAY</b>	0.20m[94.16m]		-													
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435747.94 **NORTHING:** 5004424.67 **ELEVATION:** 94.61

PROJECT: Proposed Mixed-Use Development FILE NO. : PG4216

ADVANCED BY: Track Mounted Drill Rig

REMARKS:						DAIE: D	ecemb	er 16, 2024		BH 1-24		
					S	AMPLE	_	DCPT	ESIST. (BLOWS/0 (50mm DIA. CON	E)		
SAMPLE DESCRIPTION	STRATA PLOT	(m) H		I TPE AND NO.	RECOVERY (%)	RQD	WATER CONTENT (%)	20 △ REMOULDEI ▲ UNDRAINEE 20	40 60 D SHEAR STRENG O SHEAR STRENG 40 60		PIEZOMETER CONSTRUCTION	ELEVATION (m)
	TRA.	DEPTH (m)	5	7	ECO	N OR RQD	VATE	PL (%) WAT	TER CONTENT (%		)IEZO	:LEV
GROUND SURFACE  TOPSOIL 0.23m [94 38m]	0,		<b>'</b>	_			>	20	40 60	80 '	E 0	ш
GLACIAL TILL: Compact, brown silty sand, with	$ \triangle \triangle \triangle \triangle$	=	X	AU 1			24	0				
gravel, trace clay, cobbles and boulders	\( \times  \delta   \delta   \delta   \delta   \delta  \delta  \delta   \delta   \delta    \delta	=		٩								94 –
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Dense by 3.0 m depth	A A A A A A A A A A A A A A A A A	3-	7	4								
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Grey by 4.6 m depth	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	=	7	5						× · · · · · · · · · · · · · · · · · · ·		90-
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6.71m [87.90m] End of Borehole	V V V V	=				20						88-
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **ELEVATION**: 94.07 **EASTING:** 435865.23 **NORTHING:** 5004448.81

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:						DATE: D	ecemb	er 1				OLE NO					Т
					S	AMPLE	E		20	DCPT		. (BLOW) m DIA. C	ONE)	<b>n)</b> 80			
SAMPLE DESCRIPTION	STRATA PLOT	(m)	TYPE AND NO.		RECOVERY (%)	ζΦD	WATER CONTENT (%)	<b>∆</b>	REM	DULDEI RAINED	) SHE	AR STRI AR STRE	ENGT ENGTI	H (kPa)		PIEZOMETER CONSTRUCTION	
	TRAT	DEPTH (m)	YE		E 0	N OR RQD	ATE		PL (%)	WAT	TER C	ONTENT	(%)	LL (%	)	IEZOI ONSI	
GROUND SURFACE	S		<b> -</b>	·	~	Z	<b>S</b>		20	:	40	60		80		<u> </u>	H
OPSOIL, trace sand and gravel 0.28m[93.79m]	0 0 0 0			AU 1			16		0	:				:			
<b>LACIAL TILL:</b> Compact to dense, brown silty sand, ith gravel, cobbles and boulders	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	-		₹													
in graver, cobbies and boulders	\( \times  \delta  \delta  \delta  \delta  \delta   \qu	1-	M	22	58	11-11-11-24	11		0			ļļ		i i			
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3_05m[91.02m] Compact to dense, brown <b>SILTY SAND</b> , trace gravel		3-		4						[							
		-	XI	SS 4	25	16-14-13-13 27	23		C	)		<u>.</u>		<u>.</u>			
3.66m[90.41m]	\(\rangle \times \time	=				<u></u>				:				:			
obbles and boulders	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	4 –							ļ								
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6.71m [ 87.36m ]	$\triangle \triangle \triangle \triangle$	=	X	SS	83	9-19-25-24 44	10	'	Э			ļļ.					
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 ELEVATION: 94.06 **EASTING:** 435899.31 **NORTHING:** 5004264.51

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:						DATE: D	ecemb	per 16, 2024	HOLE NO. :		
					S	AMPLE		DCPT (5	SIST. (BLOWS/0.3	<b>:</b> )	
SAMPLE DESCRIPTION	STRATA PLOT	(m) +	TYPE AND NO.		RECOVERY (%)	RQD	WATER CONTENT (%)	A LINDDAINED S	SHEAR STRENG		PIEZOMETER CONSTRUCTION
GROUND SURFACE	STRAI	DEPTH (m)	TYPE		RECO	N OR RQD	WATE		R CONTENT (%)	LL (%)	PIEZO CONS
**OPSOIL, trace clay 0.23m [93.83m],										80	
lard, brown SILTY CLAY		=	X	AU 1			39	0			
		-		7						<b>∆</b> 80 >250	
4.45~ 100.04~ 1		' <u>-</u> -	M	SS 2	100	Р	37	0		Δ00	`
1.45m[ 92.61m]1.45m[ 92.61m]	$\triangle \triangle \triangle \triangle$			က							
avel, trace sand, cobbles and boulders	\( \times  \delta   \delta   \delta  \delta  \delta   \delta   \delta	2 - 2 -	ΙXΙ	SS	50	Р	31	0			!
2.29m [91.77m]	\( \times \q	=									
LACIAL TILL: Compact, brown silty sand, with ravel, trace clay, occasional cobbles	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	-	X	SS 4	58	4-9-7-4 16	12	0			
, , , , , , , , , , , , , , , , , , ,	^ ^ ^ ^ ^	3-				10					
	^ ^ ^ ^ ^	-	X	SS 2	50	8-4-15-19	12	0			
	^ ^ ^ ^ ^	-				19					
	^ ^ ^ ^ ^	4-									
Grey by 4.6 m depth	^ ^ ^ ^ ^	- - -									
эгеу бу 4.0 т черш	^ ^ ^ ^ ^	=	M	SS 6	42	3-17-4-6	11	0			
	^ ^ ^ ^ ^	5-		တ		21					
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	A A A A A A A A	- -	$\overline{}$	_							
6.71m [ 87.35m ]	^ ^ ^ ^ ^	=	$\bigwedge$	SS	42	15-28-13-12 41	14	0			
nd of Borehole		7-									
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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435891.01 **NORTHING:** 5004133.10 **ELEVATION:** 93.78

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Track Mounted Drill Rig

REMARKS:

DATE: December 19, 2024

HOLE NO.: BH 3A-21

**REMARKS:** DATE: December 19, 2024 PEN. RESIST. (BLOWS/0.3m) **SAMPLE** DCPT (50mm DIA. CONE) 20 40 **NATER CONTENT** CONSTRUCTION ġ 8 ELEVATION (m) REMOULDED SHEAR STRENGTH (kPa) STRATA PLOT SAMPLE DESCRIPTION RECOVERY UNDRAINED SHEAR STRENGTH (kPa) TYPE AND DEPTH (m) N OR ROD 40 60 PL (%) WATER CONTENT (%) LL (%) 20 **GROUND SURFACE** 40 60 80 For soil profile refer to BH 3-21 92 2.29m [ 91.49m ] >121 Very stiff, brown SILTY CLAY 100 52 >121 100 44 3.73m [ 90.05m ] Firm, grey SILTY CLAY **SS3 △**10 75 42 >121 0 36 4.88m [ 88.90m ] 100 GLACIAL TILL: Dense, grey silty clay, trace sand, 12 0 gravel, cobbles and boulders 94 2-5-27-50 12 O 5.87m [ 87.91m ] End of Borehole 6 87

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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Track Mounted Drill Rig

REMARKS: December 19, 2024 HOLE NO.: BH 3B-21

REMARKS:						DATE:	Decemb	er 19	, 2024		HOLE N	J. :	BH 3B	5-Z1		
					S	AMPLE			■ P	EN. RE	SIST. (BLOW 50mm DIA. (	S/0.3r	n)			
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0.440, 5.0500,00	"ON	=		ġ.	(%		恒	Δ			SHEAR STR				ᇫᅙᆝ	Ē
SAMPLE DESCRIPT	ION	STRATA PLOT	Ē	TYPE AND NO.	RECOVERY (%)	ے ا	WATER CONTENT (%)	•	UNDR	AINED S	SHEAR STR	ENGT	H (kPa)		PIEZOMETER CONSTRUCTION	ELEVATION (m)
		≰	DEPTH (m)	₹	Š	N OR RQD	H &		20	4	0 60		80		STR	ΔĬ
		≱	ᇤ	YPE	Signal Signal	_ <u>R</u>	ATE	F	PL (%)	WATE	R CONTENT	「(%)	LL (%)			
	GROUND SURFACE	S			<u> </u>		>		20	4	0 60	-	80		<u> </u>	
For soil profile refer to BH 3-21			• ]					:	:	:		:				
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	3.05m [ 90.73m ]		3													J
Brown SILTY CLAY			Ť	_				:	:	:		:				
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	3.81m [ 89.97m ]		‡					:	:	:		:				9(
Grey SILTY CLAY			4-	7												90
				TW 2	100							Ė				
	4.42m [ 89.36m ]	VXX4	3													
End of Borehole			=					:	:	:		:				89
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

- Coro and Coco Chara Chook, Char

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435936.41 **NORTHING:** 5004099.07 **ELEVATION:** 93.87

PROJECT: Proposed Mixed-Use Development FILE NO. : PG4216

ADVANCED BY: Track Mounted Drill Rig

REMARKS:						DATE: [	ecemb	er 1	16, 2024	HOLE NO. :	BH 4-24	
					S	AMPLE				ESIST. (BLOWS/0.3 (50mm DIA. CONE		
SAMPLE DESCRIPTION	STRATA PLOT	DЕРТН (m)	ON OND HOYT		RECOVERY (%)	N OR RQD	WATER CONTENT (%)	Δ.	20  REMOULDED  UNDRAINED 20  PL (%) WAT	40 60 SHEAR STRENG SHEAR STRENG 40 60 ER CONTENT (%)	80 TH (kPa) FH (kPa) 80 LL (%)	PIEZOMETER CONSTRUCTION
GROUND SURFACE  10 PSOIL 0.25m [ 93 62m ]				+	<u> </u>		>		20	40 60	80 '	E 0 1
Hard, brown SILTY CLAY		- - -		AU 1			43			0		
		1— 1— - -	X	SS 2	50	Р	36		O		▲80	, 9
		2-	X	SS 3	100	Р	50			<b>A</b> 50	>249	9
Very stiff by 2.3 m depth		- - - - - -	X	SS 4	100	Р	57		29	Δ.665	179	
3.50m[90.37m] GLACIAL TILL: Compact, grey silty sand, with clay,	\D \D \D \D \d	3-	X	SS 5		Р	37 44		∆29 C	0	>121	
ravel, cobbles and boulders	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4-	X	SS 6	100	Р	31	c	0			g
	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	5—	X	SS 7	50	2-6-9-5 15	10		O			8
	A A A A A A A A A A A A A A A A	6-										8
6.71m [ 87.16m ]	A A A A A A A A A A A A A A A A A A A	- - - -	X	SS 8	67	3-3-3-2 6	14		0			
nd of Borehole		7-										8
		8-										8
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		9-										
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PAGE: 1/1

P:/Autocad Drawings/Test Hole Data Files/PG42xx/PG4216/data.sqlite 2025-03-14, 14:03 Paterson\_Template



ADVANCED BY: Track Mounted Drill Rig

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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 ELEVATION: 93.87 **EASTING:** 435936.41 **NORTHING:** 5004099.07

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** 

EMARKS:						DATE: I	Decemb	er 19				LE NO. :		A-24	}	
					S	AMPLE				DCPT (5	0mm	BLOWS/0. DIA. CON	Ε)			
SAMPLE DESCRIPTION		STRATA PLOT	DЕРТН (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	<b>△</b>			SHEAR SHEAR	60 R STRENG STRENG 60			PIEZOMETER CONSTRUCTION	FI EVATION (m)
		RAT	E	ᇤ	ုင္ပ	K	崑		PL (%)	WATE	R CON	ITENT (%)	LL (%	5)	ISN	N
GRO	OUND SURFACE	ST	<u></u>	≱	쀭	ž	≱		20	40	) 0	60	80		₩8	ū
or soil profile refer to BH 4-24			. =							: :	:		:	:		
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	2.90m [ 90.97m ]	717 7	. ]								1		•	:		(
own SILTY CLAY			3-	TWT	100											
	3.51m [ 90.36m ]		=		100						- 1		•	:		
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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

ELEVATION: 93.90 COORD. SYS.: UTM ZONE 18 **EASTING:** 435994.61 **NORTHING:** 5003956.27

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:						DATE: [	ecemb	per 16, 2024	HOLE NO.: BH 5-2	4
				$\overline{}$	S	AMPLE	1.	DCPT (5	SIST. (BLOWS/0.3m) 50mm DIA. CONE)	
SAMPLE DESCRIPTION	STRATA PLOT	H (m)	TVDE AND NO	AND NO.	RECOVERY (%)	RQD	WATER CONTENT (%)	20 40  △ REMOULDED S  ▲ UNDRAINED S  20 40	SHEAR STRENGTH (kPa) SHEAR STRENGTH (kPa)	PIEZOMETER CONSTRUCTION ELEVATION (m)
GROUND SURFACE	STRA	DEPTH (m)	2	1	RECO	N OR RQD	WATE	PL (%) WATE	ER CONTENT (%) LL (%)	PIEZO CONS ELEVA
OPSOIL 0.25m [93.65m]	/			_			39	0		
Hard, brown SILTY CLAY		=		AU 1						
		1-							>2 <b>A</b> 89	93 -
			M	SS 2	100	P	52	<b>4</b> 20	0	49 92-
		2-								
		=						▲ 29	<b>&gt;</b>	21
	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	3-								91-
ravel, cobbles and boulders	A A A 4	-	X	SS 3	42	4-5-4-6	9	0		
Grey by 3.4 m depth	A A A A A A A A A A A A A A A A A A A	=				9	''			90-
	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	4-								
Dense by 4.6 m depth	A A A 4	-								
	A A A A	5-	X	SS 4		5-10-29-24 39	9	0		
5.36m [88.54m]	A A A A									
ind of Borehole		=								
ractical refusal to augering at 5.36 m depth		6-								88 -
		=								
		7-								87-
		=								
		=								
		8-								86-
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		9-								85-
		10		Ш						84-

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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **ELEVATION**: 94.24 **EASTING:** 435422.68 **NORTHING:** 5003879.79

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:					DATE: D	ecemb	er 17	, 2024		HOLE	E NO. :	BH 6	-24		
				5	SAMPLE				PEN. RES DCPT (5 40	0mm DI		E)			
SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	RQD	R CONTENT (%)	Δ		ULDED S RAINED S 40	HEAR S	STRENG 60	80		PIEZOMETER CONSTRUCTION	ELEVATION (m)
GROUND SURFACE	STRA	DEPT	TYPE	RECO	N OR RQD	WATER (%	ı	PL (%)	WATEF 40	CONT	<b>ENT (%</b> )	LL (%	)	PIEZC	ELEV.
TOPSOIL, trace clay 0.05m[94.19m] GLACIAL TILL: Compact, brown sandy clay, with silt,	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	. =	₩ 7			39		20	0		00	00			94 -
gravel, cobbles and boulders	A A A A A A A A A A A A A A A A A A A A	1-	SS 2	100	P	30			0						93 -
	A A A A A A A A A A A A A A A A A A A A	2-	SS 3	50	3-3-3-2 6	14		0							92-
	\times \t	3—													
	A A A A A A A A A A A A A A A A A A A A	- - - - - -	SS 4 SS	92	6-8-3-2 11	13		0							91-
4 <u>.</u> 57m[89.67m]	A A A A A A A A A A A A A A A A A A A A	4-													90 -
GLACIAL TILL: Compact grey silty sand, with gravel, cobbles and boulders	A A A A A A A A A A A A A A A A A A A	5-	SS 5	83	8-13-18-19 31	9	0								89-
6 <u>.10m [ 88.14</u> m ]	A A A A A A A A A A A A A A A A A A A	6—													
GLACIAL TILL: Compact, grey sand, with cobbles and boulders  6.71m [87.53m]  End of Borehole	\(\rangle \times \rangle \times \ran	- - - -	SS 6	100	2-3-16-43 19	19		0							88-
Lita of Borefloie		7-													87 -
		8-													86 -
		9—													
		- - - -													85-

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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435600.21 **NORTHING:** 5004031.42 **ELEVATION:** 93.94

PROJECT: Proposed Mixed-Use Development FILE NO. : PG4216

ADVANCED BY: Track Mounted Drill Rig

REMARKS:						DATE: D	ecemb	er 1	7, 2	024			HOLI	E NO.	.:	BH 7	<b>'-24</b>		
					S	AMPLE								LOWS		1)			
SAMPLE DESCRIPTION	STRATA PLOT	(m)	ON CINE	AND NO.	RECOVERY (%)	Rab	WATER CONTENT (%)	Δ.				40 60 80  LDED SHEAR STRENGTH (kPa) INED SHEAR STRENGTH (kPa) 40 60 80				PIEZOMETER CONSTRUCTION	ELEVATION (m)		
	TRAT	DEPTH (m)	5	7	50	N OR RQD	/ATEF		PL	(%)	WA	TER	CONT	ΓENT (	(%)	LL (%	6)	IEZOI ONSI	LEVA
GROUND SURFACE TOPSOIL 0.20m [93.74m]			-	_	~	Z	>		:	20	:	40	-	60	:	80	:	<u> </u>	ш
GLACIAL TILL: Brown silty sand, with gravel, cobbles and boulders	^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^	-		AU 1			34				0								-
	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1-	X	SS 2	75	13-20-12-8 32	10		<b>)</b>										93-
- Increasing clay content by 1.5 m depth	V V V V V V V V V V V V V V V V V V V	2-	X	SS 3	75	4-8-4-4 12	11	(	0										92
- Increasing sand content by 3.0 m depth	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3-																	91
- Grey by 3.5 m depth 3.71m [90.23m]	^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^	- - - -	X	SS 4	83	1-20-23-16 43	9	c	)										-
End of Borehole		4-							: !	i.			. į			i.			90-
Practical refusal to augering at 3.71 m depth		-																	-
		5-																	89
		6-	-																88
		- - - -																	-
		7-																	87-
		8- -	-																86
		- - - - -																	-
		9-																	85 <u> </u>
		10 -													:	-	:		84

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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **EASTING:** 435672.21 **NORTHING:** 5003946.75 **ELEVATION**: 94.52

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:						DAIE: L	ecemb	oer 17, 2024			BH 8-24		
					S	AMPLE		D	N. RESIST. ( CPT (50mm	DIA. CONE	:)		
SAMPLE DESCRIPTION	STRATA PLOT	(m)	TYPE AND NO.		RECOVERY (%)	αορ	WATER CONTENT (%)	20 △ REMOUI ▲ UNDRA 20	40 LDED SHEAR NED SHEAR 40			PIEZOMETER CONSTRUCTION	ELEVATION (m)
	TRAT	DEPTH (m)	5	7	ECO	N OR RQD	ATEF	PL (%)	WATER CO	NTENT (%)	LL (%)	IEZOI	EVA
GROUND SURFACE  OPSOIL  0 18m [ 94 34m] /			F	_	~	z	<b>S</b>	20	40	60	80 '	<b>₽</b> Ω	ш
GLACIAL TILL: Compact, brown silty sand, with clay, ravel, cobbles and boulders	A A A A A A A A A A A A A A A A	-	X	AU 1			22	o					94
,	^ ^ ^ ^ 7	1-	X	SS 2	33	4-9-5-3 14	25	0					
	^ ^ ^ ^ ^	2-	X	SS 3	67	6-5-4-6 9	14	o					93
	A A A A	 - - -		4		9							92
Increasing sand content by 3.0 m depth	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3-	X	SS 4		12-50-/-/	11	O					
3.53m [ 90.99m ] and of Borehole	<u> </u>	-				50/0.08							91
ractical refusal to augering at 3.53 m depth		4-											
		5-											90
		- - - -											89
		6											
		-											88
		7-					,						
		-											87
		8- - - - -											80
		9-											
		-											8

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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Track Mounted Drill Rig

P:/Autocad Drawings/Test Hole Data Files/PG42xx/PG4216/data.sqlite 2025-03-14, 14:03 Paterson\_Template

REMARKS: December 17, 2024 HOLE NO.: BH 9-24

REMARKS:					DATE: [	Decemb	er 1	7, 20	24		HO	LE NC	<b>)</b> . :	BH 9	9-24		
				s	AMPLE					CPT (	50mm	BLOWS DIA. C					
SAMPLE DESCRIPTION	STRATA PLOT	DЕРТН (m)	TYPE AND NO.		N OR RQD	ER CONTENT (%)	Δ	20 40 60 80  △ REMOULDED SHEAR STRENGTH (kPa)  ■ UNDRAINED SHEAR STRENGTH (kPa)  20 40 60 80					MONITORING WELL CONSTRUCTION	ELEVATION (m)			
GROUND SURFACE	STRA	DEPT	TYPE	RECOVERY (%)	N OR	WATER (%)		PL (%	<b>6)</b> 20		R CON	NTENT 60	(%)	LL (%	6)	MON	ELEV
TOPSOIL 0.25m[93.93m], Hard, brown SILTY CLAY		• -	¥ X			31				0			-	:			94-
Tidid, blown die i deni		1-	SS 2 A	58	P	25			0						∆89 A	92 m <b></b> 2	_ :
1.45m[92.73m] _ Compact, brown <b>SILTY SAND</b> , with clay, gravel,						23			0								93-
cobbles and boulders	A A A A	2	SS3	100	2-5-5-3 10	17		0									92
cobbles and boulders 2.97m [91.21m]	^ ^ ^ ^ ^																92 –
BEDROCK: Excellent to good quality limestone		3-	RC 1	100	RQD 90												91-
		4-						· · · · · · · · · · · · · · · · · · ·									90-
		5-	RC 2	100	RQD 92												4.52m 89 –
6.17m [88.01m]		6	RC 3	83	RQD 74												6.05m
End of Borehole									: : :								88-
(GWL at 0.92 m depth - January 9, 2025)		7-															87 -
		-															
		8-															86-
		9-															
		-															85-
		10						:				:	:	:	:		

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## **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435719.68 **NORTHING:** 5004015.59 **ELEVATION:** 94.18

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Track Mounted Drill Rig

REMARKS: DATE: December 18, 2024 HOLE NO.: BH10-24

REMARKS:					DATE: [	Decemb	er 18	3, 2024		HOLI	E NO. :	BH10-2	4		
				S	AMPLE			■ PEN. RESIST. (BLOWS/0.3m) DCPT (50mm DIA. CONE)							
SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	Δ.	UNDR 20	4 ULDED AINED \$	SHEAR SHEAR SHEAR	MONITORING WELL CONSTRUCTION	ELEVATION (m)			
GROUND SURFACE	STR/	Eb.	Τ	REC	N O	WAT		PL (%)	WATE	R CONT	<b>FENT (%)</b> 60	LL (%)	MON	E.E.	
For soil profile refer to BH 9-24		1-						20					0.94 m ¥ 202	94 -	
		2-											1.	93 - 45m 92 -	
2.97m [91.21m] End of Borehole		3-												92 92 97 97 97 91 91 91 91 91 91 91 91 91 91 91 91 91	
Practical refusal to augering at 2.97 m depth		-													
(GWL at 0.94 m depth - January 9, 2025)		4-												90 -	
		5-												89	
		6-												88 -	
		7-												87-	
		8-												86-	
		9-												85-	
		10													

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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **EASTING:** 435692.30 **NORTHING:** 5004138.10 **ELEVATION**: 93.82

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

REMARKS:					DATE: [	ecemb	er 1	3, 202	24	H	OLE NO	: BH	111-24		
				S	AMPLE				DCF	PT (50m	(BLOWS	ONE)		Ⅎ	
SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	ER CONTENT (%)	Δ	<b>UN</b> I	MOULD Draine	ED SHEA 40	60 AR STRE AR STREM	NGTH (kF 80	Pa) Pa)	MONITORING WELL CONSTRUCTION	ELEVATION (m)
GROUND SURFACE	STR/	DEP1	TYPE	RECO	N OR	WATER (%)		PL (%	6) W	ATER Co	ONTENT (	( <b>%)</b> LL	. <b>(%)</b>	MON	ELEV
TOPSOIL 0.25m [93.57m]		. =				30			0	-10			:		3
Very stiff, brown SILTY CLAY		-	<b>X</b> ₹	2		30									3
		1_											180		93-
		' - -													3
		=		,									<b>∆</b> 89		}
		2	\\ v	100	Р	47				С	1		1.	98 m <b>⊻</b> 201	92 - 25-01-09
		-													3
		=						Δ14				<b>▲</b> 72			2
		3											>121	3 6	91-
Grey by 3.4 m depth		=	X v	75	Р	26 22			0			Δ72			<u>-</u> - -
3.66m[90.16m] LACIAL TILL: Loose to dense, grey silty sand, with	V V V V	=													90 -
ay, gravel, cobbles and boulders	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4-	V V	67	0-1-1-/	15		0							
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	-			2									4	.58m
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	_	\\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\	67	2-3-3-5	12		0							89 -
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5—			6										
	^ ^ ^ ^ /	=													
	^ ^ ^ ^	6—													88 -
0.50 (0.700 )	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	=	٧٧	42	13-21-50-/	11		0	•					6	i.10m
6.53m [87.29m] nd of Borehole	V V V V	-			71/0.23	''		Ŭ							
		7													87-
ractical refusal to augering at 6.53 m depth		=							-	•			•		
GWL at 1.98 m depth - January 9, 2025)		-													86-
		8-													
		=													
		=							-				:		85-
		9-													
		=													
		10 -							:	:			:		84 -

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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Track Mounted Drill Rig

REMARKS: December 18, 2024 HOLE NO.: BH12-24

REMARKS:					DATE: D	ecemb	18, 2024 HOLE I	NO.: BH12-24	
				S	AMPLE		PEN. RESIST. (BLO DCPT (50mm DIA.		
SAMPLE DESCRIPTION	STRATA PLOT	DЕРТН (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	20 40 6  REMOULDED SHEAR ST  UNDRAINED SHEAR ST	0 80 RENGTH (kPa) RENGTH (kPa) 0 80	MONITORING WELL CONSTRUCTION ELEVATION (m)
GROUND SURFACE	ST	ä	Ĕ	RE	0 2	¥	· · · · · · · · · · · · · · · · · · ·	0 80	COI
TOPSOIL  0.36m [ 93.57m ]  Hard, brown SILTY CLAY		1-	AU1			36	o	239	93 – 08 m¥ 2025-01-09
		2-/	SS 2	100	Р	54	25 54 <b>⊕</b>	199	92-
- Silt content increasing by 3.0 m depth 3.73m [ 90.20m ]		3	SS 3	83	Р	29	0	>121 \delta 87	91-
GLACIAL TILL: Compact, grey silty clay, with sand, gravel, cobbles and boulders  4.57m [ 89.36m ]	\( \times \) \( \t	4	SS 4	67	9-3-9-9 12	15	0		90-
<b>GLACIAL TILL:</b> Dense, grey silty sand, with gravel, cobbles and boulders, trace clay	A A A A A A A A A A A A A A A A A A A	5	SS 5	67		18	O		5.34m
	A A A A A A A A	6	9		37				88-
End of Borehole	V V V V	7-	SS	100	50-/-/-/ 50/0.08	16	0		6.86m <sub>o</sub> 7 –
(GWL at 1.08 m depth - January 9, 2025)		8-							86 -
		9-							85 -
DISCLAIMER: THE DATA PRESENTED IN THIS SHEET IS TH	IE DDOD	10	E DATE	PSO		ND THE		OODLICED THIS SUFF	84 –

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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Track Mounted Drill Rig

REMARKS: December 19, 2024 HOLE NO.: BH12A-24

REMARKS:					DATE:	Decemb	er 19,	2024		HOLI	E NO. :	BH12	A-24		
				S	AMPLE			<b>=</b> F	EN. RE	SIST. (BI	LOWS/0.3 DIA. CONE	m) )			
						þ		20		0	60	80		_	
CAMPLE DESCRIPTION	5		Š.	%		WATER CONTENT (%)	Δ	REMO	ULDED	SHEAR	STRENG			PIEZOMETER CONSTRUCTION	(E
SAMPLE DESCRIPTION	STRATA PLOT	E	TYPE AND NO.	RECOVERY (%)	٩	Ö.	<b>A</b>	UNDR	AINED :	SHEAR	STRENGT	H (kPa)			ELEVATION (m)
	¥	E	₹	🖔	8	ା <sup>ଅ</sup> ଚ	_	20	4	10	60	80		STR	¥
	₹	DEPTH (m)	YE	Signal Signal	N OR RQD	ATE	P	L (%)	WATE	R CONT	TENT (%)	LL (%)		S EZ	
GROUND SURFACE	S			<u>~</u>	Z	>		20	4	10	60	80		<u> </u>	ш
For soil profile refer to BH 12-24							:	- 1	:		1 1	:			
		=											:		93
		=					:	:	:			:			
		1_													
		1													
		3													92
		=					:					:			02
		2													
									:			:	:		
		1					:	Ė	:	<u> </u>		i	:		9
		=													9
3.05m [ 90.45m ]		٦					:					:			
rown SILTY CLAY, with high silt content	TXX	3-													
TOWN SILT I CEAT, WITH HIGH SIIL CONTENT			T WT	96			:					:			_
2 00 [ 00 70 ]		=					:								9
3.80m[89.70m]	V V V	. =					:	:	:			:			
SEACIAL FILE	A A A A A A A A A A A A A A A A A A A	4 =	TW 2	62						1 1 1 1					
4.42m [ 89.08m ]	A A A A	=					:								
nd of Borehole								1							89
		3													
		5							!	Programme (		T			
		=					:					: :			
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		3							:			:			
		6-													
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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Track Mounted Drill Rig

REMARKS: Date: December 19, 2024 HOLE NO.: BH12B-24

REMARKS:					DATE:	Decemb	er 19, 20	24	НОІ	LE NO. :	BH12B	-24	
				S	AMPLE	<u> </u>		DCPT		BLOWS/0.3 DIA. CONE 60			
SAMPLE DESCRIPTION	STRATA PLOT	(m) +	TYPE AND NO.	RECOVERY (%)	Rab	WATER CONTENT (%)	△ RE	MOULDED DRAINED	SHEAF	R STRENG R STRENG	TH (kPa)	PIEZOMETER CONSTRUCTION	ELEVATION (m)
	TRAI	DEPTH (m)	YPE	ECO	N OR RQD	WATE	PL (	6) WAT	ER CON	NTENT (%)	LL (%)	IEZO CONS	FK
GROUND SURF. For soil profile refer to BH 12-24	ACE 0		<u> </u>	<u> </u>	_ Z	>	2	20	40	60	80	10	- ш
Tot soil profile refer to Bit 12-24													93-
		=											95
		1-											
		=											00
													92-
		2-							<u>.</u>				
		=					:						
2.74m [ 90.7	[6m]	_					:	· · · · · · · · · · · · · · · · · · ·					91-
Brown SILTY CLAY		3-	L WT	92									
3.35m [ 90.1	5m]	1 =					:						
End of Borehole		=											90-
		4-						: :					
		=					:						
		=					<u>.</u>						89-
		5-											
		=						ii i i					88-
							:						
		6-											
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		=											86-
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development FILE NO. : PG4216

ADVANCED BY: Track Mounted Drill Rig

EMARKS:						DATE: D	ecemb	er 1	9, 2024		HOL	E NO. :	BH13-	24		
					S	AMPLE						LOWS/0.3		$\top$		
					_		k		20	40	)	60	80		z	_
SAMPLE DESCRIPTION	STRATA PLOT			IYPE AND NO.	RECOVERY (%)		WATER CONTENT (%)	△				STRENGT STRENGT			PIEZOMETER CONSTRUCTION	(w) NOITAN
	ATA	DEPTH (m)	1	A A	OVE	N OR RQD	ER C		20	4(	0	60	80		STR	Ė
GROUND SURFACE	STR	퓜	}	<u> </u>	REC	Ō	WAT		PL (%)	40	-	60 FENT (%)	LL (%)		Sez	<u> </u>
ard to very stiff, brown SILTY CLAY			V	SS 1	50	4-4-4-3	36			0	-					
		=	$\Lambda$	Š	30	8										
		-									-			100		9
		1— -												109 >249		9.
ACIAL TILL: Compact, brown silty clay, with sand,	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \			,												
avel, cobble sand boulders	A A A A	-	X	SS 2	92	1-2-8-9	14		0		- - - -					
,	\( \times  \delta   \delta   \delta  \delta  \delta   \delta   \delta	2-	/ \			10										9
	\( \times  \q	-	V	SS 3	400	2445	40									
	A A A A	=	$\backslash$	SS	100	3-4-4-5 8	13		0		-					
ilty sand, with clay, gravel, cobbles and boulders	\( \triangle \tr	3-		_												9
3 3 m donth	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	-	X	SS 4	67	5-8-18-16 26	12		0							
3.66m [90.30m] d of Borehole	\ \ \ \ \ \ \ \ \ \	=				20					-					
		4-														9
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PAGE: 1/1

P:/Autocad Drawings/Test Hole Data Files/PG42xx/PG4216/data.sqlite 2025-03-14, 14:03 Paterson\_Template



#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **EASTING:** 435792.21 **NORTHING:** 5004358.09 **ELEVATION**: 93.96

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** ADVANCED BY: Track Mounted Drill Rig

EMARKS:					DATE:	Decemb	er 19	9, 2024		HOL	_E NO. :	BH1	3A-2	4	
				S	AMPLE				DCPT (5	50mm	BLOWS/0. DIA. CON	E)			
SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	Δ.			SHEAF SHEAR	60 R STRENC STRENG 60			PIEZOMETER CONSTRUCTION	FI FVATION (m)
	RAT	Ħ	Ŗ,	ြင္ယ	%	IE I		PL (%)	WATE	R CON	ITENT (%	) LL (	<b>6</b> )	ISNO	X
GROUND SURFACE	ST	<u> </u>	₽	22	z	*		20	4(	0	60	80		≣ ੪	ū
or soil profile refer to BH 13-24											:				
rown SILTY CLAY		=	_												
		-	WT 1	62					: :	:	:		:		
0.99m [92.97m] nd of Borehole	<i>YXX</i> 4	1-													93
ind of Boreffole		=								-	:		:		
		_													
		_ =											i		92
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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 436034.93 **NORTHING:** 5004037.45 **ELEVATION:** 93.95

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Track Mounted Drill Rig

REMARKS: December 19, 2024 HOLE NO.: BH14-24

REMARKS:					DATE: D	ecemb	oer 19	9, 2024	Н	DLE NO. :	BH14-24		
				S	SAMPLE			<b>■</b> F	PEN. RESIST. DCPT (50mr		IE)		
SAMPLE DESCRIPTION	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	Δ		40 ULDED SHEA AINED SHEA 40	AR STRENG	80 80	PIEZOMETER CONSTRUCTION	ELEVATION (m)
	TRA	EPT	YPE	ECO	R	VATE		PL (%)	WATER CO	ONTENT (%	6) LL (%)	)IEZC	.rev
GROUND SURFACE For soil profile refer to TP 6	<u> </u>			-		>		20	40	60	80		
Tor soil profile refer to 11 o		=											
		=											
		1-											93-
		=											
		2											92-
2.29m [ 91.66m	- 17/7	1								· · · · · · · · · · · · · · · · · · ·	>121		
Very stiff, brown SILTY CLAY, with high silt content		= '	SS 1	83	Р	36		22  -	—¦° 0 △43				
2.97m [ 90.98m	]	3—									· · · · · · · · · · · · · · · · · · ·		91-
GLACIAL TILL: Dense to compact, brown silty clay,	A A A A		SS 2	50	5-12-6-3	11		0		· · · · · · · · · · · · · · · · · · ·	>121		
with sand, gravel, cobbles and boulders	\( \times \q	1	\\\	50	18	''		0		<u></u>			
	A A A A	1											90 -
	\(\rangle \times \rangle \times \rangle \times \rangle \times \rangle \times \rangle \rangle \times \rangle \r	4 7	SS 3	33	4-3-2-2 5	10		)					
- Silty sand by 4.2 m depth	\( \times  \delta  \q	7			3								
only cand by the macpan	\( \delta \q \delta \de		SS 4	42	23-26-20-16	12		0					
5.18m [ 88.77m	I	5-1/	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		46	· <del>-</del>							89-
End of Borehole										İ			
		6-											88-
		7-											87 -
		=											
		8-											86-
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		10											84_

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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Track Mounted Drill Rig

REMARKS: December 19, 2024 HOLE NO.: BH14A-24

REMARKS:					DATE:	Decemb	er 19, 2	2024		HOLE N	10. :	BH14	A-24	1	
				S	AMPLE					ST. (BLOV )mm DIA.	CONE)	1			
SAMPLE DESCRIPTION	STRATA PLOT	(m)	TYPE AND NO.	RECOVERY (%)	N OR RQD	WATER CONTENT (%)	△ R		JLDED S	HEAR STE	RENGT RENGT			PIEZOMETER CONSTRUCTION	ELEVATION (m)
	IRA	DEPTH (m)	YPE	SO	S. I	ATEI	PL	. (%)	WATER	CONTEN	IT (%)	LL (%)		IEZO ONS.	LEVA
GROUND SURFACE	Ś		<u> </u>	~	Z	<b>       </b>		20	40	60	0	80		<b>₽</b> Ω	ш
For soil profile refer to TP 6							:				:	:			
										*********					
		1_													93
															92
2.29m [ 91.66m ]		2-					· · · · · · · · · · · · · · · · · · ·					:			32
Brown SILTY CLAY															
2.90m [ 91.05m ]		=	T MT	96							1				
End of Borehole	7 7 7 1 7	3													91
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		4-													90
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		]													89
		5-													03
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		9-					<u>:</u>								85
		10										:			84

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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

ELEVATION: 94.53 COORD. SYS.: UTM ZONE 18 **EASTING:** 436079.41 **NORTHING:** 5003734.86

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** 

**ADVANCED BY:** Excavator

P:/Autocad Drawings/Test Hole Data Files/PG42xx/PG4216/data.sqlite 2025-03-14, 10:44 Paterson\_Template

EMARKS:	, ,				DATE:	Decemb	er 10	6, 202	24		HOLE	: NO. :	11	1-24		
				;	SAMPLE	1.			DC	CPT (50	mm D	.OWS/0. IA. CON	E)			
SAMPLE DESCRIPTION	STRATA PLOT	DЕРТН (m)	TYPE AND NO.	RECOVERY (%)	SQD	WATER CONTENT (%)	Δ.		/IOULI		HEAR :	60 STRENC STRENG 60		Pa) Pa)	PIEZOMETER CONSTRUCTION	EI EVATION (m)
	RA	ᇤ	Æ		N OR RQD	ATE		PL (%	6) V	NATER	CONT	ENT (%	) LL	. (%)	EZO ONS	1
GROUND SURFACE	·Ω		<u> </u>	<u>~</u>	Z	>			0	40		60	80	<u>'</u>	<u> </u>	ū
DPSOIL0.20m[94.33m]																
iff, brown SILTY CLAY		-														
		-	<u>.</u>	-		34				0						
		-							<b>∆</b> 22					<b>▲</b> 88		9
									Δ22		<u>.</u>			▲00		
		_														
		1—														
		-							:	:						
1.40m [ 93.12m ]		-														
ACIAL TILL: Compact, brown silty clay, some	<u> </u>	_						: :			į					
vel, occasional cobbles and boulder	\[ \times  \q	-														
	$ \begin{array}{c c} & & & & & & \\ & & & & & & \\ & & & & & $	_														
	$ \begin{picture}(20,0) \put(0,0){\line(1,0){10}} \put(0,$	-														
2.15m [ 92.38m ]	$ \begin{picture}(20,0) \put(0,0){\line(1,0){12}} \put(0,$	2-	ن 🗏	2 2		27			0							
d of Test Pit	V V V V	-														
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st pit terminated on bedrock surface		-														,
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groundwater infiltration was observed upon		-														
mpletion of the test pit		3-														
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Supplemental Geotechnical Investigation

5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435766.77 **NORTHING:** 5003865.03 **ELEVATION:** 95.89

PROJECT: Proposed Mixed-Use Development FILE NO. : PG4216

**ADVANCED BY:** Excavator

P:/Autocad Drawings/Test Hole Data Files/PG42xx/PG4216/data.sqlite 2025-03-14, 10:44 Paterson\_Template

REMARKS: December 16, 2024 HOLE NO.: TP 2-24

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					SA	MPLE								WS/0.3 . CONE				
							F		2	20		40		80	80		_	
SAMPLE DESCRIPTION	6		TYPE AND NO.		8		WATER CONTENT (%)	Δ						RENG			PIEZOMETER CONSTRUCTION	ELEVATION (m)
	STRATA PLOT	DEPTH (m)	N ON O		RECOVERY (%)	N OR RQD	ည် (န	•		<b>IDR/</b> 20		<b>SHE</b> 40		RENGT 30	<b>H (kPa</b> 80	)	METE	
	RA	蓝	Ä		ဒ္ဌ	OR F			PL (					NT (%)	LL (	%)	EZOI	ĕ
GROUND SURFACE	်	<u> </u>	_	-	22	z				20		40	- 6	0	80		≣ ੪	ᇳ
TOPSOIL 0.15m [ 95.74m ]															i			
GLACIAL TILL: Compact to very dense, brown silty	^ ^ ^ ^ /	_							: 	ļ								
sand, some gravel, cobbles and boulders	$ \begin{picture}(20,0) \put(0,0){\line(1,0){10}} \put(0,$	-													:			
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- Increasing boulder content by 2.7 m depth	^ ^ ^ ^ ^	_													:			
	0 0 0 0	-																00
3.00m [ 92.89m ]	A A A 4	3-								į.,	<u>.</u>		. <u>.</u>					93
End of Test Pit		-																
Practical refusal to excavation at 3.00 m depth		_							: :				. [					
		-																
Groundwater infiltration was observed the bottom of		-								<u>.</u>								
the test pit		_									1		i		i			
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**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

ELEVATION: 97.15 COORD. SYS.: UTM ZONE 18 **EASTING:** 435639.53 **NORTHING:** 5003762.56

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** 

**ADVANCED BY:** Excavator

EMARKS:					DATE: I	Decemb	er 1	6, 2024		HOLE NO. :	TP 3-24	
				s	AMPLE			DC	PT (50	ST. (BLOWS/0. mm DIA. CON	E)	
SAMPLE DESCRIPTION	PO		9.	(%)		WATER CONTENT (%)	Δ			60 IEAR STRENG		A TION
OAMI EE DEGOMI HON	STRATA PLOT	DEPTH (m)	TYPE AND NO.	RECOVERY (%)	Rad	(%)	<b>A</b>	20	40	EAR STRENG 60	80	PIEZOMETER CONSTRUCTION
GROUND SURFACE	STRA	DEPT	TYPE	RECC	N OR RQD	WATE		PL (%) W	<b>VATER</b> 40	<b>CONTENT (%)</b> 60	) LL (%)	PIEZC
DPSOIL 0.20m [96.95m]		٠ _										9
ACIAL TILL: Compact, brown silty sand, some	^ ^ ^ ^ ^	-										
vel and cobbles, occasional boulders	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	-										
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	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	-										
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1.95m [ 95.20m ]	0 0 0 0	-										
of Test Pit		2-										
t nit terminated on hadrack curface		-						<u> </u>				9
t pit terminated on bedrock surface		-							:			
groundwater infiltration was observed upon		-										
npletion of the test pit		-							<u>.</u>			
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

**ADVANCED BY:** Excavator

P:/Autocad Drawings/Test Hole Data Files/PG42xx/PG4216/data.sqlite 2025-03-14, 10:44 Paterson\_Template

REMARKS: Date: December 16, 2024 HOLE NO.: TP 4-24

EMARKS:					DATE: [	Decemb	er 10	6, 2024		HOLE N	0.: <b>T</b> I	P 4-24		
				S	AMPLE					IST. (BLOW 0mm DIA. (				
						EN		20	40	60		30		_
SAMPLE DESCRIPTION	STRATA PLOT	<u>_</u>	TYPE AND NO.	RECOVERY (%)		WATER CONTENT (%)	△			HEAR STR			PIEZOMETER CONSTRUCTION	(m) NOITV/10
	ATA	DEPTH (m)	A AN	OVE	N OR RQD	ER C		20 PL (%)	40	60	8	30	OME	I
GROUND SURFACE	STR	DEP	₹	REC	ō	WAT		20	40	CONTENT 60	1 (%) L	<b>-L (%)</b>	SEZ	Ū
OPSOIL														
0_25m[94.69m] ompact, brown <b>SILT</b> , trace clay and sand		-												
ompact, brown <b>SiL1</b> , trace day and sand		-												
		_	<u> </u>			33			0					
		-												
1.00m [ 93.94m ]		-												9
ACIAL TILL: Loose, brown silt, some clay and	A A A A	1—												
vel	^ ^ ^ ^ ^	-												
	A A A A	-									:			
	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	-												
	A A A A	-												
	A A A A	-	G 2			26		0						ç
2.05m [92.89m] d of Test Pit	A A A A	2-												
d of restric		-												
st pit terminated on bedrock surface		_												
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oundwater infiltration was observed at 1.30 m pth		-												
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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **ELEVATION**: 94.65 **EASTING:** 435320.86 **NORTHING:** 5003704.85

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216 ADVANCED BY:** Excavator

REMARKS:					DATE: I	Decemb	er 10	6, 2024	HOLE NO. :	TP 5-24	
				S	AMPLE			DCPT	SIST. (BLOWS/0.3 (50mm DIA. CONE	Ε)	
	F		<u>o</u>	(%		EN EN	Δ		40 60 SHEAR STRENG	80 TH (kPa)	NO .
SAMPLE DESCRIPTION	STRATA PLOT	(E)	TYPE AND NO.	RECOVERY (%)	8	WATER CONTENT (%)	<u> </u>	UNDRAINED	SHEAR STRENG		PIEZOMETER CONSTRUCTION
	RAT/	DEPTH (m)	Ή.	8	N OR RQD	ATER (		PL (%) WATI	ER CONTENT (%)		EZON
GROUND SURFACE	S	<u> </u>		2	z			20 4	40 60	80	≣8 i
OPSOIL0.20m[94.45m]		٠ -									
ompact, brown <b>SILT</b> , some sand											
		_									
		_	<u> </u>			25		0			9
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		1-									
1.25m [ 93.40m ]		-									
nd of Test Pit		_									
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est pit terminated on bedrock surface											
o groundwater infiltration was observed upon		-									
ompletion of the test pit		2-									
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COORD. SYS.: UTM ZONE 18

#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**ELEVATION**: 95.32 **EASTING:** 435449.30 **NORTHING:** 5003642.99

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** 

**ADVANCED BY:** Excavator

EMARKS:					l	DATE: [	Decemb	er 10	6, 2	024		Н	OLE	NO.	: 1	P 6	-24		
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ACIAL TILL: Compact, brown silty sand, with	\(\rangle \times \rangle \rang	-	-						<u>.</u>	. j			. į.						
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PAGE: 1/1

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#### **SOIL PROFILE AND TEST DATA**

**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

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**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435565.10 **NORTHING:** 5003574.64 **ELEVATION:** 96.93

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Excavator

FILE NO.: PG4216

DATE: December 16, 2024 HOLE NO.: TP 7-24

REMARKS:					DATE: [	Decemb	er 1	6, 2024		НО	LE NO. :	TP 7-24		
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435618.80 **NORTHING:** 5003684.74 **ELEVATION:** 97.72

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Excavator

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**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435764.56 **NORTHING:** 5003694.37 **ELEVATION:** 95.94

PROJECT: Proposed Mixed-Use Development

ADVANCED BY: Excavator

FILE NO.: PG4216

REMARKS: DATE: December 16, 2024 HOLE NO.: TP 9-24

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	TRAT	DEPTH (m)	YPE ,	00	N OR RQD	ATEF	ı	PL (%)	WATE	R CONT	ΓENT (%)	LL (%)	IEZOI	LEVA
GROUND SURFACE  TOPSOIL  0.15m [95.79m]	S			<u>~</u>	Z	>	:	20	40	)	60	80 '	P 0	ш
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**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 ELEVATION: 97.38 **EASTING:** 435773.40 **NORTHING:** 5003504.87

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216 ADVANCED BY:** Excavator

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	RAI	븞	Æ	ပ္ပြဲ	N OR RQD	ATE	PL (%)	WATER	CONTENT (%)		EZO ONS	2
GROUND SURFACE	တ	<u> </u>	F	2	z	>	20	40	60	80	ੂ ਨੂੰ	
<b>PPSOIL</b> 0.20m [97.18m]												
ACIAL TILL: Compact, brown silty sand, some	A A A A	_										
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**Supplemental Geotechnical Investigation** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **ELEVATION**: 94.87 **EASTING:** 435928.90 **NORTHING:** 5003701.30

PROJECT: Proposed Mixed-Use Development FILE NO.: **PG4216** 

**ADVANCED BY:** Excavator

EMARKS:					DATE:	Decemb	er 1	6, 2024		HOLE NO.	: TP11-24	
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GROUND SURFACE	STI	吕	₽	쀭	ž	×		20	40	60	80	
OPSOIL .												
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ACIAL TILL: Compact, brown silty sand, clay, with		_				40						
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**Supplemental Geotechnical Investigation** 

FILE NO.:

**PG4216** 

5970 and 6038 Ottawa Street, Ottawa, Ontario

COORD. SYS.: UTM ZONE 18 **ELEVATION**: 95.92 **EASTING:** 435480.15 **NORTHING:** 5003517.81 PROJECT: Proposed Mixed-Use Development

**ADVANCED BY:** Excavator

HOLE NO.: TP12-24

REMARKS:					I	DATE: [	Decemb	er 1	6, 202	24		HC	LE N	10. :	TP	<b>'12</b> -	24		
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GROUND SURFACE	STRA:	DEPTI	TYPE	:	RECO	N OR RQD	WATE		PL (%			R CO	NTEN 60		<b>LI</b>	L (%)		PIEZO	ELEV
TOPSOIL					7					)	4	U	00	J	80	J			
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1.90m [94.02m] End of Test Pit	<u> </u>	2-							<u>.</u>										94 –
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Test pit terminated on bedrock surface		-	1												<u>.</u>				
Groundwater infiltration was observed at 1.40 m		-	1																
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**Supplemental Geotechnical Investigation** 5970 and 6038 Ottawa Street, Ottawa, Ontario

**COORD. SYS.:** UTM ZONE 18 **EASTING:** 435577.21 **NORTHING:** 5003410.85 **ELEVATION:** 96.69

PROJECT: Proposed Mixed-Use Development FILE NO.: PG4216

ADVANCED BY: Excavator

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REMARKS:				9	DATE: [		1			ST. (BLOWS/	: TP13-24		Т
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GROUND SURFACE	S			22	ž	<b>*</b>		20	40	60	80	_ ≣ 8	$\downarrow$
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9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.237 FILE NO. PG4216

REMARKS: HOLE NO.

BORINGS BY: CME 55 Power Auger					DATE:	2021 [	March 19		HOL	E NO.	BH 10	)-21
SAMPLE DESCRIPTION	PLOT		SAN	IPLE	ı	DEPTH	ELEV.			Blows	s / 0.3m Cone	3 WELL
GROUND SURFACE	STRATA	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)			Conte		MONITORING WEL
TOPSOIL	30 \( \hat{\lambda} \lambd	AU	1			0-	-94.24					
Very stiff, brown <b>SILTY CLAY</b>		SS	2	100	7	1-	-93.24					
GLACIAL TILL: Compact silt, some sand and clay, trace gravel	83	SS	3	100	16	2-	-92.24					
<b>GLACIAL TILL:</b> Dense, brown silty sand with gravel, cobbles and boulders		ss	4	8	+50		04.04					
- some running sand present by 3.5m depth3.	66 \^.^.	ss	5	100	34	3-	-91.24					
End of Borehole (GWL @ 0.15m - March 31, 2021)												
								20 Shea ▲ Undist		60 ength △ Re		100

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

**EASTING: NORTHING: ELEVATION:** 94.237 FILE NO. **PG4216** Geodetic DATUM: **REMARKS:** HOLE NO. **BH 1S-21** BORINGS BY: CME 55 Power Auger DATE: 2021 March 19 MONITORING WELL CONSTRUCTION STRATA PLOT **SAMPLE** Pen. Resist. Blows / 0.3m **DEPTH** ELEV. • 50 mm Dia. Cone **SAMPLE DESCRIPTION** (m) (m) N VALUE or RQD RECOVERY NUMBER Water Content % **GROUND SURFACE** 80 20 0+94.24**TOPSOIL** 0.30 1+93.24 Very stiff, brown SILTY CLAY GLACIAL TILL: Compact silt, some sand and clay, trace gravel 2 + 92.24<u>2.13</u> End of Borehole (GWL @ 0.16m - March 31, 2021) 40 60 80 100 Shear Strength (kPa) ▲ Undisturbed  $\triangle$  Remoulded

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 93.908 FILE NO. PG4216

REMARKS:
BORINGS BY: CME 75 Power Auger

DATE: 2021 March 19

BH 2-21

BORINGS BY: CME 75 Power Aug	ger					DATE:	2021 N	March 19				В	H 2-2	1
SAMPLE DESCRIPTION		PLOT		SAN	IPLE	1	DEPTH	ELEV.	Pen. Re			ws / 0 . Con		WELL
GROUND SURFACE		STRATAF	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)				tent %		MONITORING WEL
OPSOIL	0.25	^^^^	***		<u> </u>		0-	-93.91						
	_ 5.25		AU	1										<u> </u>
ery stiff to stiff, brown <b>SILTY</b>			SS	2	92	4	1-	-92.91						որ արևարկանի արևարդությունը արևարդությունը արևարդությունը արևարդությունը արևարդությունը արևարդությունը արևարդու
	_ 2.21		SS	3	100	2	2-	-91.91						
GLACIAL TILL: Brown silty clay come sand, gravel, cobbles and	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		ss	4	75	4								
come sand, gravel, cobbles and coulders	\		ss	5	17	3	3-	-90.91						
End of Borehole	3.66	\^^^^		3	17	3								
GWL @ 0.32m - March 31, 2021	)													
									20	40	60		30 10	00

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 92.787 FILE NO. PG4216

REMARKS: HOLE NO.

BORINGS BY: CME 75 Power Aug	jer				DATE:	2021 [	March 19	)	HOLI	ENO. BH	l 3-21
SAMPLE DESCRIPTION	PLOT		SAN	/PLE	1	DEPTH	ELEV.			Blows / 0.3	3m merr
CDOUND SUDEACE	STRATA	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)	0 W		Content %	NITORING
GROUND SURFACE		AU	1	<u> </u>		0-	92.79	20	40	60 80	
Very stiff to stiff brown <b>SILTY</b>		SS	2	100	6	1-	-91.79				
Very stiff to stiff, brown <b>SILTY CLAY</b>		SS	3	50	4	2-	-90.79				
	3.12	ss	4	75	3	3-	-89.79				
Stiff, grey SILTY CLAY	3.66	SS	5	100	2						
End of Borehole								20 Shea ▲ Undist		60 80 ength (kPa	)

9 Auriga Drive, Ottawa, Ontario K2E 7T9

#### **SOIL PROFILE AND TEST DATA**

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.514 FILE NO. PG4216

DATUM: Geodetic HOLE NO.

BORINGS BY: CME 55 Power Auger					DATE:	2021 [	March 22	BH 4D-21
SAMPLE DESCRIPTION  GROUND SURFACE	PLOT		SAN	//PLE	1	DEPTH	ELEV.	Pen. Resist. Blows / 0.3m  ■ 50 mm Dia. Cone
	STRATA	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)	Pen. Resist. Blows / 0.3m  ■ 50 mm Dia. Cone  ○ Water Content %  20 40 60 80
<b>TOPSOIL</b> <u>0</u> .:	18 \^^^^	AU	1			0-	94.51	<del>      </del>
Very stiff to stiff, brown <b>SILTY CLAY</b> , trace sand		ss	2	58	5	1-	-93.51	
2.3	36	SS	3	50	11	2-	-92.51	
GLACIAL TILL: Brown silty clay with sand, gravel, cobbles and boulders	, , , , , , , , , , , , , , , , , , ,	ss	4	58	20	3-	91.51	
<u>3.</u> End of Borehole	38 \^^^^	ss	5	8	+50			
Practical refusal to augering at 3.38m depth								
(GWL @ 0.29m - March 31, 2021)								
								20 40 60 80 100 Shear Strength (kPa)  ▲ Undisturbed △ Remoulded

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

**EASTING: NORTHING: ELEVATION:** 94.514 FILE NO. **PG4216** Geodetic DATUM: **REMARKS:** HOLE NO. **BH 4S-21** BORINGS BY: CME 55 Power Auger DATE: 2021 March 22 MONITORING WELL CONSTRUCTION STRATA PLOT **SAMPLE** Pen. Resist. Blows / 0.3m **DEPTH** ELEV. • 50 mm Dia. Cone **SAMPLE DESCRIPTION** (m) % RECOVERY (m) N VALUE or RQD NUMBER Water Content % **GROUND SURFACE** 80 20 0+94.51**TOPSOIL** 0.18 Very stiff to stiff, brown SILTY CLÁY, trace sand 1 + 93.512 + 92.51End of Borehole (GWL @ 0.30m - March 31, 2021) 40 60 80 100 Shear Strength (kPa) ▲ Undisturbed  $\triangle$  Remoulded

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.212 FILE NO. PG4216

REMARKS: HOLE NO.

REMARKS:
BORINGS BY: CME 55 Power Auger

DATE: 2021 March 22

BH 5-21

BORINGS BY: CME 55 Power Auger	GS BY: CME 55 Power Auger				DATE:	2021 N	/larch 22	BH 5-21				21
SAMPLE DESCRIPTION			SAN	IPLE		DEPTH		Pen. Resist. Blows / 0.3m  ■ 50 mm Dia. Cone				WELL
	STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)	0 <b>N</b>	ater (	Conte	nt %	MONITORING WELL
GROUND SURFACE	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	<b>X</b>	_	~		0-	-94.21	20	40	60	80	Σ
TOPSOIL 0	.23 \\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	AU	1									
/ery stiff to stiff, brown <b>SILTY</b> <b>CLAY,</b> trace sand		SS	2	58	5	1-	-93.21					
		SS	3	50	5		-92.21					
2	.49	SS	4	100	6	2-	-92.21					
GLACIAL TILL: Brown silty clay with sand, gravel, cobbles and boulders						3-	-91.21 ·					
3 End of Borehole	.66	SS	5	42	7							
(GWL @ 0.30m - March 31, 2021)												
								20 Shea		60 ength		100

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.044 FILE NO. PG4216

DATUM: Geodetic HOLE NO.

BORINGS BY: CME 55 Power Auger					DATE:	2021 I	March 22		HOLE		BH 6-2	1
SAMPLE DESCRIPTION	PLOT		SAN	/IPLE	T	DEPTH	ELEV.		esist. Blows / 0.3m 0 mm Dia. Cone			3 WELL
	STRATA	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)	0 W		Content	%	MONITORING WEL
GROUND SURFACE TOPSOIL	.25 \^.^.^			<u> </u>		0-	94.04	20	40	60	80	+
<u>v</u>		AU	1									<u>*</u>
Very stiff to stiff, brown <b>SILTY</b> <b>CLAY</b>		SS	2	100	5	1-	93.04					
		SS	3	33	1	2-	-92.04					
GLACIAL TILL: Compact to	.21	ss	4	75	9							
dense, brown silty clay with sand, gravel, cobbles and boulders						3-	-91.04					
<u>3</u> End of Borehole	.66 \^.^.	ss	5	50	33							
(GWL @ 0.26m - March 31, 2021)												
								20 Shea ▲ Undistu		60 ength (k △ Rem	Pa)	00

NORTHING:

SOIL PROFILE AND TEST DATA

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

Geodetic

**ELEVATION**: 93.623

FILE NO. PG4216

DATUM: REMARKS:

**EASTING:** 

HOLE NO.

REMARKS: BORINGS BY: CME 75 Power Auger					DATE:	2021 N	March 23	}	HOL	E NO		3H 7E	)-21
SAMPLE DESCRIPTION	PLOT		SAN	/IPLE		DEPTH (m)	ELEV.	Pen. Resist. Blows / 0.3m  ● 50 mm Dia. Cone				G WELL	
	STRATA	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(111)	(m)	O Water Content %					MONITORING WELL
GROUND SURFACE		AU	1	<u>«</u>		0-	93.62	20	40	6	0	80	
		SS	2	33	9	1-	-92.62						
Very stiff to stiff, brown <b>SILTY CLAY</b>		ss	3	17	4	2-	-91.62						
		ss	4	50	6	3-	-90.62						
GLACIAL TILL: Brown silty clay with sand, gravel, cobbles and boulders 3.66 End of Borehole	5 (2)	ss	5	67	6								
								20 Shea ▲ Undist	40 60 80 100 ear Strength (kPa) sturbed △ Remoulded				

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

40

▲ Undisturbed

Shear Strength (kPa)

60

80

 $\triangle$  Remoulded

100

**Geotechnical Investigation** Proposed Residential Development - Eagleson Road

Ottawa, Ontario **EASTING: NORTHING: ELEVATION: 93.623** FILE NO. **PG4216** DATUM: Geodetic **REMARKS:** HOLE NO. **BH 7S-21** BORINGS BY: CME 55 Power Auger DATE: 2021 March 23 MONITORING WELL CONSTRUCTION STRATA PLOT **SAMPLE** Pen. Resist. Blows / 0.3m **DEPTH** ELEV. **SAMPLE DESCRIPTION** • 50 mm Dia. Cone (m) % RECOVERY (m) N VALUE or RQD NUMBER Water Content % **GROUND SURFACE** 80 20 40 0+93.62Very stiff to stiff, brown **SILTY CLAY** 1 + 92.622+91.62 End of Borehole

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.047 FILE NO. PG4216

REMARKS: HOLE NO.

**BH 8-21** BORINGS BY: CME 75 Power Auger 2021 March 23 DATE: MONITORING WELL CONSTRUCTION STRATA PLOT **SAMPLE** Pen. Resist. Blows / 0.3m **DEPTH** ELEV. **SAMPLE DESCRIPTION** 50 mm Dia. Cone (m) (m) N VALUE or RQD RECOVERY NUMBER Water Content % **GROUND SURFACE** 80 20 0+94.05**TOPSOIL** 0.25 1 Very stiff to stiff, brown SILTY 1+93.05SS 2 100 2 CLÁY SS 3 100 12 2 + 92.052.13 GLACIAL TILL: Compact, brown silty clay with sand, gravel, SS 4 58 23 cobbles and boulders 3 + 91.053.20 **GLACIAL TILL:** Compact, brown SS 5 58 24 silty sand with gravel, clay, cobbles and boulders 3.66 End of Borehole (GWL @ 0.49m - March 31, 2021) 20 40 60 80 100 Shear Strength (kPa) ▲ Undisturbed  $\triangle$  Remoulded

9 Auriga Drive, Ottawa, Ontario K2E 7T9

#### **SOIL PROFILE AND TEST DATA**

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.209 FILE NO. PG4216

REMARKS: HOLE NO.

**BH 9-21** BORINGS BY: CME 55 Power Auger DATE: 2021 March 23 MONITORING WELL CONSTRUCTION STRATA PLOT **SAMPLE** Pen. Resist. Blows / 0.3m **DEPTH** ELEV. **SAMPLE DESCRIPTION** 50 mm Dia. Cone (m) (m) N VALUE or RQD RECOVERY NUMBER Water Content % **GROUND SURFACE** 80 20 40 0 + 94.21**TOPSOIL** 1 1 + 93.21Very stiff to stiff, brown SILTY SS 2 100 2 CLÁY SS 3 75 2 2 + 92.21GLACIAL TILL: Compact, brown SS 4 50 23 silty sand with gravel, clay, cobbles and boulders 3 + 91.213.10 \^^^^ \\ SS 5 8 +50 End of Borehole (GWL @ 0.68m - March 31, 2021) 20 40 60 80 100 Shear Strength (kPa) ▲ Undisturbed  $\triangle$  Remoulded

SOIL PROFILE AND TEST DATA

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

Geodetic

NORTHING: ELEVATION: 94.076

FILE NO. PG4216

DATUM:

**EASTING:** 

REMARKS: BORINGS BY: CME 55 Power Auger					DATE:	2021 N	March 23		HOLE NO.	BH10D	-21
SAMPLE DESCRIPTION	РСОТ		SAN	/IPLE	ı	DEPTH	ELEV.	Pen. Resist. Blows / 0.3m  • 50 mm Dia. Cone			3 WELL
	STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)		ater Conte		MONITORING WELI CONSTRUCTION
GROUND SURFACE	ST		N	REC	z°	0-	-94.08	20	40 60	80	MON
TOPSOIL0.23	3	AU	1			0	94.00				
Very stiff to stiff, brown <b>SILTY CLAY</b>		SS	2	75	8	1-	-93.08				
GLACIAL TILL: Clayey silt, some sand, gravel, cobbles and	3	ss	3	100	4	2-	-92.08				
boulders		SS	4	100	6	3-	-91.08				
3.66	· ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^ ^	SS	5	75	7						
End of Borehole  (GWL @ 0.72m - March 31, 2021)											
								20 Shea ▲ Undistr	40 60 r <b>Strength</b> urbed △ Re		00

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

**EASTING: NORTHING: ELEVATION:** 94.076 FILE NO. **PG4216** Geodetic DATUM: **REMARKS:** HOLE NO. BH10S-21 BORINGS BY: CME 55 Power Auger DATE: 2021 March 23 MONITORING WELL CONSTRUCTION STRATA PLOT **SAMPLE** Pen. Resist. Blows / 0.3m **DEPTH** ELEV. • 50 mm Dia. Cone **SAMPLE DESCRIPTION** (m) % RECOVERY (m) N VALUE or RQD NUMBER Water Content % **GROUND SURFACE** 80 20 0 + 94.08**TOPSOIL** 1+93.08Very stiff to stiff, brown SILTY CLÁY 2 + 92.08End of Borehole (GWL @ 0.49m - March 31, 2021) 20 40 60 80 100 Shear Strength (kPa) ▲ Undisturbed  $\triangle$  Remoulded

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Residential Development - Eagleson Road Ottawa, Ontario

▲ Undisturbed

 $\triangle$  Remoulded

EASTING: NORTHING: ELEVATION: 92.938 FILE NO.

PG4216

REMARKS: HOLE NO.

HOLE NO. BH11-21 BORINGS BY: CME 55 Power Auger DATE: 2021 March 23 MONITORING WELL CONSTRUCTION STRATA PLOT SAMPLE Pen. Resist. Blows / 0.3m **DEPTH** ELEV. **SAMPLE DESCRIPTION** • 50 mm Dia. Cone (m) (m) N VALUE or RQD RECOVERY NUMBER Water Content % **GROUND SURFACE** 80 20 40 0 + 92.941 Very stiff to stiff, brown SILTY CLÁY 1 + 91.94SS 2 100 4 SS 3 67 6 2 + 90.94Stiff, grey SILTY CLAY 4 SS 83 3 3 + 89.943.35 SS 5 75 15 GLACIAL TILL: Grey silty clay with sand, gravel, cobbles and boulders 3.66 End of Borehole (GWL @ 0.13m - March 31, 2021) 40 60 80 100 Shear Strength (kPa)

**SOIL PROFILE AND TEST DATA** 

**Geotechnical Investigation** 

Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

FILE NO.

**PG4216** 

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**ELEVATION**: 94.63 **EASTING:** NORTHING: DATUM:

Ground surface elevations were referenced to a geodetic datum.

REMARKS: BORINGS BY: Hydraulic Shovel					DATE:	2019 F	ebruary	27	HOL	E NO.		ГР 1	
SAMPLE DESCRIPTION	PLOT		SAN	IPLE	Ι	DEPTH (m)	ELEV. (m)	Pen. R ● 50					TER
	STRATA	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(111)	(111)	0 W	ater	Cont	tent	%	PIEZOMETER
Ground Surface	ဟ		Z	2	Z	0-	-94.63	20	40	60	)	80	1 5
TOPSOIL	). <u>30</u>												
	\^^^^	G	1										
		_				1-	93.63						
GLACIAL TILL: Loose to	\^^^^												
compact, grey-brown silty sand with clay, gravel and cobbles		G	2								- 1 - 0 - 1		
- some rootlest at upper 0.2m depth		_				2-	-92.63						
grey by 2.0m depth		G	3										
		G	4			3-	-91.63						
	\^^^^												
	3. <b>7</b> 0 \\ \^\^\^\\\	G	5										
End of Test Pit (Groundwater infiltration at 3.5m													
depth)													
								20 Shea ▲ Undist		_	h (kl		⊣ 100

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St.

9 Auriga Drive, Ottawa, Ontario K2E 7T9

Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.42

DATUM: Ground surface elevations were referenced to a geodetic datum.

PG4216

REMARKS: BORINGS BY: Hydraulic Shovel					DATE:	2019 I	February	27	HOLI	E NO.	2
SAMPLE DESCRIPTION	PLOT		SAN	/IPLE	I	DEPTH		Pen. F		. Blows/0.3เ Dia. Cone	
	STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(111)	(111)	0 V	/ater (	Content %	PIEZOMETER
Ground Surface	S		2	2	2		94.42	20	40	60 80	
<b>TOPSOIL</b>	0.30										
Very stiff, grey-brown <b>SILTY</b> <b>CLAY</b>		G	1			1-	-93.42				>130
- some rootlets at upper 0.2m depth - grey by 1.0m depth		G	2				00.12				>130
2	2.00	G	3			2-	-92.42				
<b>GLACIAL TILL:</b> Grey silty clay with sand, gravel, cobbles and boulders		G	4								
boulders		G G	5			3-	91.42				
3 End of Test Pit	3.70	<u></u>	7								
(TP dry upon completion)											
								20 Shea ▲ Undis		60 80 ength (kPa) △ Remoulde	<b>100</b>

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**EASTING:** 

NORTHING: ELEVATION: 94.48

**DATUM**: Ground surface elevations were referenced to a geodetic datum.

FILE NO.

**PG4216** 

REMARKS: BORINGS BY: Hydraulic Shovel						DATE:	2019 I	- ebruary	27	HOLE	NO. TP 3	
SAMPLE DESCRIPTION		PLOT		SAM	IPLE		DEPTH (m)		Pen. R		Blows/0.3m Dia. Cone	TER
		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(111)	(111)	0 W	/ater C	ontent %	PIEZOMETER
Ground Surface		S		Z	A.	Z	0-	94.48	20	40	60 80	ļ
TOPSOIL	0.30											
			_ G	1							>1	30
							4	02.40				
Very stiff, brown <b>SILTY CLAY</b>			G	2			1-	-93.48				
grey by 1.6m depth			_ G	3								
			-									
			_				2-	92.48				
	2.50		- G	4								
	^ ^ ^		_ G	5								
GLACIAL TILL: Grey silty clay with sand, gravel, cobbles and boulders	\^ \^ \^		_				3-	91.48				
33414310	3.50 \hat{\hat{\hat{\hat{\hat{\hat{\hat{		_ G	6								
End of Test Pit			_									
Water infiltration at base of test oit)												
									20 Shea		60 80 1 ngth (kPa) △ Remoulded	00

**SOIL PROFILE AND TEST DATA** 

FILE NO.

**PG4216** 

**Geotechnical Investigation** Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**EASTING:** 

DATUM:

ELEVATION: 93.63 NORTHING:

Ground surface elevations were referenced to a geodetic datum.

REMARKS: BORINGS BY: Hydraulic Shovel						DATE:	2019 F	ebruary	27	HOLE NO	TP 4	_
SAMPLE DESCRIPTION		PLOT		SAN	IPLE		DEPTH			esist. Bl	ows/0.3m . Cone	TER
		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)	0 W	later Cor	ntent %	PIEZOMETER
Ground Surface		S		ž	REC	z°	0-	-93.63	20	40 6	80	_
TOPSOIL	0.30							00.00				
											>1	30
			G -	1								
			_ G	2			1-	-92.63			>1	30 z
			- -									
Very stiff, brown <b>SILTY CLAY</b>			_ G	3							>1	30
			_									
			_ G	4			2-	91.63				
			_									
			_ G 	5								
grey by 2.8m depth								00.00				
			G	6			3-	-90.63				
			_									
 End of Test Pit	<u>3</u> .70		<b>G</b> –	7								
(Groundwater infiltration at 1.0m depth)												
acpui)												
									20	40 6	0 80 1	00

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

NORTHING: ELEVATION: 94.28

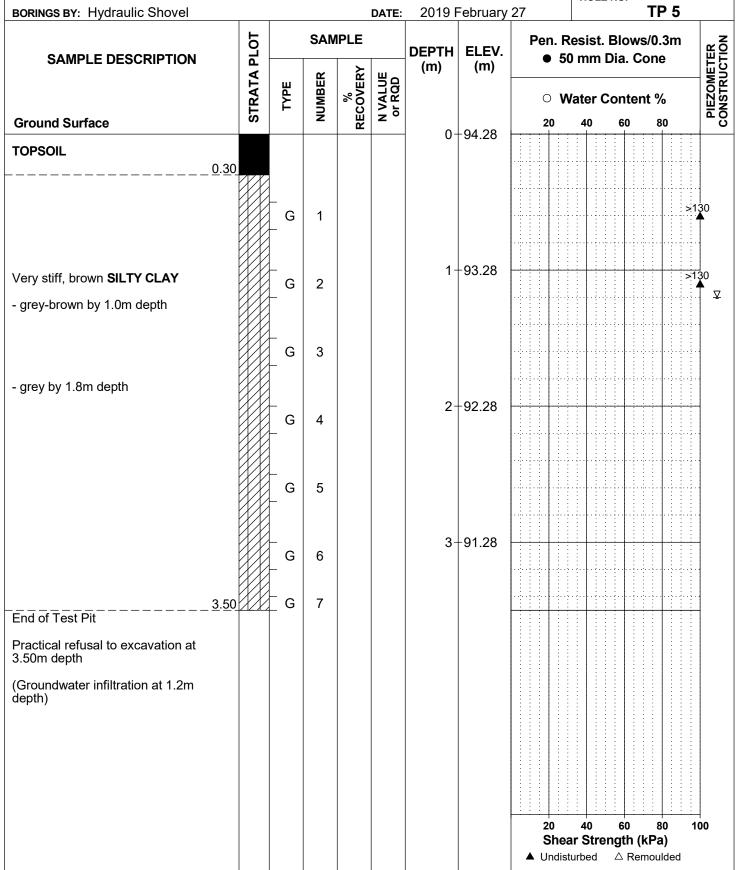
**DATUM:** Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**PG4216** 

REMARKS:



SOIL PROFILE AND TEST DATA

FILE NO.

**PG4216** 

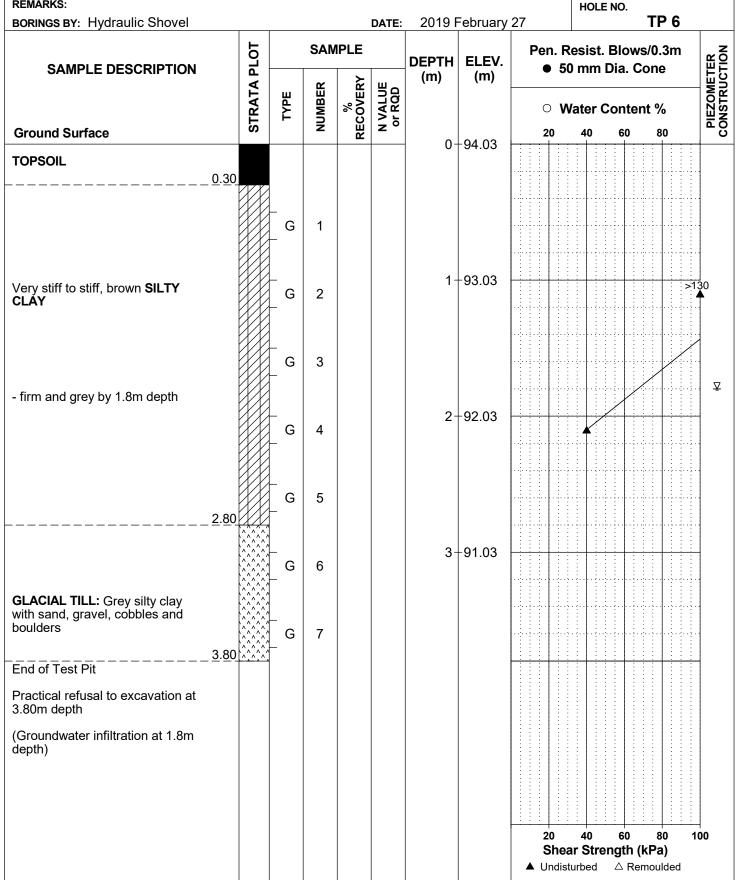
**Geotechnical Investigation** Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**NORTHING:** ELEVATION: 94.03

Ground surface elevations were referenced to a geodetic datum.

DATUM: **REMARKS:** 



**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

NORTHING: ELEVATION: 94.46

**DATUM**: Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**PG4216** 

REMARKS:

BORINGS BY: Hydraulic Shovel		_		04-	4DI -						4 B:	10. 0	
SAMPLE DESCRIPTION		V PLO			IPLE	 	DEPTH (m)	ELEV. (m)			t. Blov n Dia. (	vs/0.3m Cone	ETER
		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD			0 V	Vater	Conte	ent %	PIEZOMETER
Ground Surface		0)			2		0-	94.46	20	40	60	80	퇶
TOPSOIL	<u>0.30</u>												
			G	1									
Very stiff, brown <b>SILTY CLAY</b>			G	2			1-	93.46					
			G	3									
- grey and with sand by 1.8m depth			G	4			2-	-92.46					
			G	5									
Grey <b>CLAYEY SILT</b>	3.00 3.20		G	6			3-	91.46					-
<b>GLACIAL TILL:</b> Grey silty clay with sand, gravel, cobbles and boulders	3.70		G	7									
End of Test Pit  Practical refusal to excavation at													
3.70m depth (Groundwater infiltration at 1.8m depth)													

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

NORTHING: ELEVATION: 94.53

Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**PG4216** 

REMARKS:

**EASTING:** 

DATUM:

BORINGS BY: Hydraulic Shovel	1					DATE:	2019 F	ebruary	27			TP	8	1
SAMPLE DESCRIPTION		PLOT		SAN	IPLE		DEPTH (m)	ELEV. (m)	Pen. R ● 50		. Blov Dia. (		m	TER
Ground Surface		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(111)	(111)	○ W	/ater	Conte	ent % 80		PIEZOMETER
TOPSOIL	<u>0.30</u>	7/2					0-	94.53						
			G	1										
Very stiff, brown SILTY CLAY								00.50						
- grey-brown by 0.8m depth			G	2			1-	-93.53						
- grey by 1.4m depth			G	3										
			_	3									1 1 1 1 1 1 2 2 2 3 4 2 3 4 4 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	Ž
			G	4			2-	-92.53						
	2.80		G	5										
			G	6			3-	91.53						
<b>GLACIAL TILL:</b> Grey silty clay with sand, gravel, cobbles and boulders			G	7										
End of Test Pit	_ <u>3</u> . <u>80</u>	<u>`^^^^</u>												
Practical refusal to excavation at 3.80m depth														
(Groundwater infiltration at 1.9m depth)														
									20 Shea		60 ength △ R	80 (kPa) emould		00

**SOIL PROFILE AND TEST DATA** 

**Geotechnical Investigation** 

Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**EASTING: NORTHING:** ELEVATION: 97.18

Ground surface elevations were referenced to a geodetic datum.

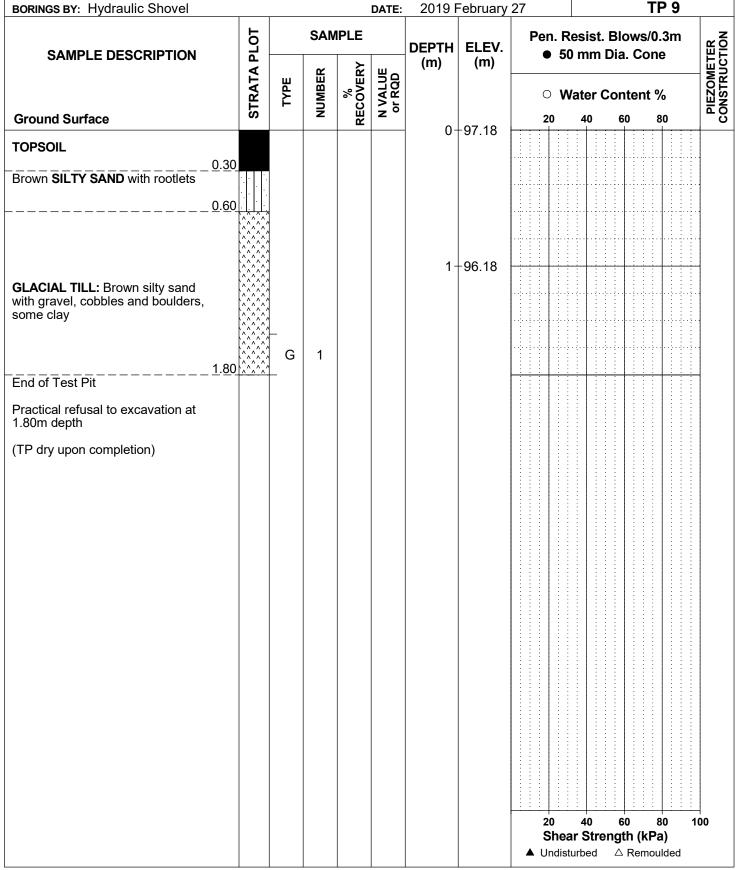
FILE NO. **PG4216** 

**REMARKS:** 

DATUM:

HOLE NO.

TP9



**SOIL PROFILE AND TEST DATA** 

**Geotechnical Investigation** 

Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**ELEVATION**: 97.48 **EASTING:** NORTHING: Ground surface elevations were referenced to a geodetic datum. DATUM:

FILE NO. **PG4216** 

DEMARKS.

REMARKS:									HOLE N		
BORINGS BY: Hydraulic Shovel		1			DATE:	2019	February	27		TP 9A	1
SAMPLE DESCRIPTION	STRATA PLOT			IPLE ≿	ш	DEPTH (m)	ELEV. (m)		Resist. B 0 mm Di	lows/0.3m a. Cone	PIEZOMETER CONSTRUCTION
	TRAT/	TYPE	NUMBER	% RECOVERY	N VALUE or RQD			0 V	/ater Co	ntent %	PIEZOM
Ground Surface	0)			2	_		97.48	20	40	60 80	်
TOPSOIL	0.30										
Brown <b>SILTY SAND</b> , trace rootlets											.
	).60   \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\										
						1-	96.48				
											.
<b>GLACIAL TILL:</b> Light brown to grey silty sand with clay, gravel, cobbles and boulders											
cobbles and boulders											
	\^^^^ \^^^^					2-	95.48				
							00.40				.
End of Test Pit	2.40 \^^^^										
Practical refusal to excavation at 2.40m depth											
(TP dry upon completion)											
								20 She		60 80 1 gth (kPa)	100
								▲ Undis		\ Remoulded	

**SOIL PROFILE AND TEST DATA** 

9 Auriga Drive, Ottawa, Ontario K2E 7T9

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

**EASTING:** ELEVATION: 95.63 NORTHING: FILE NO. **PG4216** DATUM: Ground surface elevations were referenced to a geodetic datum. **REMARKS:** HOLE NO. **TP10 BORINGS BY:** Hydraulic Shovel 2019 February 27 DATE: STRATA PLOT **SAMPLE** Pen. Resist. Blows/0.3m PIEZOMETER CONSTRUCTION DEPTH ELEV. • 50 mm Dia. Cone **SAMPLE DESCRIPTION** (m) (m) % RECOVERY N VALUE or RQD NUMBER Water Content % **Ground Surface** 80 20 40 0+95.63**TOPSOIL** 0.35 Very stiff, red-brown SILTY CLÁY, some sand, trace G 1 organics 0.65 GLACIAL TILL: Brown silty sand 2 G with gravel and sand End of Test Pit Practical refusal to excavation at 0.90m depth (TP dry upon completion) 40 60 80 100 Shear Strength (kPa) ▲ Undisturbed  $\triangle$  Remoulded

**SOIL PROFILE AND TEST DATA** 

**Geotechnical Investigation** Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**ELEVATION**: 94.45 NORTHING:

DATUM: Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**PG4216** 

**EASTING:** 

REMARKS:

BORINGS BY: Hydraulic Shovel						DATE:	2019 I	February	27	HOL	ENO. TP1	11
SAMPLE DESCRIPTION		PLOT			IPLE		DEPTH (m)	ELEV. (m)			. Blows/0.3 Dia. Cone	m H
Ground Surface		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(,		○ W	ater	Content %	PIEZOMETER
FILL: Sand with cobbles	0.25						0-	-94.45				
GLACIAL TILL: Loose to			_ _ _ _	1			1-	-93.45				
compact, grey-brown silty sand with clay, gravel, cobbles and boulders			G	2								
			G	3			2-	92.45				
End of Test Pit	2.80		G	4								
Practical refusal to excavation at 2.80m depth  (TP dry upon completion)												
									20 Shea ▲ Undist		60 80 ength (kPa) △ Remoulde	<b>100</b>

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

NORTHING: ELEVATION: 93.96

Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**PG4216** 

REMARKS:

**EASTING:** 

DATUM:

		<u>ю</u> .		SAN	<b>IPLE</b>		DEPTH	ELEV.	Pen. R					<u>~</u>
SAMPLE DESCRIPTION		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)			Dia. Conte			PIEZOMETER
Ground Surface		S		Z	Ä	Z	0-	-93.96	20	40	60	80	)	<u>n</u>
TOPSOIL	_0.20	-1111		1				33.30						▩
oose, brown <b>SANDY SILT</b> , ome clay			: ≅ :V	0	F.4	_	1-	-92.96						
	<u>1.22</u>		ss	2	54	5	'	92.90						▩
			ss	3	67	39	2-	-91.96						
			V ss	4	71	23	_	01.00						
				7	' '	20	3-	-90.96						▩
Dense to compact, brown <b>SILTY</b>			ss	5	58	14								
SAND, trace gravel			ss	6	71	32	4-	-89.96						
			·\\\											▩
			ss	7		24	5-	88.96						▩
	5.94		ss	8	100	3								
	<u> </u>		∯ ∭ss	9	67	42	6-	-87.96						▓
				9	07	42	_	00.00						▩
							/-	-86.96						
GLACIAL TILL: Dense to very lense, grey silty sand with gravel			∬ ss	10	91	82	8-	-85.96						
ind clay			<u> </u>					00.00						
			^ ^				9-	-84.96						
	<u>9</u> .75		ss	11		7								
and of Borehole	_ <u></u>	A												اخدر
GWL @ 0.61m - Dec. 28, 2018)														
									20	40	60	80	) 1	00
										ar Str	ength		)	•

**SOIL PROFILE AND TEST DATA** 

**Geotechnical Investigation** Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**ELEVATION**: 94.23 NORTHING:

DATUM: Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**REMARKS:** 

**EASTING:** 

**PG4216** 

ORINGS BY: CME 55 Power Aug		ТО		SAN	1PLE	DATE:		Decembe		esist. Blo	BH 2 ows/0.3m	~
SAMPLE DESCRIPTION		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)		) mm Dia /ater Con		PIEZOMETER
Fround Surface		S	•	N	REC	zō		04.00	20	40 6	0 80	▮ឨ
OPSOIL	0.25			1			0-	-94.23				
tiff to very stiff brown SILTY			ss	2	92	4	1-	-93.23				
tiff to very stiff, brown <b>SILTY</b> <b>LAY</b> some sand			<u>//</u>								1	
	_ <u>2</u> . <u>13</u>		17				2-	-92.23				
LACIAL TILL: Brown silty clay			ss	3	8	12	3-	-91.23				
ith sand, gravel, cobbles and bulders			ss	4	42	35		01.20				
	4. <u>32</u>						4-	-90.23				
nd of Borehole												
ractical refusal to augering at 32m depth												
GWL @ 0.77m - Dec. 28, 2018)												
									20	40 6 ir Strengt		↓ 00

**SOIL PROFILE AND TEST DATA** 

9 Auriga Drive, Ottawa, Ontario K2E 7T9

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

EASTING: NORTHING: ELEVATION: 94.76

DATUM: Ground surface elevations were referenced to a geodetic datum.

REMARKS:

BORINGS BY: CME 55 Power Auger

DATE: 2018 December 13

BH 3

BORINGS BY: CME 55 Power Aug	jer					DATE:	2018 [	Decembe	er 13 <b>BH 3</b>
SAMPLE DESCRIPTION		LOT		SAN	IPLE		DEPTH		Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone
OAMI LE DEGORII HOR		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)	Pen. Resist. Blows/0.3m  • 50 mm Dia. Cone   O Water Content %  20 40 60 80
Ground Surface		STR		N D N	ECO	N N			20 40 60 80
TOPSOIL	0.15		×		œ		0-	94.76	20 40 60 80
	. 0.15		<b>⊗</b> AU	1					
			72 17						
Stiff to firm, brown SILTY CLAY			∦ ss	2	92	6	1-	93.76	
			11						
	0.40		∦ ss	3	92	6		00.76	
	_ <u>2</u> . <u>13</u>		<u>/</u> ∆ <del>=</del> SS	4	0	50+	2-	92.76	<b></b>
		[^^^^/ [^^^^/		-		30.			
							3-	91.76	
<b>LACIAL TILL:</b> Brown sandy silt ith clay, gravel, cobbles and		\^^^ <i>^</i>	∜ ss	5	67	23			
oulders		\^^^^ \^^^^	$\sqrt[4]{}$						
		\^^^^ \^^^^	2				4-	90.76	
 nd of Borehole	<u>4.42</u>	\^^^^							
ractical refusal to augering at 42m depth									
GWL @ 0.62m - Dec. 28, 2018)									
,									
									20 40 60 80 100 Shear Strongth (kPa)
									Shear Strength (kPa)  ▲ Undisturbed △ Remoulded

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**EASTING:** 

**REMARKS:** 

DATUM:

NORTHING: ELEVATION: 97.71

Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**PG4216** 

REMARKS: BORINGS BY: CME 55 Power Aug	er				DATE:	2018 [	Decembe	er 13	HOL	E NO. B	H 4
SAMPLE DESCRIPTION	PLOT		SAN	/IPLE		DEPTH (m)	ELEV. (m)			. Blows/0. Dia. Cond	3m
Ground Surface	STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(111)	(···/	○ W	ater	Content %	3m
OSPOIL	0.30	<b>※</b>				0-	-97.71	20	70		×
	\^^^	- AU	1								<b> </b>
	\^^^ \^^^	ŝ∯ ss	2	71	40	1-	-96.71		·		::   :
	\^^^ \^^^		_								
	\^^^ \^^^	ŝ̂∭ ss	3	71	68	2-	-95.71				×
GLACIAL TILL: Very dense,				7.4	50.		95.71				
prown silty sand with gravel, sobbles and boulders	\^^^	ss. ∫	4	71	50+						:-:  :-:-:-
obbles and bodiders	\^^^ \^^^	Â√ ss	5	88	73	3-	-94.71				
	\^^^ \^^^			00	13						
	\^^^					4-	-93.71				
	4.85 \\\^^^	^^ ^^▼ SS	6	100	50+						
 End of Borehole	_ <u>4</u> . <u>85^,</u> ^			100	301						
Practical refusal to augering at .85m depth											
GWL @ 1.10m - Dec. 28, 2018)											
,											
								20	40		0 100
								Shea  ▲ Undist		ength (kPa △ Remou	

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation

Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**EASTING:** NORTHING: ELEVATION: 97.45

DATUM: Ground surface elevations were referenced to a geodetic datum.

FILE NO. PG4216

EMARKS: DRINGS BY: CME 55 Power Auge	er		i			DATE:	2018 [	Decembe	er 13	HOL	E NO.	BH 5	
SAMPLE DESCRIPTION		PLE DESCRIPTION		SAN	1PLE		DEPTH	ELEV.				ws/0.3m Cone	띪
			TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(m)	(m)				ent %	PIEZOMETER
round Surface		STRATA	-	3	ZEC	N P			20	40	60		∣≣
OPSOIL OPSOIL	0.30		<u></u>		<u> </u>		0-	-97.45					
	0.30	XX.	<b>⊗</b> AU	1									
ery stiff, brown <b>CLAYEY SILT</b>	<u>1</u> .01		≅ SS	2		50+							
nd of Borehole	1.01	<i>AZX</i> 2					1-	-96.45					
ractical refusal to augering at 01m depth													
BH dry upon completion)													
									20	40	60	80	100
									Shea ▲ Undist		ength △ F	ı (kPa)	

**SOIL PROFILE AND TEST DATA** 

**Geotechnical Investigation** Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**ELEVATION**: 94.70 **EASTING:** NORTHING: DATUM: Ground surface elevations were referenced to a geodetic datum.

FILE NO. **PG4216** 

**REMARKS:** 

HOLE NO.

BORINGS BY: CME 55 Power Aug	jer					DATE:	2018 [	Decembe			BH 6	
SAMPLE DESCRIPTION		PLOT			/IPLE		DEPTH (m)	ELEV. (m)		esist. Bl mm Dia	lows/0.3m a. Cone	TER
Ground Surface		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD		. ,	○ W		ntent %	PIEZOMETER
TOPSOIL	<u>0.3</u> 0	-1 L-1 ·1.		1			0-	-94.70				
Compact to loose, brown SANDY SILT, trace clay			≋ √ss	2	67	12	1-	-93.70				
Brown SILTY CLAY	1.93 2.19		ss	3	92	5	2-	92.70				
			ss	4	88	59	3-	91.70				
GLACIAL TILL: Very dense, prown silty sand with gravel, cobbles and boulders			∑ ss	5	71	68	4-	-90.70				
			ss	6	100	50	5-	-89.70				
	<u>5.69</u>		≅ SS	7	100	50+						
End of Borehole												
Practical refusal to augering at i.69m depth												
GWL @ 0.73m - Dec. 28, 2018)												
									20 Shea ▲ Undist	r Streng	60 80 1  th (kPa)  Remoulded	100

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation

Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

**EASTING:** 

**REMARKS:** 

NORTHING: ELEVATION: 94.88

**DATUM**: Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

**PG4216** 

temarks: s <b>orings by</b> :  CME 55 Power Auge	er					DATE:	2018 [	Decembe	er 13	HOLI	E NO.	BH 7	
SAMPLE DESCRIPTION		PLOT		SAN	IPLE	П	DEPTH				. Blow Dia. C	s/0.3m Cone	ER
		STRATA F			% RECOVERY	N VALUE or RQD	(m)	(m)	0 W	later (	Conte	nt %	PIEZOMETER
Ground Surface		•	×	NUMBER	2	_	0-	94.88	20	40	60	80	
OPSOIL	0.25	`^^^	§ AU	1									
GLACIAL TILL: Brown silty sand	\^^	`^^^	<u> </u>										
vith clay and gravel	\^^	`^^^	∖ ss	2	67	9	1-	-93.88					
	1.73		ב צss	3	50	50+							
ind of Borehole		^^											
Practical refusal to augering at .73m depth													
GWL @ 0.83m - Dec. 28, 2018)													
5 VV L (@ 0.00111													
									20	40	60	80	- 100
											ength		.00
							1		▲ Undist			moulded	

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

NORTHING: ELEVATION: 94.03

**DATUM**: Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

FILE NO.

PG4216

REMARKS:

DRINGS BY: CME 55 Power Au	uger					DATE:	2018 [	Decembe	er 13		BH 8	
SAMPLE DESCRIPTION	I	PLOT			/IPLE		DEPTH (m)	ELEV. (m)			Blows/0.3m Dia. Cone	TER
round Surface		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(,		○ W	ater (	Content %	PIEZOMETER
OPSOIL	0.25		<b>X</b>				0-	94.03	20	70		
ery stiff, brown <b>SILTY CLAY</b>			SS	2	79	13	1-	-93.03				
	<u> </u>		ss	3	96	6	2-	-92.03				
LACIAL TILL: Brown silty sand th gravel, cobbles, boulders	I		ss Ss Ss	5	50	36	3-	91.03				
nd of Borehole	<u>4.17</u> _		^				4-	-90.03				
ractical refusal to augering at 17m depth												
GWL @ 1.28m - Dec. 28, 2018												

**SOIL PROFILE AND TEST DATA** 

Geotechnical Investigation Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

9 Auriga Drive, Ottawa, Ontario K2E 7T9

NORTHING: ELEVATION: 93.78

**DATUM**: Ground surface elevations were referenced to a geodetic datum.

HOLE NO.

**PG4216** 

FILE NO.

REMARKS:

BORINGS BY: CME 55 Power Auger		-1			DATE:	2018 [	Decembe	er 13 HOLE NO.
SAMPLE DESCRIPTION	PLOT			IPLE		DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m  ■ 50 mm Dia. Cone
Ground Surface	STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD			Pen. Resist. Blows/0.3m  • 50 mm Dia. Cone  Cone  Water Content %  20 40 60 80
TOPSOIL 0.2	5	AU	1			0-	-93.78	
Very stiff, brown <b>SILTY CLAY</b>		ss	2	96	4	1-	-92.78	
<u>2.2</u>	9	ss	3	83	Р	2-	-91.78	
Brown <b>SANDY SILT,</b> trace clay 3.0	5	ss ss ss	5	88	P 7	3-	-90.78	
Loose, brown <b>SILTY SAND</b> with		ss	6	79	9	4-	-89.78	
gravel, trace clay		ss	7	71	9	5-	-88.78	
<u>6.1</u>	0	ss	8	100	26	6-	87.78	
Compact to dense, grey <b>SILTY</b>		ss	9	62	14	7-	-86.78	
SAND		∬ SS	10	62	36	8-	85.78	
9.7	5	ss	11	42	31	9-	84.78	
Dynamic Cone Penetration Test commenced at 9.75m depth. 10.1 End of Borehole						10-	83.78	
Practical DCPT refusal at 10.17m depth								
(GWL @ 0.70m - Dec. 28, 2018)								20 40 60 80 100
								Shear Strength (kPa)  ▲ Undisturbed △ Remoulded

**SOIL PROFILE AND TEST DATA** 

**Geotechnical Investigation** 

Proposed Development - Eagleson Road at Ottawa St. Ottawa, Ontario

FILE NO.

9 Auriga Drive, Ottawa, Ontario K2E 7T9

ELEVATION: 93.37 **EASTING: NORTHING:** DATUM: Ground surface elevations were referenced to a geodetic datum.

**PG4216** 

REMARKS: BORINGS BY: CME 55 Power Auge	er	_				DATE:	2018 [	Decembe	er 13	HU	LE NO		ВН	10	$\top$
SAMPLE DESCRIPTION		PLOT		SAMPLE			DEPTH (m)	ELEV. (m)	Pen. R ● 50					m	TER
		STRATA PLOT	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	(,	(,	0 W	ater	Cor	nten	t %		PIEZOMETER
Ground Surface		S		ž	REC	z°	0	-93.37	20	40	6	<b>60</b>	80		_
OPSOIL	0.30		& AU	1				93.37							
			ss	2	96	6	1-	-92.37							
ery stiff, brown SILTY CLAY			ss	3	54	Р	2-	-91.37	Δ					1,	
grey by 2.9m depth			ss	4	100	Р		00.07	Δ					1	
							3-	90.37	Δ			<b>A</b>			
			ss	5	58	10	4-	-89.37							
			ss	6	83	37	5-	-88.37							
:	<u>5.41</u>		∑ ∑ss	7	96	50+									
		^^^^ ^^^^^	ss	8	83	75	6-	-87.37							
LACIAL TILL: Very dense, grey		^^^^ ^^^^^					7-	-86.37							
ilty sand with gravel, cobbles nd boulders		^^^^^ ^^^^^	∑ss	9	92		8-	-85.37							
		^^^^ ^^^^^													
		^^^^^ ^^^^^	=	10	100	50+	9-	-84.37						1 1 1 2 - 4 - 3 - 3 - 4 - 3 -	
nd of Borehole	9.75	<u>^^^^^</u>													
GWL @ 0.48m - Dec. 28, 2018)															
									20	40		50	80		00
									Shea  Indist		_		<b>(Pa)</b> noulde		

### **SYMBOLS AND TERMS**

### **SOIL DESCRIPTION**

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the strength of cohesionless soils is the relative density, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm.

Relative Density	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory vane tests, penetrometer tests, unconfined compression tests, or occasionally by Standard Penetration Tests.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

### **SYMBOLS AND TERMS (continued)**

### **SOIL DESCRIPTION (continued)**

Cohesive soils can also be classified according to their "sensitivity". The sensitivity is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil.

Terminology used for describing soil strata based upon texture, or the proportion of individual particle sizes present is provided on the Textural Soil Classification Chart at the end of this information package.

#### **ROCK DESCRIPTION**

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NXL size core. However, it can be used on smaller core sizes, such as BX, if the bulk of the fractures caused by drilling stresses (called "mechanical breaks") are easily distinguishable from the normal in situ fractures.

RQD %	ROCK QUALITY
90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

#### SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard Penetration Test (SPT))
TW	-	Thin wall tube or Shelby tube
PS	-	Piston sample
AU	-	Auger sample or bulk sample
WS	-	Wash sample
RC	-	Rock core sample (Core bit size AXT, BXL, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.
Р	-	Split-spoon pushed through sampling interval which was also tested using a vane apparatus and resulted in a obtaining a sample of disturbed material (i.e., blow-counts not reflective of undisturbed, in-situ soils and not considered relevant)

### SYMBOLS AND TERMS (continued)

#### **GRAIN SIZE DISTRIBUTION**

MC% - Natural moisture content or water content of sample, %

Liquid Limit, % (water content above which soil behaves as a liquid)
 PL - Plastic limit, % (water content above which soil behaves plastically)

PI - Plasticity index, % (difference between LL and PL)

Dxx - Grain size which xx% of the soil, by weight, is of finer grain sizes

These grain size descriptions are not used below 0.075 mm grain size

D10 - Grain size at which 10% of the soil is finer (effective grain size)

D60 - Grain size at which 60% of the soil is finer

Cc - Concavity coefficient =  $(D30)^2 / (D10 \times D60)$ 

Cu - Uniformity coefficient = D60 / D10

Cc and Cu are used to assess the grading of sands and gravels:

Well-graded gravels have: 1 < Cc < 3 and Cu > 4 Well-graded sands have: 1 < Cc < 3 and Cu > 6

Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded.

Cc and Cu are not applicable for the description of soils with more than 10% silt and clay

(more than 10% finer than 0.075 mm or the #200 sieve)

#### **CONSOLIDATION TEST**

p'<sub>o</sub> - Present effective overburden pressure at sample depth

p'c - Preconsolidation pressure of (maximum past pressure on) sample

Ccr - Recompression index (in effect at pressures below p'c)
Cc - Compression index (in effect at pressures above p'c)

OC Ratio Overconsolidaton ratio =  $p'_c/p'_o$ 

Void Ratio Initial sample void ratio = volume of voids / volume of solids

Wo - Initial water content (at start of consolidation test)

#### PERMEABILITY TEST

Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.

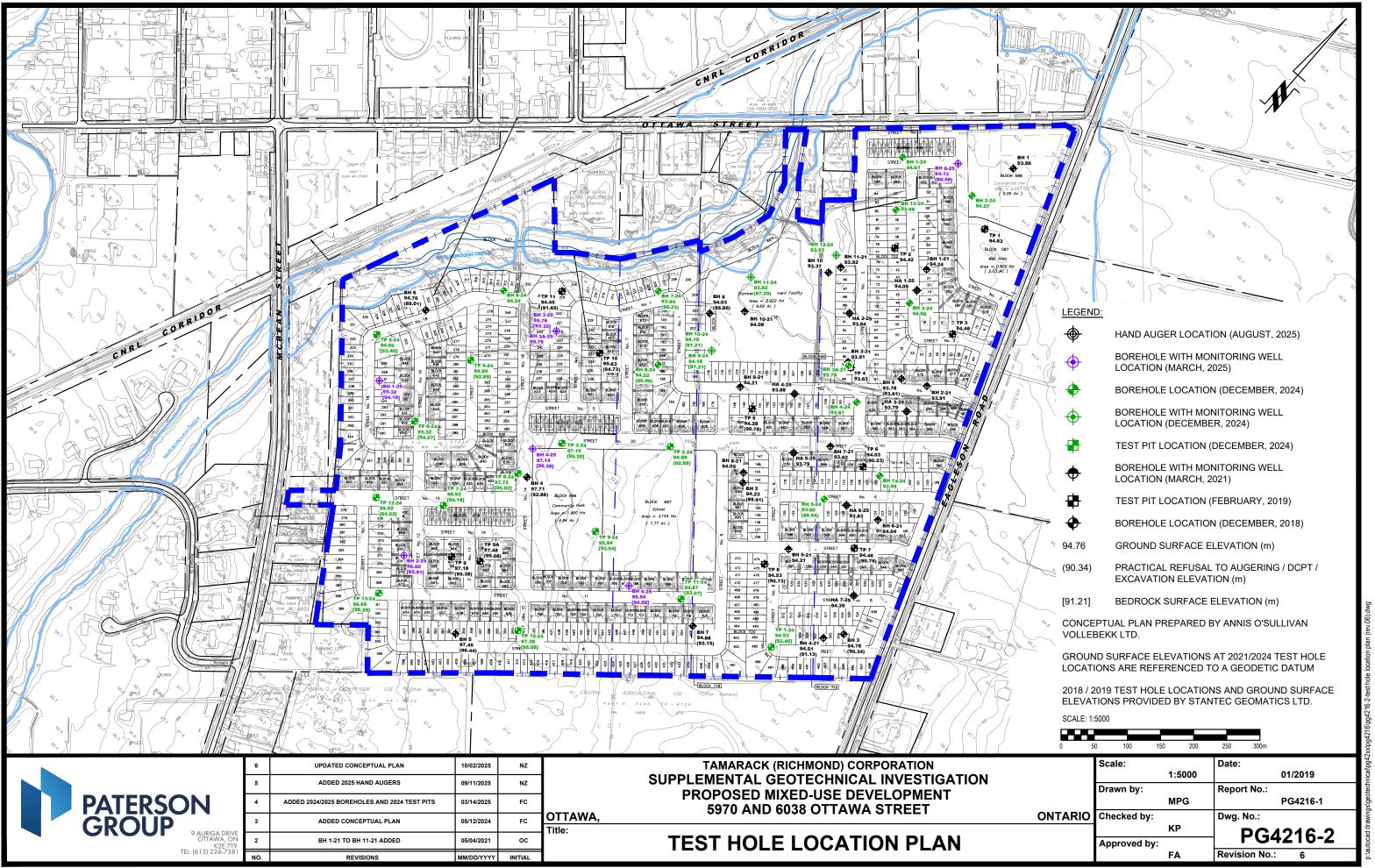
### SYMBOLS AND TERMS (continued)

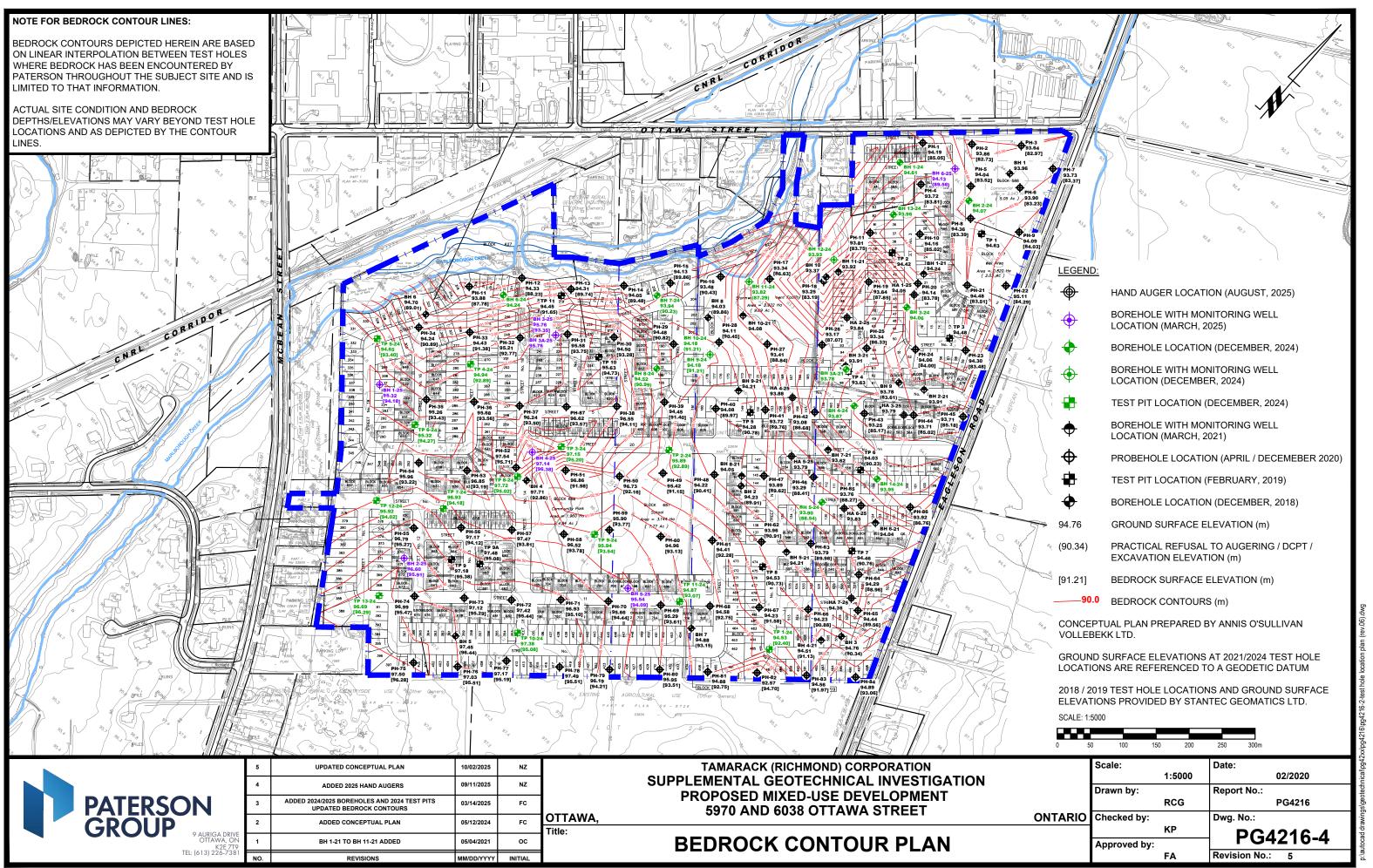
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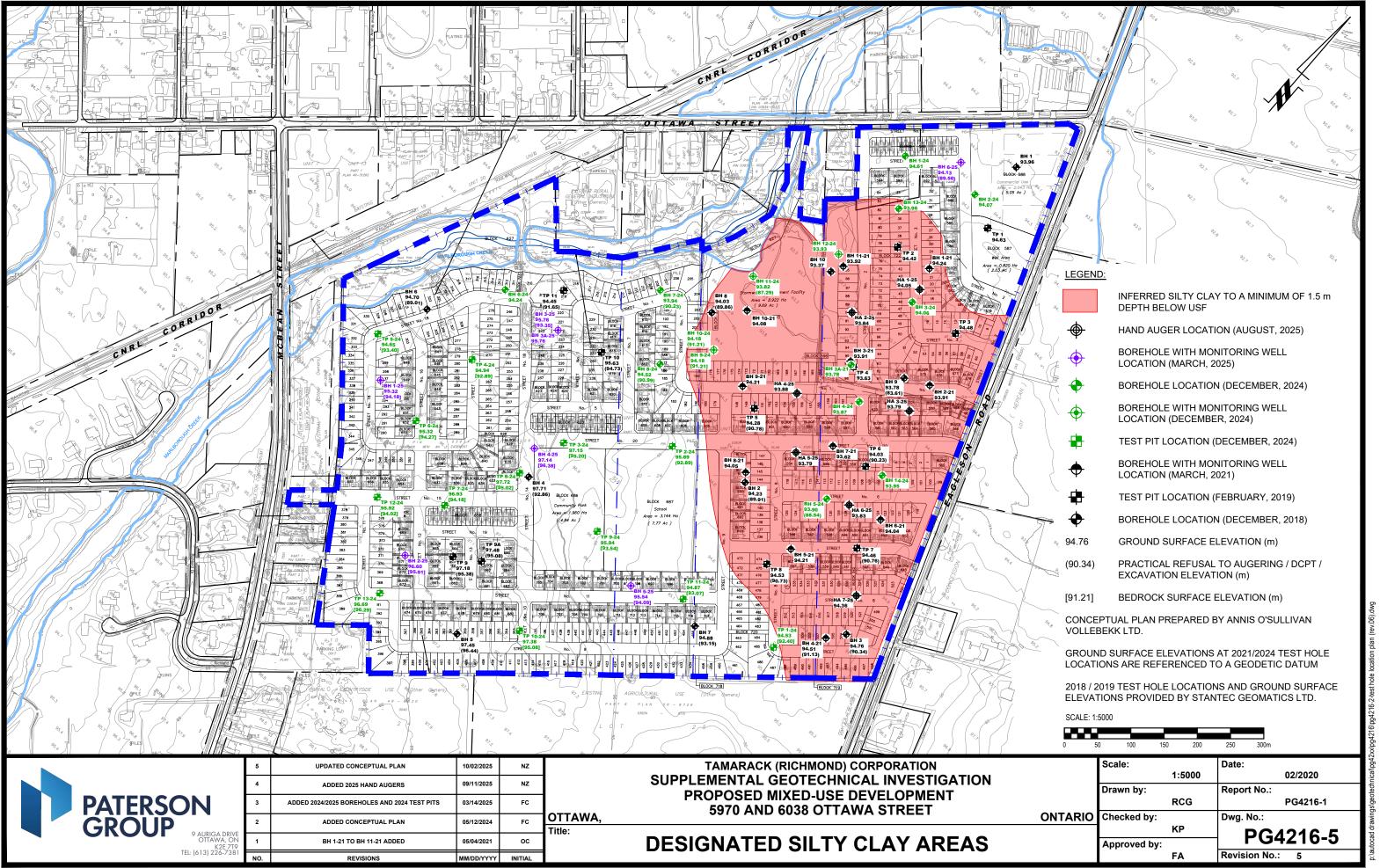


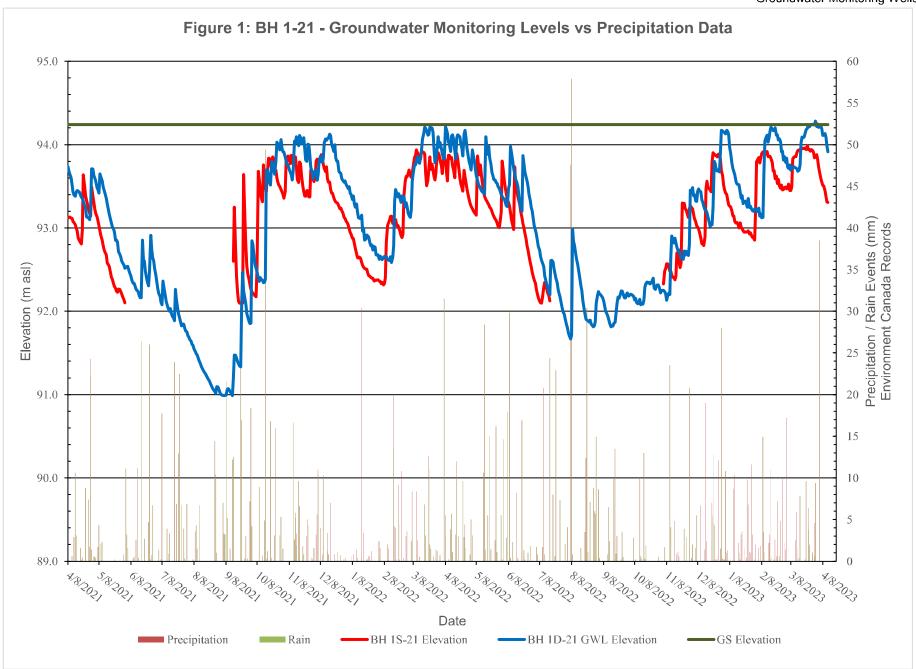
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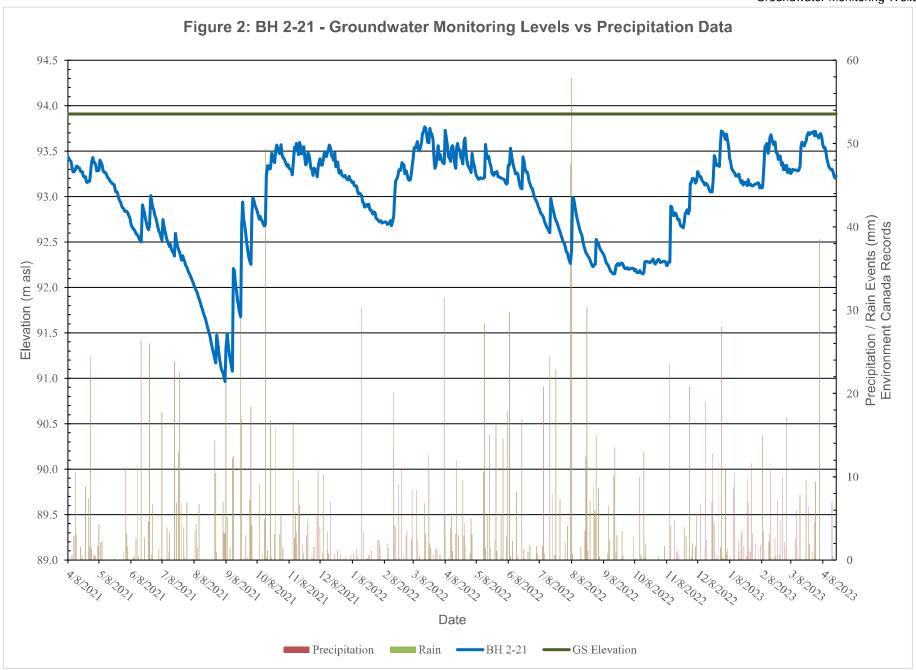




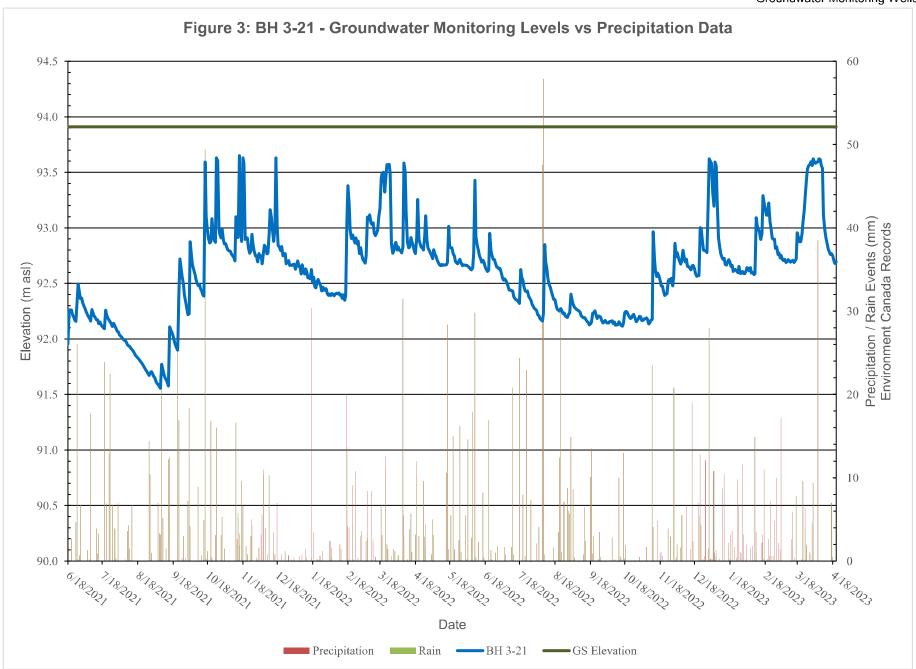




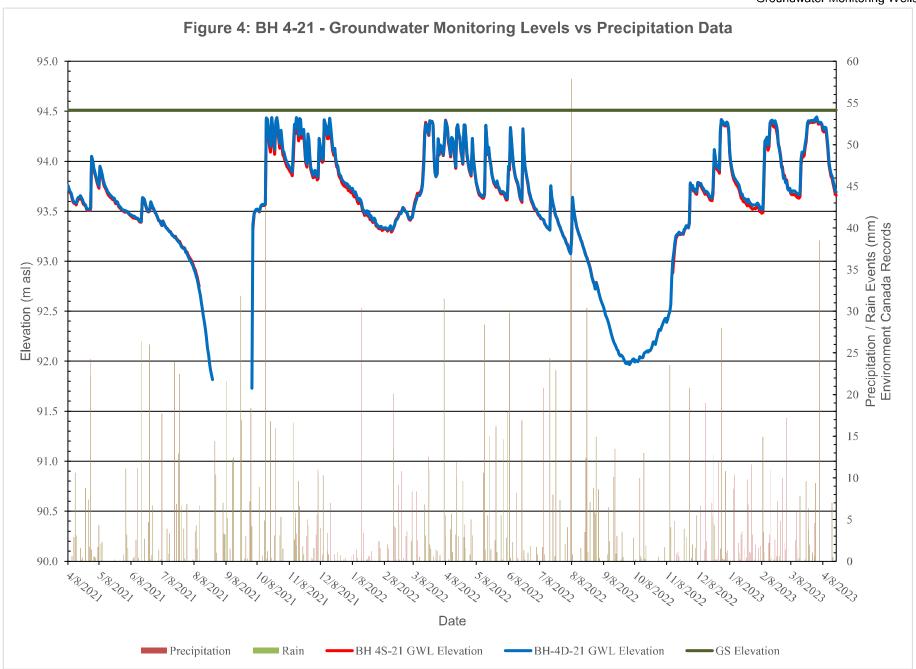




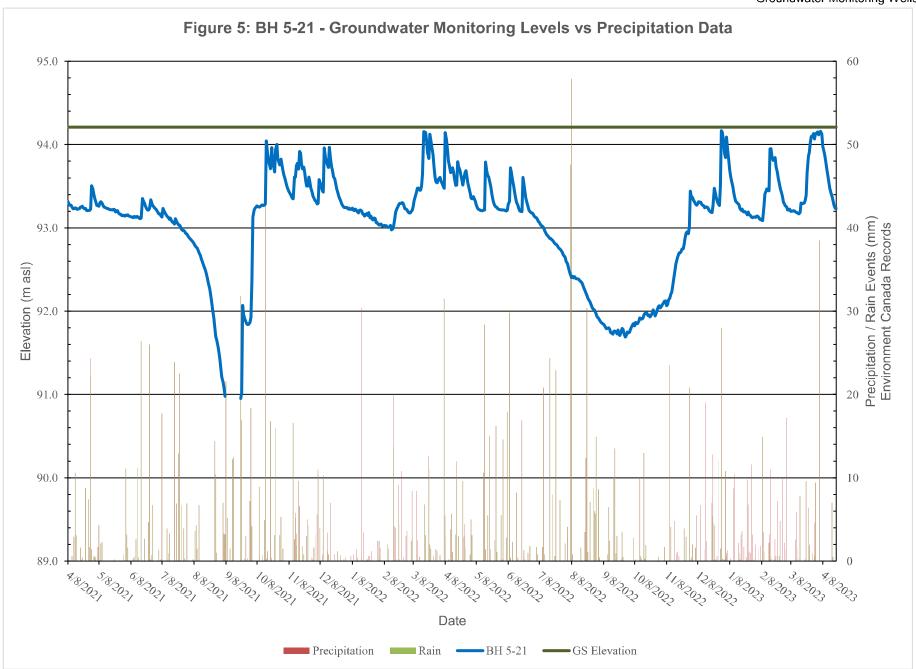




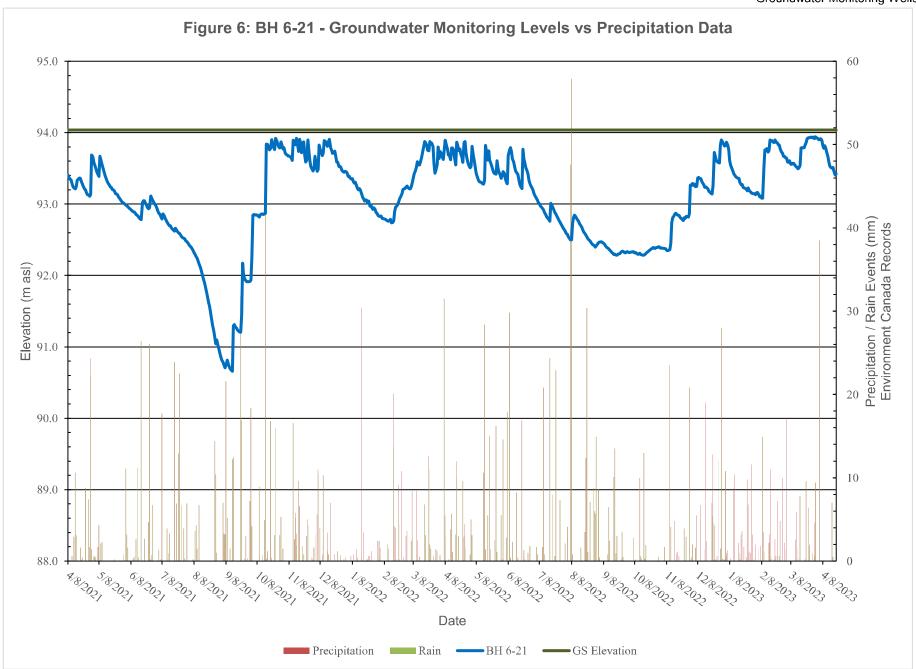




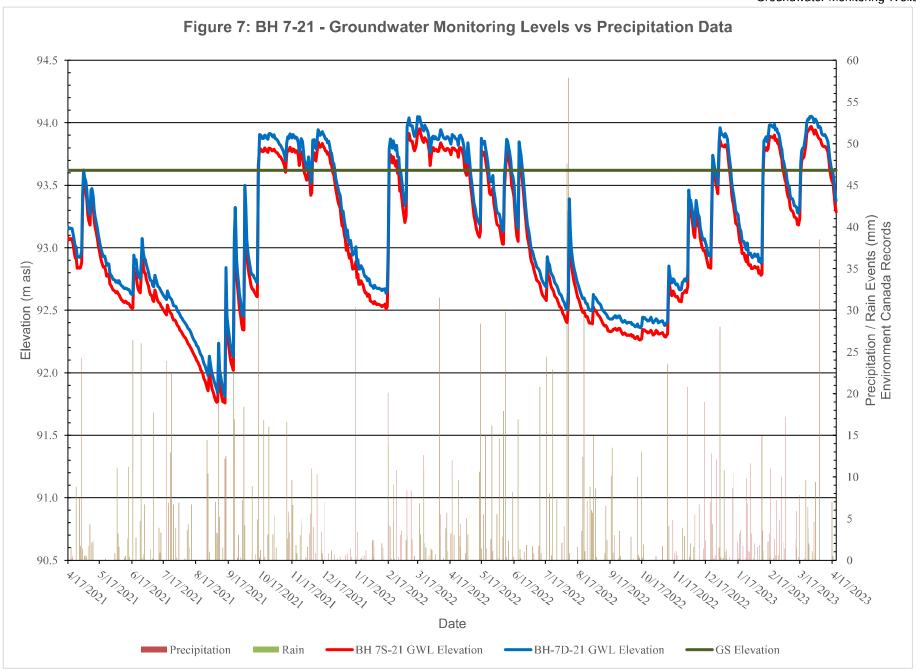




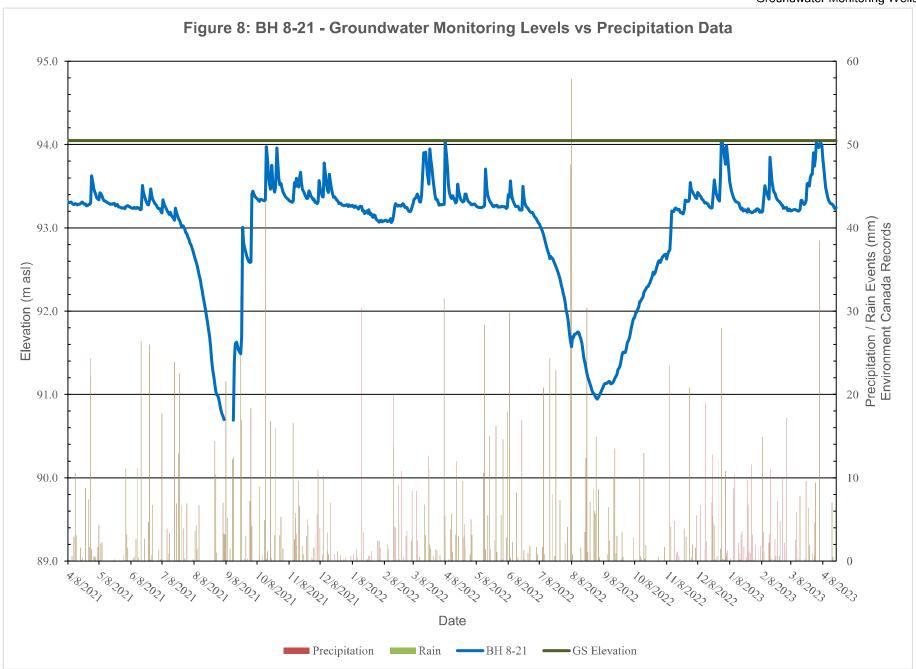




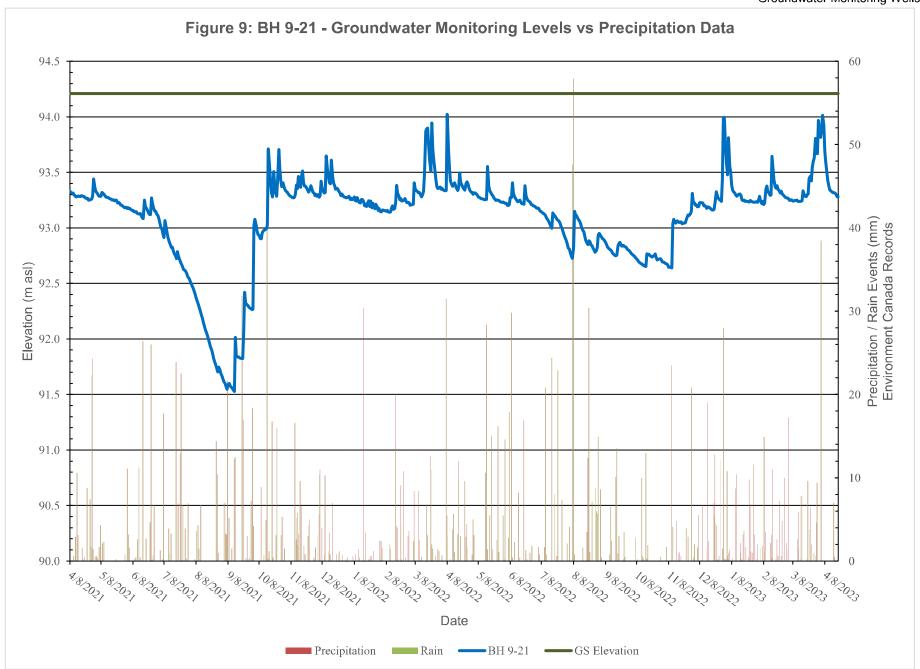




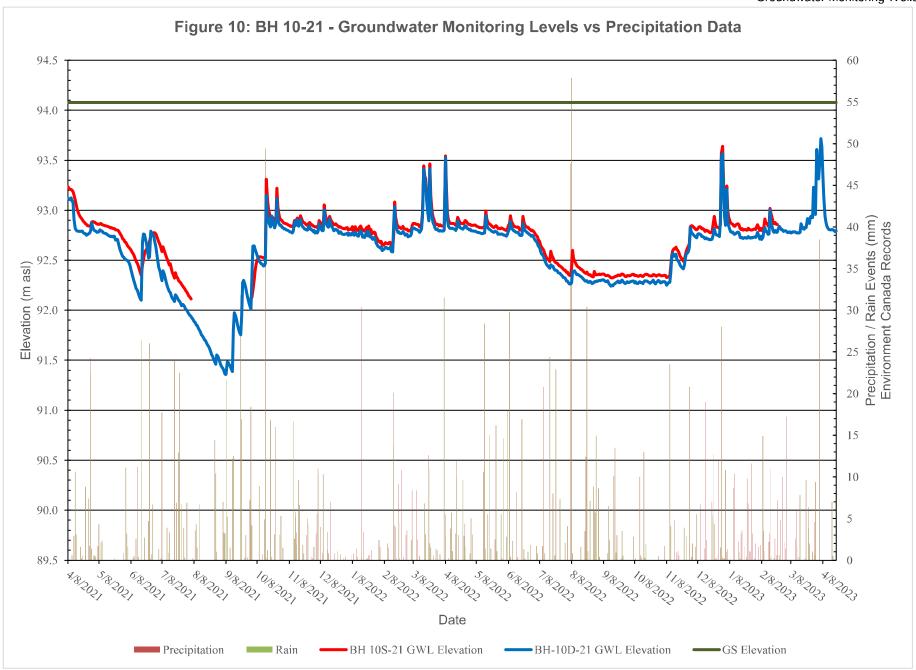




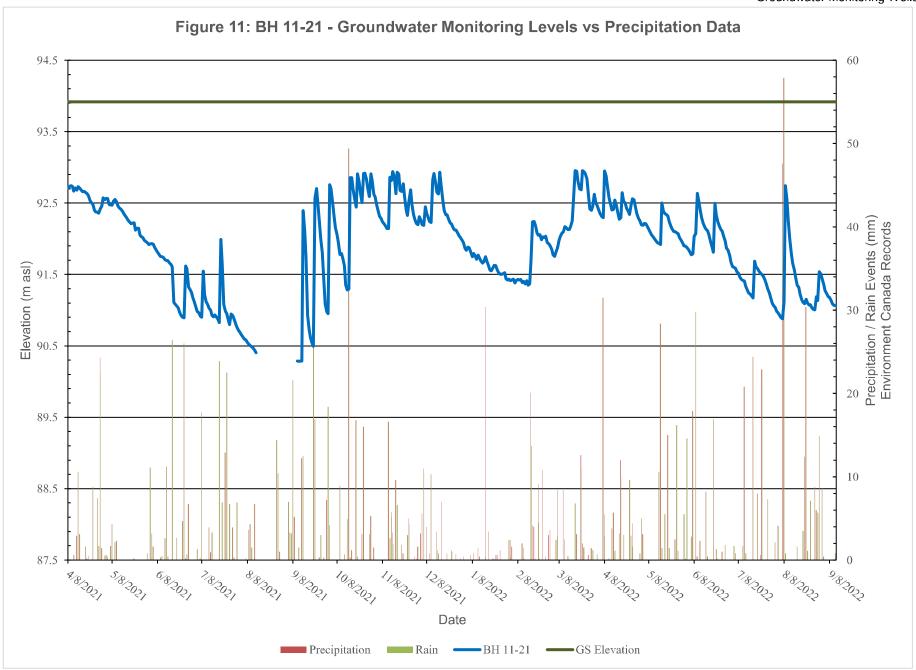




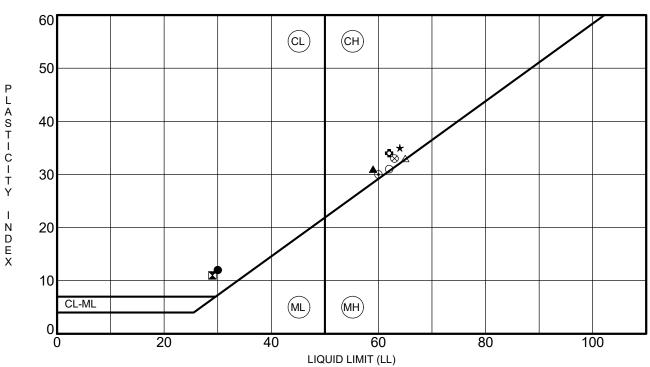












5	Specimen Ident	ification	LL	PL	PI	Fines	Classification
•	BH 3-25	SS2	30	18	12		CL - Inorganic clays of low plasticity
	BH11-24	SS3	29	18	11		CL - Inorganic clays of low plasticity
lack	HA 1-25	G1	59	28	31		CH - Inorganic clays of high plasticity
*	HA 2-25	G1	64	29	35		CH - Inorganic clays of high plasticity
$\odot$	HA 3-25	G1	60	30	30		CH - Inorganic clays of high plasticity
٥	HA 4-25	G1	62	28	34		CH - Inorganic clays of high plasticity
0	HA 5-25	G1	62	31	31		CH - Inorganic clays of high plasticity
Δ	HA 6-25	G1	65	32	33		CH - Inorganic clays of high plasticity
$\otimes$	HA 7-25	G1	63	30	33		CH - Inorganic clays of high plasticity

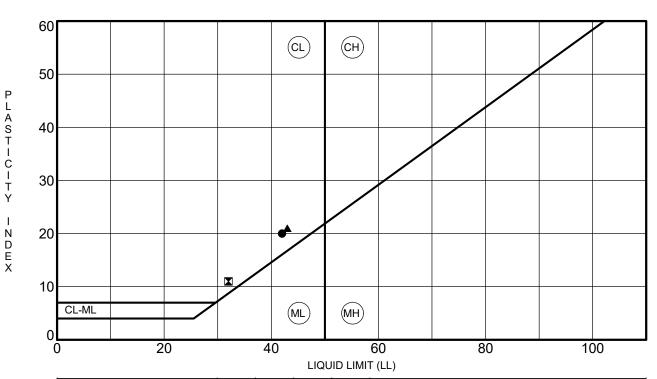
CLIENTTaggart InvestmentsFILE NO.PG4216PROJECTGeotechnical Investigation - Proposed Mixed-UseDATE2 Sep 25

Development - 5970 and 6038 Ottawa Street



9 Auriga Drive Ottawa, Ontario K2E 7T9 TEL: (613) 226-7381

ATTERBERG LIMITS' RESULTS



3	Specimen Identification			PL	PI	Fines	Classification
•	BH 3-24	SS2	42	22	20		CL - Inorganic clay with low plasticity
×	BH 5-24	SS2	32	21	11		CL - Inorganic clay with low plasticity
<b>A</b>	TP 1-24	G1	43	22	21		CL - Inorganic clay with low plasticity

CLIENT Tamarack (Richmond) Corporation FILE NO. PG4216

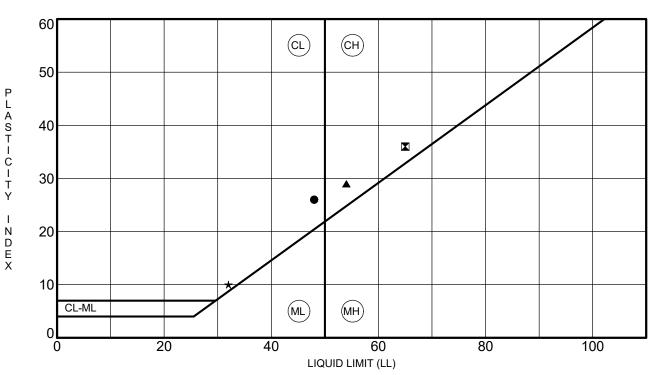
PROJECT Geotechnical Investigation - Proposed Mixed-Use DATE 14 Mar 25

Development - 5970 and 6038 Ottawa Street



9 Auriga Drive Ottawa, Ontario K2E 7T9 TEL: (613) 226-7381

ATTERBERG LIMITS' RESULTS



Specimen Identification			LL	PL	PI	Fines	Classification
•	BH 3A-21	SS2	48	22	26		CL - Inorganic clays of low plasticity
	BH 4-24	SS4	65	29	36		CH - Inorganic clays of high plasticity
	BH12-24	SS2	54	25	29		CH - Inorganic clays of high plasticity
*	BH14-24	SS1	32	22	10		CL - Inorganic clays of low plasticity
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CLIENT Tamarack ("Richmond") Corporation
PROJECT Geotechnical Investigation - Proposed Mixed-Use

FILE NO. \_\_\_\_

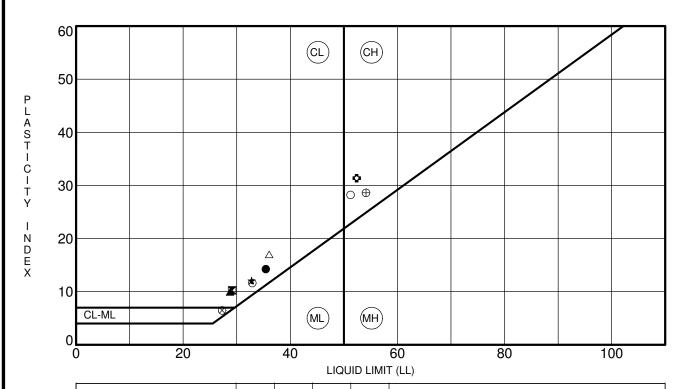
PG4216 14 Jan 25

Development - 5970 and 6038 Ottawa Street



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ATTERBERG LIMITS' RESULTS



Specimen Identification		ication	LL	PL	PI	Fines	Classification		
•	BH 2	SS 2	35	21	14		CL - Inorganic clay with low plasticity		
	BH 3	SS 3	29	19	10		CL - Inorganic clay with low plasticity		
<b>A</b>	BH 8	SS 2	29	19	10		CL - Inorganic clay with low plasticity		
*	TP 2	G 3	33	21	12		CL - Inorganic clay with low plasticity		
•	TP 3	G 4	33	21	12		CL - Inorganic clay with low plasticity		
0	TP 4	G 6	52	21	31		CH - Inorganic clays of high plasticity		
0	TP 5	G 6	51	23	28		CH - Inorganic clays of high plasticity		
Δ	TP 6	G 5	36	19	17		CL - Inorganic clay with low plasticity		
$\otimes$	TP 7	G 6	27	21	6		CL-ML - Inorganic silt with some clay with low p	las	ticity
$\oplus$	TP 8	G 5	54	26	29		CH - Inorganic clays of high plasticity		

CLIENTTaggart ConstructionFILE NO.PG4216PROJECTGeotechnical Investigation - ProposedDATE27 Feb 19

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Development - Eagleson Road at Ottawa St.

Consulting Engineers ATTERBERG LIMITS'
RESULTS

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