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ADEQUACY OF SERVICES REPORT

FOR

BARRHAVEN CONSERVANCY DEVELOPMENT CORPORATION

BARRHAVEN CONSERVANCY EAST PHASE 5

CITY OF OTTAWA

PROJECT NO.: 20-1180

DECEMBER 2022 – 1ST SUBMISSION

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**ADEQUACY OF SERVICES REPORT
FOR
BARRHAVEN CONSERVANCY DEVELOPMENT CORPORATION

BARRHAVEN CONSERVANCY EAST PHASE 5**

**CITY OF OTTAWA
PROJECT NO: 20-1180**

1.0 INTRODUCTION

David Schaeffer Engineering Limited (DSEL) has been retained to update an Adequacy of Services Report (AES) in support of the Barrhaven Conservancy “Phase 5” development area on behalf of Barrhaven Conservancy Development Corporation (BCDC). This area (being referred to as “Phase 5”) is part of a previously approved draft plan of subdivision within the greater Barrhaven Conservancy development area (City file no. D07-16-20-0021).

The overall Conservancy land area is approximately 139.7 ha (all land use components) and is located within the City of Ottawa urban boundary in the Barrhaven ward. As illustrated in **Figure 1**, the site is located north of the Jock River, east of Highway 416, west of Greenbank Road (and the Kennedy-Burnett Stormwater Facility), and south of both McKenna Casey Drive and Strandherd Drive.

The focus of this report is for the **Conservancy East (Phase 5)** draft plan area consisting of vacant land that is located east of the existing Foster Ditch, which bisects the overall BCDC landholdings, and west of Borrisokane Road. The subject lands are an approximately 19.4 ha irregular parcel including parts of 3288, and 3300 Borrisokane Road. Of this, approximately 13.82 ha in area (including right-of-ways environmental areas and open space) are considered in the servicing review with the proposed updated development draft plan **Figure 2B** provided in the **Drawings** section of this report for reference. Also provided is Figure 2A which illustrates the portion of the prior approved draft plan and the “Phase 5” area being revised. The development area is planned to be developed with a mix of detached single homes, townhomes, park blocks, open spaces and a road network.

The Conservancy East Phase 5 development area is outside of the Jock River 100-year limit as confirmed by the Rideau Valley Conservation Authority (RVCA). Refer to the RVCA confirmation letter in **Appendix D**. The 100-year regulatory flood line is demonstrated in Drawing 1 (Grading) and Drawing 3 (Stormwater) in the **Appendix**.

The objective of this report is to provide sufficient detail to demonstrate that the updated development plan area can be supported by municipal services.

1.1 Existing Conditions

The **Conservancy East (Phase 5)** property is relatively flat with the existing elevations ranging from ~91.5 m in the north to 91 m in the south. All existing flows are either overland to the Jock River or conveyed to the Jock River by way of the Foster Ditch and Borrisokane Road ditches which is adjacent to the subject property. The property is within the Jock River watershed and is under the jurisdiction of the RVCA.

1.2 Summary of Pre-Consultation

The following provides a summary of the pre-consultation:

1.2.1 Ministry of the Environment, Conservation and Parks (MECP)

Prior consultations associated with the Conservancy Phase 2-4 development east of Borrisokane Road were previously undertaken for the approval of those phases of the development area.

A pre-consultation with the local MECP office has not yet been completed for the balance of the Conservancy development area until the functional design details and requirements have been finalized with the City of Ottawa.

1.2.2 Rideau Valley Conservation Authority (RVCA)

Multiple consultations, analysis and submissions were coordinated with the RVCA to establish that the development area is outside of the Jock River 100-year limit. See the RVCA documentation in **Appendix D** for reference.

1.3 Existing Permits / Approvals

Key approvals associated with the advancement of development of the Barrhaven Conservancy area, are presented in the following table. The most relevant approvals are the Environmental Compliance Approval (ECA) for the South Nepean Collector sanitary trunk sewer as well as sanitary sewer ECA for the Conservancy development area east of Borrisokane Road. The documents are provided in **Appendix A** for reference.

Table 1A: Existing Permits / Approvals

Agency	Approval Type	Approval Number	Remarks
Ministry of the Environment, Conservation and Parks (MECP)	Environmental Compliance Approval	# 8129-AB7LDF (June 23, 2016)	South Nepean Collector existing approval (sanitary outlet for development area)
(MECP)	Environmental Compliance Approval	# 4357-CHMQEM (Sept. 1, 2022)	Sanitary and storm sewer approvals for Conservancy lands east of Borrisokane Road
MECP	Permit to take Water	#5633-C2RQPL (May 26, 2021)	Water taking from Building Excavation, Site Servicing, SWMW, In-Water Works, Poned Surface Water
Rideau Valley Conservation Authority (RVCA)	RVCA Letter of Permission under O.Reg. 174/06	RV5-4419	Letter of permission related to placement of fill within a regulated area.

1.4 Required Permits / Approvals

The City of Ottawa must approve detailed engineering design drawings and reports prior to future construction of the municipal infrastructure identified in this report. This will occur as part of the Plan of Subdivision application process and detailed design.

Based on pre-consultation with City staff, the additional approvals and permits listed in the following table are expected to be required prior to construction of the municipal infrastructure detailed herein. Please note that other permits and approvals may be required, as detailed in the other studies to be submitted as part of the Plan of Subdivision application (e.g. *Tree Conservation Report, Environmental Impact Statement, Phase 1 Environmental Site Assessment, Headwater Drainage Feature Assessment, etc.*)

Table 1B: Required Permits/Approvals

Agency	Permit/Approval Required	Trigger	Remarks
MECP	Environmental Compliance Approval	Construction of new sanitary and storm sewers throughout the subdivision.	The MECP will review the sanitary and storm sewer design through the City of Ottawa transfer of review process.
MECP	Environmental Compliance Approval	Implementation of oil-grit separator units and LIDs for quality control.	The MECP will review the stormwater management appurtenance design through the City of Ottawa transfer of review process.

MECP	Permit to Take Water	Construction of proposed land uses (e.g. basements for residential homes) and services.	Pumping of groundwater may be required during construction, given groundwater conditions and proposed land uses and on-site/off-site municipal infrastructure.
City of Ottawa	MECP Form 1 – Record of Watermains Authorized as a Future Alteration.	Construction of watermains throughout the subdivision	The City of Ottawa will review the watermains on behalf of the MECP through the Form 1 – Record of Watermains Authorized as a Future Alteration.
RVCA	Permit under Ontario Regulation 174/06, RVCA's Development, Interference with Wetlands and Alterations to Shorelines and Watercourses Regulation	Grading (proposed development & potential temporary access roads) within the subject lands (i.e. crossing of Fraser-Clarke Watercourse)	Supporting applications and documentation as required through consultation with the RVCA.
RVCA	Outlets to Jock River	In conjunction with issuance of MECP applications	Supporting applications and documentation as required through consultation with the RVCA.
RVCA	Alteration to Watercourses	As necessary through consultation with the RVCA	Supporting applications and documentation as required through consultation with the RVCA.
City of Ottawa	Commence Work Notification (CWN)	Construction of new sanitary and storm sewers throughout the subdivision	The City of Ottawa will issue a commence work notification for construction of the sanitary and storm sewers once an approval is issued by the MECP.

2.0 GUIDELINES, PREVIOUS STUDIES, AND REPORTS

2.1 Existing Studies, Guidelines, and Reports

The following studies were utilized in the preparation of this report.

- Ottawa Sewer Design Guidelines, City of Ottawa, *SDG002*, October 2012 (*City Standards*)

- Technical Bulletin ISDTB-2014-01
City of Ottawa, February 5, 2014
(ITSB-2014-01)
- Technical Bulletin PIEDTB-2016-01
City of Ottawa, September 6, 2016
(PIEDTB-2016-01)
- Technical Bulletin ISTB-2018-01
City of Ottawa, March 21, 2018
(ISTB-2018-01)
- Technical Bulletin ISTB-2018-04
City of Ottawa, June 27, 2018
(ISTB-2018-04)
- Ottawa Design Guidelines – Water Distribution
City of Ottawa, July 2010.
(*Water Supply Guidelines*)
 - Technical Bulletin ISD-2010-2
City of Ottawa, December 15, 2010.
(ISD-2010-2)
 - Technical Bulletin ISDTB-2014-2
City of Ottawa, May 27, 2014.
(ISDTB-2014-2)
 - Technical Bulletin ISTB-2018-02 / ISTB-2019-02
City of Ottawa, March 21, 2018 / July 08, 2019
(ISTB-2018-02 / ISTB-2019-02)
- Design Guidelines for Sewage Works,
Ministry of the Environment, Conservation and Parks, 2008. (formerly MOECC)
(*MECP Design Guidelines*)
- Stormwater Planning and Design Manual,
Ministry of the Environment, March 2003.
(*SWMP Design Manual*)
- City of Ottawa Official Plan,
adopted by Council 2003.
(*Official Plan*)
- City of Ottawa Secondary Plan – Former Nepean – South Nepean Urban Area –
Areas 9 and 10,
Adopted by Council 2003.
(*Secondary Plan*)

- South Nepean Collector: Phase 2 Hydraulics Review / Assessment Technical Memorandum
Novatech, August 2015
(*Novatech SNC Memo*)
- South Nepean Collector: Phase 2 Preliminary Design Report,
Novatech, March 2016
(*Novatech SNC Design Report*)
- Strandherd Drive Widening Project, South Nepean Collector: Phase 3 Sanitary Flow Calculations
Novatech, May 2019
(*2019 Novatech SNC Design Report*)
- Hydraulic Potable Water Assessment for Barrhaven Conservancy Development Corporation, March 2021
(*Stantec Hydraulic Analysis*)
- Jock River Reach One Subwatershed Study
Stantec, 2007
(*Jock River SWS*)
- Geotechnical Investigation, Proposed Residential Development, Conservancy Lands East, Ottawa, Ontario
Paterson Group, September 24, 2019 (Project No. PG5036-1)
(*Geotechnical Report*)
- Environmental Impact Statement for Barrhaven Conservancy East
Kilgour & Associates Ltd., July 29, 2020
(*Kilgour EIS*)
- Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River): Water Distribution System Analysis, Stantec, June 2, 2022
(*Stantec Hydraulic Analysis - East*)
- Adequacy of Services Report for Barrhaven Conservancy Development Corporation, Barrhaven Conservancy East
David Schaeffer Engineering Ltd., July 2021
(*DSEL East FSR*)
- Design Brief for Barrhaven Conservancy East – Phase 2, 3, & Jock River
David Schaeffer Engineering Ltd., June 2022
(*DSEL East Design Brief*)

3.0 WATER SUPPLY SERVICING

3.1 Existing Water Supply Services

The subject property is located adjacent to the City of Ottawa's Pressure Zone (PZ) 3SW (previously known as PZ BARR). PZ SUC services the lands that are east of the subject property, as well as south of the Jock River.

The City of Ottawa has recently reconfigured the pressure zones servicing Barrhaven and the South Urban Community (SUC) in order to improve reliability and efficiency and to increase pumping capacity to accommodate for future growth in the area. Work is ongoing. There are three pumping stations servicing Zone 3SW and Zone SUC as follows: the Fallowfield Road Pumping Station (FRPS), the Barrhaven Pumping Station (BPS) and the Ottawa South Pumping Station (OSPS).

There are future trunk watermains proposed in the vicinity of the subject property (i.e. along Greenbank Road) which will provide water service to development lands to the east and south of Conservancy East. These services will be further extended to provide the requisite water supply to the development area.

3.2 Water Supply Servicing Design

Stantec Consulting Limited was retained to perform a hydraulic assessment for the Conservancy East Lands. The ***Hydraulic Potable Water Assessment for Barrhaven Conservancy Development Corporation (Stantec Hydraulic Analysis)*** prepared by Stantec (March 2021) previously supported the advancement of the Conservancy East lands east of Borrisokane road. Subsequently, as part of the detailed design for the approved phases east of Borrisokane Road, Stantec prepared an updated study "***Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River): Water Distribution System Analysis*** (June 2022 – ***Stantec Hydraulic Analysis - East***) which is enclosed in ***Appendix B*** for reference. Note that phasing references have changed for the development area and the "Phase 4" area referenced in the Stantec study represents the "Phase 5" which is the subject of this updated draft plan – See Figure 1-2 of the ***Stantec Hydraulic Analysis – East*** study for reference. As well, the layout analyzed in the Stantec analysis differs slightly but is not expected to impact the serviceability.

The analysis reviewed the system requirements of the development area on the west and east sides of Borrisokane Road but only the detailed design of the areas east of Borrisokane Road were advanced to detailed design.

The proposed water servicing layout is presented in ***Figure 3***.

The following table summarizes the relevant Water Supply Design Criteria which will be employed in the design of the subject property.

Table 2A: Water Supply Design Criteria

Design Parameter	Value
<i>Extracted from Section 4: Ottawa Design Guidelines, Water Distribution (July 2010)</i>	
Residential – Detached Single	3.4 p/unit
Residential – Townhome/ Semi	2.7 p/unit
Residential – Apartment	1.8 p/unit
Minimum Watermain Size	150 mm diameter
Minimum Depth of Cover	2.4 m from top of watermain to finished grade
During normal operating conditions desired operating pressure is within	350 kPa and 480kPa
During fire flow operating pressure must not drop below	140 kPa
<i>Stantec Hydraulic Analysis, Stantec, July 20, 2017 for Population Exceeding 3000 Persons</i>	
Residential – Detached Single	180 L/cap/day
Residential – Rear Lane Town	198 L/cap/day
Residential – Back-to-Back	198 L/cap/day
Outdoor Water Demand	1049 L/unit/day (single detached)
Basic Day	Population x Demand
Max Day	Basic Day + Outdoor Water Demand

3.2.1 Fire Flow Demand

Fire Flow requirements are established in the boundary condition request found in **Appendix B** as prepared by Stantec. Based on anticipated unit configurations and separations the City’s fire flow cap of 10,000 L/min for single dwellings and traditional townhomes as outlined in *ISDTB-2014-02* does not apply and separation of fire areas with units of ordinary construction, as well as architectural elements, are required to meet target fire flows. The fire flows are calculated in accordance with the Fire Underwriters Survey’s Water Supply for Public Fire Protection Guideline (1999). Detailed FUS calculations can be found in the Stantec reporting.

3.2.2 Boundary Conditions

To support the preparation of a hydraulic analysis for the subdivision, boundary conditions were provided by the City of Ottawa for the anticipated water demands and are summarized in the following table. See **Appendix B** for full details of the boundary condition request submitted.

Table 2B: Boundary Conditions (from *Stantec Hydraulic Analysis – East report*)

HGL (m) - Zone SUC Servicing Conditions			
Demand Scenario	Two Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	150.0	150.0	
PKHR	144.2	144.0	
AVDY +FF	138.7	135.1	
MXDY +FF	137.0	133.2	
Demand Scenario	Two Connections with Upgrades ⁽⁴⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	149.5	149.5	
PKHR	144.1	144.1	
AVDY +FF	138.6	139.8	
MXDY +FF	136.8	138.1	
Demand Scenario	Three Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	Connection 3 ⁽³⁾
AVDY	149.5	149.5	149.5
PKHR	144.5	144.4	142.0
AVDY +FF	138.6	135.1	137.4
MXDY +FF	137.1	133.4	134.8

(1) Ground elevation at Connection 1 (Chapman Mills Drive) = 92.80 m
(2) Ground elevation at Connection 2 (Danson Gardens Grv / Darjeeling Ave) 91.80 m
(3) Ground elevation at Connection 3 (Flagstaff Dr) 92.10 m
(4) Upgrades to existing water distribution required to increase HGL at Connection 2; upscale existing 203mm diameter watermain on Danson Gardens Grv to a 305mm watermain
(5) For scenarios where ultimate conditions will include three connections, the boundary conditions for two connections (without upgrades) were used when only connections 1 and 2 are in place (i.e. for modelling Phases 2 and 3.

3.2.3 Water Demand Calculations

A summary of water demands for the subject site is presented in the following table as derived from the criteria above and the *Stantec Hydraulic Analysis* found in **Appendix B**.

Table 2C: Water Demand Estimate

	Unit Count Conservancy East	Pop ⁽¹⁾	AVDY ⁽²⁾ (L/s)	OWD ⁽³⁾ (L/s)	MXDY ⁽⁴⁾ (L/s)	PKHR ⁽⁵⁾ (L/s)
Single Family	782	2,659	8.62	9.49	21.55	47.38
Townhouse	606	1,636	5.30	0	13.25	29.17
Totals	1,388⁽⁶⁾	4,296	13.92	9.49	34.80	76.55

(1) Population per unit is 3.4 for Single Family and 2.7 for Townhomes
(2) AVDY = Average Day
(3) OWD (outdoor water demand) = 1,049 L/unit/day for Singles
(4) MXDY = Maximum Day
(5) PKHR = Peak Hour
(6) Total unit count may vary slightly from final layouts but are estimated to be within +/-2.5%.
(7) See Stantec Hydraulic Analysis in **Appendix B** for details.

3.3 Summary of Hydraulic Modeling Analysis

A watermain analysis has been prepared to confirm that the network is sized adequately, which is the greater of maximum day plus fire and maximum hour. City review comments on the current **Stantec Hydraulic Analysis - East** note that 'Option B' is the preferred system configuration (sizing and layout for three connections) and those results are presented below. For full details of the assessments refer to the **Stantec Hydraulic Analysis - East**, enclosed in **Appendix B**.

System Pressures

The modeling indicates that the development can be adequately serviced by the proposed watermain network. Modeled service pressures for the development are summarized the following table. The detailed pipe and junction tables are contained in the **Stantec Hydraulic Analysis**, enclosed in **Appendix B**.

Table 2D: Summary of Available System Pressures

	AVDY Maximum Pressure		Peak Hour Demand Minimum Pressure	
	kPA	psi	kPA	psi
Option B – 3 Connections	559	81.14 (J55, J60)	482	69.83 (J103)

*Note: See model results in the Appendix D of the **Stantec Hydraulic Analysis** memo (buildout of all phases).*

The generally accepted best practice is to design new water distribution systems to operate between 350 kPa (50 psi) and 480 kPa (70 psi) as outlined in the City of Ottawa Design Guidelines. Where pressures exceed 80psi pressure reducing valves (PRV) shall be implemented as per the Ontario Building Code.

Available Fire Flows

The minimum allowable pressure under fire flow conditions is 140 kPa (20 psi) at the location of the fire. A summary of the available fire flows is presented in the following table. The detailed fire flow reports are found in the **Stantec Hydraulic Analysis - East** enclosed in **Appendix B**.

Table 2E: Summary of Available Fire Flows

	Required Fire Flow (L/s)	Minimum Available Flow (L/s)	Junction ID
Option B – 3 Connections	217	250	J86, J87

*Note: See model results in the Appendix D of the **Stantec Hydraulic Analysis – East** memo (buildout of all phases). Exception is the phase west of Borrisokane Road where the anticipated cul-de-sac fire flow node can be managed by procedures noted in ISDTB-2018-02 (See Section 3.2 of Stantec report)*

As shown in the above table, the model predicts the network will be able to provide all required fire flows. Detailed results are included in the ***Stantec Hydraulic Analysis - East***, enclosed in ***Appendix B***.

System Reliability

Various major watermain failure scenarios were reviewed by Stantec. Some scenarios resulted in potential reliability issues which have been resolved in the updated watermain layout with additional looping in the northwest area of the design. See discussion in Section 3.3 of the ***Stantec Hydraulic Analysis -East***.

3.4 Water Supply Conclusion

The subject lands are have been reviewed by Stantec to confirm that servicing is feasible from the SUC pressure zone. Future watermain extensions from Nepean Town Centre development areas, being constructed as part of Phase 2-4 approvals, will facilitate servicing to the Conservancy East Phase 5 lands via watermain extension along the future Chapman Mills Drive extension and through the Claridge “Burnett Lands” development area. Future modelling at the detailed design stage will confirm phasing of the extensions of trunk watermains and sizing of the local watermain network. The proposed water supply design will conform to all relevant City and MECP Guidelines and Policies.

4.0 WASTEWATER SERVICING

4.1 Existing Wastewater Services

Per the ***South Nepean Collector (SNC) Wastewater Servicing Study and Functional Design Report*** by Dillon in October 2003 (***Dillon SNC Report***), the subject property is tributary to the South Nepean Collector (SNC) sewer as urban development land.

The SNC (previously called the Jock River Collector) sewer operates north of the subject property within Strandherd Drive prior to travelling south down a Chapman Mills Drive (CMD) and then turns eastward within the future CMD right-of-way (ROW).

The ***South Nepean Collector Phase 2: Hydraulics Review / Assessment*** memo was prepared by Novatech Engineering Consultants on August 20, 2015 (***Novatech SNC Memo***) to provide an update to the sanitary design flows for Phase 2 of the South Nepean Collector, as previously documented in the ***South Nepean Collector (SNC) – Functional Design Report and Update*** by Dillon in 2012 (***Dillon SNC Report and Update***).

4.2 South Nepean Collector Phase 3 – Preliminary Design

The 2015 ***Novatech SNC Memo*** contemplated that the Conservancy Phase 1 development area (north of the Fraser-Clarke Watercourse) would be serviced by the 900 mm diameter SNC sewer running adjacent to the property within the future extension of CMD. This is represented by area “A6-E” within the “***Sanitary Drainage Areas and Land Use – Fig.1***” plan within the 2015 Novatech memo (note that the actual tributary area and population varied slightly).

For the Phase 3 extension of the SNC, Novatech has prepared another review of sanitary flows within their technical memorandum titled “***Strandherd Drive Widening Project, South Nepean Collector Phase 3: Sanitary Flow Calculations***” May 30, 2019 (***2019 Novatech SNC Memo***). The memorandum along with the design sheet calculations from the Novatech memo are provided in ***Appendix C*** for reference along with DSEL annotations on key items in the figure and design sheets. The updated “***Sanitary Drainage Areas and Land Use – Fig.1***” (May 2019) plan is essentially reflective of the same tributary information that was provided in the 2015 study (the plan has been marked up to reflect the Conservancy areas as a frame of reference). The associated design sheet also reflects updated City wastewater design criteria that was not accounted for in the 2015 study and is discussed further in the following section.

Report excerpts are provided in ***Appendix C*** for the SNC Phase 2 analysis as well as draft information associated with the Phase 3 extension. The location of the SNC sewer is shown in ***Figure 4***.

4.3 Wastewater Design

The subject property is planned to be serviced by an internal gravity sanitary sewer system that is to generally follow the local road network. The wastewater servicing plan can be seen in **Drawing 4**.

The prior report proposed that the drainage area of the SNC sanitary sewer be expanded to include the entirety of the Conservancy property. The sewer network will connect to the off-site SNC sanitary sewer within the future CMD at existing manhole 'SANMH8' as identified in the Novatech SNC Phase 2 design Drawing No. 20 provided in **Appendix C** for reference (City contract number ISD14-2033). As noted in the prior section, the 2015 **Novatech SNC Memo** was derived flows based on the City guideline parameters of the time (namely 350 L/capita/day, infiltration allowance of 0.28 L/s/ha and commercial properties at 50,000 L/ha/d). The following table summarizes the new City design guidelines and criteria to be applied to the **Conservancy East** sewer design as well for the determination of the projected flows to be tributary to the SNC along the frontage of the Conservancy Phase 1 development area.

Table 3: Wastewater Design Criteria

Design Parameter	Value
Current Design Guidelines	
Residential - Single Family	3.4 p/unit
Residential – Townhome/ Semi	2.7 p/unit
Residential – Apartment	1.8 p/unit
Average Daily Demand	280 L/d/person
Peaking Factor	Harmon's Peaking Factor. Max 4.0, Min 2.0
Commercial / Institutional Flows	28,000 L/ha/day
Commercial / Institutional Peak Factor	1.5
Infiltration and Inflow Allowance	0.33 L/s/ha
Park Flows	28,000 L/ha/d
Park Peaking Factor	1.0
Sanitary sewers are to be sized employing the Manning's Equation	$Q = \frac{1}{n} AR^{2/3} S^{1/2}$
Minimum Sewer Size	200mm diameter
Minimum Manning's 'n'	0.013
Minimum Depth of Cover	2.5m from crown of sewer to grade
Minimum Full Flowing Velocity	0.6m/s
Maximum Full Flowing Velocity	3.0m/s
<i>Extracted from Sections 4 and 6 of the City of Ottawa Sewer Design Guidelines, October 2012, and associated Technical Bulletins.</i>	

The sanitary design sheet for the lands east of Borrisokane Road is provided in **Appendix C** for reference. Within that design sheet the area and flows from the lands west of Borrisokane Road are highlighted where flows enter that development area at the westward stub from MH10A. That design sheet projected a flow of 77.81 L/s. Based on the updated Phase 5 draft plan, and updated concept plans for the

Conservancy West development area (west of the Foster Ditch), the flows shown at the eastern limit of Phase 5 (see Phase 5 design sheet in **Appendix C**) is now ~68.96 L/s at MH 532A. As such, downstream systems are sufficient and no negative impacts given that flows are lower than the previously projected 77.81 L/s.

4.4 Wastewater Servicing Conclusion

The subject property will be serviced by local sanitary sewers, an on-site trunk sanitary sewer, and the off-site SNC sanitary sewer as defined in previous reports. This AES continues to confirm that the expansion of the drainage areas from the **2019 Novatech SNC Memo** to include the entirety of the subject property has no negative impacts. There is residual capacity in the downstream SNC providing sufficient capacity for the peak sanitary flows for the subject property, including external commercial and community park flows.

5.0 STORMWATER CONVEYANCE

5.1 Existing Stormwater Drainage

The subject property is within the Jock River watershed. Per the existing topography characterized in available City of Ottawa base mapping, as well as site specific survey, all flows from the subject property are ultimately conveyed to the Jock River by a series of watercourses, sheet flow and minor ditches. The Foster Ditch, Borrisokane Road roadside ditches, are the main stormwater conveyances within the Conservancy East Phase 5 property that convey stormwater to the Jock River.

5.2 Proposed Stormwater Management Strategy

As documented in the previous AES, various stormwater strategies were discussed within the Master Infrastructure Review (MIR) prepared in parallel with the AES. Alternatives reviewed were:

Alternative 1 – Oil and Grit Separators & Treatment Train to Naturalized Wetlands*

Alternative 2 – Stormwater Management Wetland Facilities in the Floodplain

Alternative 3 – Stormwater Management Wetland Facilities out of the Floodplain

Alternative 4 – Modified Etobicoke filtration System (MEFS)

For the purposes of this AES update for Phase 5 Alternative 1 continues to be advanced as per the evaluation provided in the MIR and per discussions with the City of Ottawa on July 20, 2021. This alternative:

- A storm sewer system designed to capture at least the minimum design capture events in accordance with the amendment to the storm sewer and stormwater management elements of the Ottawa Design Guidelines – Sewer (Technical Bulletin PIEDTB-2016-01). The stormwater runoff will be treated before ultimately being released into the natural heritage features and the Jock River as per the ***Jock River Reach One Subwatershed Study*** prepared by Stantec in 2007 (***Jock River SWS***).
- All proposed units will be equipped with sump pumps due to local constraints;
- A treatment train approach to attain an Enhanced Level of Protection (80% total suspended solids (TSS) removal) per MECP guidelines consisting of:
 - Deep sump catchbasins;
 - The incorporation of infiltration-type LIDs within the right-of-way extending out from catchbasin locations (see ***Figure 5*** in the ***Figures & Drawings*** section). Future detailed grading will allow for the determination of suitable locations in order to yield optimal benefit from this LID. See Section 5.7 for additional LID discussion.

- Multiple oil and grit separators (OGS) units to provide TSS treatment with outlets that are above the 2-year event summer water levels on the Jock River;
- The storm systems will discharge the treated stormwater at multiple outlets located along the southern natural heritage corridor, connecting via channels. Discharge locations are demonstrated in the **Storm Tributary Area** plan in the **Figures & Drawings** section
- An on-site road network designed to maximize the available storage within right-of-ways for the 100-year design event, where possible; and
- An overland flow route designed to safely convey stormwater runoff flows in excess of the on-site road storage.

The design for the site proposes to have stormwater flows conveyed through the development area of the subject property via an underground sewer network. The stormwater runoff will be treated before ultimately being released into the Jock River as per the **Jock River Reach One Subwatershed Study** prepared by Stantec in 2007 (**Jock River SWS**).

The proposed stormwater design layout is shown on **Drawing 3** with the stormwater management design consisting of (similar to prior phases):

- A storm sewer system designed to capture at least the minimum design capture events in accordance with the amendment to the storm sewer and stormwater management elements of the Ottawa Design Guidelines – Sewer (Technical Bulletin PIEDTB-2016-01);
- All proposed units will be equipped with sump pumps due to local constraints;
- A treatment train approach to attain an Enhanced Level of Protection (80% total suspended solids (TSS) removal) per MECP guidelines consisting of:
 - Deep sump catchbasins to reduce catchbasin sump sediment re-suspension and optimize TSS removal;
 - Multiple oil and grit separators (OGS) units to provide TSS treatment with outlets that are above the 2-year event summer water levels of the Jock River;
 - The incorporation of infiltration-type LIDs within the right-of-way extending out from catchbasin locations (see **Figure 5** in the **Figures & Drawings** section). The future detailed grading will allow for the determination of preferred locations in order to yield optimal benefit from this LID. See Section 5.7 for additional LID discussion;

- The storm systems will discharge the treated stormwater at multiple outlets (2) located along the natural heritage corridor, connecting to the Foster Ditch via channels to support hydration of the wetlands and ultimately outletting to the Jock River. Discharge locations are demonstrated in **Drawing 3**;
- An on-site road network designed to maximize the available storage within right-of-ways for the 100-year design event, where possible, with controlled release of stormwater to the minor storm system; and
- An overland flow route designed to safely convey stormwater runoff flows in excess of the on-site road storage.

Although quantity control has not typically been required for this reach of the Jock River, as per the **Jock River SWS**, the quantity of stormwater runoff exiting from the subject property will be minimized by optimizing on-site storage in the sags of the proposed road network, which in turn minimizes the size of downstream storm sewer infrastructure. It is noted that the RVCA is currently reviewing the SWM requirements within the Jock River Reach 1 area. In consideration of this, J.F. Sabourin and Associates (JFSA) has undertaken a review of the existing quantity control recommendations and the existing, and proposed, development conditions for this area. The findings are presented in the JFSA memorandum *Review of Quantity Control Requirement for Jock River Reach 1 (March 2021)* provided in **Appendix D** which concludes that quantity controls will still not be required for this reach of the Jock River.

5.2.1 Post-Development Stormwater Management Targets

Stormwater management requirements for the proposed alternative Stormwater management scheme have been adopted from the **Jock River SWS**, **City Standards**, and the **MECP SWMP Manual**.

Given the general criteria mentioned above, the following specific standards are anticipated for stormwater management within the subject property:

- Enhanced quality treatment will be provided for stormwater runoff from the subject property, corresponding to a long-term average TSS removal efficiency of 80%, as defined by the MECP prescribed treatment levels;
- Downstream receiving watercourses will be assessed for responses to planned stormwater management outflows, and stabilization mitigation measures will be planned as required;
- Storm sewers on local roads are to be designed to provide at least a 2-year level of service without any ponding per the City's latest Technical Bulletin PIEDTB-2016-01;

- Storm sewers on collector roads are to be designed to provide at least a 5-year level of service without any ponding per the City's latest Technical Bulletin PIEDTB-2016-01;
- For less frequent storms (i.e. larger than 2-year or 5-year), the minor system sewer capture will be restricted with the use of inlet control devices to prevent excessive hydraulic surcharges;
- Under full flow conditions, the allowable velocity in storm sewers is to be no less than 0.80 m/s and no greater than 6.0 m/s;
- For the 100-year storm and for all roads, the maximum depth of water (static and/or dynamic) on streets, rear yards, public space and parking areas shall not exceed 0.35 m at the gutter;
- The major system shall be designed with sufficient capacity to allow the excess runoff of a 100-year storm to be conveyed within the public ROW, or adjacent to the ROW, provided the water level does not touch any part of the building envelope; must remain below all building openings during the stress test event (100-year + 20%); and must maintain 15 cm vertical clearance between spill elevation on the street and the ground elevation at the nearest building envelope;
- Flow across road intersections shall not be permitted for minor storms (generally 5-year or less);
- When catchbasins are installed in rear yards, safe overland flow routes are to be provided to allow the release of excess flows from such areas. A minimum of 30 cm of vertical clearance is required between the rear yard spill elevation and the ground elevation at the adjacent building envelope; and
- The product of the maximum flow depths on streets and maximum flow velocity must be less than 0.60 m²/s on all roads.

5.2.2 Quality Control

Per the **Jock River SWS**, Enhanced quality treatment will be provided for stormwater runoff from the subject property, corresponding to a long-term average TSS removal efficiency of 80%, as described by the MECP prescribed treatment levels. See Section 5.3 for quality control approach and discussion.

5.2.3 Quantity Control

As noted in the **Jock River SWS**, quantity control is not anticipated to be required for outlets to the Jock River, however, some quantity control may be provided by erosion storage, as erosion thresholds for any watercourses/outlets will be respected where required. As noted in Section 5.2, JFSA has reviewed the current/future development conditions contributing to this reach of the Jock River and concludes that quantity

control will still not be required. See “Review of Quantity Control Requirement for Jock River Reach 1 (JFSA March 2021) provided in **Appendix D**.

5.3 Stormwater Management Design

5.3.1 Treatment Train Approach

JFSA previously (June 2021 memo) prepared a review of various potential stormwater quality treatment options that were investigated for the development. These included options, and combinations of options, as summarized in the following updated table. Each of the options has an expected total suspended sediment (TSS) removal capability, varying from 5% to 88%. This review assessed how the required Enhanced Level of Protection (80% TSS removal) could be achieved when the options are used in a treatment train approach, consistent with the expected requirements of the upcoming MECP Consolidated Linear Infrastructure policy.

<u>Selection and comparison of alternatives</u>									
Method	TSS Removal	Alt. 1	Alt. 2	Alt. 3	Alt. 4	Alt. 5	Alt. 6	Alt. 7	Alt. 8
Street Sweeping (Monthly)	5%								
Street Sweeping (Weekly)	10%								
Street Sweeping (Weekly with Elgin Eagle)*	88%	x							
Curb Cut with Grass Swales	75%			X					
Curb Cut with Infiltration Trenches	80%								
Catchbasin Inserts (CB Shield)*	27%		X		X	X		X	
Deep Sump Catch Basin	25%				X		X		X
Infiltration/ Filtration Trenches**	80%				X	X	X		
Infiltration at CBs, per MOE Table 3.2 (22.5m ³ /ha)	70%								X
OGS*	50%			X					X
JellyFish*	85%							X	
SWM Pond (Wet Pond)	80%		X						
Overall Performance		88.0%	85.4%	87.5%	89.1%	85.4%	85.0%	89.1%	88.8%
Treatment Train Overall Performance = 1 - (1- TSS Removal Rate Method 1) x (1- TSS Removal Rate Method 2) x (1- TSS Removal Rate Method 3 x ...)									
*) TSS Removal as documented by ETV Canada									
**) includes the use of Etobicoke infiltration or filtration systems or other permutations of the same									

The above table provides a summary of the TSS removal for the various methods that were considered. An option of infiltration LID measures located at catchbasin locations has been added as a method, and to Alternative 8 (see further discussion regarding this method below). The options, and combinations of options, have been assessed and shown to meet or exceed the required 80% TSS target.

For the development area, the updated Alternative 8 option demonstrates an estimated TSS removal of 88.8% for that particular treatment train approach which has been discussed with City staff for the approval of prior phases and will be the design being

advanced. For the determination of the TSS removal of 70% for the infiltration LID at catchbasins, the *Table 3.2* of the MOECC (now MECP) publication entitled “*Stormwater Management Planning and Design Manual, March 2003*” sets the storage volume requirements for infiltration measures to achieve certain TSS removal rates.

Table 3.2 Water Quality Storage Requirements based on Receiving Waters^{1, 2}

Protection Level	SWMP Type	Storage Volume (m ³ /ha) for Impervious Level			
		35%	55%	70%	85%
<i>Enhanced</i> 80% long-term S.S. removal	Infiltration	25	30	35	40
	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250
<i>Normal</i> 70% long-term S.S. removal	Infiltration	20	20	25	30
	Wetlands	60	70	80	90
	Hybrid Wet Pond/Wetland	75	90	105	120
	Wet Pond	90	110	130	150
<i>Basic</i> 60% long-term S.S. removal	Infiltration	20	20	20	20
	Wetlands	60	60	60	60
	Hybrid Wet Pond/Wetland	60	70	75	80
	Wet Pond	60	75	85	95
	Dry Pond (Continuous Flow)	90	150	200	240



¹Table 3.2 does not include every available SWMP type. Any SWMP type that can be demonstrated to the approval agencies to meet the required long-term suspended solids removal for the selected protection levels under the conditions of the site is acceptable for water quality objectives. The sizing for these SWMP types is to be determined based on performance results that have been peer-reviewed. The designer and those who review the design should be fully aware of the assumptions and sampling methodologies used in formulating performance predictions and their implications for the design.

²Hybrid Wet Pond/Wetland systems have 50-60% of their permanent pool volume in deeper portions of the facility (e.g., forebay, wet pond).

The required storage volume of 22.5 m³/ha is determined for the development area prorated from the above table based on an overall imperviousness of ~62.5%. Similar to prior phases it is anticipated that the extent of the site area for Conservancy East Phase 5 can be managed with the proposed LID. For prior phases it is noted that approximately 140 lineal meters of LID per hectare of area to be treated was required. With approximately 9.0 ha of area to be treated (which excludes rear yards similar to the prior phase) this equates to 140x9.0= ~1,260 m extent of LID required. Phase 5 has approximately 1,800 m of roadway to incorporate the LID infrastructure therefore sufficient roadway is available for use.

5.3.2 Oil-Grit Separator Units (OGS)

As shown on **Drawing 3**, two (2) OGS units at locations along the southern boundary of the property, discharging to the Jock River via the existing Foster Ditch. By way of an MECP Certificate of Technology Assessment and manufacturer’s design report, the OGS units will demonstrate compliance with Enhanced Level of Protection requirements, with specific drainage area parameters for each area.

The manufacturer’s reported efficiency of TSS removal of the OGS units is expected to be based on a ‘fine distribution’ particle size distribution in conformance with the following table, unless otherwise approved by the City of Ottawa, RVCA, and MECP. The particle size distribution is the generic particle size distribution accepted by the City of Toronto per the *Wet Weather Flow Management Guidelines* (City of Toronto, 2006) as a typical average stormwater particle size distribution, and is an excerpt from Table 3.3 of the *Stormwater Management Practices Planning and Design Manual* (MOECC, 1994).

**Table 4: Typical Stormwater Particle Size Distribution & Settling Velocities
 (Source: *Stormwater Management Practices Planning and Design Manual*,
 MOECC, 1994)**

Particle Size (microns) (NURP 1983)	% of Particle Mass	Average Settling Velocities (m/s)
< 20	0 - 20	0.00000254
20 - 40	20 - 30	0.00001300
40 - 60	30 - 40	0.00002540
60 - 130	40 - 60	0.00012700
130 - 400	60 - 80	0.00059267
400 - 4000	80 - 100	0.00550333

To allow for flexibility as detailed design advances, it is proposed that any OGS unit can be selected, given that it:

- Meets the requirements set out in the preceding sections;
- Ensures no significant negative impact on the upstream storm sewer system – to be determined via hydraulic modelling at detailed design; and
- Demonstrates suitability for meeting Enhanced water quality targets via a MECP Certificate of Technology Assessment.

The preliminary OGS units proposed in the following table have been sized to treat the stormwater runoff for the tributary areas noted in order to meet MECP Enhanced Level of Protection criteria prior to discharge to the Jock River via naturalized wetlands as

shown on **Drawing 3**. The OGS total suspended removal rates and preliminary OGS unit details have been attached for reference in **Appendix D**.

Table 5: OGS Unit ID and Design Characteristics

Area and Unit ID ⁽¹⁾⁽²⁾	Drainage Area Target (ha)	Estimated Weighted C Value	Unit Treatment Capacity (L/s)	Unit Model ⁽¹⁾
Area 9 – OGS9 ⁽³⁾	7.21	0.72	212	CDS Model 4045-8
Area 10 – OGS10 ⁽³⁾	6.61	0.70	212	CDS Model 4045-8
(1) Providing at minimum 80% TSS removal for a Fine Distribution (2) See Drawing 3 for OGS unit locations (3) NOTE: the OGS numbering of OGS9 and OGS10 have been used to maintain consistency with prior functional servicing reports circulated in relation to this development area.				

The above OGS units will achieve required quality controls within the treatment train and, along with other elements, will have additional beneficial TSS mitigation.

5.3.3 Groundwater

Paterson Group has reviewed the anticipated long term groundwater condition for the development area. Paterson drawing PG5036-10A in **Appendix D** demonstrates the long term groundwater elevation across the Conservancy East Phase 5 development area. The lowest elevation of 88.70 is below the trunk sewer profiles shown in **Drawing No. 5** in the **Drawings** section and would be below any infiltration-type LID proposed within the development area.

5.4 Proposed Minor System

The subject property will be serviced by an internal gravity storm sewer system that will generally follow the local road network and proposed servicing easements as required. The drainage will be conveyed within the underground piped sewer system to headwall outlets located along the natural heritage corridor.

Street catchbasins will collect drainage from the streets and front yards, while rear yard catchbasins will capture drainage from backyards. Perforated catch basin leads will be provided in rear yards, except the last segment where they connect to the right-of-way which will be solid pipe, per City standards.

The rational method design of the minor system captures drainage for storm events up to and including the 2-year (local) and 5-year (collector) event within the subject property. The following table summarizes the standards employed in the detailed

design of the storm sewer network. The preliminary drainage area information can be found in **Drawing 3** and rational method design sheets are provided in **Appendix D**.

Table 6: Storm Sewer Design Criteria

Design Parameter	Value
Minor System Design Return Period	1:2 year (PIEDTB-2016-01) for local roads, without ponding 1:5 year (PIEDTB-2016-01) for collector roads, without ponding 1:100 year (PIEDTB-2016-01) for arterial road, without ponding
Major System Design Return Period	1:100 year
Intensity Duration Frequency Curve (IDF) 2-year storm event: A=732.951 B=6.199 C=0.810 5-year storm event: A = 998.071 B = 6.053 C = 0.814	$i = \frac{A}{(t_c + B)^C}$
Minimum Time of Concentration	10 minutes
Rational Method	$Q = CiA$
Storm sewers are to be sized employing the Manning's Equation	$Q = \frac{1}{n} AR^{2/3} S^{1/2}$
Runoff coefficient for paved and roof areas	0.9
Runoff coefficient for landscaped areas	0.2
Minimum Sewer Size	250 mm diameter
Minimum Manning's 'n' for pipe flow	0.013
Minimum Depth of Cover	1.5 m from crown of sewer to grade
Minimum Full Flowing Velocity	0.8 m/s
Maximum Full Flowing Velocity	6.0 m/s
Clearance from 100-Year Hydraulic Grade Line to Building Opening	0.30 m
Design Parameter	Value
Max. Allowable Flow Depth on Municipal Roads	35 cm above gutter (PIEDTB-2016-01)
Extent of Major System	To be contained within the municipal ROW or adjacent to the ROW provided that the water level must not touch any part of the building envelope and must remain below the lowest building opening during the stress test event (100-year + 20%) and 15cm vertical clearance is maintained between spill elevation on the street and the ground elevation at the nearest building envelope (PIEDTB-2016-01)
Stormwater Management Model	DDSWMM (release 2.1), SWMHYMO (v. 5.02) and XPSWMM (v. 10)

Model Parameters	Fo = 76.2 mm/hr, Fc = 13.2 mm/hr, DCAY = 4.14/hr, D.Stor.Imp. = 1.57 mm, D.Stor.Per. = 4.67 mm
Imperviousness	Based on runoff coefficient (C) where Percent Imperviousness = $(C - 0.2) / 0.7 \times 100\%$.
Design Storms	Chicago 3-hour Design Storms and 24-hour SCS Type II Design Storms. Maximum intensity averaged over 10 minutes.
Historical Events	July 1st, 1979, August 4th, 1988 and August 8th, 1996
Climate Change Street Test	20% increase in the 100-year, 3-hour Chicago storm
<i>Extracted from City of Ottawa Sewer Design Guidelines, October 2012, and ISSU, and based on recent residential subdivisions in City of Ottawa.</i>	

The peak design flows are calculated based on an average predicted runoff coefficient (C-value) of 0.67 and 0.80 for the development areas, 0.40 for park areas and 0.25 for grassed areas. As detailed design progresses, the runoff coefficients will be refined to reflect the proposed building envelopes, driveways and other details.

There are several trunk sewers proposed and the peak flows are described for the trunk sewers which correspond to the stormwater management design areas as summarized in the following table:

Table 7: Minor System Trunk Sewer Outlets

Area/Outlet # (from east to west)	Trunk Sewer Outlet Headwall	Peak Flow (L/s)
9 (HW9) ⁽¹⁾	1050 mm diameter @ 0.20%	894
10(HW10) ⁽¹⁾	1050 mm diameter @ 0.11%	739
(1) NOTE: the OGS numbering of OGS9 and OGS10 have been used to maintain consistency with prior functional servicing reports circulated in relation to this development area.		

The storm sewers tributary to the various outlets, and associated peak flows, are detailed in the rational method design sheet, enclosed in **Appendix D**.

The conceptual servicing layout is shown on **Drawing 2** in **Drawings**. As detailed design progresses, alignment and sizing of local storm sewers will be confirmed and additional servicing easements may be required, potentially triggering minor amendments to the proposed lot fabric in the concept plan. The preliminary sanitary and storm trunk plan and profiles are shown on **Drawing 5** in **Drawings**.

5.4.1 Hydraulic Grade Line Analysis

A preliminary hydraulic grade line (HGL) modelling analysis has been completed by JFSA to demonstrate that the HGL will be maintained below the ground surface. See the JFSA memo entitled *BCDC Phase 5 – Preliminary HGL Analysis (December 1, 2022)* in **Appendix D** for details/results. The analysis has been evaluated for various scenarios for the Jock River (as per prior City requirements) in order to assess the appropriate HGL boundary condition:

- 100-year rainfall event on the development and a 5-year spring water level on the Jock River; or
- 5-year rainfall event on the development and a 100-year spring water level on the Jock River (deemed to be the critical event).

The HGL results in JFSA's Table 1 demonstrate that the worst case scenario freeboard to the ground surface ranges from 0.64m to 0.90m for the critical event noted above. The HGL is shown on the profile **Drawing 5** for reference.

An updated HGL analysis will be completed for the proposed system at the detailed design stage, based on the above noted events, including historical design storms and climate change stress test as required. Detailed grading design and storm sewer design will be modified as required to achieve the freeboard requirements (per PIEDTB-2016-01).

5.5 Proposed Major System

Major system conveyance, or overland flow, will be provided to accommodate flows in excess of the minor system capacity. Overland flow is accommodated by generally storing stormwater up to the 100-year design event in road sags then routing additional surface flow along the road network and service easements towards the proposed stormwater outlets, discharging to the Jock River through the natural heritage corridors, as shown on **Drawing 1**. The grading design includes a saw-toothed-road design with 0.10% minimum grade from high point to high point in order to maximize available surface storage for management of flows up to the 100-year design event where possible.

5.6 Foundation Drainage (Sump Pumps)

Due to the grade raise restrictions and the proposed storm and sanitary drainage schemes, the road centerlines do not allow for standard basements with a gravity connection to the storm sewer system. As such, because of the constraints on the subject property, sump pumps are proposed to be installed for all residential blocks and residential lots.

The City of Ottawa issued Technical Bulletin *ISTB-2018-04* and *2019-02* for the amendment of the *Ottawa Design Guidelines – Sewer, Second Edition*, October 2012 with respect to the screening criteria for the use of sump pump systems for foundation drainage in Greenfield developments on sites with clay soils. Similar to the development of Conservancy Phase 1, and Conservancy East (Phase 2-4) this site has also been assessed as meeting the required criteria for the use of sump pumps.

One of the screening criterion is with respect to the hydraulic grade line (HGL) for the development wherein the system should be reviewed to demonstrate that the HGL cannot reasonably be lowered any further due to outlet restrictions. The site grading is constrained by the close proximity of the Jock River, which is the receiver of stormwater outflows, and is also constrained by grade raise restrictions for the property.

For the Barrhaven Conservancy East Phase 5 Lands the grade raise restriction varies between 1.4 m and 1.8 m. Paterson's permissible grade raise plan is contained in **Appendix E** for reference (See Section 6 for discussion). Further investigations on the property and potential surcharging or lightweight fill (LWF) underneath garages could increase the permissible grade raise and will be investigated further as part of the detailed design.

The functional grading plan for the subdivision has been prepared with the grade raise restrictions in mind with grades being kept as low as possible.

The proposed centerline of road grades, and subsequently the house grades, do not allow for standard basements with a gravity connection to the storm sewer system. As such, the subdivision will be serviced entirely by sump pumps due to site constraints imposed by grade raise restrictions, HGL elevations and the proximity to the Jock River stormwater outlet.

5.7 Low Impact Development (LID) - Infiltration

The following general Low Impact Development (LID) techniques could be considered for implementation, where possible, as part of detailed design (noting that they have to be weighed against the objectives of the City's sump pump technical bulletins):

- Rear-yard swales should be designed with minimum grades where possible, to promote infiltration;
- Rear-yard catchbasin leads should be perforated (except for the last segment connecting to the storm sewer within the ROW), to promote infiltration; and,
- Where eavestroughs are provided on residential units, they are to be directed to landscaped surfaces, to promote infiltration.
- Furthermore, the following techniques can be examined as part of detailed landscaping design of the park block; and,
- Micro-grading can be considered to promote infiltration.

Generally, the LID techniques proposed above are most suitable due to the existing clay soils and high groundwater levels. The long term groundwater anticipated is demonstrated on Paterson Drawing *PG5036-10A* in **Appendix D** as previously noted. The proposed LID infiltration measure noted in Section 5.2 will contribute some infiltration benefits as first flush stormwater is conveyed into the filtration trench. The amount of infiltration is dependent upon the surrounding soils, but the proposed design will optimize the potential on the site.

5.8 Existing Watercourses

5.8.1 Foster Ditch

The Foster Ditch borders the western boundary of the Conservancy East Phase 5 development area. It originates south of Fallowfield Road, west of Cedarview Road and flows south until it converges with the Jock River South of McKenna Casey Drive. The ditch is approximately 3200 m long and has been artificially straightened. This non-municipal drain is a fish bearing tributary of the Jock River with approximately 335 ha of catchment area. The surrounding land use is urban and vacant lands. Riparian vegetation is very sparse consisting of mostly grasses with a few shrubs.

As noted in the **Jock River SWS**, to ensure protection of the aquatic habitat north of the Jock River, a development setback should be provided for all of the tributaries. Further studies will determine the development setback, which will be the greater of: 1) regulatory floodplain; 2) meander belt width; and 3) aquatic setback, whichever is greater.

5.9 Floodplain

On November 8th, 2019 the RCVA gave permission to Barrhaven Conservancy Development Corporation to cut and fill on the subject property under permit RV5 44/19 pursuant to review under Section 28 of the Conservation Authorities Act, regulation 174/06. The application and approval by the RVCA was supported by a 2D HEC-RAS model prepared by JFSA. The JFSA model identified the existing and proposed 100-year water levels and permissible extent of fill placement.

The works pursuant to the above-mentioned permit were completed and accepted by the RVCA on May 31st 2020. Options to complete the fill area boundary as set by JFSA included building a structural face of fill (retaining wall) to the limits of the 100-year floodplain boundary, or, building a berm with the toe of slope at the 100-year floodplain boundary. A vertical structural face of fill was not seen as a desirable or practical outcome and a berm was thus constructed. As-builts for the top of berm were subsequently provided and approved by the RVCA, resulting in the May 31st approval noted above and the current 100-year floodplain boundary delineation. The toe of the berm as constructed corresponds to the approved JFSA 100-year floodplain line and the current top of berm corresponds to the as-built top of berm.

5.10 Stormwater Servicing Conclusions

The Phase 5 stormwater runoff is designed to be captured by an internal gravity sewer system that will convey flows to multiple outlet locations equipped with end of line OGS units (two). A proposed treatment train arrangement of 1.0 m deep sump catchbasins, to optimize catchbasin sump retention of solids, as well as select catchbasin locations with connected infiltration-type subdrains will provide the required quality control treatment to achieve the Enhanced Level of protection. Downstream of the storm outlets along the southern development boundary will be channels within the natural heritage corridor where flows will be conveyed to the Foster Ditch prior to discharge to the Jock River. It is anticipated that quantity control is not required for the Jock River. Notwithstanding, some quantity control by means of erosion storage will be included.

A preliminary Hydraulic Grade Line (HGL) modelling analysis has been completed for the Conservancy East (Phase 5) development area at this time and demonstrates that the HGL is maintained below the ground surface with freeboards ranging from 0.64 m to 0.90 m. Further detailed HGL review will be completed for the proposed system at the detailed design stage. Due to the grade raise restrictions, and the proposed storm and sanitary drainage layouts, the road centerlines do not allow for standard basements with a gravity connection to the storm sewer system. As such, because of the constraints on the subject property, sump pumps are proposed to be installed for all residential blocks and residential lots.

The Conservancy East phase 5 development area will be outside of the Jock River's regulatory floodplain area.

Appropriate setbacks from existing watercourse are incorporated into the draft plan based on advancement/finalizing of studies to assess the various determining criteria.

6.0 GRADING

A site grading arrangement has been developed to optimize earthworks and provide major system conveyance to the receiving outlets, and naturalized wetland facilities, which ultimately outlet to the existing Jock River drainage network. The proposed grading can be found in **Drawing 1** in **Drawings**.

The development area is outside of the Jock River regulatory flood plain limits. The site grading will be a minimum of 0.50m above the 100-year regulatory limit event of the Jock River.

6.1 Geotechnical Conditions

Paterson completed a geotechnical investigation for the Conservancy East lands as follows:

- *Geotechnical Investigation – Proposed Residential Development, Conservancy Lands East* (Paterson Group, September 24, 2019);

The existing ground surface across the site is relatively level with approximate ground surface elevation varying between 91.5 m and 91.0 m. The subsurface profile generally consists of an approximate 50 mm to 460 mm thick layer of topsoil underlain by a silty clay deposit.

Due to the presence of a silty clay deposit, permissible grade raise restrictions are recommended for this site. The recommended permissible grade raise varies between 1.4 m in the north area of the phase and 1.8 m in the south. Figure PG5036-2 '*Permissible Grade Raise Plan*' by Paterson is enclosed in **Appendix E** for reference. At the time of detailed design, efforts will be made to mitigate any exceedances and detailed review and signoff by a licensed Geotechnical Engineer will be required. Where grade raises exceed the permissible levels the Engineer will recommend appropriate measures to mitigate where required (i.e. light weight fill or pre-consolidation etc).

The following additional grading criteria and guidelines will be applied to detailed design, per **City of Ottawa Guidelines**:

- Driveway slopes will have a maximum slope of 6%;
- Grading in grassed/landscaped areas to range from 2% to 3:1, with terracing required for slopes larger than 7%;
- Swales are to be 0.15m deep with 3:1 side slopes unless otherwise indicated on the drawings; and,
- Perforated pipe will be required for drainage swales if they are less than 1.5% in slope.

The geotechnical analysis of the site, published under separate cover in support of the development applications, provides additional information about the suitability of the site for the proposed services and grading scheme. At the time of detailed design, detailed review and signoff by a licensed Geotechnical Engineer will be required.

7.0 EROSION AND SEDIMENT CONTROL

Soil erosion occurs naturally and is a function of soil type, climate and topography. The extent of erosion losses is exaggerated during construction where vegetation has been removed and the top layer of soil becomes agitated.

Prior to topsoil stripping, earthworks or underground construction, erosion and sediment controls are implemented and will be maintained throughout any construction phase.

The following specific recommendations to the Contractor will be included in contract documents.

- Limit extent of exposed soils at any given time.
- Re-vegetate exposed areas as soon as possible.
- Minimize the area to be cleared and grubbed.
- Protect exposed slopes with plastic or synthetic mulches.
- Install silt fence to prevent sediment from leaving the site and entering existing watercourses, and clean and maintain throughout construction.
- Install catchbasin inserts during construction to protect from silt entering the storm sewer system.
- Install mud mats in order to prevent mud tracking onto adjacent roadways.
- No refueling or cleaning of equipment near existing watercourses.
- No material stockpiles within 30m of existing watercourses, unless otherwise permitted by RVCA and City of Ottawa.
- Provide sediment traps and basins during dewatering.
- Plan construction at proper time to avoid flooding.
- The Contractor will, at every rainfall, complete inspections to ensure proper performance.
- Erosion and sediment controls will remain in place until the working areas have been stabilized and re-vegetated.

8.0 UTILITIES

Utility services extending to the site may require connections to multiple existing infrastructure points: consultation with Enbridge gas, Hydro Ottawa, Rogers, and Bell is required as part of the development process to confirm the servicing plan for the subject lands.

9.0 CONCLUSION AND RECOMMENDATIONS

This AES provides details on the planned on-site municipal services for the subject property and demonstrates that adequate municipal infrastructure capacity is expected to be available for the planned development area.

- The subject lands are have been reviewed by Stantec to confirm that servicing is feasible from the SUC pressure zone. Future watermain extensions from Nepean Town Centre development areas, being constructed as part of Phase 2-4 approvals, will facilitate servicing to the Conservancy East Phase 5 lands via watermain extensions along the future Chapman Mills Drive extension and through the Claridge “Burnett Lands” development area. Detailed modelling will confirm phasing of the extensions of trunk watermains and verify sizing of the local watermain network.
- The subject property will be serviced by local sanitary sewers, an on-site trunk sanitary sewer, and the off-site SNC sanitary sewer as defined in previous reports. This AES continues to confirm that the expansion of the drainage areas from the **2019 Novatech SNC Memo** to include the entirety of the subject property has no negative impacts. There is residual capacity in the downstream SNC providing sufficient capacity for the peak sanitary flows for the subject property, including external commercial and community park flows.
- Stormwater service is to be provided by capturing stormwater runoff by an internal gravity sewer system that will convey flows to various outlets along the southern boundary to the Foster Ditch which will convey flows to the Jock River. Prior to discharge from the development, any first flush stormwater will have passed through a treatment train of measures in order to provide the appropriate level of Enhanced quality control. The treatment train consists of deep sump catchbasins, LID infiltration trench at catchbasin locations and an end-of-line OGS unit. It is anticipated that quantity control will not be required for discharges to the Jock River.
- A preliminary Hydraulic Grade Line (HGL) modelling analysis has been completed at this time and demonstrates that the HGL is maintained below the ground surface. Another detailed HGL review will be completed for the proposed system at the detailed design level.

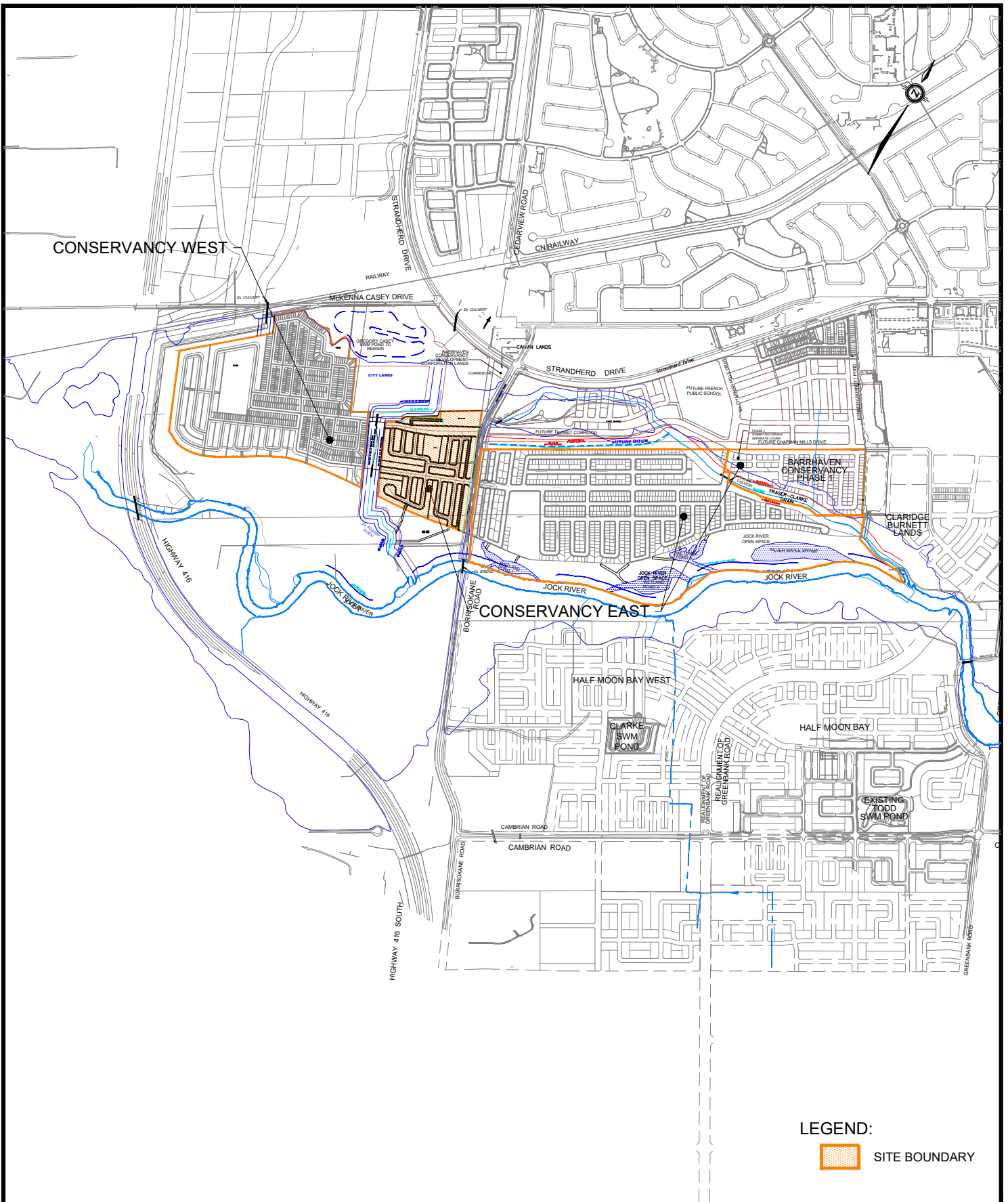
- Sump pumps are proposed to be installed for all units within residential blocks and lots;
- The proposed servicing and grading plans are anticipated to meet all City, RVCA, and MECP requirements as set out in background studies and current standards.
- Prior to detailed design of the infrastructure presented in this report, this AES will require approval under the Planning Act as supporting information for the Plan of Subdivision application. Project-specific approvals are also expected to be required for the infrastructure presented in this report from the City of Ottawa, MECP, and Rideau Valley Conservation Authority, among other agencies.

Prepared by,
David Schaeffer Engineering Ltd.



Per: Kevin L. Murphy, P.Eng.

FIGURES & DRAWINGS



LEGEND:
 SITE BOUNDARY



120 Iber Road, Unit 103
 Stittsville, ON K2S 1E9
 TEL: (613) 836-0856
 FAX: (613) 836-7183
 www.DSEL.ca

**BARRHAVEN CONSERVANCY
 EAST PHASE 5
 KEY PLAN
 CITY OF OTTAWA**

DATE: DECEMBER 2022
 SCALE: 1:20000
 PROJECT No.: 20-1180 A-5
 FIGURE: 1

SCHEDULE OF LAND USE

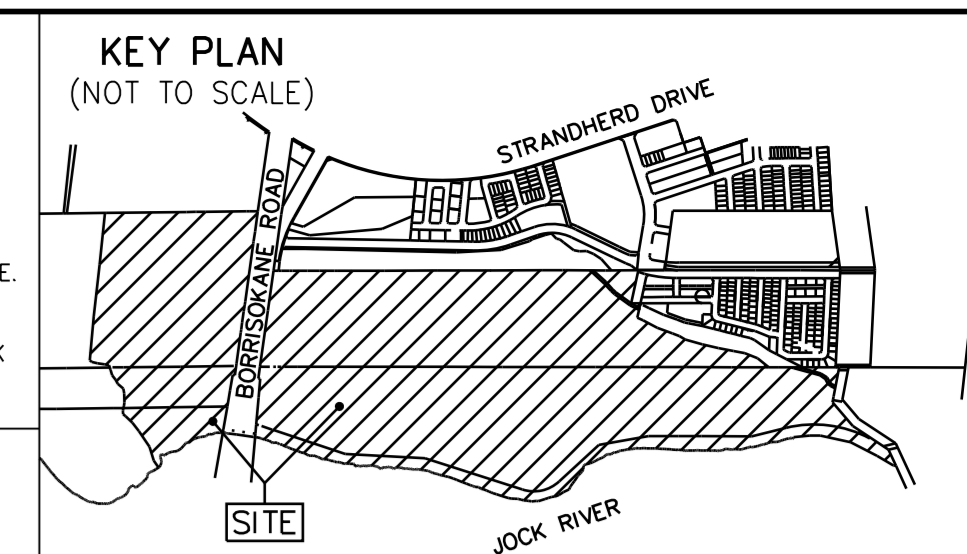
LAND USE	BLOCK(S)	AREA (Sq. m)
SINGLES (DETACHED)	1 TO 8, (BOTH INCLUSIVE), 20, 21, 24, 25, 41, 43, 45, 47, 48 AND 50 TO 59, (BOTH INCLUSIVE)	219,615.4
TOWNHOME	9 TO 19, (BOTH INCLUSIVE), 22, 23, 26, 27, 31, 32, 38, 42, 44, 46, 49 AND 135	66,745.4
REAR LANE TOWNHOME	100 TO 103, (BOTH INCLUSIVE) AND 106 TO 108, (BOTH INCLUSIVE)	27,698.3
WALKWAYS	104, 105, 110 TO 116, (BOTH INCLUSIVE) AND 134	1,815.1
OPEN SPACE	118, 119 AND 121	4,021.3
BUS LANE	132	8,651.3
FUTURE DEVELOPMENT	120	9,218.2
PARK	117, 122, 123 AND 133	42,832.3
NATURAL HERITAGE SYSTEM	124, 125 AND 126	52,086.5
JOCK RIVER OPEN SPACE	127, 128 AND 129	237,125.6
STREET WIDENING	130 AND 131	5,989.5
STREETS	STREET '7' TO STREET '37', (BOTH INCLUSIVE)	176,962.4
LANES	LANE '1' TO LANE '3', (BOTH INCLUSIVE)	4,890.3
TOTAL:		857,651.6

SUBJECT TO THE CONDITIONS, IF ANY, SET FORTH IN OUR LETTER DATED... THIS DRAFT PLAN IS APPROVED BY THE CITY OF OTTAWA UNDER SECTION 51(17) OF THE PLANNING ACT THIS... DAY OF... 2021.

SEAN MOORE, MCI, RPP, MANAGER
DEVELOPMENT REVIEW SECTION
PLANNING, INFRASTRUCTURE AND ECONOMIC DEVELOPMENT DEPARTMENT,
CITY OF OTTAWA

ADDITIONAL INFORMATION AS REQUIRED UNDER SECTION 51(17) OF THE PLANNING ACT R.S.O. 2001

(X) - AS SHOWN ON THIS DRAFT AND KEY PLAN
(O) - AS SHOWN ON THIS DRAFT AND KEY PLAN
(S) - LAND TO BE USED IN ACCORDANCE WITH THE SCHEDULE OF LAND USE
(H) - FULL MUNICIPAL SERVICES
(U) - OFFSHORE MARINE DEPOSITS OF CLAY, SILT, CLAY AND SILT, BEDROCK
OTTAWA FORMATION, LIMESTONE



DRAFT PLAN OF SUBDIVISION
PART OF LOTS 13 AND 14
CONCESSION 3 (RIDEAU FRONT)
PART OF LOTS 13 AND 14
CONCESSION 4 (RIDEAU FRONT)
GEOGRAPHIC TOWNSHIP OF NEPEAN
CITY OF OTTAWA
SCALE 1:1500
J.D. BARNES LIMITED
© COPYRIGHT 2021
DISTANCES AND/OR COORDINATES SHOWN ON THIS PLAN ARE IN METRIC UNITS AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

AREA OF DRAFT PLAN TO BE REVISED (NOW REFERRED TO AS "PHASE 5"). D07-16-20-0021

SEE FIGURE 2B FOR PROPOSED REVISIONS

**FIGURE 2A
2021 APPROVED DRAFT PLAN**

OWNER'S CERTIFICATE
BARRHAVEN CONSERVANCY DEVELOPMENT CORPORATION, BEING THE REGISTERED OWNER OF THE SUBJECT LOTS HEREBY AUTHORIZES J.D. BARNES LIMITED TO PREPARE AND SUBMIT THIS DRAFT PLAN OF SUBDIVISION FOR APPROVAL.

November 12, 2021
DATE
FRANK CARO
PRESIDENT
I HAVE THE AUTHORITY TO BIND THE CORPORATION

SURVEYOR'S CERTIFICATE
I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED ARE CORRECTLY SHOWN.

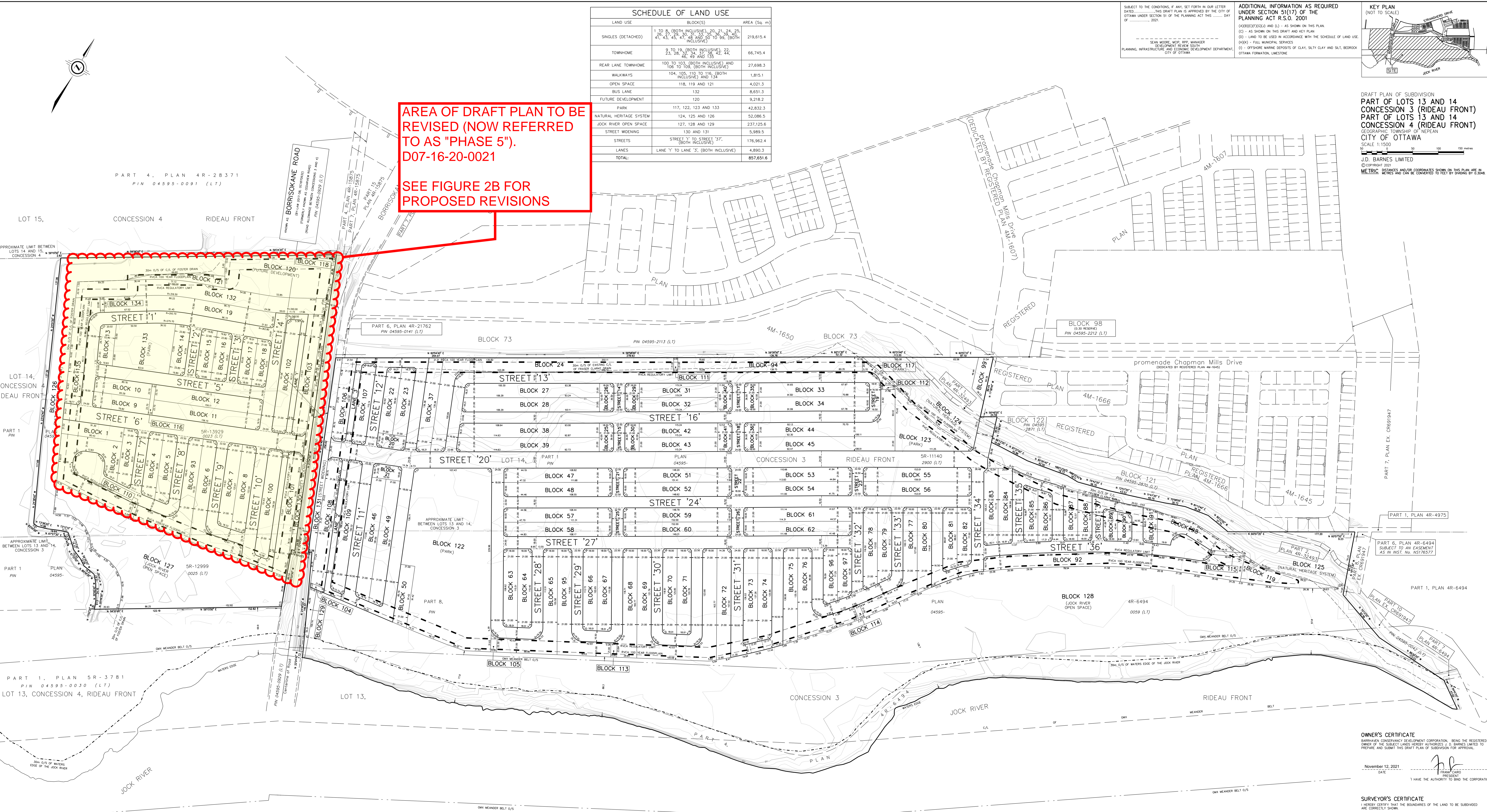
NOVEMBER 12th, 2021
DATE
C.M. FOX
ONTARIO LAND SURVEYOR

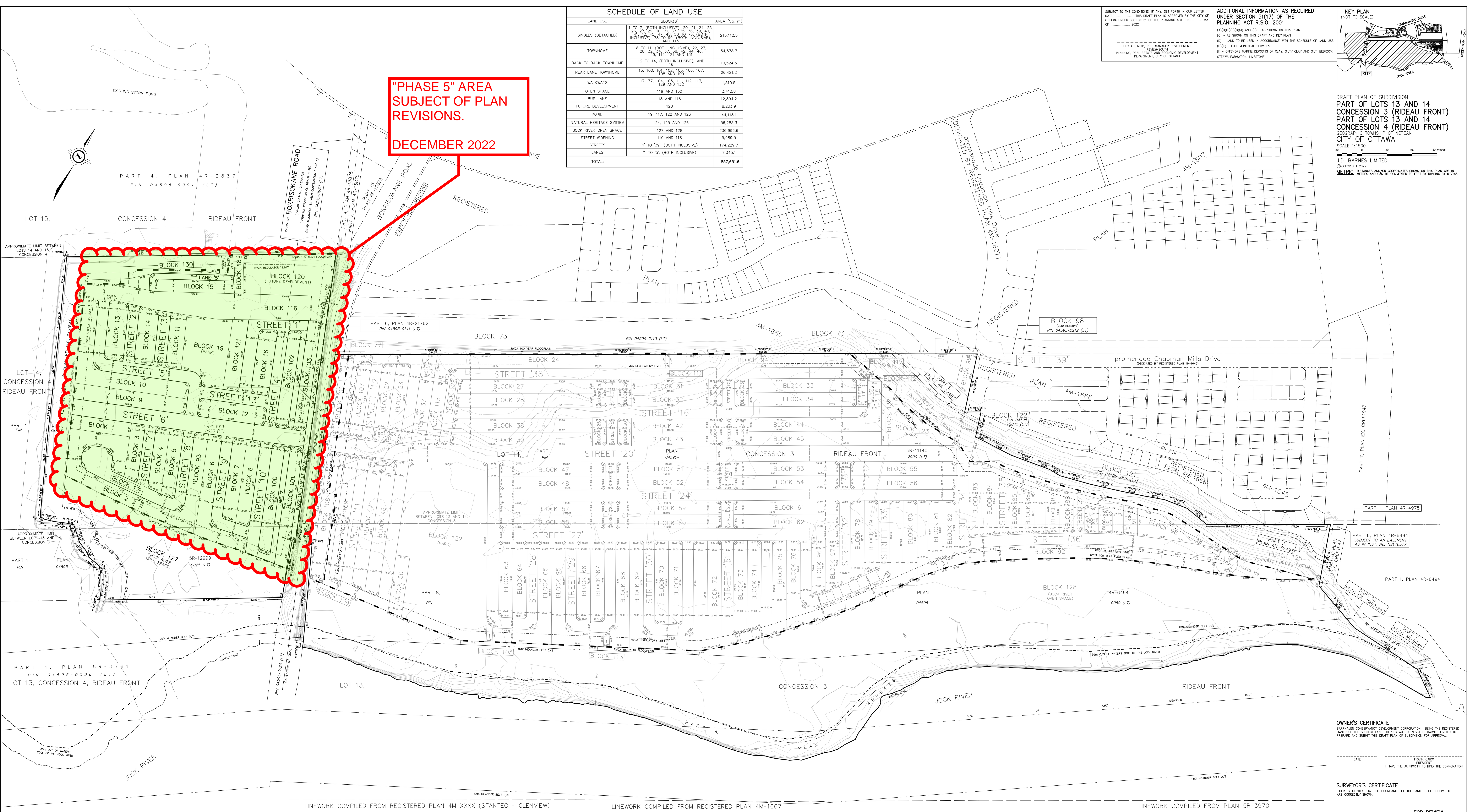


LINWORK COMPILED FROM REGISTERED PLAN 4M-XXXX (STANTEC - GLENVIEW)

LINWORK COMPILED FROM REGISTERED PLAN 4M-1667

LINWORK COMPILED FROM PLAN 5R-3970





SCHEDULE OF LAND USE		
LAND USE	BLOCK(S)	AREA (Sq. m)
SINGLES (DETACHED)	1 TO 7, (BOTH INCLUSIVE), 20, 21, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200	215,112.5
TOWNHOME	8 TO 11, (BOTH INCLUSIVE), 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200	54,578.7
BACK-TO-BACK TOWNHOME	12 TO 14, (BOTH INCLUSIVE), AND 16	10,524.5
REAR LANE TOWNHOME	15, 100, 101, 102, 103, 106, 107, 108 AND 109	26,421.2
WALKWAYS	17, 77, 104, 105, 111, 112, 113, 129 AND 132	1,510.5
OPEN SPACE	119 AND 130	3,413.8
BUS LANE	18 AND 116	12,894.2
FUTURE DEVELOPMENT	120	8,233.9
PARK	19, 117, 122 AND 123	44,118.1
NATURAL HERITAGE SYSTEM	124, 125 AND 126	56,283.3
JOCK RIVER OPEN SPACE	127 AND 128	236,996.6
STREET WIDENING	110 AND 118	5,989.5
STREETS	1 TO 39, (BOTH INCLUSIVE)	174,229.7
LANES	1 TO 5, (BOTH INCLUSIVE)	7,345.1
TOTAL:		857,651.6

ADDITIONAL INFORMATION AS REQUIRED UNDER SECTION 51(17) OF THE PLANNING ACT R.S.O. 2001

(X) - AS SHOWN ON THIS DRAFT AND KEY PLAN.
 (O) - FULL MUNICIPAL SERVICES
 (S) - LAND TO BE USED IN ACCORDANCE WITH THE SCHEDULE OF LAND USE
 (H) - OFFSHORE MARINE DEPOSITS OF CLAY, SILT, CLAY AND SILT, BEDROCK
 (M) - DISTURBED AREAS AND/OR COORDINATES SHOWN ON THIS PLAN ARE IN METRIC AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

KEY PLAN (NOT TO SCALE)

**"PHASE 5" AREA
SUBJECT OF PLAN
REVISIONS.
DECEMBER 2022**

DRAFT PLAN OF SUBDIVISION
PART OF LOTS 13 AND 14
CONCESSION 3 (RIDEAU FRONT)
PART OF LOTS 13 AND 14
CONCESSION 4 (RIDEAU FRONT)
 GEOGRAPHIC TOWNSHIP OF NEPEAN
CITY OF OTTAWA
 SCALE 1:1500
 J.D. BARNES LIMITED
 © COPYRIGHT 2022
 DISTANCES AND/OR COORDINATES SHOWN ON THIS PLAN ARE IN METRIC AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

**FIGURE 2B
2022 PROPOSED REVISED DRAFT PLAN**

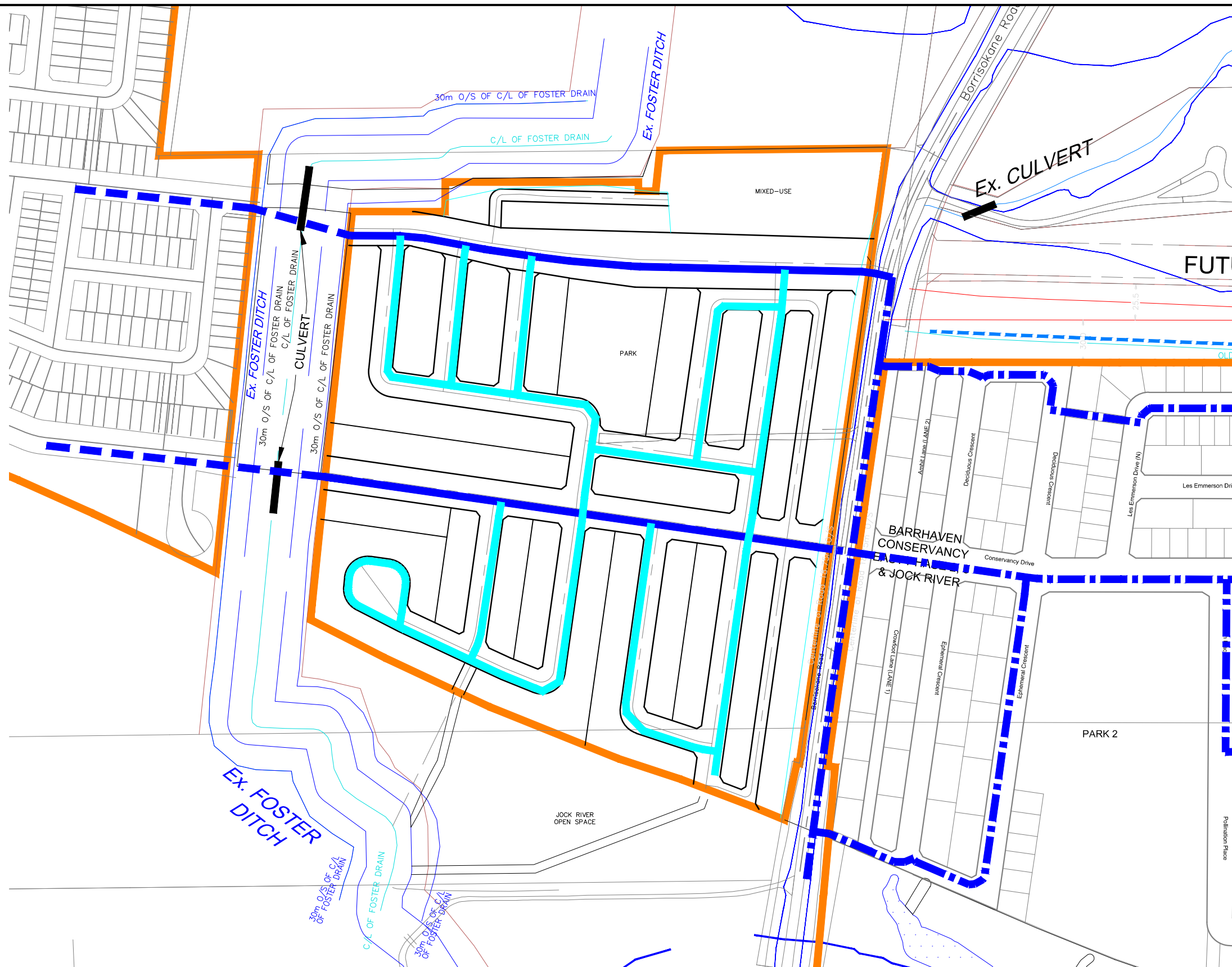
OWNER'S CERTIFICATE
 I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED ARE CORRECTLY SHOWN.

SURVEYOR'S CERTIFICATE
 I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED ARE CORRECTLY SHOWN.

FOR REVIEW
 DATE: _____
 NAME: _____
 TITLE: _____

J.D. BARNES LIMITED
 LAND INFORMATION SPECIALISTS
 655 KENNEDY ROAD, SUITE 101, KANAWA, ONTARIO L3R 9V5
 T: (416) 751-7244 F: (416) 254-8659 www.jdbarnes.com

LINEWORK COMPILED FROM REGISTERED PLAN 4M-XXXX (STANTEC - GLENVIEW) LINEWORK COMPILED FROM REGISTERED PLAN 4M-1667 LINEWORK COMPILED FROM PLAN 5R-3970



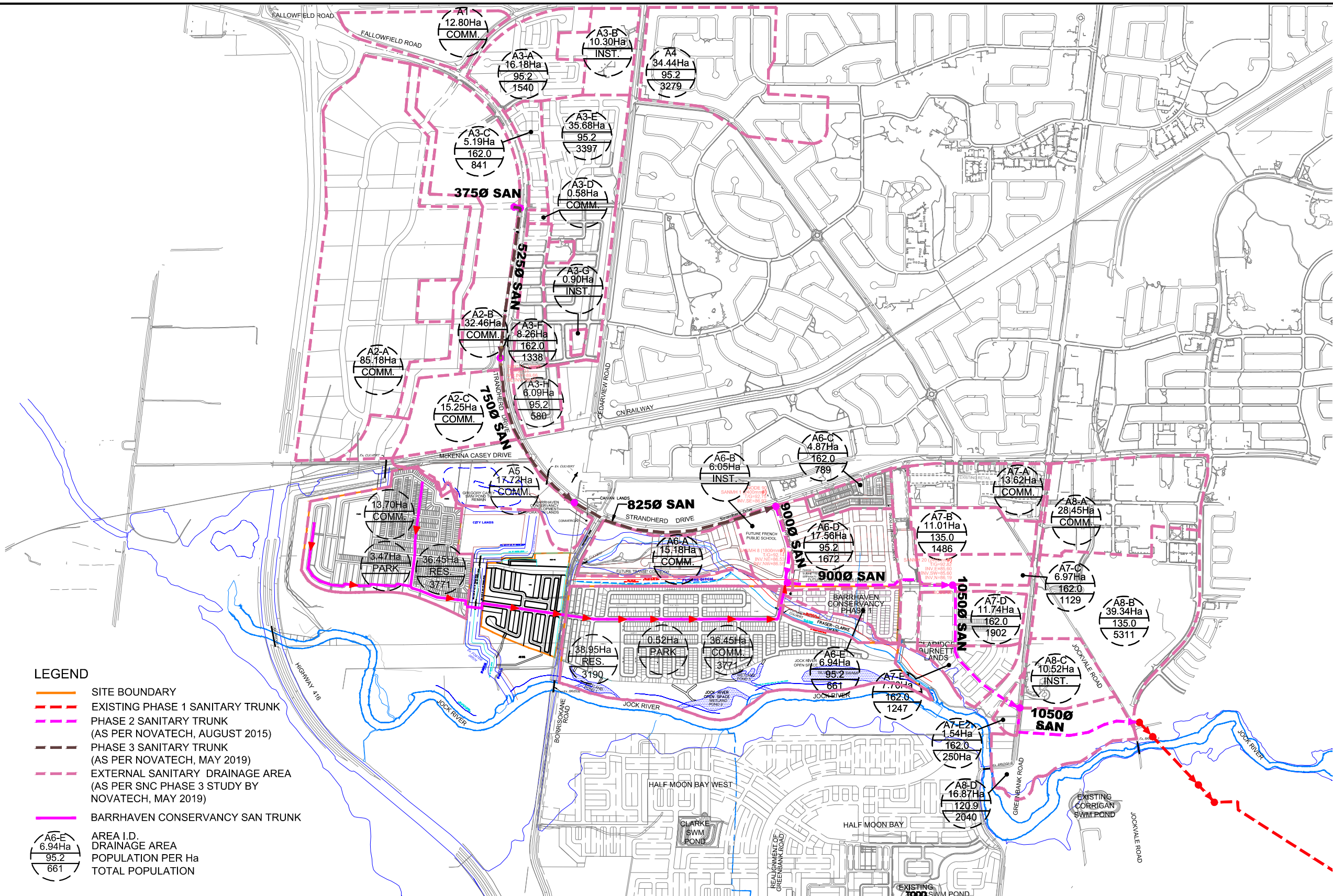
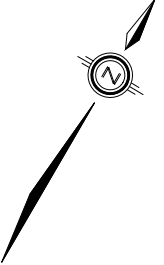
LEGEND	
	SITE BOUNDARY
	PROPOSED 300mm WATERMAIN
	PROPOSED LOCAL WATERMAIN
	EXTERNAL 300mm WATERMAIN
	FUTURE 300mm WATERMAIN



120 Iber Road, Unit 103
 Stittsville, ON K2S 1E9
 TEL: (613) 836-0856
 FAX: (613) 836-7183
 www.DSEL.ca

BARRHAVEN CONSERVANCY EAST PHASE 5
WATERMAIN SERVICING PLAN
 CITY OF OTTAWA

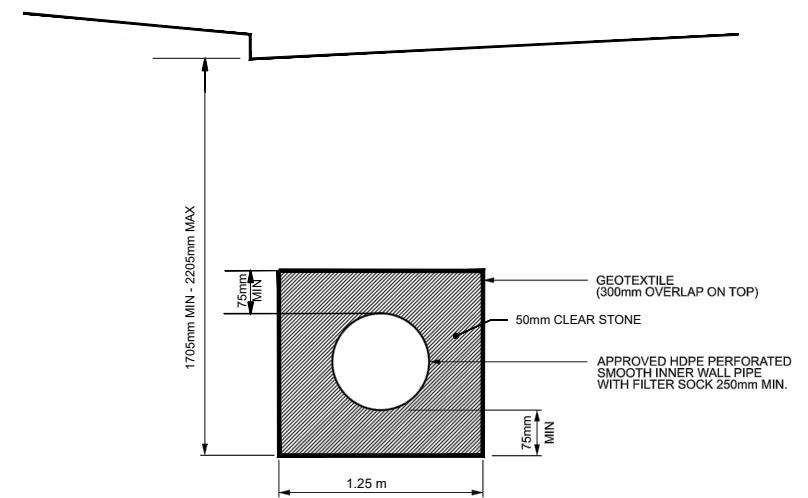
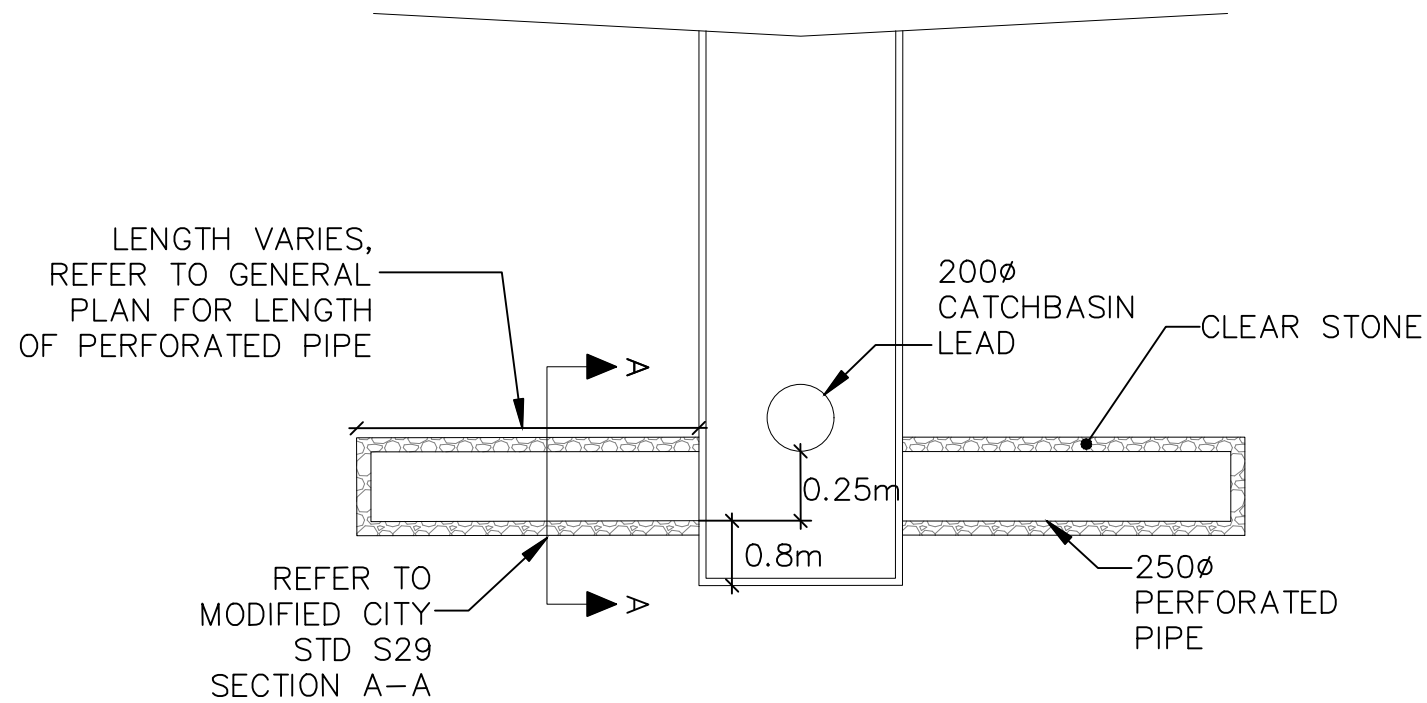
PROJECT No.:	20-1180 A-5
SCALE:	1:3000
DATE:	DECEMBER 2022
FIGURE:	3



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TEL: (613) 836-0856
FAX: (613) 836-7183
www.DSEL.ca

**BARRHAVEN CONSERVANCY EAST PHASE 5
EXTERNAL SANITARY SERVICING
CITY OF OTTAWA**

PROJECT No.:	20-1180 A-5
SCALE:	1:18000
DATE:	DECEMBER 2022
FIGURE:	4



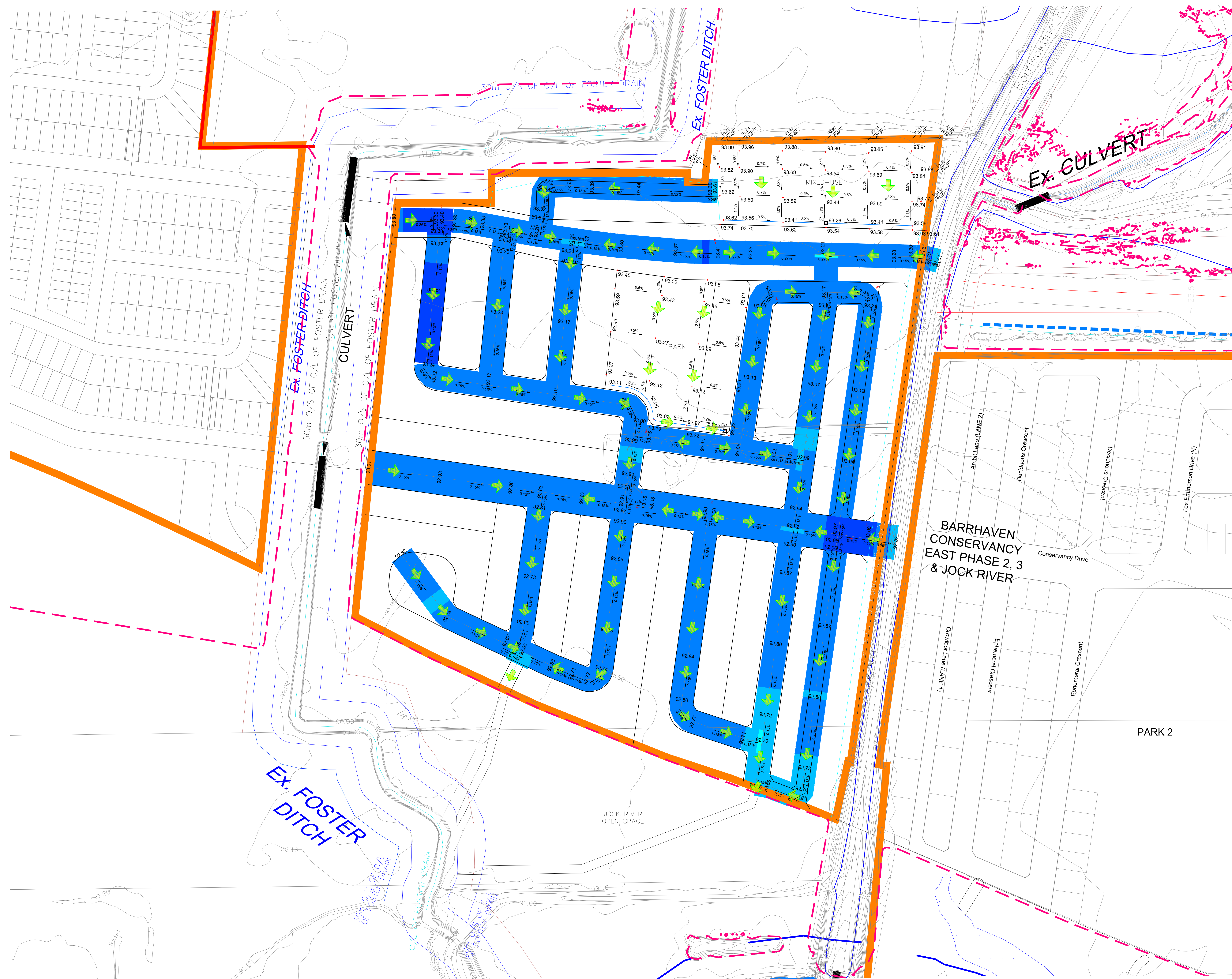
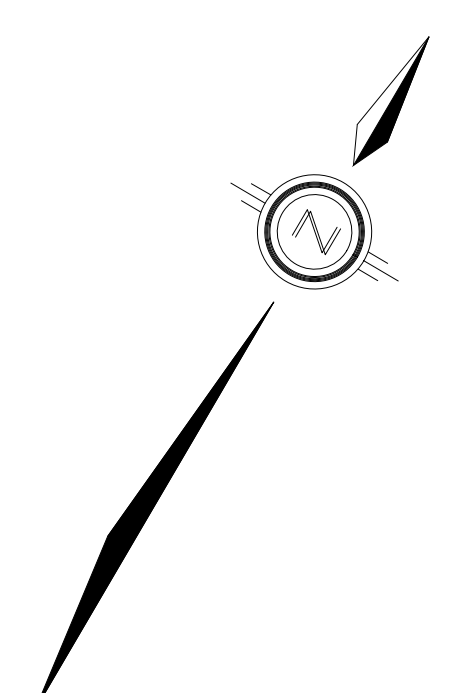
SECTION A-A: MODIFIED CITY STD S29
SCALE: N.T.S.



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Stittsville, ON K2S 1E9
TEL: (613) 836-0856
FAX: (613) 836-7183
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BARRHAVEN CONSERVANCY EAST PHASE 5
FILTRATION SYSTEM DETAILS
CITY OF OTTAWA

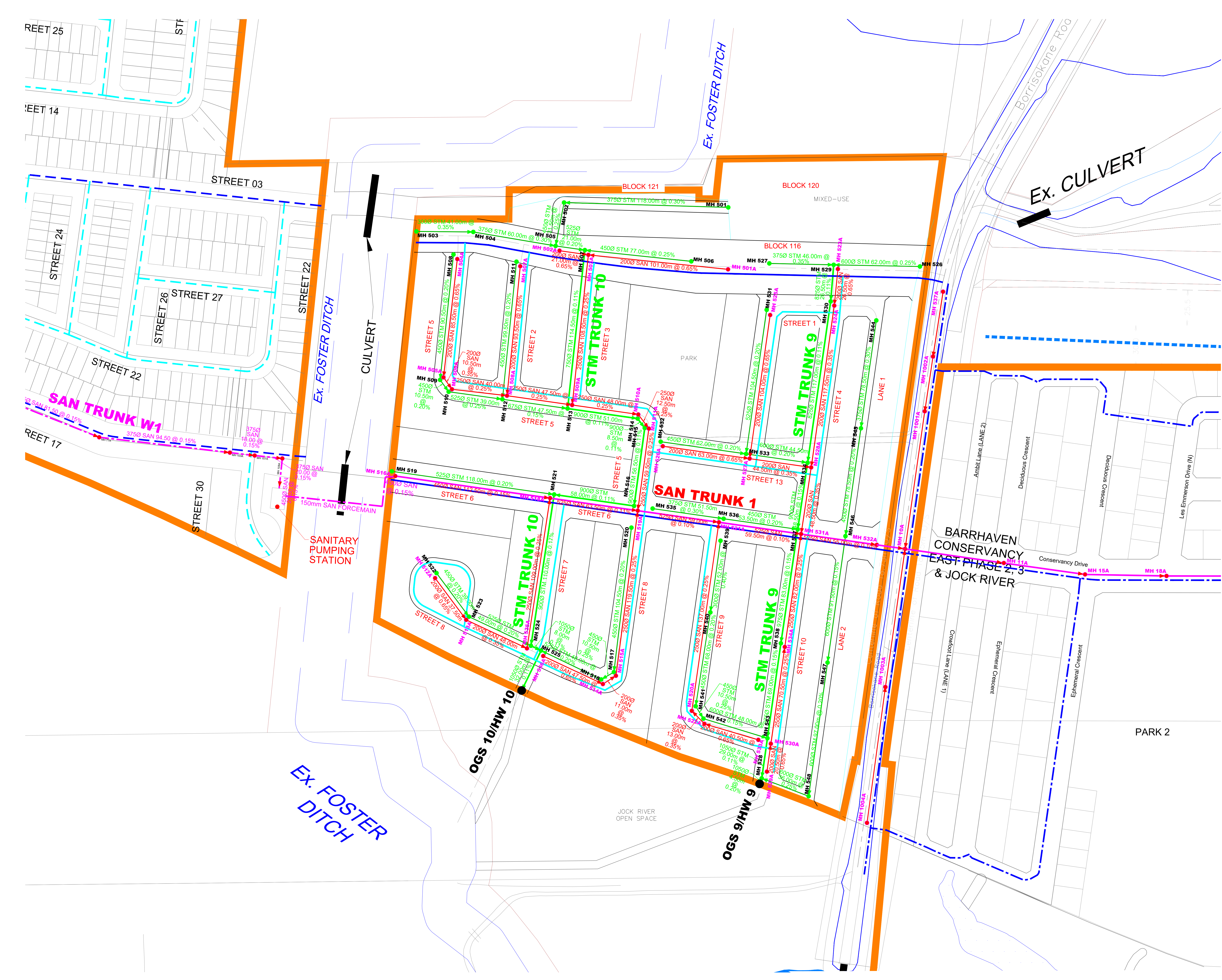
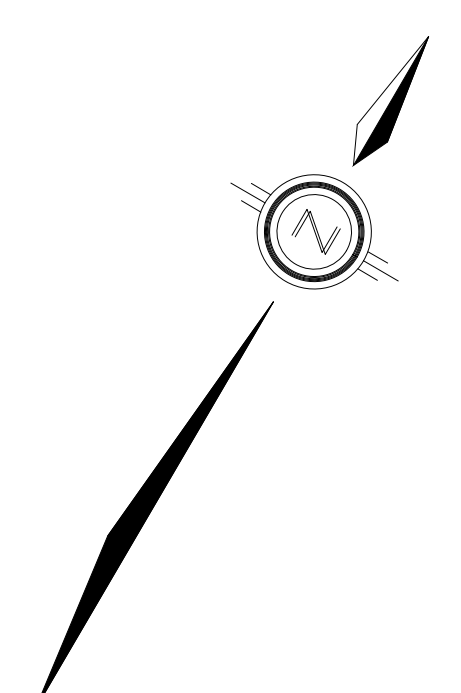
PROJECT No.:	20-1180 A-5
SCALE:	NTS
DATE:	DECEMBER 2022
FIGURE:	5



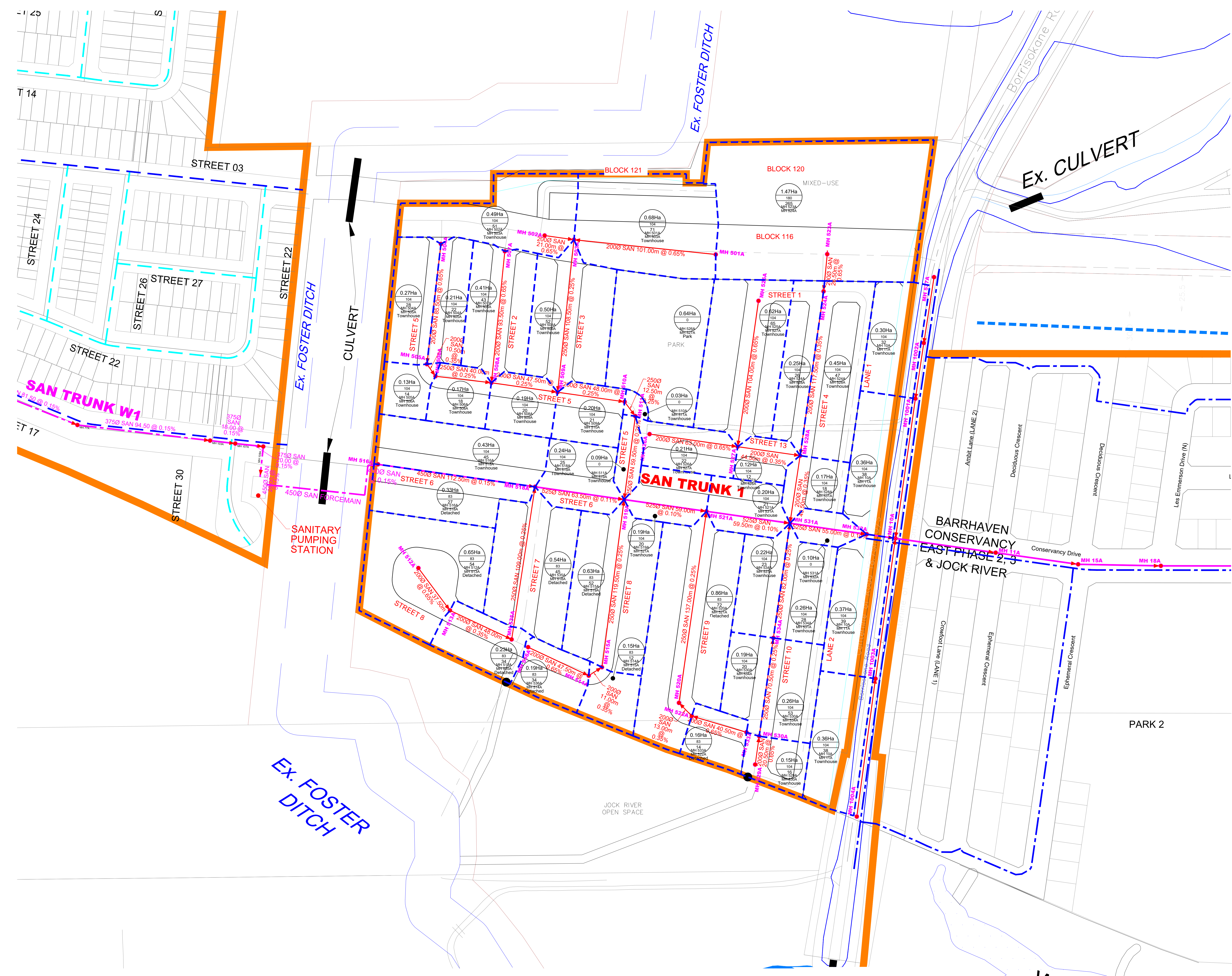
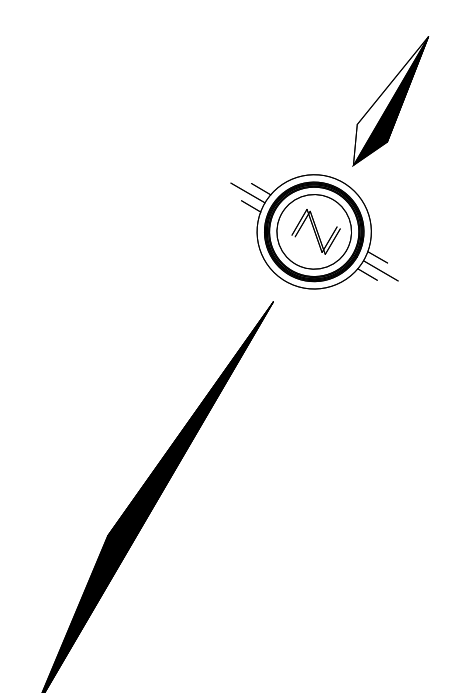
LEGEND

	SITE BOUNDARY		STORM OVERLAND FLOW ARROW
	PROPOSED CENTERLINE ELEVATION		EXISTING CONTOUR ELEVATION
	2019 RVCA APPROVED FLOODLINE BOUNDARY		PROPOSED FLOODLINE PER MARCH 2021 RVCA APPLICATION

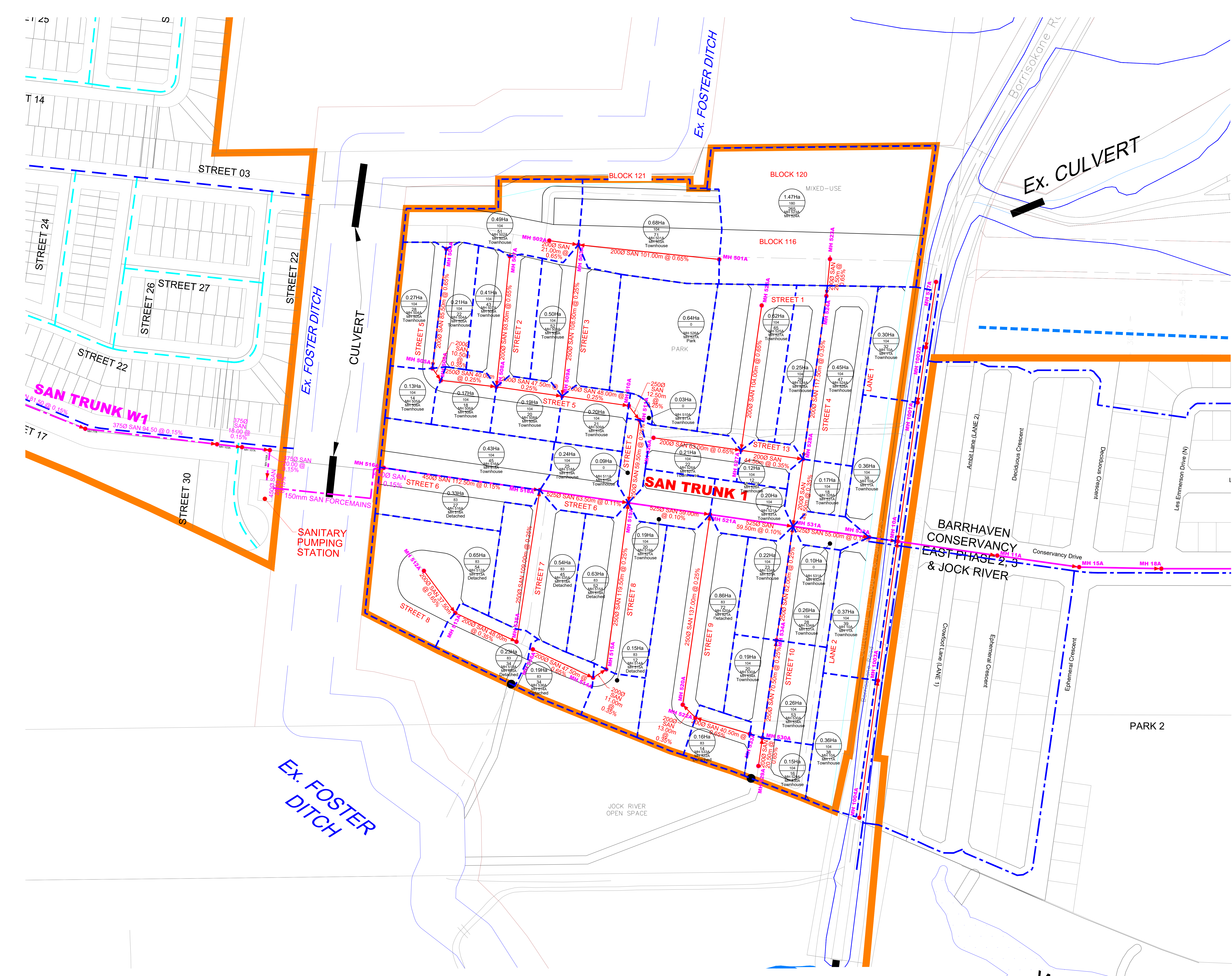
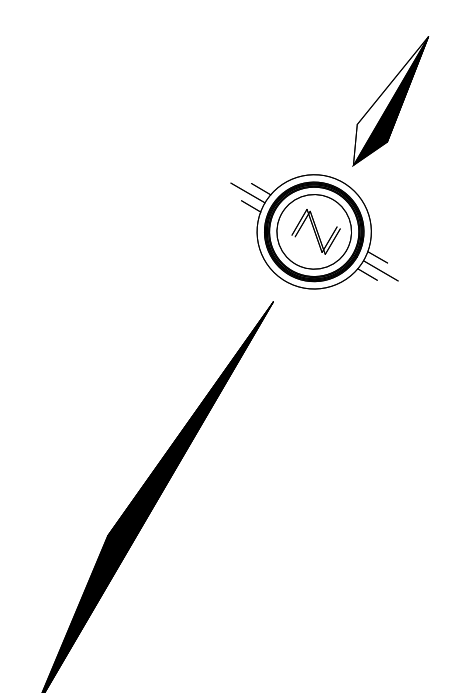
CUT-FILL DEPTH ALONG CENTER LINE:	
CUT DEPTH (m)	FILL DEPTH (m)
0.00-0.50:	0.00-0.50:
0.50-1.00:	0.50-1.00:
1.00-1.50:	1.00-1.50:
1.50-2.00:	1.50-2.00:
>2.00:	>2.00:



LEGEND	
	SITE BOUNDARY
	STORM TRUNK SEWER
	STORM LOCAL SEWER
	SANITARY TRUNK SEWER
	SANITARY LOCAL SEWER
	EXTERNAL SANITARY TRUNK SEWER
	PROPOSED 300mm WATERMAIN
	PROPOSED LOCAL WATERMAIN
	FUTURE 300mm WATERMAIN
	EXTERNAL 300mm WATERMAIN
	STORM MANHOLE
	SANITARY MANHOLE



- LEGEND**
- SITE BOUNDARY
 - SANITARY TRUNK SEWER
 - SANITARY LOCAL SEWER
 - EXTERNAL SANITARY TRUNK SEWER
 - SANITARY MANHOLE
 - SANITARY TRIBUTARY AREA
 - 0.19Ha
104
Townhouse AREA IN HECTARES
POPULATION PER HECTARE
POPULATION
 - 0.19Ha
104
Townhouse AREA TYPE



LEGEND	
	SITE BOUNDARY
	SANITARY TRUNK SEWER
	SANITARY LOCAL SEWER
	EXTERNAL SANITARY TRUNK SEWER
	SANITARY MANHOLE
	SANITARY TRIBUTARY AREA
	AREA IN HECTARES POPULATION PER HECTARE POPULATION AREA TYPE

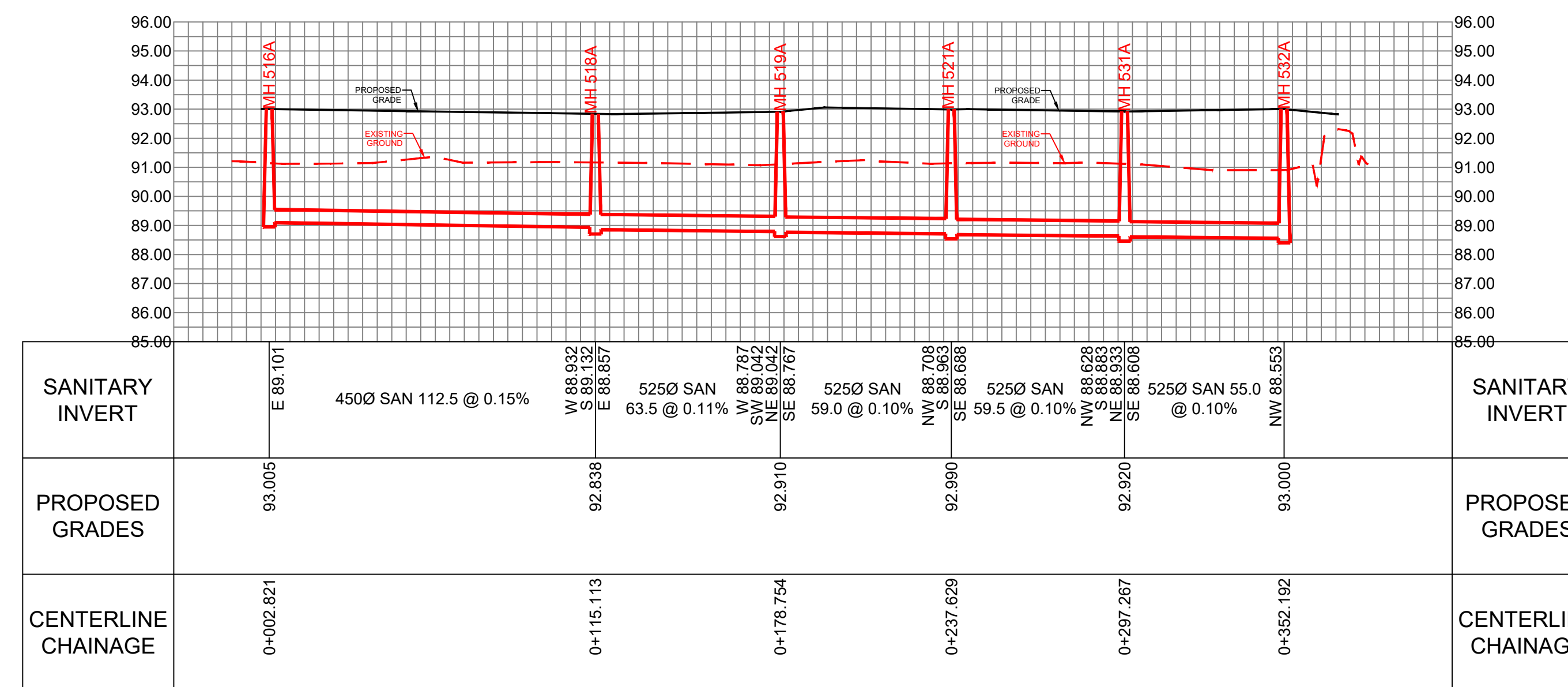


120 Iber Road, Unit 103
Stittsville, ON K2S 1E9
Tel. (613) 836-0856
Fax. (613) 836-7183
www.DSEL.ca

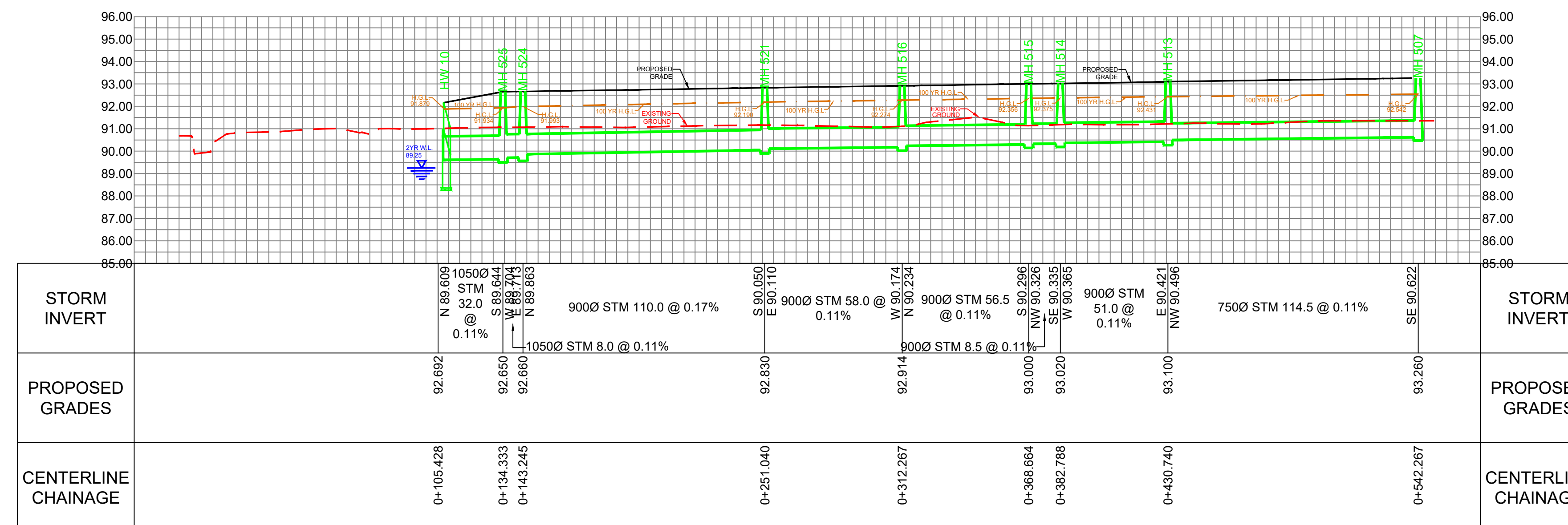
BARRHAVEN CONSERVANCY EAST PHASE 5
SANITARY TRIBUTARY AREA
CITY OF OTTAWA

PROJECT No. : 20-1180 A-5
SCALE: 1:1000
DATE: DECEMBER 2022
DRAWING No. 4

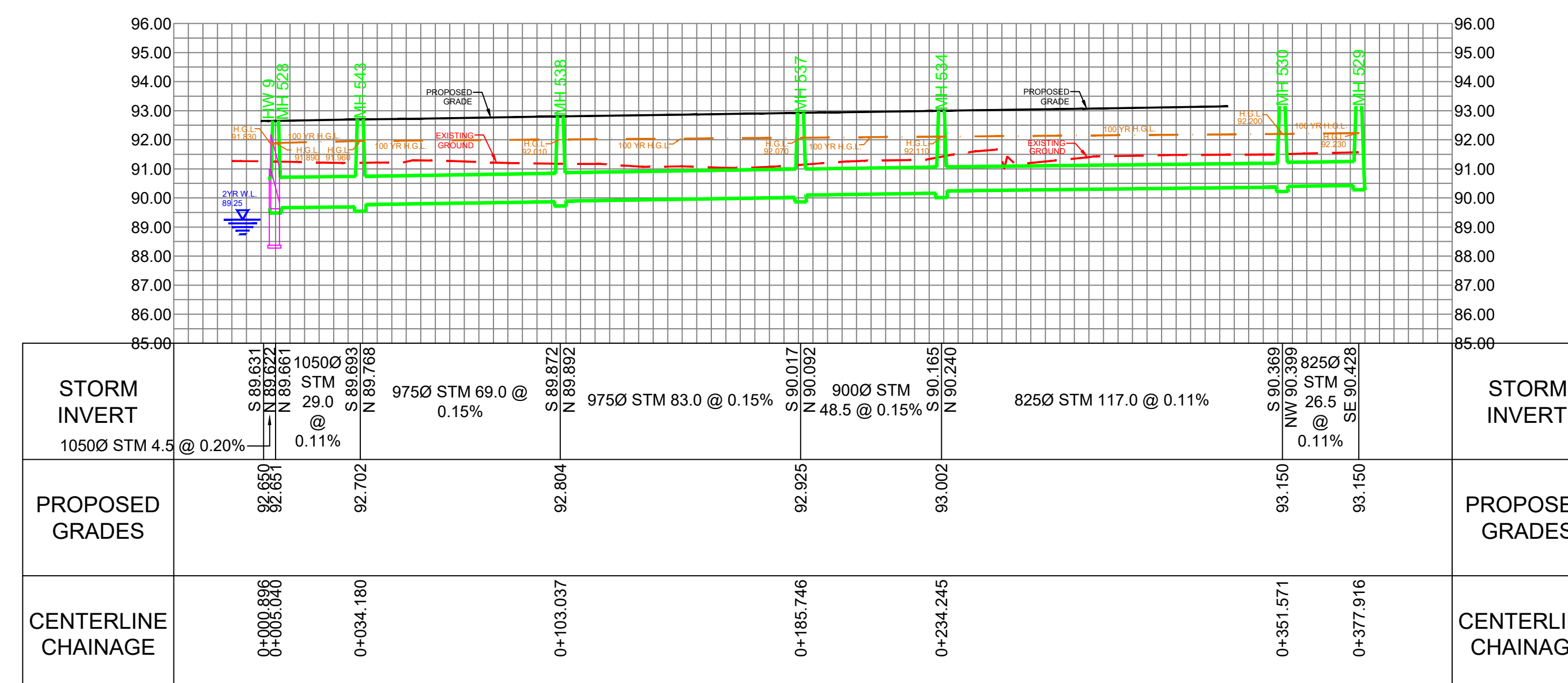
SAN TRUNK 1



STM TRUNK 10



STM TRUNK 9



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Stittville, ON K2S 1E9
Tel. (613) 836-0856
Fax. (613) 836-7183
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BARRHAVEN CONSERVANCY EAST PHASE 5
SANITARY AND STORM TRUNK PROFILES
CITY OF OTTAWA

PROJECT No. : 20-1180 A-5
SCALE: 1:1500
DATE: DECEMBER 2022
DRAWING No. 5

APPENDIX A

GENERAL

Content Copy Of Original



Ministry of the Environment and Climate Change
Ministère de l'Environnement et de l'Action en matière de changement
climatique

ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 8129-AB7LDF

Issue Date: June 23, 2016

City of Ottawa
100 Constellation Crescent West, 6th Floor
Ottawa, Ontario
K2G 6J8

Site Location: Jockvale Road and Strandherd Drive
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act , R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

sanitary sewers to be constructed in the City of Ottawa, on various vacant development lands (from Station 0+003 to Station 2+517), Greenbank Road (from Station 1+846 to Station 1+947), and Jockvale Road (from Station 2+430 to Station 2+517);

all in accordance with the application form from the City of Ottawa, dated June 22, 2016, including final plans and specifications prepared by Novatech Engineers, Planners and Landscape Architects.

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

1. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

3. The name of the appellant;
4. The address of the appellant;
5. The environmental compliance approval number;
6. The date of the environmental compliance approval;
7. The name of the Director, and;
8. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
655 Bay Street, Suite 1500

AND

The Director appointed for the
purposes of Part II.1 of the
Environmental Protection Act

Toronto, Ontario
M5G 1E5

Ministry of the Environment and
Climate Change
135 St. Clair Avenue West, 1st
Floor
Toronto, Ontario
M4V 1P5

*** Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or www.ert.gov.on.ca**

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

DATED AT TORONTO this 23rd day of June, 2016

Gregory Zimmer, P.Eng.
Director
appointed for the purposes of Part II.1 of
the *Environmental Protection Act*

AF/
c: District Manager, MOECC Ottawa
Water Supervisor, MOECC, Ottawa
M. Rick O'Connor, City Clerk, City of Ottawa
Luc Marineau, City of Ottawa
Jonathan Knoyle, City of Ottawa
Bob Dowdall, Novatech Engineers, Planners and Landscape Architects
Edson Donnelly, Novatech Engineers, Planners and Landscape Architects

ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 4357-CHMQEM
Issue Date: September 1, 2022

Barrhaven Conservancy Development Corporation
2934 Baseline Road, Suite 302
Ottawa, Ontario
K2H 1B2

Site Location: Barrhaven Conservancy East - Phase 2, 3 & Jock River
Part of Lots 13 & 14 (Rideau Front)
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

the establishment of wastewater infrastructure Works located in the City of Ottawa, consisting of the following:

- **sanitary sewers** on Les Emmerson Drive (N)(from Station 0+000.000 to Station 0+720.000), Les Emmerson Drive (S) (from Station 0+000.000 to Station 0+660.000), Conservancy Drive (from Station 0-010.000 to Station 0+973.545), Peninsula Road (from Station 0+010.058 to Station 0+703.797), Sapling Grove (from Station 0+000.000 to Station 0+528.245), Canoe Street (from Station 0+000.000 to Station 0+491.136), Deciduous Crescent (from Station 0+002.000 to Station 0+328.189), Ephemeral Crescent (from Station 0+000.000 to Station 0+492.987), Mineral Street (from Station 0+242.832 to Station 0+000.000), Pollination Place (from Station 0+002.985 to Station 0+433.904), Gallium Crescent (from Station 0+002.728 to Station 0+321.940), Syringa Court (from Station 0+000.000 to Station 0+332.328), Anemone Mews (from Station 0+242.833 to Station 0+059.755), Ainsworth Crescent (from Station 0+002.715 to Station 0+353.228), Ecology Lane (from Station 0+205.411 to Station 0+007.658), Meander Way (from Station 0+002.747 to Station 0+333.559), Elation Heights (from Station 0+000.000 to Station 0+380.000), Jollity Crescent (from Station 0+001.794 to Station 0+221.612), Euphoria Crescent (from Station 0+000.000 to Station 0+170.000), and on Borrisokane Road (from Station 0+168.736 to Station 0+507.126), all discharging to the existing South Nepean Collector sanitary sewer; and

- **storm sewers** on Les Emmerson Drive (N) (from Station 0+002.919 to Station 0+718.915), Les Emmerson Drive (S) (from Station 0-002.269 to Station 0+676.895), Conservancy Drive (from Station 0+020.468 to Station 0+961.195), Peninsula Road (from Station 0-001.986 to Station 0+705.797), Sapling Grove (from Station 0-010.000 to Station 0+526.245), Canoe Street (from Station 0+000.000 to Station 0+493.136), Deciduous Crescent (from Station 0+004.500 to Station 0+324.827), Ephemeral Crescent (from Station 0-002.063 to Station 0+495.738), Mineral Street (from Station 0+244.847 to Station 0+002.015), Pollination Place (from Station 0+000.000 to Station 0+424.262), Gallium Crescent (from Station 0+000.000 to Station 0+325.307), Syringa Court (from Station 0-001.985 to Station 0+334.348), Anemone Mews (from Station 0+244.843 to Station 0+001.982), Ainsworth Crescent (from Station 0+000.000 to Station 0+354.443), Ecology Lane (from Station 0+207.411 to Station 0+006.523), Meander Way (from Station 0+016.643 to Station 0+335.359), Elation Heights (from Station 0+000.000 to Station 0+381.539), Jollity Crescent (from Station 0+003.277 to Station 0+220.000), Euphoria Crescent (from Station 0+003.400 to Station 0+157.175), Lane 1 (Crowfoot Lane) (from Station 0-002.000 to Station 0+201.525), and on Lane 2 (Ambit Lane) (from Station 0+002.000 to Station 0+127.5060), proposed storm sewers collect flows from the subdivision and discharge to the Jock River and the Fraser-Clarke Watercourse which is an existing tributary to the Jock River;

the modification of a section of the Fraser-Clarke Watercourse to accommodate stormwater outflows from Phase 2 of the Barrhaven Conservancy East Subdivision development, for the collection and transmission of stormwater runoff for all storm events up to and including the 100-year storm event, discharging to the Jock River, consisting of the following:

- **approximately 950 metres long modified channel**, located along the northern boundary of the Barrhaven Conservancy East Phase 2, 3 & Jock River, having a channel gradient of 0.09% and 3:1 side slopes, complete with low flow path and riffle-pool sequences and erosion protection structures, including two (2) 2.4 metre by 1.2 metre box culverts under the future Canoe Street crossing, discharging to the Jock River;

the establishment of stormwater management Works to serve the Barrhaven Conservancy East – Phase 2, 3 & Jock River development, located in the City of Ottawa, for the collection, transmission, treatment and disposal of stormwater runoff from a total catchment area of 44.17 hectares, to provide Enhanced Level protection and to provide on-site retention of 22.5 cubic metres per hectare, discharging to proposed storm sewers, consisting of the following:

- **subsurface infiltration trenches (catchment area 41.81 hectares)**, located on-site within proposed roadways, having a total length of 3,514 metres, a width of 1.75 metres, a base area of 6,150 square metres, a maximum allowable storage depth of 0.40 metres and a maximum available storage volume of 1,087 cubic metres, comprised of a 75 millimetre deep clear stone layer overlying a geotextile non-woven filter fabric, complete with a 250 millimetre diameter perforated storm sub-drain installed in the clear stone layer, installed at select street catch basin manhole locations;

the establishment of stormwater management Works to serve Steeves & Rozema Enterprises Limited, located in the City of Sarnia, consisting of the following:

- **oil and grit separator (catchment area 5.52 hectares):** one (1) oil and grit separator (OGS1), CDS Model PMSU4040-8 or Equivalent Equipment, located within the Canoe Street right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 83%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Canoe Street right-of-way, discharging via a 975 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 5.59 hectares):** one (1) oil and grit separator (OGS2), CDS Model PMSU4040-8 or Equivalent Equipment, located within Servicing Block 767, providing a Predicted Net Annual Load Removal Efficiency of 82.1%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Block 767 and Meander Way right-of-way, discharging via a 975 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 6.77 hectares):** one (1) oil and grit separator (OGS3), CDS Model PMSU4045-8 or Equivalent Equipment, located within the Ainsworth Crescent right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 82.4%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 2,149 litres, a total storage volume of approximately 11,510 litres, and a maximum treatment rate of 212 litres per second, receiving inflow from the storm sewer located within the Ainsworth Crescent right-of-way, discharging via a 975 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 8.42 hectares):** one (1) oil and grit separator (OGS5), CDS Model PMSU5640-10 or Equivalent Equipment, located within Servicing Block 766, providing a Predicted Net Annual Load Removal Efficiency of 81.7%, having a sediment storage capacity of 6,672 litres, an oil storage capacity of 2,869 litres, a total storage volume of approximately 17,070 litres, and a maximum treatment rate of 255 litres per second, receiving inflow from the storm sewer located within Block 766 and the Gallium Crescent right-of-way, discharging via a 1050 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;

- **oil and grit separator (catchment area 5.46 hectares):** one (1) oil and grit separator (OGS6), CDS Model PMSU3035-8 or Equivalent Equipment, located within Servicing Block 765, providing a Predicted Net Annual Load Removal Efficiency of 80.0%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,493 litres, a total storage volume of approximately 10,210 litres, and a maximum treatment rate of 108 litres per second, receiving inflow from the storm sewer located within Block 766 and the Pollination Place right-of-way, discharging via a 900 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 5.05 hectares):** one (1) oil and grit separator (OGS7), CDS Model PMSU4040-8 or Equivalent Equipment, located within the Ephemeral Crescent right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 82.3%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Ephemeral Crescent right-of-way, discharging via a 900 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 4.52 hectares):** one (1) oil and grit separator (OGS8), CDS Model PMSU4040-8 or Equivalent Equipment, located within Servicing Block 744, providing a Predicted Net Annual Load Removal Efficiency of 82.1%, having a sediment storage capacity of 4,270 litres, an oil storage capacity of 1,970 litres, a total storage volume of approximately 10,910 litres, and a maximum treatment rate of 170 litres per second, receiving inflow from the storm sewer located within the Borrisokane Road right-of-way, discharging via a 1200 millimetre diameter outlet pipe to an outlet channel on Block 774 and connecting to the Jock River;
- **oil and grit separator (catchment area 1.63 hectares):** one (1) oil and grit separator (OGS12), CDS Model PMSU2025-5 or Equivalent Equipment, located within Servicing Block 317, providing a Predicted Net Annual Load Removal Efficiency of 81.1%, having a sediment storage capacity of 1,668 litres, an oil storage capacity of 439 litres, a total storage volume of approximately 3,330 litres, and a maximum treatment rate of 45 litres per second, receiving inflow from the storm sewer located within the Les Emmerson Drive right-of-way, discharging via a 600 millimetre diameter outlet pipe to an outlet channel connecting to the Fraser-Clarke watercourse;
- **oil and grit separator (catchment area 1.21 hectares):** one (1) oil and grit separator (OGS13), CDS Model PMSU2020-5 or Equivalent Equipment, located within the Deciduous Crescent right-of-way, providing a Predicted Net Annual Load Removal Efficiency of 80.1%, having a sediment storage capacity of 1,668 litres, an oil storage capacity of 376 litres, a total storage volume of approximately 3,150 litres, and a maximum treatment rate of 31 litres per second, receiving inflow from the storm sewer located within the Deciduous Crescent right-of-way, discharging via a 600 millimetre diameter outlet pipe to an outlet channel connecting to the Fraser-Clarke watercourse;

including erosion/sedimentation control measures during construction and all other controls and appurtenances essential for the proper operation of the aforementioned Works;

all in accordance with the submitted application and supporting documents listed in Schedule "A" forming part of this Approval.

For the purpose of this environmental compliance approval, the following definitions apply:

1. "Approval" means this entire document and any schedules attached to it, and the application;
2. "Director" means a person appointed by the Minister pursuant to section 5 of the EPA for the purposes of Part II.1 of the EPA;
3. "District Manager" means the District Manager of the appropriate local District Office of the Ministry, where the Works are geographically located;
4. "EPA" means the *Environmental Protection Act*, R.S.O. 1990, c.E.19, as amended;
5. "Equivalent Equipment" means a substituted equipment or like-for-like equipment that meets the required quality and performance standards of the approved named equipment.
6. "Ministry" means the ministry of the government of Ontario responsible for the EPA and OWRA and includes all officials, employees or other persons acting on its behalf;
7. "Owner" means Barrhaven Conservancy Development Corporation, and includes its successors and assignees;
8. "OWRA" means the *Ontario Water Resources Act*, R.S.O. 1990, c. O.40 , as amended;
9. "Works" means the sewage Works described in the Owner's application, and this Approval.

You are hereby notified that this environmental compliance approval is issued to you subject to the terms and conditions outlined below:

TERMS AND CONDITIONS

1. GENERAL CONDITIONS

1. The Owner shall ensure that any person authorized to carry out work on or operate any aspect of the Works is notified of this Approval and the conditions herein and shall take all reasonable measures to ensure any such person complies with the same.

2. Except as otherwise provided by these Conditions, the Owner shall design, build, install, operate and maintain the Works in accordance with the description given in this Approval, and the application for approval of the Works.
3. Where there is a conflict between a provision of any document in the schedule referred to in this Approval and the conditions of this Approval, the conditions in this Approval shall take precedence, and where there is a conflict between the documents in the schedule, the document bearing the most recent date shall prevail.
4. Where there is a conflict between the documents listed in Schedule "A" and the application, the application shall take precedence unless it is clear that the purpose of the document was to amend the application.
5. The conditions of this Approval are severable. If any condition of this Approval, or the application of any requirement of this Approval to any circumstance, is held invalid or unenforceable, the application of such condition to other circumstances and the remainder of this Approval shall not be affected thereby.

2. EXPIRY OF APPROVAL

1. This Approval will cease to apply to those parts of the Works which have not been constructed within five (5) years of the date of this Approval.
2. In the event that completion and commissioning of any portion of the Works is anticipated to be delayed beyond the specified expiry period, the Owner shall submit an application of extension to the expiry period, at least twelve (12) months prior to the end of the period. The application for extension shall include the reason(s) for the delay, whether there is any design change(s) and a review of whether the standards applicable at the time of Approval of the Works are still applicable at the time of request for extension, to ensure the ongoing protection of the environment.

3. CHANGE OF OWNER

1. The Owner shall notify the District Manager and the Director, in writing, of any of the following changes within thirty (30) days of the change occurring:
 - a. change of Owner;
 - b. change of address of the Owner;
 - c. change of partners where the Owner is or at any time becomes a partnership, and a copy of the most recent declaration filed under the *Business Names Act*, R.S.O. 1990, c.B17 shall be included in the notification to the District Manager; or

- d. change of name of the corporation where the Owner is or at any time becomes a corporation, and a copy of the most current information filed under the *Corporations Information Act*, R.S.O. 1990, c. C39 shall be included in the notification to the District Manager.
2. In the event of any change in ownership of the Works, other than a change to a successor municipality, the Owner shall notify in writing the succeeding owner of the existence of this Approval, and a copy of such notice shall be forwarded to the District Manager and the Director.
3. The Owner shall ensure that all communications made pursuant to this condition refer to the number at the top of this Approval.

4. OPERATION AND MAINTENANCE

1. If applicable, any proposed storm sewers or other stormwater conveyance in this Approval can be constructed but not operated until the proposed stormwater management facilities in this Approval or any other Approval that are designed to service the storm sewers or other stormwater conveyance are in operation.
2. The Owner shall make all necessary investigations, take all necessary steps and obtain all necessary approvals so as to ensure that the physical structure, siting and operations of the Works do not constitute a safety or health hazard to the general public.
3. The Owner shall undertake an inspection of the condition of the Works, at least once a year, and undertake any necessary cleaning and maintenance to ensure that sediment, debris and excessive decaying vegetation are removed from the Works to prevent the excessive build-up of sediment, oil/grit, debris and/or decaying vegetation, to avoid reduction of the capacity and/or permeability of the Works, as applicable. The Owner shall also regularly inspect and clean out the inlet to and outlet from the Works to ensure that these are not obstructed.
4. The Owner shall construct, operate and maintain the Works with the objective that the effluent from the Works is essentially free of floating and settleable solids and does not contain oil or any other substance in amounts sufficient to create a visible film, sheen, foam or discoloration on the receiving waters.
5. The Owner shall maintain a logbook to record the results of these inspections and any cleaning and maintenance operations undertaken, and shall keep the logbook at the Owner's administrative office for inspection by the Ministry. The logbook shall include the following:
 - a. the name of the Works; and
 - b. the date and results of each inspection, maintenance and cleaning, including an estimate of the quantity of any materials removed and method of clean-out of the Works.

6. The Owner shall prepare an operations manual prior to the commencement of operation of the Works that includes, but is not necessarily limited to, the following information:
 - a. operating and maintenance procedures for routine operation of the Works;
 - b. inspection programs, including frequency of inspection, for the Works and the methods or tests employed to detect when maintenance is necessary;
 - c. repair and maintenance programs, including the frequency of repair and maintenance for the Works;
 - d. contingency plans and procedures for dealing with potential spills and any other abnormal situations and for notifying the District Manager; and
 - e. procedures for receiving, responding and recording public complaints, including recording any follow-up actions taken.
7. The Owner shall maintain the operations manual current and retain a copy at the Owner's administrative office for the operational life of the Works. Upon request, the Owner shall make the manual available to Ministry staff.

5. TEMPORARY EROSION AND SEDIMENT CONTROL

1. The Owner shall install and maintain temporary sediment and erosion control measures during construction and conduct inspections once every two (2) weeks and after each significant storm event (a significant storm event is defined as a minimum of 25 mm of rain in any 24 hours period). The inspections and maintenance of the temporary sediment and erosion control measures shall continue until they are no longer required and at which time they shall be removed and all disturbed areas reinstated properly.
2. The Owner shall maintain records of inspections and maintenance which shall be made available for inspection by the Ministry, upon request. The record shall include the name of the inspector, date of inspection, and the remedial measures, if any, undertaken to maintain the temporary sediment and erosion control measures.

6. REPORTING

1. One (1) week prior to the start-up of the operation of the Works, the Owner shall notify the District Manager (in writing) of the pending start-up date.
2. The Owner shall, upon request, make all reports, manuals, plans, records, data, procedures and supporting documentation available to Ministry staff.

3. The Owner shall prepare a performance report within ninety (90) days following the end of the period being reported upon, and submit the report(s) to the District Manager when requested. The first such report shall cover the first annual period following the commencement of operation of the Works and subsequent reports shall be prepared to cover successive annual periods following thereafter. The reports shall contain, but shall not be limited to, the following information:
 - a. a description of any operating problems encountered and corrective actions taken;
 - b. a summary of all maintenance carried out on any major structure, equipment, apparatus, mechanism or thing forming part of the Works, including an estimate of the quantity of any materials removed from the Works;
 - c. a summary of any complaints received during the reporting period and any steps taken to address the complaints;
 - d. a summary of all spill or abnormal discharge events; and
 - e. any other information the District Manager requires from time to time.

7. RECORD KEEPING

1. The Owner shall retain for a minimum of five (5) years from the date of their creation, all records and information related to or resulting from the operation, maintenance and monitoring activities required by this Approval.

Schedule "A"

1. Application for Environmental Compliance Approval, dated August 9, 2022 and received on August 17, 2022, submitted by Barrhaven Conservancy Development Corporation;
2. Transfer of Review Letter of Recommendation, dated August 17, 2022 and signed by Jeff Shillington, P.Eng., Senior Project Manager, Development Review, City of Ottawa , including the following supporting documents:
 - a. Final Plans and Specifications prepared by David Schaeffer Engineering Ltd.
 - b. Pipe Data Form - Watermain, Storm Sewer, Sanitary Sewer, and Forcemain Design Supplement to Application for Approval for Water and Sewage Works.
 - c. Hydraulic Design Sheets prepared by David Schaeffer Engineering Ltd.
 - d. Stormwater Management Report prepared by David Schaeffer Engineering Ltd.
 - e. Design brief, calculations and specifications prepared by David Schaeffer Engineering Ltd.
3. Email received on August 25, 2022 from Jeff Shillington, City of Ottawa.

The reasons for the imposition of these terms and conditions are as follows:

1. Condition 1 is imposed to ensure that the Works are constructed and operated in the manner in which they were described and upon which approval was granted. This condition is also included to emphasize the precedence of conditions in the Approval and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review.
2. Condition 2 is included to ensure that, when the Works are constructed, the Works will meet the standards that apply at the time of construction to ensure the ongoing protection of the environment.
3. Condition 3 is included to ensure that the Ministry records are kept accurate and current with respect to the approved Works and to ensure that subsequent owners of the Works are made aware of the Approval and continue to operate the Works in compliance with it.
4. Condition 4 is included as regular inspection and necessary removal of sediment and excessive decaying vegetation from the Works are required to mitigate the impact of sediment, debris and/or decaying vegetation on the treatment capacity of the Works. The Condition also ensures that adequate storage is maintained in the Works at all times as required by the design. Furthermore, this Condition is included to ensure that the Works are operated and maintained to function as designed.
5. Condition 5 is included as installation, regular inspection and maintenance of the temporary sediment and erosion control measures is required to mitigate the impact on the downstream receiving watercourse during construction until they are no longer required.
6. Condition 6 is included to provide a performance record for future references, to ensure that the Ministry is made aware of problems as they arise, and to provide a compliance record for all the terms and conditions outlined in this Approval, so that the Ministry can work with the Owner in resolving any problems in a timely manner.
7. Condition 7 is included to require that all records are retained for a sufficient time period to adequately evaluate the long-term operation and maintenance of the Works.

In accordance with Section 139 of the *Environmental Protection Act*, you may by written notice served upon me and the Ontario Land Tribunal within 15 days after receipt of this notice, require a hearing by the Tribunal. Section 142 of the *Environmental Protection Act* provides that the notice requiring the hearing ("the Notice") shall state:

- a. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
- b. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

1. The name of the appellant;
2. The address of the appellant;
3. The environmental compliance approval number;
4. The date of the environmental compliance approval;
5. The name of the Director, and;
6. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

Registrar*
Ontario Land Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario
M5G 1E5
OLT.Registrar@ontario.ca

and

The Director appointed for the purposes of
Part II.1 of the *Environmental Protection Act*
Ministry of the Environment,
Conservation and Parks
135 St. Clair Avenue West, 1st Floor
Toronto, Ontario
M4V 1P5

* **Further information on the Ontario Land Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349 or 1 (866) 448-2248, or www.olt.gov.on.ca**

The above noted activity is approved under s.20.3 of Part II.1 of the *Environmental Protection Act*.

DATED AT TORONTO this 1st day of September, 2022



Aziz Ahmed, P.Eng.
Director
appointed for the purposes of Part II.1 of the
Environmental Protection Act

RR/

c: District Manager, MECP Ottawa District Office
Clerk, City of Ottawa (File No. D07-16-20-0021)
Jeff Shillington, P.Eng., Senior Project Manager, Development Review, City of Ottawa
Kevin Murphy, David Shaeffer Engineering Ltd.

APPENDIX B
WATER SUPPLY



**Barrhaven Conservancy East
(Phases 2, 3, 4 & Jock River):
Water Distribution System Analysis**

Final Report

June 2, 2022

Prepared for:

David Schaeffer Engineering Ltd.

Prepared by:

Stantec Consulting Ltd.

Revision	Description	Author		Quality Check		Independent Review	
0	Final	TAW	20211213	JS	20211214	KA	20211216
1	Final	TAW/AMG	20220512	AMG	20220516	AP	20220518
2	Final	TAW/AMG	20220602	AMG	20220602	AP	20220602



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

This document entitled **Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River): Water Distribution System Analysis** was prepared by Stantec Consulting Ltd. ("Stantec") for the account of David Schaeffer Engineering Ltd. (the "Client"). Any reliance on this document by any third party is strictly prohibited. The material in it reflects Stantec's professional judgment in light of the scope, schedule and other limitations stated in the document and in the contract between Stantec and the Client. The opinions in the document are based on conditions and information existing at the time the document was published and do not take into account any subsequent changes. In preparing the document, Stantec did not verify information supplied to it by others. Any use which a third party makes of this document is the responsibility of such third party. Such third party agrees that Stantec shall not be responsible for costs or damages of any kind, if any, suffered by it or any other third party as a result of decisions made or actions taken based on this document.

Prepared by _____
(signature)

Thomas Westwood, M.Eng., P.Eng.

Prepared by _____
(signature)

Alexandre Mineault-Guitard, M.Sc.A., ing., P.Eng.

Reviewed by _____
(signature)

Alexandre Mineault-Guitard, M.Sc.A., ing., P.Eng.

Approved by _____
(signature)

Ana Paerez, P.Eng.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Introduction
June 2, 2022

1.0 INTRODUCTION

To support David Schaeffer Engineering Ltd (DSEL) with their conceptual design submission for the Barrhaven Conservancy East development lands (Phases 2, 3, 4 and Jock River), Stantec Consulting Ltd (Stantec) was requested to provide engineering services to complete a water distribution system analysis for this proposed development located within the City of Ottawa's (City) South Urban Community (SUC). The purpose of the analysis is to confirm associated watermain sizing and redundancy needs.

For this assignment, Stantec's scope of work included the following tasks:

- 1) Reviewing background information and establishing updated water demands for the Conservancy East development area based on the most current draft plan;
- 2) Preparing and submitting a boundary condition request to the City;
- 3) Preparing a stand-alone hydraulic model of the distribution system within the Conservancy East lands using boundary conditions provided by the City. The backbone watermain planning model used for previous planning-level analyses will be used as a base;
- 4) Assessing Fire Underwriters Survey (FUS) fire flow requirements;
- 5) Setting up and running model simulations for average day (AVDY), peak hour (PKHR), and maximum day (MXDY) plus fire flow demands to identify watermain sizing and redundancy needs required for the water distribution system within the development lands to meet design criteria; and,
- 6) Documenting the approach used, findings and recommendations from the analysis.

1.1 STUDY AREA

The study area, referred to as the Barrhaven Conservancy East development lands, is located in the City's southwestern suburban neighbourhood of Barrhaven. The lands are situated between Strandherd Dr to the north, the Jock River to the south, Fraser-Clark Drain to the east, and bisected by Borrisokane Rd through the western portion. Based on the current site plan provided by DSEL (dated October 13, 2021) and additional sub-phasing information (dated March 9, 2022), the proposed development is to be subdivided into four (4) phases, which are further described in **Section 1.2**. The proposed development will comprise a total of 782 single family home (SFH) units and 606 townhouse (MLT) units (consisting of a combination of rear-lane, back-to-back and standard townhouse units) for a total estimated population of 4,295 persons.

Based on a previously completed serviceability study for these lands (Stantec Consulting Ltd., 2021), this residential community, which is currently situated adjacent to Pressure Zone 3SW (previously known as



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Zone BARR), is ultimately planned to be serviced by the future Zone SUC. In 2015, the City embarked on a large initiative to reconfigure the pressure zones servicing Barrhaven and the southern reaches of Ottawa (i.e., SUC). The City has indicated that the pressure zone reconfiguration is planned to be completed by the second quarter (Q2) of 2024. The purpose of the zone reconfiguration was to improve reliability and efficiencies, and to provide increased pumping capacity for future growth. As such, these development lands are to be serviced by two connections to the existing distribution network, both of which are currently part of Zone 3SW and will ultimately be part of Zone SUC. These include the following locations as shown in **Figure 1-1**:

- 1) The existing 305 mm stub extending from Chapman Mills Dr (east of Kennedy-Burnett Pond); and
- 2) The T-junction on the existing 203 mm watermain at Danson Gardens Grv and Darjeeling Ave.

Both connections would require crossing the Kennedy-Burnett Pond and the Fraser-Clarke Drain.

The City has also suggested that a third connection be considered, which is also illustrated in **Figure 1-1**. This potential third connection is located south of the Jock River, at a future 305 mm stub at the intersection of Flagstaff Dr and Borrisokane Rd, and would require crossing the Jock River to service the proposed development lands. The serviceability of the development lands using this third connection is also analyzed herein.

1.2 PHASING OF BARRHAVEN CONSERVANCY EAST

For the purpose of this assessment, development within Barrhaven Conservancy East, as shown in **Figure 1-2**, is assumed to occur in the following phasing order:

- 1) Phase 2 – Comprising 240 SFH units, 98 MLT units and two park areas. The townhouses in this phase are a combination of rear-lane and standard townhouse units. Phase 2 will consist of three (3) subphases:
 - Phase 2A – Comprising 102 SFH units and one park area.
 - Phase 2B – Comprising 129 SFH units.
 - Phase 2C – Comprising 9 SFH units, 98 MLT units and one park area.
- 2) Phase 3 – Comprising 128 SFH units and 197 MLT units. As with Phase 2, these townhouses are a combination of rear-lane and standard townhouse units. Phase 3 will consist of two (2) subphases:
 - Phase 2D – Comprising 42 SFH units and 47 MLT units.
 - Phase 2E – Comprising 86 SFH units and 150 MLT units.



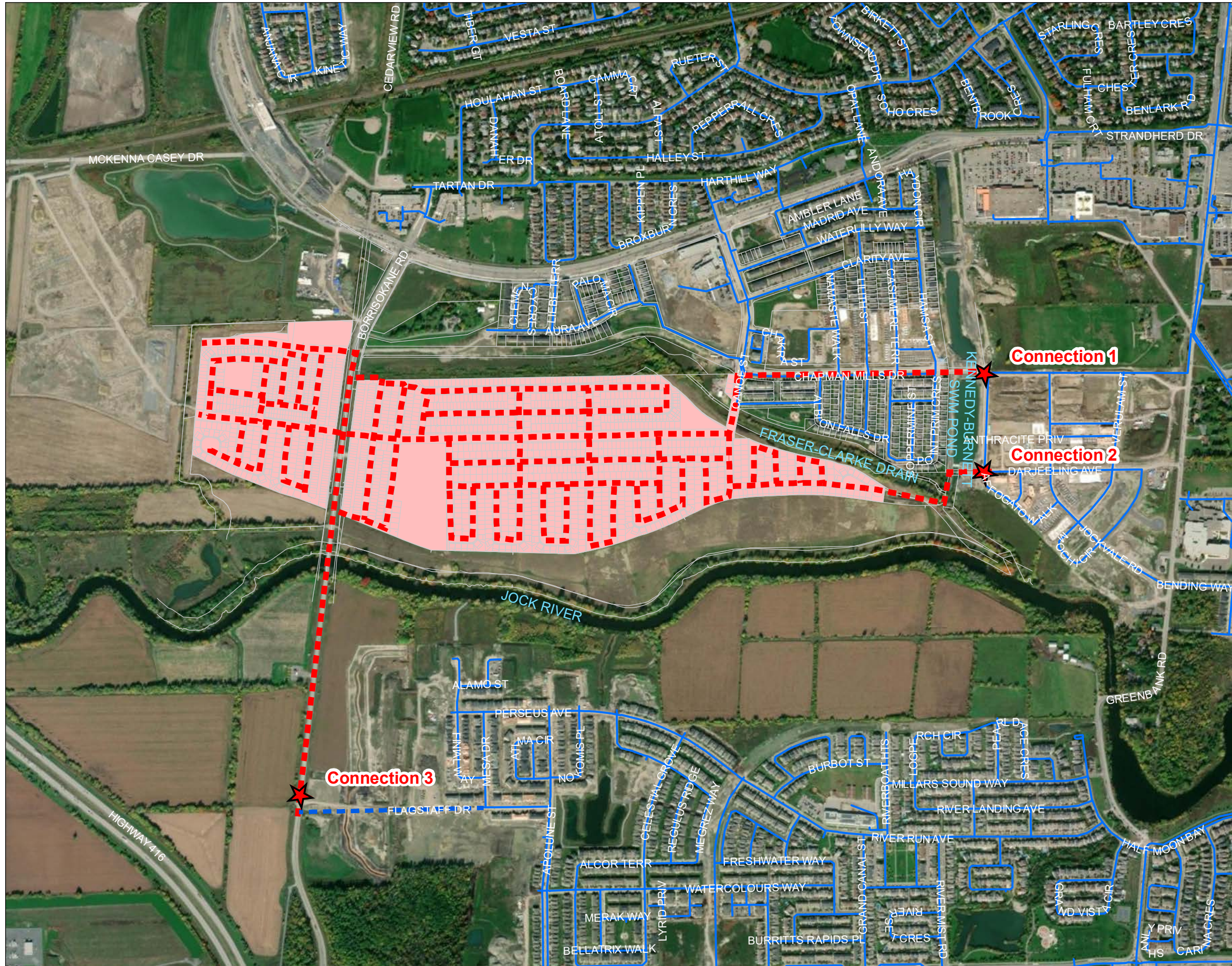
BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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- 3) Phase 4 - Comprising 86 SFH units, 311 MLT units and one park area. As with Phase 2 and 3, the townhouses in this phase are a combination of rear-lane and standard townhouse units, with additional blocks of back-to-back townhouses; and,
- 4) Jock River – Comprising 328 SFH units.

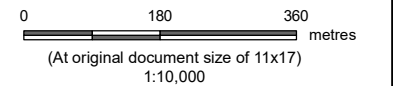
As previously mentioned, the development area will ultimately be serviced by the pressure Zone SUC, once the reconfiguration is complete (planned in Q2 of 2024). As such, the analysis and proposed watermain sizing and layout documented in this report only considers the Zone SUC servicing conditions.



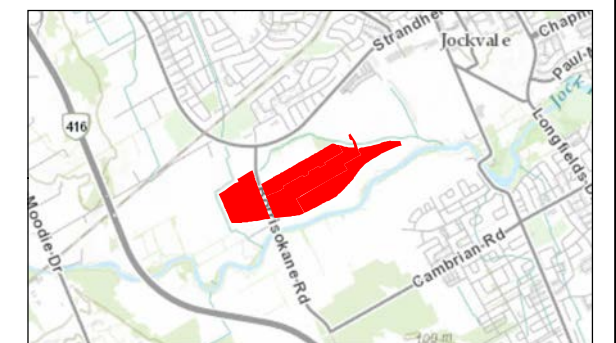


Legend

- Barrhaven Conservancy East Lands
- Property Line
- Existing Distribution Watermain
- Future Distribution Watermain
- Connection Location
- Future Watermain to Service Barrhaven Conservancy East Lands



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community
 Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



Project Location
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 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

Figure No.

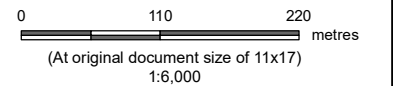
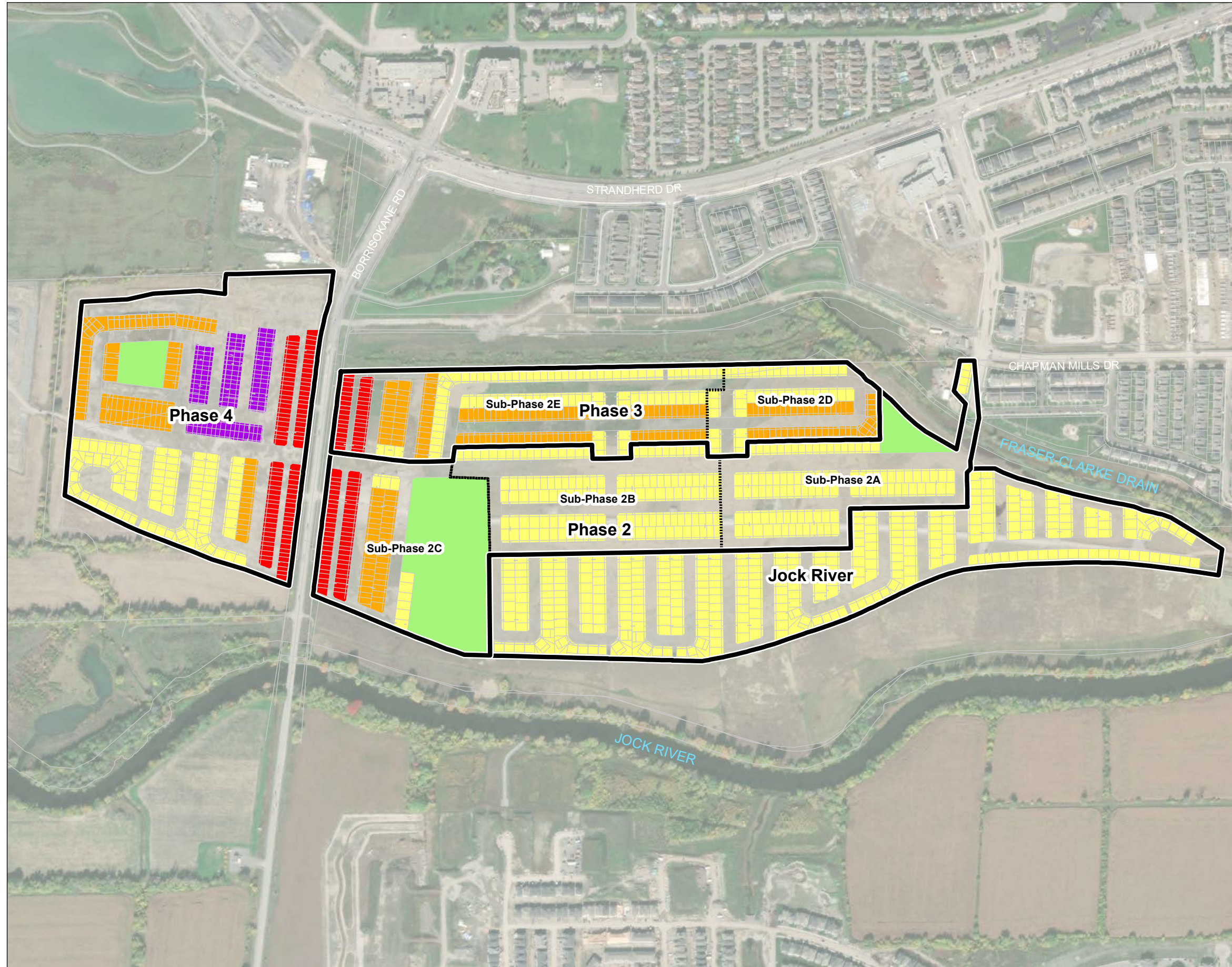
1-1

Title

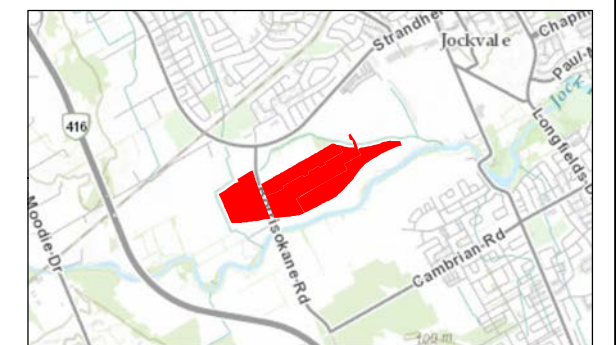
Connections to Existing Water Distribution Network

Legend

- Development Phase Boundary
- Development Sub-Phase Boundary
- Single Family Home (SFH)
- Standard Townhouse (STND TH)
- Rear-Lane Townhouse (RLTH)
- Back-to-Back Townhouse (B2B)
- Park
- Property Line



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community
 Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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1-2

Title

Phasing Plan of Barrhaven Conservancy East

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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2.0 HYDRAULIC ASSESSMENT

The City of Ottawa Water Design Guidelines (City of Ottawa, 2010) and criteria outlined in the 2013 Water Master Plan (WMP) were used to establish water demands, level of service and pressure objectives during normal and emergency conditions. As per the City's design guidelines and recently issued Technical Bulletin ISTB-2021-03, since this is a new development involving the design of new watermains, the design shall consider a required fire flow established using the calculation method published by the Fire Underwriters Survey (FUS).

2.1 SERVICEABILITY

2.1.1 System Pressures

As per the City's Water Design Guidelines, the desired range of pressure under average day (AVDY), maximum day (MXDY) and peak hour (PKHR) demands is 345 to 552 kPa (50 to 80 psi) and no less than 276 kPa (40 psi) at ground elevation (i.e., at street level). The maximum pressure at any point in the water distribution system should not exceed 552 kPa (80 psi); pressure reducing measures are required to service areas where pressures greater than 552 kPa (80 psi) are anticipated.

Under emergency fire conditions, the system must be able to supply appropriate fire flow while maintaining a residual pressure of 138 kPa (20 psi).

Figure 2-1 shows the elevations of each model junction based on the site's current grading plan. These range from 92.4 m to 93.5 m.

2.1.2 Fire Flows

The City requires a fire flow assessment to be completed to demonstrate that local watermains can provide the objective fire flows. The detailed FUS Guidelines (long method; 1999 Version) was used to calculate the objective fire flows. Based on site plan information provided, the following characteristics were considered in the FUS calculations:

- All townhouse units will be of typical construction (e.g., wood frame, limited combustible building contents); firewalls are to be added where required to meet the study area's target fire flow;
- Single family home units will generally be of typical (wood frame) construction except where a break in fire area is required to meet the study area's target fire flow. At such locations, units will be of ordinary construction as described in the FUS guidelines.
 - With side yard separation distances of < 3.0 m between SFH units, the current site layout would contain several large blocks of contiguous SFH units if all were to be of wood frame construction. It is our understanding that the current rearyard setback for all SFH



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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products will be 4.5 m, which in most locations throughout the development area results in rearyard separation distances of less than 10 m. As such, the City's cap of 10,000 L/min, as per Technical Bulletin ISDTB-2018-02, does not apply to these areas and measures such as separating fire areas with units of ordinary construction is required to meet the study area's target fire flow;

- All buildings will have 2 stories above grade (with basements more than 50% below ground level);
- Buildings are not sprinklered; and,
- Setbacks between adjacent units are greater than 3.0 m, with the exception of some proposed SFH units.
 - Per the FUS Guidelines, units with setbacks less than 3.0 m and of wood frame construction will be considered a single fire area.

Based on the latest site plan dated October 2021 and subsequent architectural changes, the required fire flow (RFF) for the governing unit design (rear-lane townhouses, RLTH) was calculated to be 13,000 L/min (217 L/s). This is based on the understanding that, as previously noted, ordinary construction SFH units will be used to separate SFH blocks into fire areas that result in RFFs no greater than 13,000 L/min. Similarly, townhouse blocks will also have firewalls to limit fire areas such that the resulting RFFs will be no greater than 13,000 L/min. The local watermains must therefore be able to provide a minimum fire flow of 13,000 L/min at a residual pressure of 20 psi. The FUS fire flow calculations for the governing unit design and to meet the target fire flow are provided in **Appendix A**.

2.1.3 Water Age

As per the City of Ottawa Design Guidelines, watermains should not be oversized as this may pose water quality degradation, assessed in terms of water age. The Design Guidelines recommend the following:

- A total travel time of 5 days or less during average day demand; and
- A maximum residence time of 8 days.

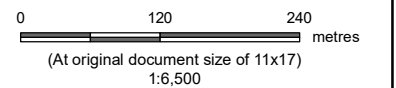
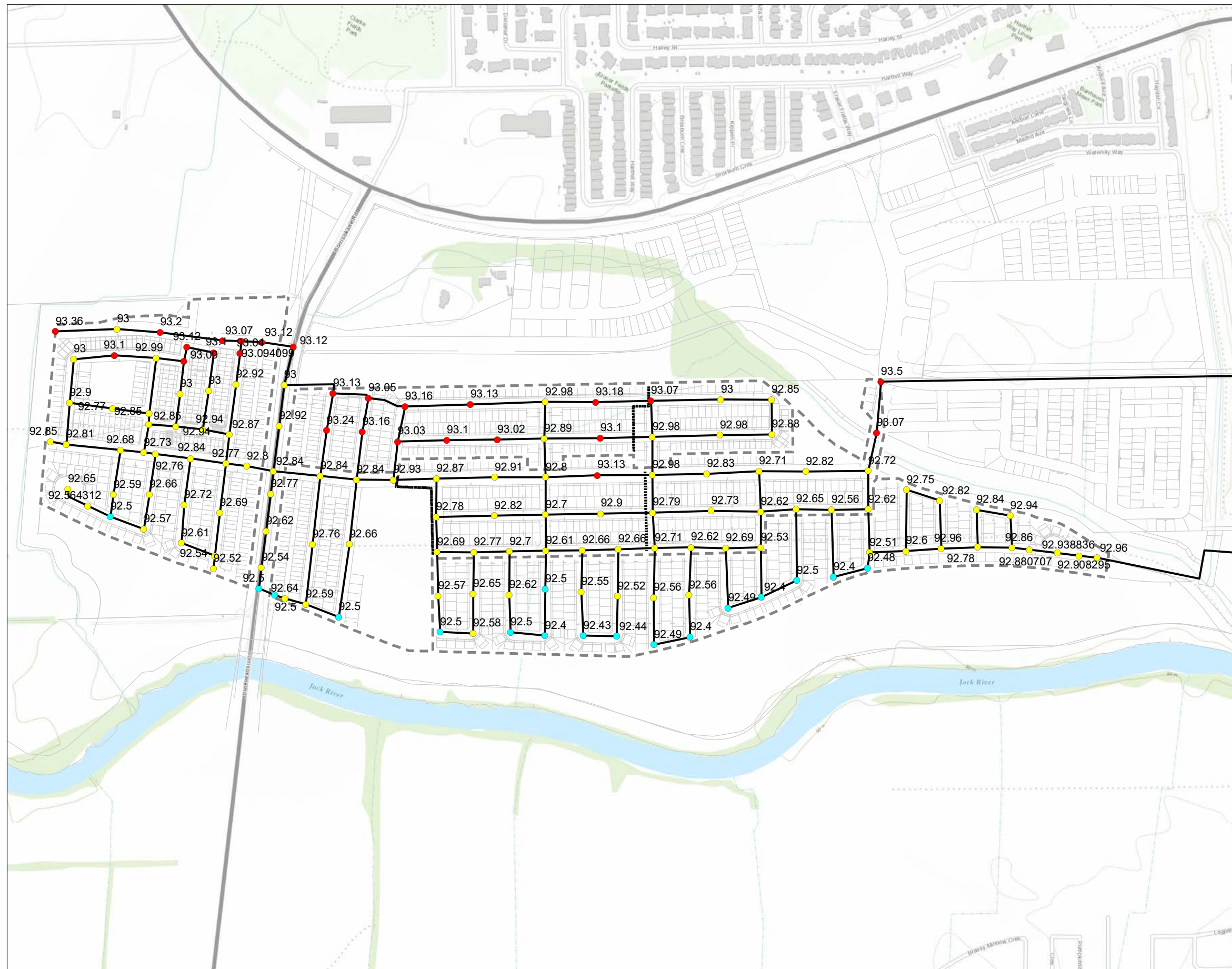


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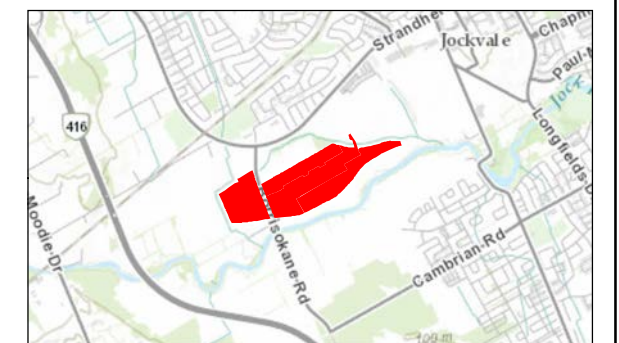
- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line
- Future Watermain

Ground Elevation (m AD)

- ≤ 92.00
- 92.01 - 92.50
- 92.51 - 93.00
- > 93.00



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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Figure No.

2-1

Title

Junction Elevations

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Hydraulic Assessment
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2.2 GROWTH PROJECTIONS

The estimated residential population for Barrhaven Conservancy East was estimated based on projected household sizes as per population densities (or persons per unit, PPU) specified in the City's Water Design Guidelines.

Table 2-1 shows the estimated number of units per phase of these development lands and the projected populations based on the distribution of residential types. The total number of units is estimated to be 1,388 with a residential population of 4,295 persons.

Table 2-1: Estimated Unit Counts and Populations for Barrhaven Conservancy East

Phase	Sub Phase	Unit Types	Units	PPU	Population
2	2A	Singles	102	3.4	347
		Towns	0	2.7	0
	2B	Singles	129	3.4	439
		Towns	0	2.7	0
	2C	Singles	9	3.4	31
		Towns	98	2.7	265
<i>Phase 2 Sub-total</i>			338	-	1,081
3	2D	Singles	42	3.4	143
		Towns	47	2.7	127
	2E	Singles	86	3.4	292
		Towns	150	2.7	405
<i>Phase 3 Sub-total</i>			325	-	967
4		Singles	86	3.4	292
		Towns	311	2.7	840
	<i>Phase 4 Sub-total</i>			397	-
Jock River (JR)		Singles	328	3.4	1,115
		Towns	0	2.7	0
	<i>JR Phase Sub-total</i>			328	-
Total			1,388		4,295



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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2.3 DEMAND PROJECTIONS

Due to the size of the service area, the criteria outlined in the City's Water Design Guidelines and recently issued Technical Bulletin ISTB-2021-03 were followed to establish water demands in Barrhaven Conservancy East. As the buildout population of the proposed development is 4,925 (i.e., greater than 3,000), the City's Water Design Guidelines refer to the MECP Guidelines for consumption rates. The MECP Guidelines provide a consumption rate range of 270 L/cap/day to 450 L/cap/day. The City's Water Design Guidelines consumption rates for subdivisions of 501 to 3,000 persons fall within that range and are therefore applicable. The demand rates and peaking factors from the Water Design Guidelines and Technical Bulletin ISTB-2021-03 were applied to the population projections presented in **Table 2-1** based on land-use.

For residential land-use, SFH and MLT units were assigned an average day (AVDY) consumption rate of 280 L/cap/d. To determine maximum day (MXDY) demands, the AVDY demands were multiplied by a residential peaking factor of 2.5. Peak hour (PKHR) demands were established by multiplying MXDY demands by a residential peaking factor of 2.2. The projected AVDY, MXDY and PKHR demands were distributed to the model nodes by phase (and sub-phase) for the corresponding demand scenario.

Estimated AVDY, MXDY and PKHR demand projections are summarized in **Table 2-2**.

Table 2-2: Estimated Demand Projections for Barrhaven Conservancy East

Phase	Sub-Phase	Unit Types	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
2	2A	Singles	1.12	2.81	6.18
		Towns	0	0	0
	2B	Singles	1.42	3.55	7.82
		Towns	0	0	0
	2C	Singles	0.10	0.25	0.55
		Towns	0.86	2.14	4.72
Phase 2 Sub-total			3.50	8.75	19.26
3	2D	Singles	0.46	1.16	2.55
		Towns	0.41	1.03	2.26
	2E	Singles	0.95	2.37	5.21
		Towns	1.31	3.28	7.22
	Phase 3 Sub-total			3.13	7.84
4	Singles	0.95	2.37	5.21	
	Towns	2.72	6.80	14.97	
	Phase 4 Sub-total			3.67	9.17
Jock River (JR)	Singles	3.61	9.04	19.88	
	Towns	0	0	0	
	JR Phase Sub-total			3.61	9.04
Total			13.92	34.80	76.55



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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2.4 MODEL DEVELOPMENT

Innovyze’s InfoWater (Suite 12.4, Update #9) was used to create a stand-alone hydraulic model of the water distribution system within the proposed development area for this analysis. The model was developed to reflect the most current site plan, including proposed watermain layout (based on proposed road alignment) and water demands.

Watermains added to the model were assigned Hazen-Williams coefficients (“C-Factors”) in accordance with the City’s Water Design Guidelines. These factors are listed in **Table 2-3**.

Table 2-3: Hazen-Williams Coefficients by Watermain Size

Watermain Diameter (mm)	Coefficient
152	100
203 - 305	110
350 - 600	120
> 600	130

2.4.1 Boundary Conditions

The proposed subdivision has two connection points to the existing water distribution system; the option to add a third connection point is also assessed (see **Section 1.1**). The boundary conditions provided by the City include hydraulic gradeline (HGL) values for Zone SUC servicing conditions. Values are provided in **Appendix B** and summarized in **Table 2-4**, and have been simulated in the hydraulic model using fixed head reservoirs to which HGLs have been applied for the respective demand scenarios.

Differences in HGL between connections 1 and 2 are observed under higher demand scenarios. While under AVDY and PKHR conditions, these differences are small (0.2 m or less), under MXDY+FF and AVDY+FF conditions, these differences increase to approximately 4 m. This is likely due to the fact that the Connection 1 is connected to a 305 mm diameter along Chapman Mills Dr, whereas Connection 2 is to a 203 mm diameter along Darjeeling Ave, and the two connection points are interconnected by a 203 mm diameter watermain along Danson Gardens Grv.

If the existing watermain along Danson Gardens Grv is upgraded to a 305 mm diameter watermain, the discrepancies in HGL between Connection 1 and Connection 2 decrease. A second set of boundary conditions (two connections with upgrades) reflecting these conditions was provided by the City.

Finally, a third set of boundary conditions was provided by the City, reflecting conditions with three connection points to the existing water distribution system. These boundary conditions do not consider any upgrades between Connection 1 and Connection 2, and as such the HGLs differ by 3.5 to 3.7 m under MXDY+FF and AVDY+FF conditions.



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Table 2-4: HGL Boundary Conditions

HGL (m)			
Zone SUC Servicing Conditions			
Demand Scenario	Two Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	150.0	150.0	
PKHR	144.2	144.0	
AVDY +FF	138.7	135.1	
MXDY+FF	137.0	133.2	
Demand Scenario	Two Connections with Upgrades ⁽⁴⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	
AVDY	149.5	149.5	
PKHR	144.1	144.1	
AVDY +FF	138.6	139.8	
MXDY+FF	136.8	138.1	
Demand Scenario	Three Connections ⁽⁵⁾		
	Connection 1 ⁽¹⁾	Connection 2 ⁽²⁾	Connection 3 ⁽³⁾
AVDY	149.5	149.5	149.5
PKHR	144.5	144.4	142.0
AVDY +FF	138.6	135.1	137.4
MXDY+FF	137.1	133.4	134.8

Notes:

- (1) Ground elevation @ Connection 1 (Chapman Mills Dr) = 92.8 m.
- (2) Ground elevation @ Connection 2 (Danson Gardens Grv / Darjeeling Ave) = 91.8 m.
- (3) Ground elevation @ Connection 3 (Flagstaff Dr) = 92.1 m.
- (4) Upgrades to existing water distribution required to increase HGL at Connection 2; upsize existing 203 mm diameter watermain on Danson Gardens Grv to a 305 mm diameter watermain.
- (5) For scenarios where ultimate conditions will include three connections, the boundary conditions for two connections (without upgrades) were used when only Connections 1 and 2 are in place, i.e., for modelling Phases 2 and 3.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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2.4.2 Proposed Watermain Sizing & Layout

Two layouts and sizing of the watermains within the proposed development are presented. The first layout (Option A) is required to service the development if only two connections to the water distribution system (Connection 1 and Connection 2) are made. The second layout (Option B) is required to service the development if a third connection (south of the Jock River) is introduced.

2.4.2.1 Option A: Watermain Sizing & Layout for Two Connections

The layout and sizing of the watermains within the proposed development for a scenario with two ultimate connections to the water distribution system are shown in **Figure 2-2**. The same layout and sizing are required with upgrades to the water distribution system along Danson Gardens Grv.

The network is proposed to consist of 152 mm, 203 mm, 305 mm, and 406 mm diameter watermains, with the 305 mm and 406 mm watermains acting as the hydraulic backbone throughout the development lands. The 406 mm diameter watermains run west from connections 1 and 2, interconnect at the east side of the development lands and continue westward across Borrisokane Rd into the Phase 4 lands. The 406 mm diameter watermains can thus serve as backbone if future developments extend further westward. Additional backbone loops added for reliability will be 305 mm watermains. The remaining local watermains will be 152 mm and 203 mm diameter watermains.

The dead-end watermain in the Phase 4 cul-de-sac is proposed to be 203 mm stepping down to 152 mm. Using the traditional “point load assumption” modelling approach to sizing new watermains, the resulting diameter that would be required to provide a fire flow of 13,000 L/min would be larger than the maximum of 152 mm specified in the City’s design guidelines for dead-end watermains. As such, to optimize sizing of this watermain, the alternative procedure outlined in Appendix I (Guidelines on Coordination of Hydrant Placement with Required Fire Flow) of the City’s Technical Bulletin ISDTB-2018-02 was employed. Additional nodes were added to the model network to represent hydrant locations, to which hydrant flows from Table 1 of Appendix I were applied. To achieve a fire flow of 13,000 L/min, two Class AA hydrants within 75 m (each with an assumed maximum flow capacity of 5,700 L/min) and an additional Class AA hydrant between 75 m and 150 m (with an assumed maximum flow capacity of 3,800 L/min) of the furthest unit along the cul-de-sac would provide a total fire flow of 15,200 L/min (i.e., > the RFF of 13,000 L/min). Other hydrant spacing combinations in accordance with the City’s Technical Bulletin ISDTB-2018-02 can also achieve a total fire flow greater than the RFF of 13,000 L/min.

For reliability, the second backbone feed (305 mm) is redirected north then west (i.e., north of the standard and rear-lane townhouse blocks) across to the Phase 4 lands. This alignment provides reliability of service to the lands west of Borrisokane Rd in the event of a failure at the current intersection of the backbone feeds situated within the Borrisokane Rd ROW. System reliability is further discussed in **Section 3.3**. West of Borrisokane Rd, the 305 mm section of backbone will serve as a service connection to future adjacent development. There are no direct Phase 4 property service connections anticipated



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along this temporary dead-end section of watermain; therefore, it is recommended that this section be isolated until it is required to provide flow to adjacent future lands.

2.4.2.2 Option B: Watermain Sizing & Layout for Three Connections

The layout and sizing of the watermains within the proposed development for a scenario with three ultimate connections to the water distribution system are shown in **Figure 2-3**.

The layout of the watermains is the same as in Option A (with two connections). However, with the third connection, the entire backbone can be reduced to 305 mm diameter watermains at all three connection points and throughout the development lands. The remaining watermain sizes are unchanged from Option A, with local watermains of 152 mm and 203 mm diameter watermains.

Based on the proposed phasing, the implementation of the third connection could be delayed until the development of the Phase 4 lands, west of Borrisokane Rd.

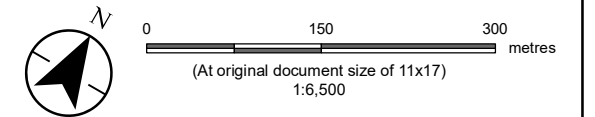


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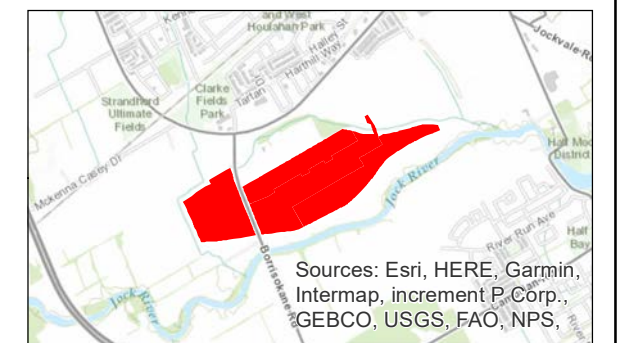
- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line

Proposed Watermain Diameter (mm)

- 152
- 203
- 305
- 406



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



Project Location
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 Water Distribution System Analysis

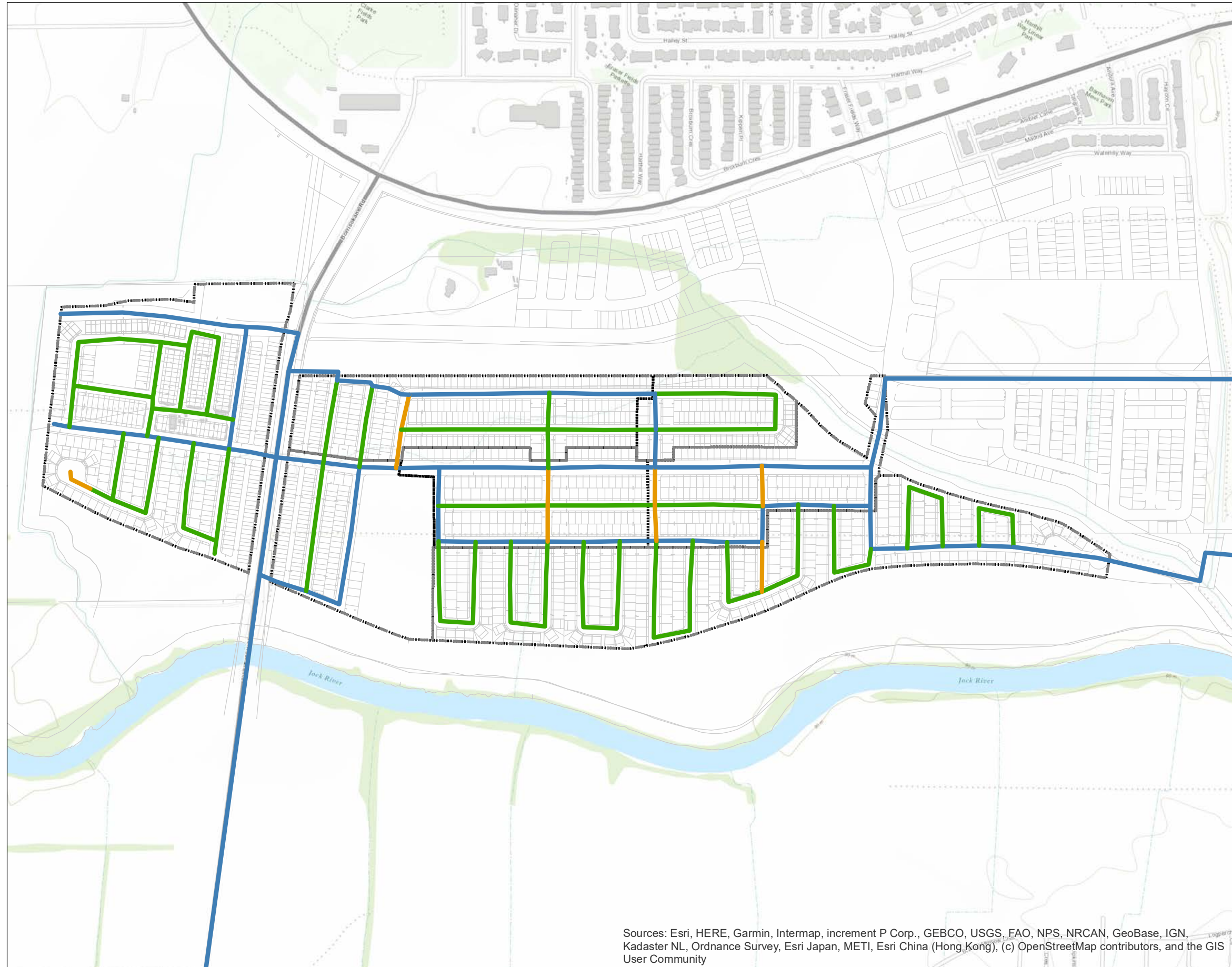
Figure No.

2-2

Title

Proposed Watermain Sizing and Layout - Option A

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

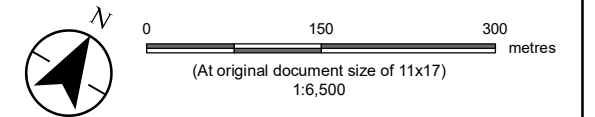


Legend

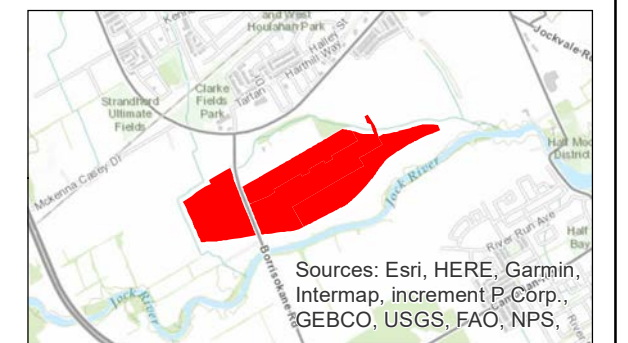
- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line

Proposed Watermain Diameter (mm)

- 152
- 203
- 305



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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 Ottawa, ON

Client/Project
 David Schaeffer Engineering Ltd
 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

Figure No.

2-3

Title

Proposed Watermain Sizing and Layout - Option B

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Hydraulic Modelling Results
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3.0 HYDRAULIC MODELLING RESULTS

Hydraulic modelling was completed for interim phasing conditions and ultimate buildout conditions of the development lands, under SUC servicing conditions, to verify how the network would respond. The following sub-sections present the modelling results under AVDY, PKHR, and MXDY+FF demands, plus under emergency conditions in the event of a watermain break at key points within the proposed network. Detailed modelling results for all scenarios are provided in **Appendix D**.

3.1 AVERAGE DAY & PEAK HOUR DEMANDS

Under AVDY demands with two connections to the water distribution system, maximum modelled pressures for each interim phase and buildout conditions are 82 psi. With three connections to the water distribution system, maximum modelled pressures for each interim phase and buildout conditions are 81 to 82 psi. These maximum pressures exceed the City's maximum pressure objective of 80 psi. As per the Ontario Building Code (OBC) in areas that may be occupied, the static pressure at any fixture shall not exceed 80 psi. Where pressures do exceed 80 psi, pressure control measures such as pressure reducing valves (PRVs) shall be considered.

Under PKHR demands with two connections to the water distribution system, minimum modelled pressures for each interim phase and buildout conditions are 72 psi. With three connections to the water distribution system, minimum modelled pressures for each interim phase and buildout conditions are also 72 psi. These pressures fall within the desired pressure range of 50 to 80 psi.

3.2 MAXIMUM DAY PLUS FIRE FLOW

MXDY+FF demands were applied for the two connections and the three connections scenarios. Each phase was included sequentially to verify network response as the development phases are constructed and occupied.

With two connections to the water distribution system, available fire flow throughout each interim phase and buildout conditions were above the required 13,000 L/min throughout the network. Likewise, with three connections to the water distribution system, available fire flow throughout each interim phase and buildout conditions were above the required 13,000 L/min throughout the network. To optimize the sizing of the dead-end watermain in the Phase 4 cul-de-sac and to reduce potential water quality issues associated with a large-diameter dead-end watermain, the alternative procedure outlined in Appendix I of ISDTB-2018-02 was employed, as described in **Section 2.4.2**. As such, placing two Class AA hydrants within 75 m of the furthest unit along the cul-de-sac (each with an assumed maximum flow capacity of 5,700 L/min), and two Class AA hydrants between 75 m and 150 m (each with an assumed maximum flow capacity of 3,800 L/min), satisfies the fire flow requirement of 13,000 L/min for units along this cul-de-



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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sac. Other hydrant spacing combinations in accordance with the City's Technical Bulletin ISDTB-2018-02 may be implemented to achieve a total fire flow greater than the RFF of 13,000 L/min.

These results show that the proposed watermain sizing and layout meet serviceability requirements with two connections to the water distribution system without requiring further upstream upgrades along Danson Gardens Grv. Nonetheless, these upgrades would still be beneficial, as they would provide consistent HGLs between the two connection points.

3.3 RELIABILITY

As per the City of Ottawa Design Guidelines, the system must be able to provide average day demand plus fire flow (AVDY+FF) while meeting serviceability requirements during a major failure (i.e., watermain break). To assess reliability and resiliency against major failures, a number of reliability scenarios were completed to confirm sufficient pressure and flow can be achieved during a major failure. These scenarios included the following and are shown in **Figure 3-1** (for Option A, with two connections) and in **Figure 3-2: Reliability Analysis Watermain Break Locations – Option B**

(for Option B, with three connections):

- 1) **Break Scenario 1** – Break in the backbone watermain from Connection 1;
- 2) **Break Scenario 2** – Break in the backbone watermain from Connection 2;
- 3) **Break Scenario 3** – Break in backbone watermain at the northwest edge of the large Phase 2 (Phase 2C) park;
- 4) **Break Scenario 4** – Break along the east-west backbone watermain, immediately west of Borrisokane Rd;
- 5) **Break Scenario 5** – Break in the south backbone watermain through Phase 2 lands, immediately east of Borrisokane Rd;
- 6) **Break Scenario 6** – Break in the north backbone watermain through Phase 2 lands, immediately west of Borrisokane Rd; and,
- 7) **Break Scenario 7** (for Option B only) – Break in the backbone watermain from Connection 3 (crossing the Jock River).

Under break scenario 1, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 2, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

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Under break scenario 3, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections). However, under the current phasing plan, a second feed would still be required to service the Phase 2 service area west of the park area (sub-phase 2C; containing more than 50 properties) in the interim until sufficient looping can be provided through the subsequent Phase 3 (sub-phase 2E) if current phasing plans remain unchanged. It is thus recommended that sub-phase 2C be developed after sub-phase 2E, to provide sufficient looping. However, some alternatives could be considered to proceed with sub-phase 2C in accordance with the City's Design Guidelines. First, if the construction of the rear-lane townhouse units (refer to **Figure 1-2**) under sub-phase 2C are delayed until sufficient looping is provided, 55 units will remain as part of sub-phase 2C. As per the City's Design Guidelines, up to 75 units can be serviced on a temporary basis by a dead-end water (or a single feed in this case), given that all pressure and demand objectives are met, and it will be looped by a future phase within 2 years. As such, the 55 units could be serviced from the single feed, given that a second loop is provided in a timely matter (2 years). Alternatively, delaying the construction of 6 other units, in addition to the rear-lane townhouses, to avoid the creation of a vulnerable service area (i.e., less than 50 units under sub-phase 2C until sufficient looping is provided) could be considered.

Under break scenario 4, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 5, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 6, all junctions meet their respective required fire flows, both for Option A (with two connections) and Option B (with three connections).

Under break scenario 7, all junctions meet their respective required fire flows. This break scenario would only occur if Option B (watermain layout with third connection across the Jock River) is selected.

3.4 WATER AGE

Water age is calculated as the total pipe volume divided by the AVDY demand. For sizing Option A (with two connections), water age is highest (1.56 days) when Phase 2 is built. The water age upon buildout is 0.67 days. For sizing Option B (with three connections), water age is highest (1.05 days) when Phase 2 is built. The water age upon buildout is 0.57 days.

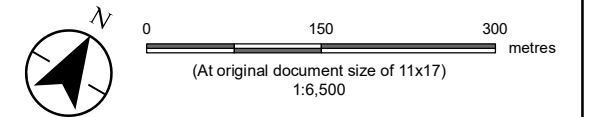
It should be noted that no water age boundary conditions at the connection points were available, therefore the total water age from the source or last point of rechlorination cannot be assessed. Nonetheless, this analysis shows that the residence time of water within the development lands does not exceed the limits per the City's Design Guidelines, and as such water age issues within the development are not anticipated.

Detailed calculations are provided in **Appendix E**.

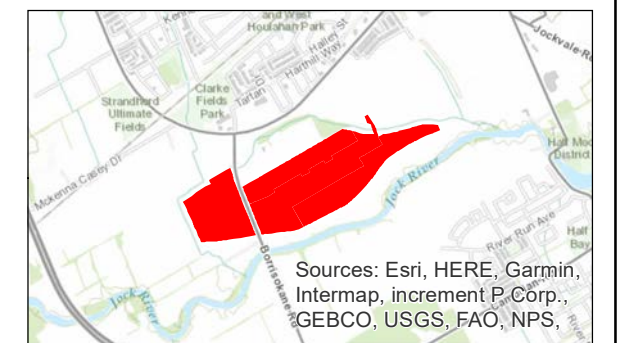


Legend

- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line
- Proposed Watermain Diameter (mm)**
- 152
- 203
- 305
- 406
- Watermain Break Locations



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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 Water Distribution System Analysis

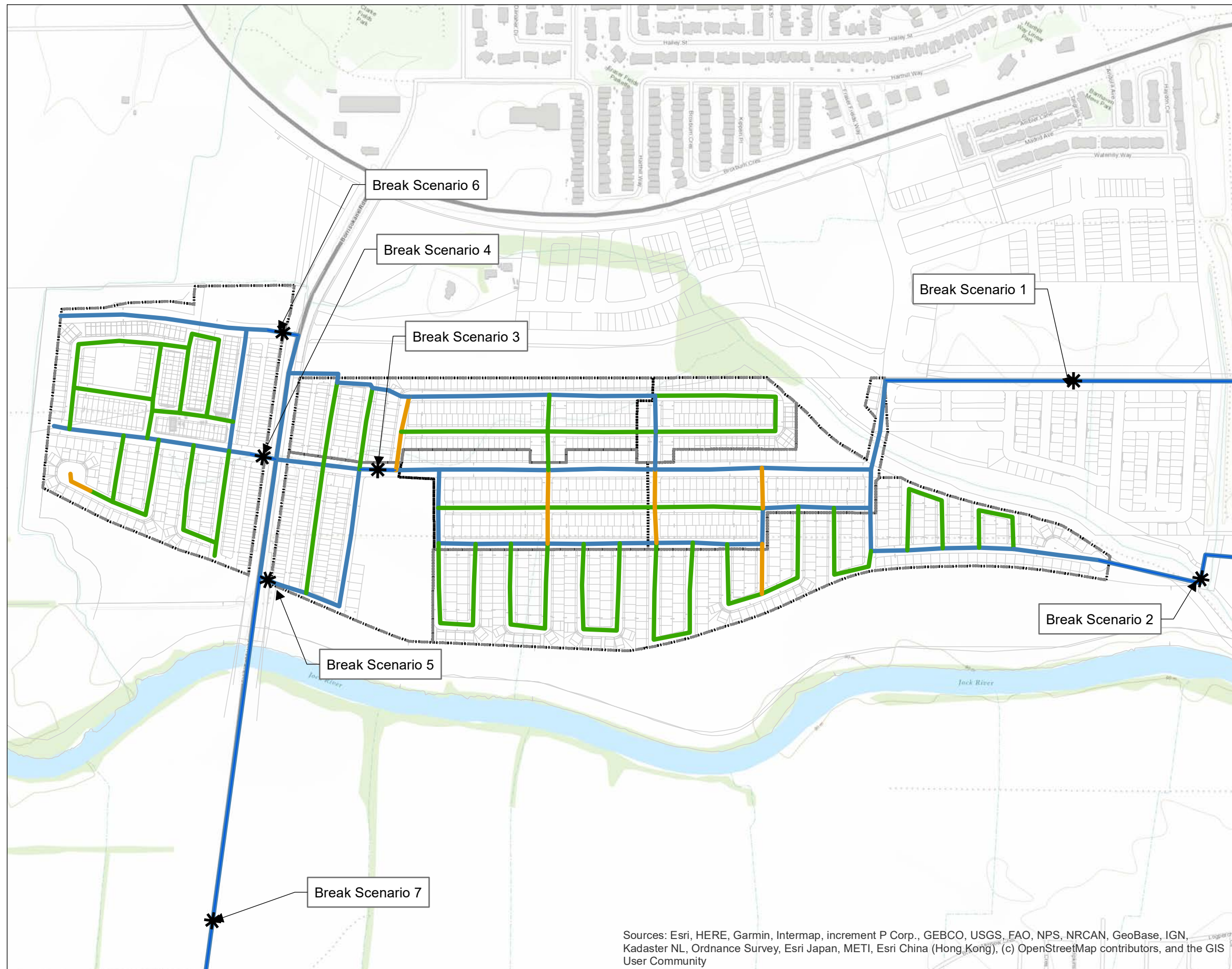
Figure No.

3-1

Title

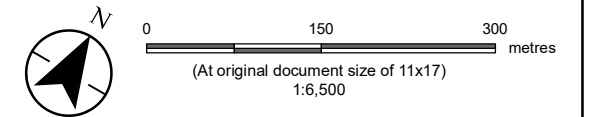
Reliability Analysis Watermain Break Locations - Option A

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

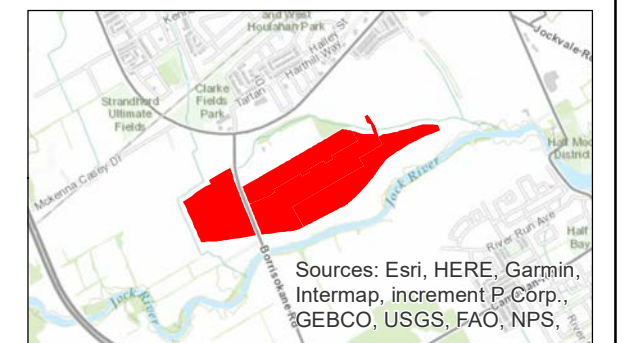


Legend

- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line
- Proposed Watermain Diameter (mm)**
- 152
- 203
- 305
- Watermain Break Locations



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



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 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

Figure No.

3-2

Title

Reliability Analysis Watermain Break Locations - Option B

Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Conclusion and Recommendations
June 2, 2022

4.0 CONCLUSION AND RECOMMENDATIONS

A water distribution system hydraulic analysis was completed for the Barrhaven Conservancy East development lands. The purpose of this analysis was to confirm associated watermain sizing and redundancy needs. Based on the hydraulic analysis, the following conclusions and recommendations were made:

- Based on the most current site plan layout, the estimated AVDY, MXDY and PKHR demand projections for the development lands are 13.92 L/s, 34.80 L/s, and 76.55 L/s, respectively.
- The required fire flow for the governing unit design (rear-lane townhouses) was calculated to be 13,000 L/min (217 L/s). This is based on the understanding that ordinary construction single-family housing (SFH) units will be used to separate SFH blocks into fire areas that result in required fire flows (RFFs) no greater than 13,000 L/min. Similarly, townhouse blocks will have firewalls to limit fire areas such that the resulting RFFs will be no greater than 13,000 L/min.
 - As the watermain sizing presented herein is based on an RFF of 13,000 L/min, the final design of the units should meet the requirements for this RFF, per the FUS Guidelines.
- Two watermain layout and sizing are proposed:
 - The first option (Option A) would involve two connections to the water distribution system; the proposed sizing is recommended to include 406 mm diameter watermains from connections 1 and 2 as the hydraulic backbone of the network, with 305 mm diameter watermains for backbone looping. Watermains along local right-of-ways would be 152 mm and 203 mm diameter watermains. The dead-end watermain in the cul-de-sac at the western extent of the development would be 152 mm diameter. The proposed layout and sizing is shown in **Figure 2-2**.
 - The second option (Option B) would involve three connections to the water distribution system, with the third connection requiring crossing the Jock River. The proposed layout within the development is similar to Option A; the proposed sizing is recommended to include 305 mm diameter watermains for the hydraulic backbone of the network. Watermains along local right-of-ways would be 152 mm and 203 mm diameter watermains. The dead-end watermain in the cul-de-sac at the western extent of the development would be 152 mm diameter. The proposed layout and sizing is shown in **Figure 2-3**.
- The serviceability of the development lands was analysed, considering that they would be serviced by the pressure zone SUC.
- As part of the currently proposed watermain layout, the backbone watermain is proposed to extend west along the northern edge of the Phase 4 lands to serve as a service connection to future adjacent development. There are no direct Phase 4 property service connections



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

Conclusion and Recommendations
June 2, 2022

anticipated along this temporary dead-end section of watermain; therefore, it is recommended that this section be isolated until it is required to provide flow to adjacent future lands.

- Under AVDY demand conditions, model results using boundary conditions provided by the City exceed the allowable maximum pressure of 80 psi in accordance with the City of Ottawa Design Guidelines. As per the OBC, the static pressure at any fixture shall not exceed 80 psi, in areas that may be occupied. Where pressures do exceed 80 psi, pressure control measures such as PRVs installed immediately downstream of the isolation valve to the home/building shall be considered.
- Under PKHR demand conditions, the minimum pressures are in accordance with the City's system pressure requirements.
- Under MXDY+FF demand conditions, the target required fire flow of 13,000 L/min can be achieved through the proposed network for all phases when the alternative procedure outlined in the Appendix I of ISDTB-2018-02 is applied to the dead-end watermain in the western extent of the development lands.
- If Option A (servicing with two connections) is selected, watermain upgrades along Danson Gardens Grv are recommended to provide similar HGLs at the two connection points under a fire flow scenario.
- To satisfy and improve system reliability in the event of an emergency break scenario at key points in the network, a second backbone feed was redirected north then west (i.e., north of the standard and rear-lane townhouse blocks) across to the Phase 4 lands. However, under the current phasing plan, a second feed would still be required to service the Phase 2 service area west of the park area (sub-phase 2C, containing more than 50 properties) in the interim until sufficient looping can be provided through the subsequent Phase 3 (sub-phase 2E) if current phasing plans remain unchanged. It is recommended that sub-phase 2C be developed after sub-phase 2E, to provide sufficient looping. However, sub-phase 2C could be serviced on an interim basis, from the single feed, given that the rear-lane townhouses proposed under sub-phase 2C are delayed until sufficient looping is provided. Alternatively, delaying the construction of 6 other units, in addition to the rear-lane townhouses, could be considered for sub-phase 2C, to avoid the creation of a vulnerable service area.
- A water age analysis shows that the residence time of water within the development lands does not exceed the limits per the City's Design Guidelines. No water age boundary conditions at the connection points were available, therefore the total water age from the source or last point of rechlorination cannot be assessed. As such, water age issues within the development are not anticipated.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

References
June 2, 2022

5.0 REFERENCES

City of Ottawa. (2010). *Ottawa Design Guidelines - Water Distribution*. Ottawa.

City of Ottawa. (2018). *Technical Bulletin ISTB-2018-02*. Ottawa.

City of Ottawa. (2021). *Technical Bulletin ISTB-2021-03*. Ottawa.

Stantec Consulting Ltd. (2013). *City of Ottawa 2013 Water Master Plan*. Ottawa.

Stantec Consulting Ltd. (2021). *Hydraulic Potable Water Assessment for Barrhaven Conservancy Development Corporation*. Ottawa.



BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

June 2, 2022

Appendix A FUS CALCULATION





FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: November 29, 2021
 Data inputted by: Jasmin Sidhu, P.Eng.
 Data reviewed by:

Fire Flow Calculation #: 1
 Building Type/Description/Name: Residential

*Single family house (SFH) block based on draft site plan dated September 20, 2021. Area assumes largest SFH unit size.
 Minimum spatial separation between the backs of adjacent units is <10m (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).
 Notes: Assumed wood frame construction.
 Target fire flow = 13,000 L/min. Requires a maximum of 2 adjacent consecutive wood frame construction units.*

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method

Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Framing Material						
			Wood Frame	1.5	Wood Frame	1.5	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
Fire resistive construction (> 2 hrs)	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Floor Space Area						
			Single Family	2	Single Family	2	Units		
			Townhouse - indicate # of units	1					
			Other (Comm, Ind, Apt etc.)	1					
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):						2	2
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			158	158	Area in Square Meters (m ²)		
					Square Metres (m2)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			632	632			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						8,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	6,800	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.75	m	5,100	
			Side Yard (Left)	0 to 3.0m	0.25				
			Rear Yard	3.1 to 10.0m	0.2				
			Side Yard (Right)	0 to 3.0m	0.25				
			Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow (above) in L/s:						200	
		Required Duration of Fire Flow (hrs)						2.50	
		Required Volume of Fire Flow (m³)						1,800	



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: November 29, 2021
 Data inputted by: Jasmin Sidhu, P.Eng.
 Data reviewed by:

Fire Flow Calculation #: 2
 Building Type/Description/Name: Residential

*Single family house (SFH) block based on draft site plan dated September 20, 2021. Area assumes largest SFH unit size.
 Notes: Minimum spatial separation between the backs of adjacent units is <10m (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).
 Assumed ordinary construction (i.e., more than 2/3 of the buildings' exterior walls are made of brick or masonry veneer).
 Target fire flow = 13,000 L/min. Requires a maximum of 5 adjacent consecutive ordinary construction units.*

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method

Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Framing Material						
			Wood Frame	1.5	Ordinary construction	1	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
Fire resistive construction (> 2 hrs)	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Floor Space Area						
			Single Family	5	Single Family	5	Units		
			Townhouse - indicate # of units	1					
			Other (Comm, Ind, Apt etc.)	1					
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			2	2	Storeys		
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			158	158	Area in Square Meters (m ²)		
					Square Metres (m2)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			1,580	1,580			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.75	m	5,738	
			Side Yard (Left)	0 to 3.0m	0.25				
			Rear Yard	3.1 to 10.0m	0.2				
			Side Yard (Right)	0 to 3.0m	0.25				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						13,000	
		Total Required Fire Flow (above) in L/s:						217	
		Required Duration of Fire Flow (hrs)						2.75	
		Required Volume of Fire Flow (m³)						2,145	



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: May 10, 2022
 Data inputted by: Christène Razafimaharo
 Data reviewed by: Jasmin Sidhu, P.Eng.

Fire Flow Calculation #: 3
 Building Type/Description/Name: Residential

*Notes: Maximum GFA for a STND TH fire area to achieve a required fire flow (RFF) of 13,000 L/min.
 Target fire flow = 13,000 L/min. Fire walls required at the back to achieve RFF.*

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method									
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Framing Material							
		Coefficient related to type of construction (C)	Wood Frame	1.5	Wood Frame	1.5	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
Fire resistive construction (> 2 hrs)	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Floor Space Area							
		Type of Housing	Single Family	1	Townhouse - indicate # of units	5	Units		
			Townhouse - indicate # of units	5					
			Other (Comm, Ind, Apt etc.)	1					
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			2	2	Storeys		
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			82	82	Area in Square Meters (m ²)		
					Square Metres (m2)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			820	820			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.6	m	4,590	
			Side Yard (Left)	3.1 to 10.0m	0.2				
			Rear Yard	Fire Wall	0.1				
			Side Yard (Right)	3.1 to 10.0m	0.2				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						12,000	
		Total Required Fire Flow (above) in L/s:						200	
		Required Duration of Fire Flow (hrs)						2.50	
		Required Volume of Fire Flow (m³)						1,800	



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: September 29, 2021
 Data inputted by: Tom Westwood, P.Eng
 Data reviewed by: Jasmin Sidhu, P.Eng.

Fire Flow Calculation #: 4
 Building Type/Description/Name: Residential

Notes: Governing rear lane townhouse (RLTH) block based on draft site plan dated September 20, 2021. Block consists of 5 RLTH units where minimum spatial separation between the backs of adjacent units is <10m and gross floor area of block is > 600m² (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method									
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Framing Material						
			Wood Frame	1.5	Wood Frame	1.5	m		
			Ordinary construction	1					
			Non-combustible construction	0.8					
			Fire resistive construction (< 2 hrs)	0.7					
Fire resistive construction (> 2 hrs)	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Floor Space Area						
			Single Family	1	Townhouse - indicate # of units	5	Units		
			Townhouse - indicate # of units	5					
Other (Comm, Ind, Apt etc.)	1								
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			2	2	Storeys		
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):			78	78	Area in Square Meters (m ²)		
		Square Metres (m2)							
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):			780	780			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0	
			Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0	
			Sprinkler not fully supervised or N/A	0					
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.7	m	5,355	
			Side Yard (Left)	3.1 to 10.0m	0.2				
			Rear Yard	3.1 to 10.0m	0.2				
			Side Yard (Right)	3.1 to 10.0m	0.2				
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						13,000	
		Total Required Fire Flow (above) in L/s:						217	
		Required Duration of Fire Flow (hrs)						2.75	
		Required Volume of Fire Flow (m³)						2,145	



FUS Fire Flow Calculation

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 1999

Stantec Project #: 163401660
 Project Name: Barrhaven Conservancy
 Date: September 29, 2021
 Data inputted by: Tom Westwood, P.Eng
 Data reviewed by: Jasmin Sidhu, P.Eng.

Fire Flow Calculation #: 5
 Building Type/Description/Name: Residential

Notes: Governing back-to-back townhouse (B2B TH) block based on draft site plan dated September 20, 2021. Block consists of 10 B2B TH units separated by one fire wall, resulting in a fire area comprising 5 units with no spatial separation between the backs of adjacent units and gross floor area of block is > 600m² (i.e., City's cap of 10,000 L/min as per Technical Bulletin ISDTB-2018-02 does not apply).
 Target Fire Flow = 13,000 L/min. Fire walls required at the back to achieve target fire flow.

Table A: Fire Underwriters Survey Determination of Required Fire Flow - Long Method								
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Wood Frame	1.5	Wood Frame	1.5	m	
			Ordinary construction	1				
			Non-combustible construction	0.8				
			Fire resistive construction (< 2 hrs)	0.7				
			Fire resistive construction (> 2 hrs)	0.6				
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Single Family	1	Townhouse - indicate # of units	5	Units	
			Townhouse - indicate # of units	5				
			Other (Comm, Ind, Apt etc.)	1				
			2.2	# of Storeys				
3	Enter Average Floor Area of Unit or Block of Units	Average Floor Area (A) (non-fire resistive construction):		78	78	Area in Square Meters (m ²)		
				Square Metres (m2)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x Average Floor Area of Unit x # of Units):		780	780			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) (F = 220 * C * √A) Round to nearest 1,000 L/min						9,000
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning						
5.1	Choose Combustibility of Building Contents	Occupancy content hazard reduction or surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	7,650
			Limited combustible	-0.15				
			Combustible	0				
			Free burning	0.15				
			Rapid burning	0.25				
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler reduction	Adequate Sprinkler conforms to NFPA13	-0.3	None	0	N/A	0
			None	0				
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is not standard or N/A	0	N/A	0
			Water supply is not standard or N/A	0				
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler not fully supervised or N/A	0	N/A	0
			Sprinkler not fully supervised or N/A	0				
5.3	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	3.1 to 10.0m	0.2	0.6	m	4,590
			Side Yard (Left)	20.1 to 30.1m	0.1			
			Rear Yard	3.1 to 10.0m	0.2			
			Side Yard (Right)	20.1 to 30.1m	0.1			
			Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:					
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow (above) in L/s:						200
		Required Duration of Fire Flow (hrs)						2.50
		Required Volume of Fire Flow (m ³)						1,800

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

June 2, 2022

Appendix B BOUNDARY CONDITIONS

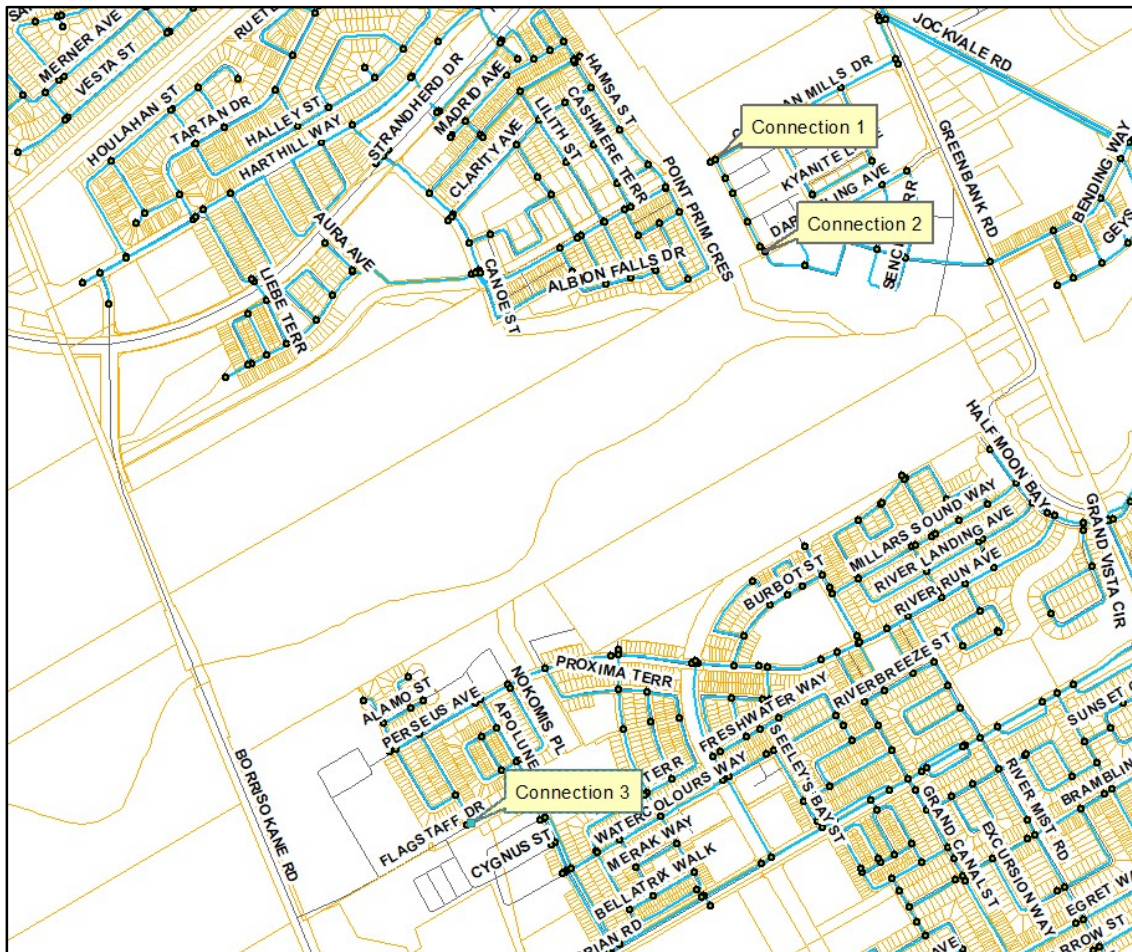


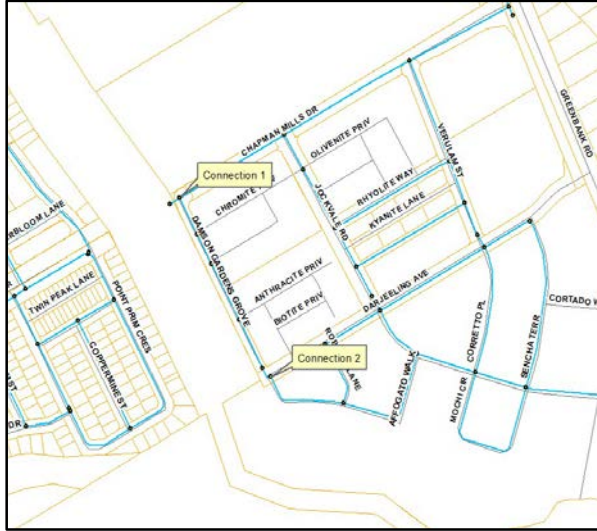
Boundary Conditions Barrhaven Conservancy East

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	832	13.87
Maximum Daily Demand	2,080	34.67
Peak Hour	4,576	76.27
Fire Flow Demand #1	13,000	216.67

Location





Results – Existing Conditions BSDY

Connection 1 – Chapman Mills Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.6	70.7
Basic Day plus Fire 1	131.2	54.5

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	95.1
Peak Hour	142.5	72.1
Basic Day plus Fire 1	127.6	50.9

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.6	94.5
Peak Hour	142.1	71.1
Basic Day plus Fire 1	129.9	53.7

Ground Elevation = 92.1 m

Results – Existing Conditions MXDY

Connection 1 – Chapman Mills Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	91.2
Peak Hour	137.6	63.7
Max Day plus Fire 1	140.1	67.2

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	92.7
Peak Hour	137.6	65.0
Max Day plus Fire 1	136.4	63.4

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	156.8	91.9
Peak Hour	136.9	63.7
Max Day plus Fire 1	137.7	64.8

Ground Elevation = 92.1 m

Results – SUC Zone Reconfiguration BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	80.7
Peak Hour	147.3	77.5
Basic Day plus Fire 1	138.6	65.1

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	82.1
Peak Hour	147.3	78.9
Basic Day plus Fire 1	135.1	61.5

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	81.5
Peak Hour	146.9	77.9
Basic Day plus Fire 1	137.4	64.3

Ground Elevation = 92.1 m

Results – SUC Zone Reconfiguration MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	78.3
Peak Hour	144.5	73.4
Max Day plus Fire 1	137.1	62.9

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	79.7
Peak Hour	144.4	74.7
Max Day plus Fire 1	133.4	59.1

Ground Elevation = 91.8 m

Connection 3 – Flagstaff Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.5	78.8
Peak Hour	142.0	71.0
Max Day plus Fire 1	134.8	60.6

Ground Elevation = 92.1 m

Notes

1. As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a. If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
 - b. Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

Disclaimer

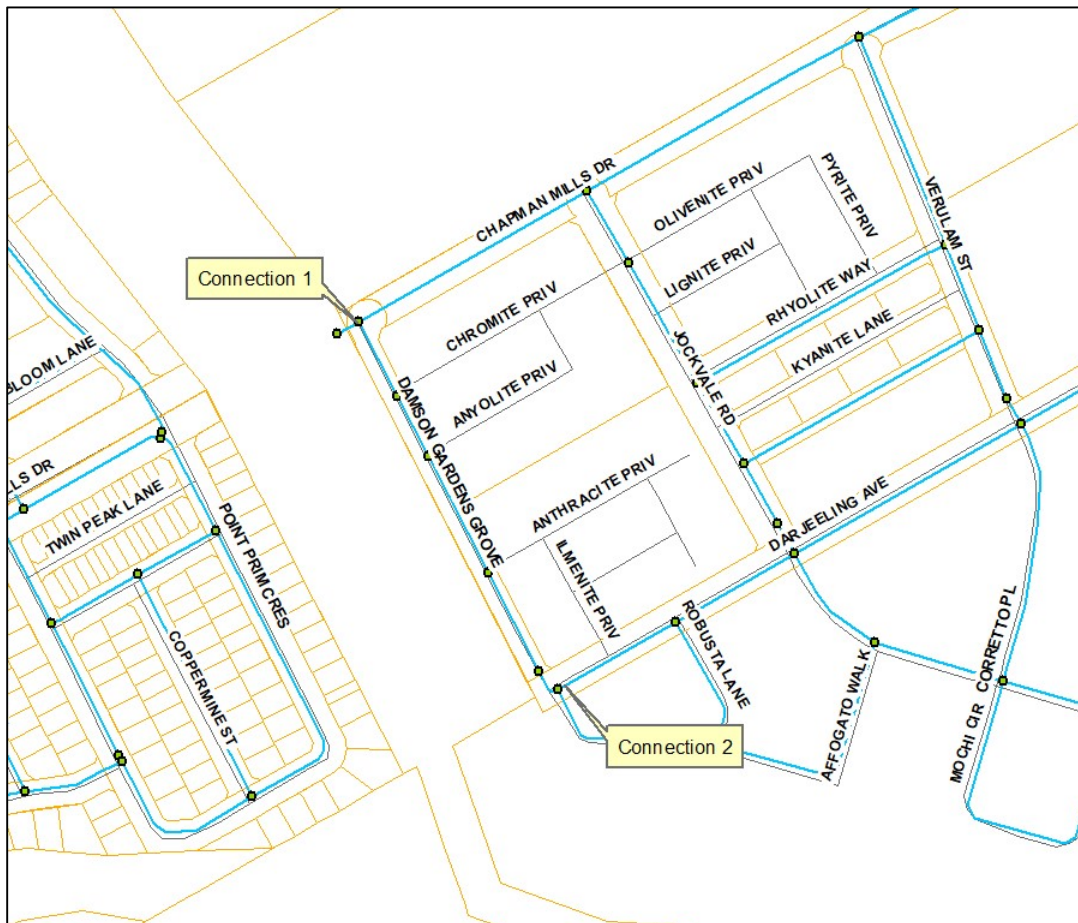
The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Boundary Conditions Barrhaven Conservancy East

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	832	13.87
Maximum Daily Demand	2,080	34.67
Peak Hour	4,576	76.27
Fire Flow Demand #1	13,000	216.67

Location



Scenario 1 Results – Existing Conditions BSDY

Connection 1 – Chapman Mills Dr.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.5	70.7
Basic Day plus Fire 1	131.0	54.2

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	95.1
Peak Hour	142.5	72.1
Basic Day plus Fire 1	127.4	50.6

Ground Elevation = 91.8 m

Scenario 1 Results – Existing Conditions MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	91.2
Peak Hour	137.5	63.5
Max Day plus Fire 1	139.6	66.5

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	92.6
Peak Hour	137.5	64.9
Max Day plus Fire 1	135.8	62.5

Ground Elevation = 91.8 m

Scenario 1 Results – SUC Zone Reconfiguration BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	150.0	81.3
Peak Hour	147.6	77.9
Basic Day plus Fire 1	138.7	65.2

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	150.0	82.8
Peak Hour	147.6	79.2
Basic Day plus Fire 1	135.1	61.5

Ground Elevation = 91.8 m

Scenario 1 Results – SUC Zone Reconfiguration MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	148.6	79.3
Peak Hour	144.2	73.0
Max Day plus Fire 1	137.0	62.8

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	148.6	80.7
Peak Hour	144.0	74.2
Max Day plus Fire 1	133.2	58.8

Ground Elevation = 91.8 m

Scenario 2 Results – Existing Conditions BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.5	70.7
Basic Day plus Fire 1	131.2	54.5

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	158.7	93.7
Peak Hour	142.5	70.7
Basic Day plus Fire 1	132.4	57.6

Ground Elevation = 91.8 m

Scenario 2 Results – Existing Conditions MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	91.2
Peak Hour	137.5	63.6
Max Day plus Fire 1	139.9	66.9

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.0	92.6
Peak Hour	137.5	65.0
Max Day plus Fire 1	141.1	70.1

Ground Elevation = 91.8 m

Scenario 2 Results – SUC Zone Reconfiguration BSDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	80.7
Peak Hour	147.3	77.5
Basic Day plus Fire 1	138.6	65.1

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	149.5	80.7
Peak Hour	147.3	77.5
Basic Day plus Fire 1	139.8	68.2

Ground Elevation = 91.8 m

Scenario 2 Results – SUC Zone Reconfiguration MXDY**Connection 1 – Chapman Mills Dr.**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	78.3
Peak Hour	144.1	72.9
Max Day plus Fire 1	136.8	62.6

Ground Elevation = 92.8 m

Connection 2 – Danson Gardens Grove / Darjeeling Ave.

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.9	79.7
Peak Hour	144.1	74.3
Max Day plus Fire 1	138.1	65.8

Ground Elevation = 91.8 m

Notes

1. The watermain on Darjeeling Ave. was upsized to a 300mm diameter pipe between Danson Gardens Grove and Jockvale Road during Scenario 2 for modelling purposes.
2. As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a. If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
 - b. Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

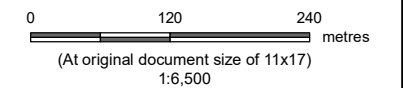
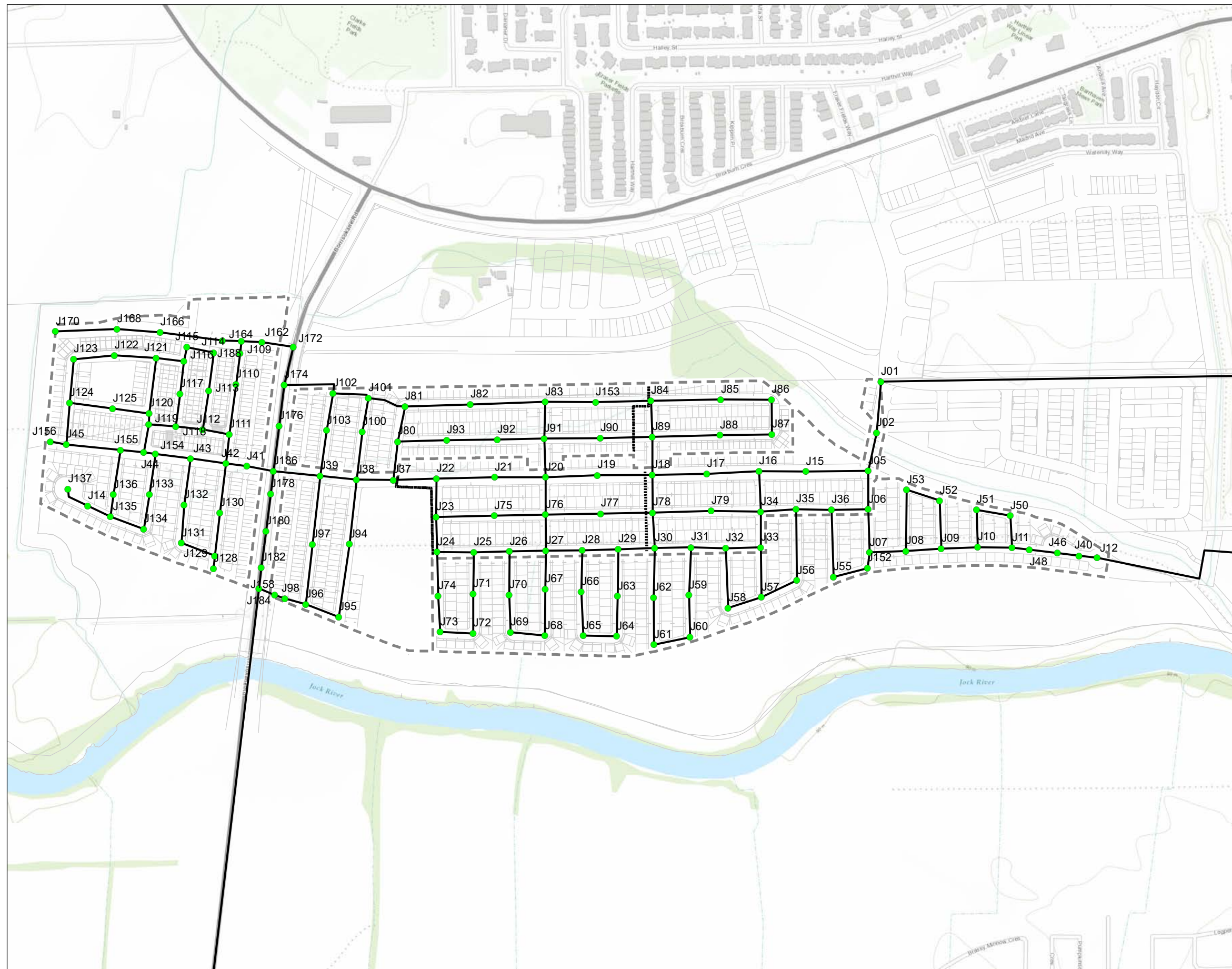
June 2, 2022

Appendix C JUNCTION IDS

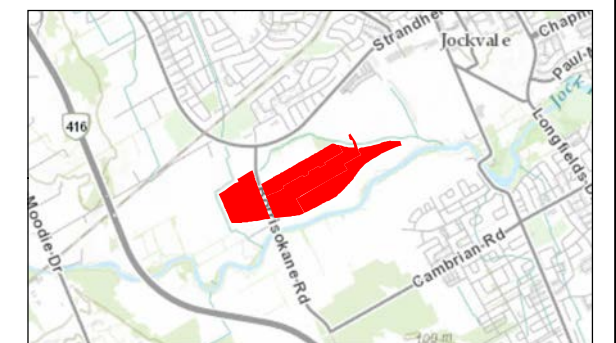


Legend

- Development Phase Boundary
- Development Sub-Phase Boundary
- Property Line
- Future Watermain
- Model Junction



Notes
 1. Coordinate System: NAD 1983 CSRS MTM 9
 2. Background: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



Project Location
 Ottawa, ON

Client/Project
 David Schaeffer Engineering Ltd
 Barrhaven Conservancy East (Phases 2, 3, 4 & Jock River)
 Water Distribution System Analysis

Figure No.

C1

Title

Junction IDs

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

June 2, 2022

Appendix D MODEL RESULTS



Option A - 2 Connections - Phase 2 (2A, 2B, 2C)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
<i>Maximum</i>	0.11	150.00	81.74	<i>Maximum</i>	0.11	149.50	81.23	<i>Maximum</i>	0.11	150.00	81.73
<i>Minimum</i>	0.00	150.00	80.32	<i>Minimum</i>	0.00	149.50	79.61	<i>Minimum</i>	0.00	149.99	80.31
J01	0.07	150.00	80.32	J01	0.07	149.50	79.61	J01	0.07	150.00	80.31
J02	0.00	150.00	80.93	J02	0.00	149.50	80.22	J02	0.00	150.00	80.92
J05	0.07	150.00	81.43	J05	0.07	149.50	80.72	J05	0.07	150.00	81.42
J06	0.07	150.00	81.57	J06	0.07	149.50	80.86	J06	0.07	150.00	81.56
J07	0.11	150.00	81.73	J07	0.11	149.50	81.01	J07	0.11	150.00	81.72
J08	0.11	150.00	81.60	J08	0.11	149.50	80.89	J08	0.11	150.00	81.59
J09	0.11	150.00	81.59	J09	0.11	149.50	80.38	J09	0.11	150.00	81.08
J10	0.11	150.00	81.34	J10	0.11	149.50	80.63	J10	0.11	150.00	81.34
J11	0.11	150.00	81.23	J11	0.11	149.50	80.52	J11	0.11	150.00	81.22
J12	0.11	150.00	81.09	J12	0.11	149.50	80.38	J12	0.11	150.00	81.08
J15	0.07	150.00	81.28	J15	0.07	149.50	80.57	J15	0.07	149.99	81.28
J158	0.00	150.00	81.74	J158	0.00	149.50	81.03	J158	0.00	149.99	81.73
J16	0.07	150.00	81.44	J16	0.07	149.50	80.73	J16	0.07	149.99	81.43
J17	0.07	150.00	81.27	J17	0.07	149.50	80.56	J17	0.07	149.99	81.26
J178	0.08	150.00	81.36	J178	0.08	149.50	80.64	J178	0.08	149.99	81.35
J18	0.07	150.00	81.08	J18	0.07	149.50	80.35	J18	0.07	149.99	81.05
J180	0.08	150.00	81.57	J180	0.08	149.50	80.86	J180	0.08	149.99	81.56
J182	0.08	150.00	81.69	J182	0.08	149.50	80.97	J182	0.08	149.99	81.68
J184	0.08	150.00	81.74	J184	0.08	149.50	81.03	J184	0.08	149.99	81.73
J186	0.08	150.00	81.26	J186	0.08	149.50	80.54	J186	0.08	149.99	81.25
J19	0.10	150.00	80.84	J19	0.10	149.50	80.13	J19	0.10	149.99	80.84
J20	0.10	150.00	81.31	J20	0.10	149.50	80.60	J20	0.10	149.99	81.31
J21	0.10	150.00	81.16	J21	0.10	149.50	80.45	J21	0.10	149.99	81.15
J22	0.10	150.00	81.21	J22	0.10	149.50	80.50	J22	0.10	149.99	81.21
J23	0.10	150.00	81.34	J23	0.10	149.50	80.63	J23	0.10	149.99	81.33
J24	0.10	150.00	81.47	J24	0.10	149.50	80.76	J24	0.10	149.99	81.46
J25	0.10	150.00	81.36	J25	0.10	149.50	80.64	J25	0.10	149.99	81.35
J26	0.10	150.00	81.45	J26	0.10	149.50	80.74	J26	0.10	149.99	81.45
J27	0.10	150.00	81.58	J27	0.10	149.50	80.87	J27	0.10	149.99	81.58
J28	0.10	150.00	81.51	J28	0.10	149.50	80.80	J28	0.10	149.99	81.51
J29	0.10	150.00	81.51	J29	0.10	149.50	80.80	J29	0.10	149.99	81.51
J30	0.07	150.00	81.44	J30	0.07	149.50	80.73	J30	0.07	149.99	81.43
J31	0.07	150.00	81.57	J31	0.07	149.50	80.86	J31	0.07	149.99	81.56
J32	0.07	150.00	81.47	J32	0.07	149.50	80.76	J32	0.07	149.99	81.46
J33	0.07	150.00	81.70	J33	0.07	149.50	80.99	J33	0.07	149.99	81.69
J34	0.07	150.00	81.57	J34	0.07	149.50	80.86	J34	0.07	149.99	81.56
J35	0.07	150.00	81.53	J35	0.07	149.50	80.82	J35	0.07	149.99	81.52
J36	0.07	150.00	81.65	J36	0.07	149.50	80.94	J36	0.07	149.99	81.65
J37	0.08	150.00	81.13	J37	0.08	149.50	80.42	J37	0.08	149.99	81.12
J38	0.08	150.00	81.26	J38	0.08	149.50	80.54	J38	0.08	149.99	81.25
J39	0.08	150.00	81.29	J39	0.08	149.50	80.54	J39	0.08	149.99	81.25
J40	0.11	150.00	81.12	J40	0.11	149.50	80.41	J40	0.11	150.00	81.11
J46	0.11	150.00	81.16	J46	0.11	149.50	80.45	J46	0.11	150.00	81.16
J48	0.11	150.00	81.20	J48	0.11	149.50	80.49	J48	0.11	150.00	81.20
J75	0.10	150.00	81.28	J75	0.10	149.50	80.57	J75	0.10	149.99	81.28
J76	0.10	150.00	81.45	J76	0.10	149.50	80.74	J76	0.10	149.99	81.45
J77	0.10	150.00	81.17	J77	0.10	149.50	80.46	J77	0.10	149.99	81.16
J78	0.07	150.00	81.33	J78	0.07	149.50	80.62	J78	0.07	149.99	81.32
J79	0.07	150.00	81.41	J79	0.07	149.50	80.70	J79	0.07	149.99	81.41
J94	0.08	150.00	81.51	J94	0.08	149.50	80.80	J94	0.08	149.99	81.50
J95	0.08	150.00	81.74	J95	0.08	149.50	81.03	J95	0.08	149.99	81.73
J96	0.08	150.00	81.61	J96	0.08	149.50	80.90	J96	0.08	149.99	81.60
J97	0.08	150.00	81.37	J97	0.08	149.50	80.66	J97	0.08	149.99	81.36
J98	0.00	150.00	81.54	J98	0.00	149.50	80.83	J98	0.00	149.99	81.53

Option A - 2 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.15	150.00	81.73	Maximum	0.15	149.50	81.62	Maximum	0.15	149.99	81.71
Minimum	0.00	149.99	80.32	Minimum	0.00	149.49	79.61	Minimum	0.00	149.98	80.30
J01	0.07	150.00	80.32	J01	0.07	149.50	79.61	J01	0.07	149.99	80.30
J02	0.00	150.00	80.93	J02	0.00	149.50	80.22	J02	0.00	149.99	80.91
J05	0.07	150.00	81.42	J05	0.07	149.50	80.71	J05	0.07	149.99	81.41
J06	0.07	150.00	81.57	J06	0.07	149.50	80.86	J06	0.07	149.99	81.55
J07	0.11	150.00	81.72	J07	0.11	149.50	81.01	J07	0.11	149.99	81.71
J08	0.11	150.00	81.60	J08	0.11	149.50	80.88	J08	0.11	149.99	81.58
J09	0.11	150.00	81.98	J09	0.11	149.50	80.37	J09	0.11	149.99	81.07
J10	0.11	150.00	81.34	J10	0.11	149.50	80.63	J10	0.11	149.99	81.33
J100	0.15	149.99	80.80	J100	0.15	149.49	80.08	J100	0.15	149.98	80.78
J101	0.15	149.99	80.85	J101	0.15	149.49	80.24	J101	0.15	149.98	80.93
J102	0.15	149.99	80.84	J102	0.15	149.49	80.13	J102	0.15	149.98	80.82
J103	0.15	149.99	80.68	J103	0.15	149.49	79.97	J103	0.15	149.98	80.66
J11	0.11	150.00	81.23	J11	0.11	149.50	80.52	J11	0.11	149.99	81.22
J12	0.11	150.00	81.09	J12	0.11	149.50	80.31	J12	0.11	149.99	81.08
J15	0.07	150.00	81.28	J15	0.07	149.50	80.57	J15	0.07	149.98	81.26
J153	0.15	149.99	80.77	J153	0.15	149.49	80.06	J153	0.15	149.98	80.75
J158	0.00	149.99	81.73	J158	0.00	149.49	81.02	J158	0.00	149.98	81.71
J16	0.07	150.00	81.44	J16	0.07	149.50	80.73	J16	0.07	149.98	81.42
J17	0.07	150.00	81.27	J17	0.07	149.50	80.56	J17	0.07	149.98	81.25
J174	0.15	149.99	81.02	J174	0.15	149.49	80.31	J174	0.15	149.98	81.00
J176	0.15	149.99	81.14	J176	0.15	149.49	80.43	J176	0.15	149.98	81.12
J178	0.08	149.99	81.35	J178	0.08	149.49	80.64	J178	0.08	149.98	81.33
J18	0.07	150.00	81.05	J18	0.07	149.50	80.34	J18	0.07	149.98	81.03
J180	0.08	149.99	81.56	J180	0.08	149.49	80.85	J180	0.08	149.98	81.54
J182	0.08	149.99	81.68	J182	0.08	149.49	80.97	J182	0.08	149.98	81.66
J184	0.08	149.99	81.73	J184	0.08	149.49	81.02	J184	0.08	149.98	81.71
J186	0.08	149.99	81.25	J186	0.08	149.49	80.54	J186	0.08	149.98	81.23
J19	0.10	150.00	80.84	J19	0.10	149.50	80.13	J19	0.10	149.98	80.82
J20	0.10	150.00	81.31	J20	0.10	149.50	80.60	J20	0.10	149.98	81.29
J21	0.10	150.00	81.15	J21	0.10	149.50	80.44	J21	0.10	149.98	81.13
J22	0.10	150.00	81.21	J22	0.10	149.50	80.50	J22	0.10	149.98	81.19
J23	0.10	150.00	81.34	J23	0.10	149.50	80.63	J23	0.10	149.98	81.32
J24	0.10	150.00	81.46	J24	0.10	149.50	80.75	J24	0.10	149.98	81.44
J25	0.10	150.00	81.35	J25	0.10	149.50	80.64	J25	0.10	149.98	81.33
J26	0.10	150.00	81.45	J26	0.10	149.50	80.74	J26	0.10	149.98	81.43
J27	0.10	150.00	81.58	J27	0.10	149.50	80.87	J27	0.10	149.98	81.56
J28	0.10	150.00	81.51	J28	0.10	149.50	80.80	J28	0.10	149.98	81.49
J29	0.10	150.00	81.51	J29	0.10	149.50	80.80	J29	0.10	149.98	81.49
J30	0.07	150.00	81.44	J30	0.07	149.50	80.73	J30	0.07	149.98	81.42
J31	0.07	150.00	81.56	J31	0.07	149.50	80.85	J31	0.07	149.98	81.55
J32	0.07	150.00	81.47	J32	0.07	149.50	80.75	J32	0.07	149.98	81.45
J33	0.07	150.00	81.69	J33	0.07	149.50	80.98	J33	0.07	149.98	81.68
J34	0.07	150.00	81.57	J34	0.07	149.50	80.85	J34	0.07	149.98	81.55
J35	0.07	150.00	81.52	J35	0.07	149.50	80.81	J35	0.07	149.98	81.51
J36	0.07	150.00	81.65	J36	0.07	149.50	80.94	J36	0.07	149.98	81.64
J37	0.08	149.99	81.12	J37	0.08	149.49	80.41	J37	0.08	149.98	81.10
J38	0.08	149.99	81.25	J38	0.08	149.49	80.54	J38	0.08	149.98	81.23
J39	0.08	149.99	81.25	J39	0.08	149.49	80.54	J39	0.08	149.98	81.23
J40	0.11	150.00	81.12	J40	0.11	149.50	80.40	J40	0.11	149.99	81.11
J46	0.11	150.00	81.16	J46	0.11	149.50	80.45	J46	0.11	149.99	81.15
J48	0.11	150.00	81.20	J48	0.11	149.50	80.49	J48	0.11	149.99	81.19
J75	0.10	150.00	81.28	J75	0.10	149.50	80.57	J75	0.10	149.98	81.26
J76	0.10	150.00	81.45	J76	0.10	149.50	80.74	J76	0.10	149.98	81.43
J77	0.10	150.00	81.17	J77	0.10	149.50	80.46	J77	0.10	149.98	81.15
J78	0.07	150.00	81.32	J78	0.07	149.50	80.61	J78	0.07	149.98	81.30
J79	0.07	150.00	81.41	J79	0.07	149.50	80.70	J79	0.07	149.98	81.39
J80	0.15	149.99	80.98	J80	0.15	149.49	80.27	J80	0.15	149.98	80.96
J81	0.15	149.99	80.80	J81	0.15	149.49	80.08	J81	0.15	149.98	80.78
J82	0.15	149.99	80.84	J82	0.15	149.49	80.13	J82	0.15	149.98	80.82
J83	0.15	149.99	81.05	J83	0.15	149.49	80.34	J83	0.15	149.98	81.03
J84	0.15	150.00	80.92	J84	0.15	149.50	80.21	J84	0.15	149.98	80.90
J85	0.15	149.99	81.02	J85	0.15	149.49	80.31	J85	0.15	149.98	81.00
J86	0.15	149.99	81.24	J86	0.15	149.49	80.53	J86	0.15	149.98	81.22
J87	0.15	149.99	81.19	J87	0.15	149.49	80.48	J87	0.15	149.98	81.17
J88	0.15	149.99	81.05	J88	0.15	149.49	80.34	J88	0.15	149.98	81.03
J89	0.15	150.00	81.05	J89	0.15	149.50	80.34	J89	0.15	149.98	81.03
J90	0.15	150.00	80.88	J90	0.15	149.50	80.17	J90	0.15	149.98	80.86
J91	0.15	149.99	81.18	J91	0.15	149.49	80.47	J91	0.15	149.98	81.16
J92	0.15	149.99	80.99	J92	0.15	149.49	80.29	J92	0.15	149.98	80.98
J93	0.15	149.99	80.88	J93	0.15	149.49	80.17	J93	0.15	149.98	80.86
J94	0.08	149.99	81.51	J94	0.08	149.49	80.80	J94	0.08	149.98	81.49
J95	0.08	149.99	81.73	J95	0.08	149.49	81.02	J95	0.08	149.98	81.71
J96	0.08	149.99	81.61	J96	0.08	149.49	80.90	J96	0.08	149.98	81.59
J97	0.08	149.99	81.36	J97	0.08	149.49	80.65	J97	0.08	149.98	81.34
J98	0.00	149.99	81.53	J98	0.00	149.49	80.82	J98	0.00	149.98	81.52

Option A - 2 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4				Option B - 3 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.15	150.00	81.72	Maximum	0.15	149.50	81.71	Maximum	0.15	149.49	81.00
Minimum	0.00	149.99	80.31	Minimum	0.00	149.49	79.60	Minimum	0.00	149.48	79.59
J01	0.07	149.99	80.31	J01	0.07	149.49	79.60	J01	0.07	149.49	79.59
J02	0.00	149.99	80.92	J02	0.00	149.49	80.21	J02	0.00	149.49	80.20
J05	0.07	149.99	81.42	J05	0.07	149.49	80.72	J05	0.07	149.49	80.70
J06	0.07	149.99	81.56	J06	0.07	149.49	80.85	J06	0.07	149.49	80.84
J07	0.11	149.99	81.72	J07	0.11	149.49	81.01	J07	0.11	149.49	81.00
J08	0.11	149.99	81.59	J08	0.11	149.49	80.88	J08	0.11	149.49	80.87
J09	0.11	150.00	81.08	J09	0.11	149.50	80.37	J09	0.11	149.49	80.36
J10	0.11	150.00	81.34	J10	0.11	149.50	80.63	J10	0.11	149.49	80.62
J100	0.15	149.99	80.79	J100	0.15	149.49	80.07	J100	0.15	149.48	80.06
J101	0.15	149.99	80.84	J101	0.15	149.49	80.23	J101	0.15	149.48	80.22
J102	0.15	149.99	80.83	J102	0.15	149.49	80.12	J102	0.15	149.48	80.11
J103	0.15	149.99	80.67	J103	0.15	149.49	79.96	J103	0.15	149.48	79.95
J109	0.11	149.99	80.96	J109	0.11	149.49	80.24	J109	0.11	149.48	80.23
J111	0.11	150.00	81.22	J111	0.11	149.50	80.51	J111	0.11	149.48	80.50
J110	0.11	149.99	81.33	J110	0.11	149.49	80.42	J110	0.11	149.48	80.40
J111	0.11	149.99	81.20	J111	0.11	149.49	80.49	J111	0.11	149.48	80.47
J112	0.11	149.99	81.10	J112	0.11	149.49	80.39	J112	0.11	149.48	80.37
J113	0.11	149.99	81.01	J113	0.11	149.49	80.30	J113	0.11	149.48	80.29
J114	0.11	149.99	80.87	J114	0.11	149.49	80.16	J114	0.11	149.48	80.15
J115	0.11	149.99	80.84	J115	0.11	149.49	80.13	J115	0.11	149.48	80.12
J116	0.11	149.99	80.88	J116	0.11	149.49	80.17	J116	0.11	149.48	80.16
J117	0.11	149.99	81.01	J117	0.11	149.49	80.35	J117	0.11	149.48	80.34
J118	0.11	149.99	81.10	J118	0.11	149.49	80.39	J118	0.11	149.48	80.37
J119	0.11	149.99	81.23	J119	0.11	149.49	80.51	J119	0.11	149.48	80.50
J12	0.11	150.00	81.08	J12	0.11	149.50	80.37	J12	0.11	149.49	80.37
J120	0.11	149.99	81.23	J120	0.11	149.49	80.31	J120	0.11	149.48	80.30
J121	0.11	149.99	81.03	J121	0.11	149.49	80.32	J121	0.11	149.48	80.31
J122	0.11	149.99	80.87	J122	0.11	149.49	80.16	J122	0.11	149.48	80.15
J123	0.11	149.99	81.01	J123	0.11	149.49	80.30	J123	0.11	149.48	80.29
J124	0.11	149.99	81.15	J124	0.11	149.49	80.44	J124	0.11	149.48	80.43
J125	0.11	149.99	81.34	J125	0.11	149.49	80.63	J125	0.11	149.48	80.62
J128	0.00	149.99	81.69	J128	0.00	149.49	80.98	J128	0.00	149.48	80.97
J129	0.11	149.99	81.67	J129	0.11	149.49	80.96	J129	0.11	149.48	80.94
J130	0.11	149.99	81.45	J130	0.11	149.49	80.74	J130	0.11	149.48	80.73
J131	0.11	149.99	81.57	J131	0.11	149.49	80.86	J131	0.11	149.48	80.84
J132	0.11	149.99	81.41	J132	0.11	149.49	80.70	J132	0.11	149.48	80.69
J133	0.11	149.99	81.50	J133	0.11	149.49	80.78	J133	0.11	149.48	80.77
J134	0.11	149.99	81.62	J134	0.11	149.49	80.91	J134	0.11	149.48	80.90
J135	0.11	149.99	81.72	J135	0.11	149.49	81.01	J135	0.11	149.48	81.00
J136	0.11	149.99	81.59	J136	0.11	149.49	80.88	J136	0.11	149.48	80.87
J137	0.11	149.99	81.51	J137	0.11	149.49	80.80	J137	0.11	149.48	80.79
J14	0.11	149.99	81.63	J14	0.11	149.49	80.92	J14	0.11	149.48	80.91
J15	0.07	149.99	81.28	J15	0.07	149.49	80.57	J15	0.07	149.48	80.55
J153	0.15	149.99	80.76	J153	0.15	149.49	80.05	J153	0.15	149.48	80.04
J154	0.11	149.99	81.30	J154	0.11	149.49	80.64	J154	0.11	149.48	80.63
J155	0.11	149.99	81.47	J155	0.11	149.49	80.75	J155	0.11	149.48	80.74
J156	0.00	149.99	81.23	J156	0.00	149.49	80.51	J156	0.00	149.48	80.50
J158	0.00	149.99	81.72	J158	0.00	149.49	81.01	J158	0.00	149.48	81.00
J16	0.07	149.99	81.43	J16	0.07	149.49	80.72	J16	0.07	149.48	80.71
J162	0.00	149.99	80.84	J162	0.00	149.49	80.13	J162	0.00	149.48	80.12
J164	0.00	149.99	80.91	J164	0.00	149.49	80.20	J164	0.00	149.48	80.19
J166	0.00	149.99	80.73	J166	0.00	149.49	80.02	J166	0.00	149.48	80.01
J168	0.00	149.99	81.00	J168	0.00	149.49	80.30	J168	0.00	149.48	80.29
J17	0.07	149.99	81.26	J17	0.07	149.49	80.55	J17	0.07	149.48	80.54
J170	0.00	149.99	80.50	J170	0.00	149.49	79.79	J170	0.00	149.48	79.78
J172	0.00	149.99	80.84	J172	0.00	149.49	80.13	J172	0.00	149.48	80.12
J174	0.15	149.99	81.01	J174	0.15	149.49	80.39	J174	0.15	149.48	80.39
J176	0.15	149.99	81.13	J176	0.15	149.49	80.42	J176	0.15	149.48	80.40
J178	0.08	149.99	81.34	J178	0.08	149.49	80.63	J178	0.08	149.48	80.62
J18	0.07	149.99	81.04	J18	0.07	149.49	80.33	J18	0.07	149.48	80.32
J180	0.08	149.99	81.55	J180	0.08	149.49	80.84	J180	0.08	149.48	80.83
J182	0.08	149.99	81.67	J182	0.08	149.49	80.96	J182	0.08	149.48	80.95
J184	0.08	149.99	81.72	J184	0.08	149.49	81.01	J184	0.08	149.48	81.00
J188	0.08	149.99	81.24	J188	0.08	149.49	80.53	J188	0.08	149.48	80.52
J188	0.00	149.99	80.88	J188	0.00	149.49	80.17	J188	0.00	149.48	80.16
J19	0.10	149.99	80.83	J19	0.10	149.49	80.12	J19	0.10	149.48	80.11
J20	0.10	149.99	81.30	J20	0.10	149.49	80.59	J20	0.10	149.48	80.58
J21	0.10	149.99	81.14	J21	0.10	149.49	80.43	J21	0.10	149.48	80.42
J22	0.10	149.99	81.20	J22	0.10	149.49	80.49	J22	0.10	149.48	80.48
J23	0.10	149.99	81.33	J23	0.10	149.49	80.62	J23	0.10	149.48	80.60
J24	0.10	149.99	81.46	J24	0.10	149.49	80.74	J24	0.10	149.48	80.73
J25	0.10	149.99	81.34	J25	0.10	149.49	80.61	J25	0.10	149.48	80.60
J26	0.10	149.99	81.44	J26	0.10	149.49	80.73	J26	0.10	149.48	80.72
J27	0.10	149.99	81.57	J27	0.10	149.49	80.86	J27	0.10	149.48	80.85
J28	0.10	149.99	81.50	J28	0.10	149.49	80.79	J28	0.10	149.48	80.78
J29	0.10	149.99	81.50	J29	0.10	149.49	80.78	J29	0.10	149.48	80.78
J30	0.07	149.99	81.43	J30	0.07	149.49	80.72	J30	0.07	149.48	80.71
J31	0.07	149.99	81.56	J31	0.07	149.49	80.85	J31	0.07	149.48	80.83
J32	0.07	149.99	81.46	J32	0.07	149.49	80.75	J32	0.07	149.48	80.74
J33	0.07	149.99	81.69	J33	0.07	149.49	80.98	J33	0.07	149.48	80.96
J34	0.07	149.99	81.56	J34	0.07	149.49	80.85	J34	0.07	149.48	80.84
J35	0.07	149.99	81.52	J35	0.07	149.49	80.81	J35	0.07	149.48	80.79
J36	0.07	149.99	81.65	J36	0.07	149.49	80.94	J36	0.07	149.48	80.92
J37	0.08	149.99	81.11	J37	0.08	149.49	80.40	J37	0.08	149.48	80.39
J38	0.08	149.99	81.24	J38	0.08	149.49	80.53	J38	0.08	149.48	80.52
J39	0.08	149.99	81.24	J39	0.08	149.49	80.53	J39	0.08	149.48	80.52
J40	0.11	150.00	81.11	J40	0.11	149.50	80.40	J40	0.11	149.49	80.40
J41	0.11	149.99	81.30	J41	0.11	149.49	80.59	J41	0.11	149.48	80.57
J42	0.11	149.99	81.34	J42	0.11	149.49	80.63	J42	0.11	149.48	80.62
J4											

Option A - 2 Connections - Buildout (Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4 + Jock River)				Option A - 2 Connections with Upgrades on Danson Gardens Drv - Buildout (Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4 + Jock River)				Option B - 3 Connections - Buildout (Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4 + Jock River)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	150.00	81.97	0.15	Maximum	149.50	81.95	0.15	Maximum	149.50	81.95	0.15
Minimum	0.00	79.60	0.00	Minimum	0.00	79.60	0.00	Minimum	0.00	79.60	0.00
J01	0.07	149.99	80.31	J01	0.07	149.49	79.60	J01	0.07	149.48	79.58
J02	0.00	149.99	80.92	J02	0.00	149.49	80.21	J02	0.00	149.48	80.19
J05	0.07	149.99	81.42	J05	0.07	149.49	80.77	J05	0.07	149.48	80.69
J06	0.07	149.99	81.56	J06	0.07	149.49	80.85	J06	0.07	149.48	80.83
J07	0.11	149.99	81.72	J07	0.11	149.49	81.00	J07	0.11	149.48	80.99
J08	0.11	149.99	81.59	J08	0.11	149.49	80.88	J08	0.11	149.48	80.86
J09	0.11	149.99	81.66	J09	0.11	149.49	80.97	J09	0.11	149.48	80.95
J10	0.11	149.99	81.33	J10	0.11	149.49	80.62	J10	0.11	149.48	80.61
J100	0.15	149.98	80.78	J100	0.15	149.48	80.07	J100	0.15	149.47	80.05
J101	0.15	149.98	80.94	J101	0.15	149.48	80.23	J101	0.15	149.47	80.21
J102	0.15	149.98	80.82	J102	0.15	149.48	80.11	J102	0.15	149.47	80.10
J103	0.15	149.98	80.67	J103	0.15	149.48	79.96	J103	0.15	149.47	79.94
J109	0.11	149.98	80.95	J109	0.11	149.48	80.24	J109	0.11	149.47	80.22
J11	0.11	149.99	81.22	J11	0.11	149.49	80.51	J11	0.11	149.48	80.50
J110	0.11	149.98	81.32	J110	0.11	149.48	80.61	J110	0.11	149.47	80.59
J111	0.11	149.98	81.19	J111	0.11	149.48	80.48	J111	0.11	149.47	80.46
J112	0.11	149.98	81.09	J112	0.11	149.48	80.38	J112	0.11	149.47	80.36
J113	0.11	149.98	81.01	J113	0.11	149.48	80.30	J113	0.11	149.47	80.29
J114	0.11	149.98	80.86	J114	0.11	149.48	80.15	J114	0.11	149.47	80.14
J115	0.11	149.98	80.84	J115	0.11	149.48	80.12	J115	0.11	149.47	80.11
J116	0.11	149.98	80.88	J116	0.11	149.48	80.17	J116	0.11	149.47	80.15
J117	0.11	149.98	81.01	J117	0.11	149.48	80.31	J117	0.11	149.47	80.29
J118	0.11	149.98	81.09	J118	0.11	149.48	80.38	J118	0.11	149.47	80.36
J119	0.11	149.98	81.22	J119	0.11	149.48	80.51	J119	0.11	149.47	80.49
J12	0.11	150.00	81.08	J12	0.11	149.50	80.37	J12	0.11	149.49	80.36
J120	0.11	149.98	81.22	J120	0.11	149.48	80.44	J120	0.11	149.47	80.42
J121	0.11	149.98	81.02	J121	0.11	149.48	80.31	J121	0.11	149.47	80.29
J122	0.11	149.98	80.86	J122	0.11	149.48	80.15	J122	0.11	149.47	80.14
J123	0.11	149.98	81.01	J123	0.11	149.48	80.30	J123	0.11	149.47	80.28
J124	0.11	149.98	81.15	J124	0.11	149.48	80.44	J124	0.11	149.47	80.42
J125	0.11	149.98	81.33	J125	0.11	149.48	80.62	J125	0.11	149.47	80.61
J128	0.00	149.98	81.69	J128	0.00	149.48	80.98	J128	0.00	149.47	80.96
J129	0.11	149.98	81.66	J129	0.11	149.48	80.95	J129	0.11	149.47	80.93
J130	0.11	149.98	81.45	J130	0.11	149.48	80.74	J130	0.11	149.47	80.72
J131	0.11	149.98	81.56	J131	0.11	149.48	80.85	J131	0.11	149.47	80.83
J132	0.11	149.98	81.40	J132	0.11	149.48	80.69	J132	0.11	149.47	80.68
J133	0.11	149.98	81.49	J133	0.11	149.48	80.78	J133	0.11	149.47	80.76
J134	0.11	149.98	81.62	J134	0.11	149.48	80.91	J134	0.11	149.47	80.89
J135	0.11	149.98	81.72	J135	0.11	149.48	81.01	J135	0.11	149.47	80.99
J136	0.11	149.98	81.59	J136	0.11	149.48	80.88	J136	0.11	149.47	80.86
J137	0.11	149.98	81.50	J137	0.11	149.48	80.79	J137	0.11	149.47	80.78
J14	0.11	149.98	81.63	J14	0.11	149.48	80.91	J14	0.11	149.47	80.90
J15	0.07	149.99	81.27	J15	0.07	149.49	80.56	J15	0.07	149.48	80.54
J152	0.11	149.99	81.76	J152	0.11	149.49	81.05	J152	0.11	149.48	81.03
J153	0.15	149.98	80.75	J153	0.15	149.48	80.75	J153	0.15	149.47	80.73
J154	0.11	149.98	81.35	J154	0.11	149.48	80.64	J154	0.11	149.47	80.62
J155	0.11	149.98	81.46	J155	0.11	149.48	80.75	J155	0.11	149.47	80.73
J156	0.00	149.98	81.22	J156	0.00	149.48	80.51	J156	0.00	149.47	80.49
J158	0.00	149.98	81.71	J158	0.00	149.48	80.97	J158	0.00	149.47	80.95
J16	0.07	149.99	81.43	J16	0.07	149.49	80.72	J16	0.07	149.48	80.70
J162	0.00	149.98	80.84	J162	0.00	149.48	80.13	J162	0.00	149.47	80.11
J164	0.00	149.98	80.91	J164	0.00	149.48	80.20	J164	0.00	149.47	80.18
J168	0.00	149.98	80.92	J168	0.00	149.48	80.23	J168	0.00	149.47	80.20
J168	0.00	149.98	81.01	J168	0.00	149.48	80.30	J168	0.00	149.47	80.28
J17	0.07	149.99	81.25	J17	0.07	149.49	80.54	J17	0.07	149.47	80.53
J170	0.00	149.98	80.49	J170	0.00	149.48	79.78	J170	0.00	149.47	79.77
J172	0.00	149.98	80.36	J172	0.00	149.48	80.13	J172	0.00	149.47	80.11
J174	0.15	149.98	81.01	J174	0.15	149.48	80.30	J174	0.15	149.47	80.28
J176	0.15	149.98	81.12	J176	0.15	149.48	80.41	J176	0.15	149.47	80.39
J178	0.08	149.98	81.33	J178	0.08	149.48	80.62	J178	0.08	149.47	80.61
J18	0.07	149.99	81.04	J18	0.07	149.49	80.33	J18	0.07	149.47	80.31
J180	0.08	149.98	81.55	J180	0.08	149.48	80.84	J180	0.08	149.47	80.82
J182	0.08	149.98	81.66	J182	0.08	149.48	80.95	J182	0.08	149.47	80.94
J184	0.08	149.98	81.72	J184	0.08	149.48	81.01	J184	0.08	149.47	80.99
J186	0.08	149.98	81.23	J186	0.08	149.48	80.52	J186	0.08	149.47	80.51
J188	0.00	149.98	80.87	J188	0.00	149.48	80.16	J188	0.00	149.47	80.15
J19	0.10	149.99	80.82	J19	0.10	149.49	80.11	J19	0.10	149.47	80.10
J20	0.10	149.98	81.29	J20	0.10	149.48	80.58	J20	0.10	149.47	80.57
J21	0.10	149.98	81.14	J21	0.10	149.48	80.43	J21	0.10	149.47	80.41
J22	0.10	149.98	81.19	J22	0.10	149.48	80.48	J22	0.10	149.47	80.47
J23	0.10	149.98	81.32	J23	0.10	149.48	80.61	J23	0.10	149.47	80.59
J24	0.10	149.98	81.45	J24	0.10	149.48	80.74	J24	0.10	149.47	80.72
J25	0.10	149.98	81.34	J25	0.10	149.48	80.62	J25	0.10	149.47	80.61
J26	0.10	149.98	81.44	J26	0.10	149.48	80.72	J26	0.10	149.47	80.71
J27	0.10	149.98	81.56	J27	0.10	149.48	80.85	J27	0.10	149.47	80.84
J28	0.10	149.98	81.45	J28	0.10	149.48	80.78	J28	0.10	149.47	80.76
J29	0.10	149.99	81.40	J29	0.10	149.49	80.78	J29	0.10	149.47	80.77
J30	0.07	149.99	81.42	J30	0.07	149.49	80.71	J30	0.07	149.47	80.69
J31	0.07	149.99	81.55	J31	0.07	149.49	80.84	J31	0.07	149.47	80.82
J32	0.07	149.99	81.45	J32	0.07	149.49	80.74	J32	0.07	149.47	80.72
J33	0.07	149.99	81.68	J33	0.07	149.49	80.97	J33	0.07	149.48	80.95
J34	0.07	149.99	81.55	J34	0.07	149.49	80.84	J34	0.07	149.48	80.83
J35	0.07	149.99	81.51	J35	0.07	149.49	80.80	J35	0.07	149.48	80.78
J36	0.07	149.99	81.64	J36	0.07	149.49	80.93	J36	0.07	149.48	80.91
J37	0.08	149.98	81.11	J37	0.08	149.48	80.40	J37	0.08	149.47	80.38
J38	0.08	149.98	81.23	J38	0.08	149.48	80.52	J38	0.08	149.47	80.51
J39	0.08	149.98	81.23	J39	0.08	149.48	80.52	J39	0.08	149.47	80.51
J40	0.11	149.98	81.11	J40	0.11	149.48	80.40	J40	0.11	149.46	80.39
J41	0.11	149.98	81.29	J41	0.11	149.48	80.58	J41	0.11	149.47	80.56
J42	0.11										

Option A - 2 Connections - Phase 2 (2A, 2B, 2C)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.58	144.12	73.34	Maximum	0.58	144.09	72.31	Maximum	0.58	144.07	72.15
Minimum	0.00	144.09	71.96	Minimum	0.00	144.06	71.90	Minimum	0.00	144.93	71.81
J01	0.39	144.12	71.96	J01	0.39	144.08	71.90	J01	0.39	144.01	71.81
J02	0.00	144.11	72.56	J02	0.00	144.08	72.51	J02	0.00	143.99	72.38
J05	0.39	144.10	73.05	J05	0.39	144.08	73.01	J05	0.39	143.97	72.85
J06	0.39	144.10	73.18	J06	0.39	144.08	73.15	J06	0.39	143.96	72.99
J07	0.58	144.10	73.34	J07	0.58	144.08	73.31	J07	0.58	143.96	73.15
J08	0.58	144.10	73.21	J08	0.58	144.08	73.18	J08	0.58	143.97	73.02
J09	0.58	144.10	72.70	J09	0.58	144.08	72.67	J09	0.58	143.97	72.51
J10	0.58	144.10	72.95	J10	0.58	144.08	72.93	J10	0.58	143.97	72.77
J11	0.58	144.10	72.84	J11	0.58	144.08	72.82	J11	0.58	143.97	72.66
J12	0.58	144.10	72.70	J12	0.58	144.09	72.68	J12	0.58	143.98	72.52
J15	0.39	144.10	72.90	J15	0.39	144.07	72.86	J15	0.39	143.96	72.70
J158	0.00	144.09	73.33	J158	0.00	144.06	73.30	J158	0.00	143.93	73.11
J16	0.39	144.10	73.05	J16	0.39	144.07	73.01	J16	0.39	143.95	72.85
J17	0.39	144.09	72.86	J17	0.39	144.07	72.84	J17	0.39	143.95	72.67
J178	0.44	144.09	72.85	J178	0.44	144.06	72.91	J178	0.44	143.93	72.72
J18	0.39	144.09	72.66	J18	0.39	144.07	72.62	J18	0.39	143.94	72.45
J180	0.44	144.09	73.16	J180	0.44	144.06	73.13	J180	0.44	143.93	72.94
J182	0.44	144.09	73.26	J182	0.44	144.06	73.24	J182	0.44	143.93	73.05
J184	0.44	144.09	73.33	J184	0.44	144.06	73.30	J184	0.44	143.93	73.11
J186	0.44	144.09	72.85	J186	0.44	144.06	72.81	J186	0.44	143.93	72.62
J19	0.56	144.09	72.45	J19	0.56	144.06	72.41	J19	0.56	143.94	72.23
J20	0.56	144.09	72.91	J20	0.56	144.06	72.88	J20	0.56	143.93	72.69
J21	0.56	144.09	72.75	J21	0.56	144.06	72.72	J21	0.56	143.93	72.53
J22	0.56	144.09	72.81	J22	0.56	144.06	72.77	J22	0.56	143.93	72.59
J23	0.56	144.09	72.94	J23	0.56	144.06	72.90	J23	0.56	143.93	72.72
J24	0.56	144.09	73.07	J24	0.56	144.06	73.03	J24	0.56	143.93	72.84
J25	0.56	144.09	72.95	J25	0.56	144.06	72.92	J25	0.56	143.93	72.73
J26	0.56	144.09	73.05	J26	0.56	144.06	73.01	J26	0.56	143.93	72.83
J27	0.56	144.09	73.18	J27	0.56	144.06	73.14	J27	0.56	143.93	72.96
J28	0.56	144.09	73.11	J28	0.56	144.06	73.07	J28	0.56	143.94	72.89
J29	0.56	144.09	73.11	J29	0.56	144.06	73.07	J29	0.56	143.94	72.89
J30	0.39	144.09	73.04	J30	0.39	144.06	73.00	J30	0.39	143.94	72.83
J31	0.39	144.09	73.17	J31	0.39	144.06	73.13	J31	0.39	143.94	72.96
J32	0.39	144.09	73.07	J32	0.39	144.07	73.03	J32	0.39	143.94	72.86
J33	0.39	144.09	73.30	J33	0.39	144.07	73.26	J33	0.39	143.95	73.09
J34	0.39	144.09	73.17	J34	0.39	144.07	73.14	J34	0.39	143.95	72.97
J35	0.39	144.10	73.13	J35	0.39	144.07	73.10	J35	0.39	143.95	72.93
J36	0.39	144.10	73.27	J36	0.39	144.07	73.23	J36	0.39	143.96	73.07
J37	0.44	144.09	72.72	J37	0.44	144.06	72.69	J37	0.44	143.93	72.50
J38	0.44	144.09	72.85	J38	0.44	144.06	72.81	J38	0.44	143.93	72.62
J39	0.44	144.09	72.85	J39	0.44	144.06	72.81	J39	0.44	143.93	72.62
J40	0.58	144.10	72.73	J40	0.58	144.08	72.71	J40	0.58	143.97	72.55
J46	0.58	144.10	72.77	J46	0.58	144.08	72.75	J46	0.58	143.97	72.59
J48	0.58	144.10	72.81	J48	0.58	144.08	72.79	J48	0.58	143.97	72.63
J75	0.56	144.09	72.86	J75	0.56	144.06	72.84	J75	0.56	143.93	72.66
J76	0.56	144.09	73.05	J76	0.56	144.06	73.02	J76	0.56	143.93	72.83
J77	0.56	144.09	72.77	J77	0.56	144.06	72.73	J77	0.56	143.94	72.55
J78	0.39	144.09	72.93	J78	0.39	144.06	72.89	J78	0.39	143.94	72.71
J79	0.39	144.09	73.01	J79	0.39	144.07	72.98	J79	0.39	143.94	72.81
J94	0.44	144.09	73.11	J94	0.44	144.06	73.07	J94	0.44	143.93	72.88
J95	0.44	144.09	73.33	J95	0.44	144.06	73.30	J95	0.44	143.93	73.11
J96	0.44	144.09	73.21	J96	0.44	144.06	73.17	J96	0.44	143.93	72.98
J97	0.44	144.09	72.96	J97	0.44	144.06	72.93	J97	0.44	143.93	72.74
J98	0.00	144.09	73.14	J98	0.00	144.06	73.10	J98	0.00	143.93	72.91

Option A - 2 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)				Option B - 3 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E)			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.53	144.04	73.12	Maximum	0.53	144.05	72.25	Maximum	0.53	143.88	72.89
Minimum	0.00	143.94	71.84	Minimum	0.00	143.98	71.85	Minimum	0.00	143.64	71.58
J01	0.39	144.04	71.84	J01	0.39	144.04	71.85	J01	0.39	143.85	71.58
J02	0.00	144.01	72.42	J02	0.00	144.04	72.45	J02	0.00	143.81	72.13
J05	0.39	144.00	72.80	J05	0.39	144.03	72.94	J05	0.39	143.77	72.58
J06	0.39	144.00	73.04	J06	0.39	144.03	73.09	J06	0.39	143.77	72.71
J07	0.58	144.00	73.19	J07	0.58	144.04	73.25	J07	0.58	143.78	72.89
J08	0.58	144.00	73.06	J08	0.58	144.04	73.13	J08	0.58	143.80	72.78
J09	0.58	144.00	72.55	J09	0.58	144.04	72.65	J09	0.58	143.81	72.28
J10	0.58	144.00	72.81	J10	0.58	144.05	72.88	J10	0.58	143.82	72.56
J100	0.83	143.94	72.20	J100	0.83	143.98	72.24	J100	0.83	143.65	71.77
J101	0.83	143.94	72.35	J101	0.83	143.98	72.40	J101	0.83	143.65	71.93
J102	0.83	143.94	72.24	J102	0.83	143.98	72.29	J102	0.83	143.64	71.81
J103	0.83	143.94	72.08	J103	0.83	143.98	72.13	J103	0.83	143.64	71.65
J11	0.58	144.00	72.70	J11	0.58	144.05	72.77	J11	0.58	143.84	72.47
J12	0.58	144.00	72.55	J12	0.58	144.06	72.65	J12	0.58	143.86	72.38
J15	0.39	143.99	72.74	J15	0.39	144.02	72.78	J15	0.39	143.74	72.39
J153	0.83	143.95	72.17	J153	0.83	143.98	72.22	J153	0.83	143.65	71.75
J158	0.00	143.95	73.13	J158	0.00	143.98	73.18	J158	0.00	143.64	72.71
J16	0.39	143.98	72.86	J16	0.39	144.01	72.93	J16	0.39	143.72	72.51
J17	0.39	143.97	72.70	J17	0.39	144.00	72.74	J17	0.39	143.69	72.30
J174	0.83	143.94	72.42	J174	0.83	143.98	72.47	J174	0.83	143.64	72.00
J176	0.83	143.95	72.54	J176	0.83	143.98	72.59	J176	0.83	143.64	72.11
J178	0.44	143.95	72.75	J178	0.44	143.98	72.83	J178	0.44	143.64	72.32
J18	0.39	143.96	72.47	J18	0.39	143.99	72.52	J18	0.39	143.67	72.05
J180	0.44	143.95	72.96	J180	0.44	143.98	73.01	J180	0.44	143.64	72.54
J182	0.44	143.95	73.08	J182	0.44	143.98	73.13	J182	0.44	143.64	72.65
J184	0.44	143.95	72.81	J184	0.44	143.98	72.86	J184	0.44	143.64	72.71
J186	0.44	143.95	72.65	J186	0.44	143.98	72.70	J186	0.44	143.64	72.22
J19	0.56	143.95	72.25	J19	0.56	143.99	72.30	J19	0.56	143.66	71.83
J20	0.56	143.95	72.72	J20	0.56	143.99	72.77	J20	0.56	143.66	72.30
J21	0.56	143.95	72.55	J21	0.56	143.98	72.61	J21	0.56	143.66	72.14
J22	0.56	143.95	72.61	J22	0.56	143.98	72.66	J22	0.56	143.65	72.20
J23	0.56	143.95	72.74	J23	0.56	143.98	72.79	J23	0.56	143.66	72.33
J24	0.56	143.95	72.87	J24	0.56	143.98	72.92	J24	0.56	143.66	72.46
J25	0.56	143.95	72.76	J25	0.56	143.98	72.81	J25	0.56	143.66	72.35
J26	0.56	143.95	72.86	J26	0.56	143.99	72.91	J26	0.56	143.66	72.45
J27	0.56	143.95	72.99	J27	0.56	143.99	73.04	J27	0.56	143.67	72.58
J28	0.56	143.95	72.92	J28	0.56	143.99	72.97	J28	0.56	143.67	72.52
J29	0.56	143.96	72.92	J29	0.56	143.99	72.97	J29	0.56	143.68	72.52
J30	0.39	143.96	72.86	J30	0.39	143.99	72.90	J30	0.39	143.68	72.46
J31	0.39	143.96	72.99	J31	0.39	144.00	73.04	J31	0.39	143.69	72.60
J32	0.39	143.97	72.89	J32	0.39	144.00	72.94	J32	0.39	143.70	72.51
J33	0.39	143.97	73.13	J33	0.39	144.00	73.17	J33	0.39	143.71	72.75
J34	0.39	143.97	73.00	J34	0.39	144.01	73.05	J34	0.39	143.72	72.64
J35	0.39	143.98	72.97	J35	0.39	144.02	73.02	J35	0.39	143.73	72.62
J36	0.39	143.99	73.11	J36	0.39	144.02	73.16	J36	0.39	143.75	72.77
J37	0.44	143.95	72.53	J37	0.44	143.98	72.57	J37	0.44	143.65	72.10
J38	0.44	143.95	72.65	J38	0.44	143.98	72.70	J38	0.44	143.65	72.23
J39	0.44	143.95	72.65	J39	0.44	143.98	72.70	J39	0.44	143.65	72.22
J40	0.58	144.00	72.58	J40	0.58	144.06	72.68	J40	0.58	143.87	72.40
J46	0.58	144.00	72.63	J46	0.58	144.06	72.71	J46	0.58	143.86	72.43
J48	0.58	144.00	72.67	J48	0.58	144.05	72.78	J48	0.58	143.84	72.45
J75	0.56	143.95	72.69	J75	0.56	143.98	72.73	J75	0.56	143.66	72.27
J76	0.56	143.95	72.85	J76	0.56	143.99	72.91	J76	0.56	143.66	72.45
J77	0.56	143.95	72.58	J77	0.56	143.99	72.63	J77	0.56	143.67	72.17
J78	0.39	143.96	72.74	J78	0.39	143.99	72.79	J78	0.39	143.68	72.34
J79	0.39	143.97	72.84	J79	0.39	144.00	72.89	J79	0.39	143.70	72.45
J80	0.83	143.94	72.38	J80	0.83	143.98	72.43	J80	0.83	143.65	71.96
J81	0.83	143.95	72.20	J81	0.83	143.98	72.24	J81	0.83	143.65	71.77
J82	0.83	143.95	72.24	J82	0.83	143.98	72.29	J82	0.83	143.65	71.82
J83	0.83	143.95	72.45	J83	0.83	143.98	72.50	J83	0.83	143.65	72.03
J84	0.80	143.95	72.33	J84	0.80	143.98	72.38	J84	0.80	143.65	71.91
J85	0.80	143.95	72.42	J85	0.80	143.98	72.47	J85	0.80	143.65	72.01
J86	0.80	143.95	72.64	J86	0.80	143.98	72.69	J86	0.80	143.65	72.22
J87	0.80	143.95	72.60	J87	0.80	143.98	72.64	J87	0.80	143.65	72.18
J88	0.80	143.95	72.45	J88	0.80	143.98	72.50	J88	0.80	143.65	72.04
J89	0.80	143.95	72.46	J89	0.80	143.99	72.51	J89	0.80	143.66	72.04
J90	0.83	143.95	72.29	J90	0.83	143.98	72.33	J90	0.83	143.65	71.87
J91	0.83	143.95	72.56	J91	0.83	143.98	72.63	J91	0.83	143.65	72.16
J92	0.83	143.95	72.40	J92	0.83	143.98	72.44	J92	0.83	143.65	71.97
J93	0.83	143.94	72.28	J93	0.83	143.98	72.33	J93	0.83	143.65	71.86
J94	0.44	143.95	72.91	J94	0.44	143.98	72.96	J94	0.44	143.65	72.48
J95	0.44	143.95	73.13	J95	0.44	143.98	73.18	J95	0.44	143.64	72.71
J96	0.44	143.95	73.01	J96	0.44	143.98	73.05	J96	0.44	143.64	72.58
J97	0.44	143.95	72.76	J97	0.44	143.98	72.81	J97	0.44	143.64	72.34
J98	0.00	143.95	72.93	J98	0.00	143.98	72.98	J98	0.00	143.64	72.51

Option A - 2 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4				Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4				Option B - 3 Connections - Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4			
Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)	Junction ID	Demand (L/s)	Head (m)	Pressure (psi)
Maximum	0.83	143.89	73.23	Maximum	0.83	143.79	71.71	Maximum	0.83	143.84	72.34
Minimum	0.00	143.78	71.69	Minimum	0.00	143.79	71.71	Minimum	0.00	142.51	69.91
J01	0.39	143.99	71.77	J01	0.39	143.98	71.78	J01	0.39	143.49	71.07
J02	0.00	143.96	72.35	J02	0.00	143.97	72.35	J02	0.00	143.36	71.49
J05	0.39	143.94	72.91	J05	0.39	143.95	72.93	J05	0.39	143.26	71.84
J06	0.39	143.94	72.96	J06	0.39	143.96	72.98	J06	0.39	143.26	71.98
J07	0.58	143.94	73.12	J07	0.58	143.97	73.15	J07	0.58	143.34	72.26
J08	0.58	143.95	72.99	J08	0.58	143.97	73.03	J08	0.58	143.41	72.24
J09	0.58	143.95	72.66	J09	0.58	143.98	72.55	J09	0.58	143.49	71.83
J10	0.58	143.95	72.75	J10	0.58	143.99	72.80	J10	0.58	143.57	72.24
J100	0.83	143.80	71.99	J100	0.83	143.81	72.01	J100	0.83	142.59	70.26
J101	0.83	143.80	72.14	J101	0.83	143.81	72.16	J101	0.83	142.59	70.42
J102	0.83	143.79	72.02	J102	0.83	143.81	72.05	J102	0.83	142.57	70.28
J103	0.83	143.79	71.87	J103	0.83	143.81	71.89	J103	0.83	142.56	70.12
J109	0.59	143.79	72.14	J109	0.59	143.80	72.16	J109	0.59	142.53	70.36
J11	0.58	143.96	72.64	J11	0.58	144.00	72.70	J11	0.58	143.64	72.19
J110	0.59	143.79	72.31	J110	0.59	143.80	72.33	J110	0.59	142.53	70.53
J111	0.59	143.79	72.38	J111	0.59	143.80	72.40	J111	0.59	142.53	70.59
J112	0.59	143.78	72.27	J112	0.59	143.80	72.30	J112	0.59	142.52	70.48
J113	0.59	143.78	72.19	J113	0.59	143.79	72.21	J113	0.59	142.51	70.39
J114	0.59	143.78	72.04	J114	0.59	143.79	72.07	J114	0.59	142.51	70.24
J115	0.59	143.78	72.02	J115	0.59	143.79	72.04	J115	0.59	142.51	70.21
J116	0.59	143.78	72.06	J116	0.59	143.79	72.08	J116	0.59	142.51	70.26
J117	0.59	143.78	72.01	J117	0.59	143.79	72.03	J117	0.59	142.51	70.39
J118	0.59	143.78	72.27	J118	0.59	143.79	72.29	J118	0.59	142.51	70.47
J119	0.59	143.78	72.40	J119	0.59	143.79	72.42	J119	0.59	142.51	70.60
J12	0.58	143.97	72.51	J12	0.58	144.03	72.60	J12	0.58	143.84	72.34
J120	0.59	143.78	72.40	J120	0.59	143.79	72.42	J120	0.59	142.51	70.60
J121	0.59	143.78	72.20	J121	0.59	143.79	72.22	J121	0.59	142.51	70.49
J122	0.59	143.78	72.04	J122	0.59	143.79	72.07	J122	0.59	142.51	70.24
J123	0.59	143.78	72.19	J123	0.59	143.79	72.21	J123	0.59	142.51	70.38
J124	0.59	143.78	72.33	J124	0.59	143.79	72.35	J124	0.59	142.51	70.51
J125	0.59	143.78	72.51	J125	0.59	143.79	72.54	J125	0.59	142.51	70.71
J128	0.00	143.78	72.88	J128	0.00	143.80	72.90	J128	0.00	142.52	71.08
J129	0.59	143.78	72.85	J129	0.59	143.80	72.87	J129	0.59	142.52	71.05
J130	0.59	143.78	72.64	J130	0.59	143.80	72.66	J130	0.59	142.52	70.84
J131	0.59	143.78	72.75	J131	0.59	143.80	72.77	J131	0.59	142.52	70.95
J132	0.59	143.78	72.59	J132	0.59	143.80	72.61	J132	0.59	142.52	70.80
J133	0.59	143.78	72.67	J133	0.59	143.80	72.69	J133	0.59	142.51	70.87
J134	0.59	143.78	72.80	J134	0.59	143.79	72.82	J134	0.59	142.51	71.00
J135	0.59	143.78	72.90	J135	0.59	143.79	72.92	J135	0.59	142.51	71.09
J136	0.59	143.78	72.77	J136	0.59	143.80	72.79	J136	0.59	142.51	70.97
J137	0.59	143.78	72.69	J137	0.59	143.79	72.70	J137	0.59	142.51	70.88
J14	0.59	143.78	72.81	J14	0.59	143.79	72.83	J14	0.59	142.51	71.00
J15	0.39	143.91	72.63	J15	0.39	143.93	72.65	J15	0.39	143.12	71.51
J153	0.83	143.82	71.99	J153	0.83	143.83	72.01	J153	0.83	142.71	70.41
J154	0.59	143.78	72.51	J154	0.59	143.80	72.53	J154	0.59	142.52	70.73
J155	0.59	143.78	72.65	J155	0.59	143.80	72.67	J155	0.59	142.51	70.84
J156	0.00	143.78	72.40	J156	0.00	143.80	72.43	J156	0.00	142.51	70.60
J158	0.00	143.79	72.92	J158	0.00	143.81	72.94	J158	0.00	142.52	71.12
J16	0.39	143.89	72.40	J16	0.39	143.90	72.42	J16	0.39	143.02	71.52
J162	0.00	143.79	72.03	J162	0.00	143.80	72.05	J162	0.00	142.54	70.25
J164	0.00	143.79	72.10	J164	0.00	143.80	72.12	J164	0.00	142.54	70.32
J166	0.00	143.79	71.92	J166	0.00	143.80	71.94	J166	0.00	142.54	70.13
J168	0.00	143.79	72.09	J168	0.00	143.80	72.11	J168	0.00	142.54	70.42
J17	0.39	143.87	72.55	J17	0.39	143.88	72.57	J17	0.39	142.90	71.18
J170	0.00	143.79	71.69	J170	0.00	143.80	71.71	J170	0.00	142.54	69.91
J172	0.00	143.79	72.03	J172	0.00	143.81	72.05	J172	0.00	142.54	70.26
J174	0.83	143.79	72.40	J174	0.83	143.81	72.42	J174	0.83	142.55	70.43
J176	0.83	143.79	72.32	J176	0.83	143.81	72.34	J176	0.83	142.54	70.54
J178	0.44	143.79	72.53	J178	0.44	143.81	72.55	J178	0.44	142.54	70.75
J18	0.39	143.84	72.31	J18	0.39	143.86	72.33	J18	0.39	142.78	70.80
J180	0.44	143.79	72.15	J180	0.44	143.81	72.17	J180	0.44	142.53	70.95
J182	0.44	143.79	72.88	J182	0.44	143.81	72.88	J182	0.44	142.53	71.06
J184	0.44	143.79	72.92	J184	0.44	143.81	72.94	J184	0.44	142.52	71.11
J186	0.44	143.79	72.61	J186	0.44	143.81	72.65	J186	0.44	142.54	70.85
J188	0.00	143.79	72.07	J188	0.00	143.80	72.09	J188	0.00	142.54	70.29
J19	0.56	143.83	72.08	J19	0.56	143.85	72.10	J19	0.56	142.76	70.55
J20	0.56	143.83	72.54	J20	0.56	143.84	72.56	J20	0.56	142.73	70.98
J21	0.56	143.82	72.31	J21	0.56	143.84	72.40	J21	0.56	142.72	70.81
J22	0.56	143.81	72.42	J22	0.56	143.83	72.44	J22	0.56	142.71	70.84
J23	0.56	143.82	72.56	J23	0.56	143.83	72.58	J23	0.56	142.73	71.01
J24	0.56	143.82	72.69	J24	0.56	143.84	72.71	J24	0.56	142.74	71.16
J25	0.56	143.82	72.82	J25	0.56	143.84	72.84	J25	0.56	142.76	71.07
J26	0.56	143.83	72.68	J26	0.56	143.84	72.70	J26	0.56	142.78	71.19
J27	0.56	143.83	72.82	J27	0.56	143.85	72.84	J27	0.56	142.80	71.34
J28	0.56	143.84	72.75	J28	0.56	143.85	72.77	J28	0.56	142.82	71.31
J29	0.56	143.84	72.40	J29	0.56	143.86	72.42	J29	0.56	142.85	71.35
J30	0.39	143.85	72.70	J30	0.39	143.87	72.72	J30	0.39	142.88	71.31
J31	0.39	143.86	72.84	J31	0.39	143.87	72.86	J31	0.39	142.91	71.49
J32	0.39	143.87	72.75	J32	0.39	143.88	72.77	J32	0.39	142.95	71.44
J33	0.39	143.88	72.99	J33	0.39	143.89	73.01	J33	0.39	142.98	71.72
J34	0.39	143.89	72.88	J34	0.39	143.90	72.90	J34	0.39	143.02	71.65
J35	0.39	143.90	72.86	J35	0.39	143.92	72.88	J35	0.39	143.10	71.71
J36	0.39	143.92	73.01	J36	0.39	143.94	73.04	J36	0.39	143.17	71.95
J37	0.44	143.81	72.32	J37	0.44	143.82	72.35	J37	0.44	142.64	70.67
J38	0.44	143.80	72.44	J38	0.44	143.81	72.46	J38	0.44	142.59	70.72
J39	0.44	143.80	72.44	J39	0.44	143.81	72.46	J39	0.44	142.56	70.69
J40	0.58	143.97	72.54	J40	0.58	144.02	72.62	J40	0.58	143.80	72.30
J41	0.59	143.79	72.46	J41	0.59	143.80	72.51	J41	0.59	142.53	70.70
J42	0.59	143.79	72.52	J42	0.59	143.80	72.55	J42	0.59	142.53	70.73
J43	0.59	143.78									

Option A - 2 Connections - Buildout (Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4 + Jock River)
Table with columns: Junction ID, Demand (L/s), Head (m), Pressure (psi)
Rows J01 to J96

Option A - 2 Connections with Upgrades on Danson Gardens Drv - Buildout (Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4 + Jock River)
Table with columns: Junction ID, Demand (L/s), Head (m), Pressure (psi)
Rows J01 to J96

Option B - 3 Connections - Buildout (Phase 2 (2A, 2B, 2C) + Phase 3 (2D, 2E) + Phase 4 + Jock River)
Table with columns: Junction ID, Demand (L/s), Head (m), Pressure (psi)
Rows J01 to J96

Option A - 2 Connections - Phase 2 (2A, 2B, 2C)					Option A - 2 Connections with Upgrades on Danson Gardens Grv - Phase 2 (2A, 2B, 2C)					Option B - 3 Connections - Phase 2 (2A, 2B, 2C)				
Junction ID	Base Demand (MXDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)	Junction ID	Base Demand (MXDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)	Junction ID	Base Demand (MXDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)
J01	0.18	13,000	59.93	64,000	J01	0.18	13,000	62.43	66,000	J01	0.18	13,000	51.23	28,000
J02	0.00	13,000	60.51	64,000	J02	0.00	13,000	63.01	66,000	J02	0.00	13,000	51.51	28,000
J05	0.18	13,000	61.00	64,000	J05	0.18	13,000	63.51	66,000	J05	0.18	13,000	51.87	28,000
J06	0.18	13,000	61.17	64,000	J06	0.18	13,000	63.67	66,000	J06	0.18	13,000	52.01	28,000
J07	0.27	13,000	61.40	64,000	J07	0.27	13,000	63.90	66,000	J07	0.27	13,000	52.34	28,000
J08	0.27	13,000	61.35	65,000	J08	0.27	13,000	63.84	67,000	J08	0.27	13,000	52.46	28,000
J09	0.27	13,000	60.94	65,000	J09	0.27	13,000	63.43	67,000	J09	0.27	13,000	52.27	28,000
J10	0.27	13,000	61.31	67,000	J10	0.27	13,000	63.80	69,000	J10	0.27	13,000	52.97	29,000
J11	0.27	13,000	61.33	68,000	J11	0.27	13,000	63.82	70,000	J11	0.27	13,000	53.36	30,000
J12	0.27	13,000	61.61	75,000	J12	0.27	13,000	64.10	77,000	J12	0.27	13,000	54.89	32,000
J15	0.18	13,000	60.14	55,000	J15	0.18	13,000	62.64	57,000	J15	0.18	13,000	49.27	25,000
J158	0.00	13,000	53.27	29,000	J158	0.00	13,000	55.77	30,000	J158	0.00	13,000	33.14	16,000
J16	0.18	13,000	59.88	52,000	J16	0.18	13,000	62.38	53,000	J16	0.18	13,000	48.40	24,000
J17	0.18	13,000	59.23	48,000	J17	0.18	13,000	61.79	50,000	J17	0.18	13,000	46.84	23,000
J178	0.20	13,000	54.17	31,000	J178	0.20	13,000	56.67	32,000	J178	0.20	13,000	33.01	18,000
J18	0.18	13,000	58.59	46,000	J18	0.18	13,000	61.09	47,000	J18	0.18	13,000	45.74	22,000
J180	0.20	13,000	53.53	30,000	J180	0.20	13,000	56.03	30,000	J180	0.20	13,000	32.83	16,000
J182	0.20	13,000	53.25	29,000	J182	0.20	13,000	55.78	30,000	J182	0.20	13,000	32.86	16,000
J184	0.20	13,000	53.24	29,000	J184	0.20	13,000	55.74	30,000	J184	0.20	13,000	33.00	16,000
J186	0.20	13,000	54.90	33,000	J186	0.20	13,000	57.40	34,000	J186	0.20	13,000	33.32	16,000
J19	0.25	13,000	57.93	43,000	J19	0.25	13,000	60.43	45,000	J19	0.25	13,000	44.33	21,000
J20	0.25	13,000	58.04	42,000	J20	0.25	13,000	60.54	43,000	J20	0.25	13,000	44.19	21,000
J21	0.25	13,000	57.50	40,000	J21	0.25	13,000	60.00	41,000	J21	0.25	13,000	43.19	20,000
J22	0.25	13,000	57.21	39,000	J22	0.25	13,000	59.71	40,000	J22	0.25	13,000	42.93	20,000
J23	0.25	13,000	56.74	37,000	J23	0.25	13,000	59.24	38,000	J23	0.25	13,000	43.28	20,000
J24	0.25	13,000	56.19	35,000	J24	0.25	13,000	58.69	36,000	J24	0.25	13,000	43.20	20,000
J25	0.25	13,000	55.70	34,000	J25	0.25	13,000	58.20	35,000	J25	0.25	13,000	43.14	20,000
J26	0.25	13,000	55.73	34,000	J26	0.25	13,000	58.23	35,000	J26	0.25	13,000	43.53	21,000
J27	0.25	13,000	56.06	35,000	J27	0.25	13,000	58.58	36,000	J27	0.25	13,000	44.21	21,000
J28	0.25	13,000	55.86	34,000	J28	0.25	13,000	58.36	35,000	J28	0.25	13,000	44.36	21,000
J29	0.25	13,000	55.98	34,000	J29	0.25	13,000	58.48	35,000	J29	0.25	13,000	44.80	21,000
J30	0.18	13,000	56.30	35,000	J30	0.18	13,000	58.80	36,000	J30	0.18	13,000	45.41	22,000
J31	0.18	13,000	56.47	36,000	J31	0.18	13,000	58.97	37,000	J31	0.18	13,000	45.83	22,000
J32	0.18	13,000	56.69	38,000	J32	0.18	13,000	59.19	38,000	J32	0.18	13,000	46.25	22,000
J33	0.18	13,000	57.55	39,000	J33	0.18	13,000	60.05	40,000	J33	0.18	13,000	47.30	23,000
J34	0.18	13,000	58.47	43,000	J34	0.18	13,000	60.97	44,000	J34	0.18	13,000	48.53	24,000
J35	0.18	13,000	58.79	44,000	J35	0.18	13,000	61.29	46,000	J35	0.18	13,000	49.22	25,000
J36	0.18	13,000	59.70	49,000	J36	0.18	13,000	62.20	50,000	J36	0.18	13,000	50.43	26,000
J37	0.20	13,000	56.39	37,000	J37	0.20	13,000	58.89	38,000	J37	0.20	13,000	39.38	19,000
J38	0.20	13,000	55.90	35,000	J38	0.20	13,000	58.40	36,000	J38	0.20	13,000	36.58	17,000
J39	0.20	13,000	55.42	34,000	J39	0.20	13,000	57.92	35,000	J39	0.20	13,000	34.87	17,000
J40	0.27	13,000	61.54	73,000	J40	0.27	13,000	64.02	75,000	J40	0.27	13,000	54.54	32,000
J46	0.27	13,000	61.47	71,000	J46	0.27	13,000	63.96	73,000	J46	0.27	13,000	54.12	31,000
J48	0.27	13,000	61.38	69,000	J48	0.27	13,000	63.86	71,000	J48	0.27	13,000	53.63	30,000
J75	0.25	13,000	47.92	23,000	J75	0.25	13,000	50.42	24,000	J75	0.25	13,000	35.45	17,000
J76	0.25	13,000	52.46	28,000	J76	0.25	13,000	54.96	29,000	J76	0.25	13,000	40.42	19,000
J77	0.25	13,000	47.31	23,000	J77	0.25	13,000	49.81	24,000	J77	0.25	13,000	35.92	17,000
J78	0.18	13,000	52.96	29,000	J78	0.18	13,000	55.46	30,000	J78	0.18	13,000	42.00	20,000
J79	0.18	13,000	49.39	25,000	J79	0.18	13,000	51.89	25,000	J79	0.18	13,000	38.99	18,000
J94	0.20	13,000	53.82	30,000	J94	0.20	13,000	56.32	31,000	J94	0.20	13,000	34.35	17,000
J95	0.20	13,000	53.35	29,000	J95	0.20	13,000	55.85	30,000	J95	0.20	13,000	33.61	16,000
J96	0.20	13,000	53.38	29,000	J96	0.20	13,000	55.88	30,000	J96	0.20	13,000	33.46	16,000
J97	0.20	13,000	44.98	21,000	J97	0.20	13,000	47.48	22,000	J97	0.20	13,000	24.84	14,000
J98	0.00	13,000	53.12	29,000	J98	0.00	13,000	55.62	30,000	J98	0.00	13,000	33.06	16,000

Option B - 3 Connections - Break Scenario 7 (Connection 3 Break)				
Junction ID	Base Demand (AVDY; L/s)	Required Fire Flow (L/min)	Residual Pressure (psi)	Available Fire Flow at Residual 20 psi (L/min)
J01	0.07	13,000	50.75	28,000
J02	0.00	13,000	50.85	28,000
J05	0.07	13,000	51.07	28,000
J08	0.07	13,000	51.13	28,000
J07	0.11	13,000	51.35	28,000
J08	0.11	13,000	51.37	28,000
J09	0.11	13,000	51.02	28,000
J10	0.11	13,000	51.58	28,000
J100	0.15	13,000	35.85	17,000
J101	0.15	13,000	41.53	20,000
J102	0.15	13,000	40.97	20,000
J103	0.15	13,000	34.84	17,000
J109	0.11	13,000	38.31	18,000
J11	0.11	13,000	51.74	29,000
J110	0.11	13,000	38.43	18,000
J111	0.11	13,000	38.95	19,000
J112	0.11	13,000	33.79	17,000
J113	0.11	13,000	25.70	15,000
J114	0.11	13,000	24.48	14,000
J115	0.11	13,000	27.00	15,000
J116	0.11	13,000	29.75	16,000
J117	0.11	13,000	28.09	15,000
J118	0.11	13,000	32.53	16,000
J119	0.11	13,000	33.78	17,000
J12	0.11	13,000	52.90	32,000
J120	0.11	13,000	32.39	16,000
J121	0.11	13,000	29.71	16,000
J122	0.11	13,000	23.29	14,000
J123	0.11	13,000	23.35	14,000
J124	0.11	13,000	29.83	16,000
J125	0.11	13,000	27.27	15,000
J128	0.00	6,000	51.65	13,000
J129	0.11	13,000	22.95	14,000
J130	0.11	13,000	26.54	15,000
J131	0.11	13,000	23.03	14,000
J132	0.11	13,000	26.43	15,000
J133	0.11	13,000	25.95	15,000
J134	0.11	13,000	22.85	14,000
J135	0.11	13,000	23.25	14,000
J136	0.11	13,000	25.15	14,000
J137	0.11	6,000	32.53	7,000
J14	0.11	6,000	50.15	12,000
J15	0.07	13,000	49.53	25,000
J152	0.11	13,000	46.54	23,000
J153	0.15	13,000	42.30	20,000
J154	0.11	13,000	38.63	18,000
J155	0.11	13,000	35.48	17,000
J156	0.00	13,000	31.86	16,000
J158	0.00	13,000	39.85	19,000
J16	0.07	13,000	47.76	24,000
J162	0.00	13,000	38.51	19,000
J164	0.00	13,000	36.88	18,000
J166	0.00	13,000	31.75	16,000
J168	0.00	13,000	28.63	15,000
J17	0.07	13,000	46.35	23,000
J170	0.00	13,000	23.26	14,000
J172	0.00	13,000	39.11	19,000
J174	0.15	13,000	40.56	19,000
J176	0.15	13,000	40.26	19,000
J178	0.08	13,000	40.57	19,000
J18	0.07	13,000	45.44	22,000
J180	0.08	13,000	40.03	19,000
J182	0.08	13,000	39.80	19,000
J184	0.08	13,000	39.81	19,000
J186	0.08	13,000	41.19	20,000
J188	0.00	13,000	38.32	18,000
J19	0.10	13,000	43.99	21,000
J20	0.10	13,000	44.59	22,000
J21	0.10	13,000	43.91	21,000
J22	0.10	13,000	44.32	22,000
J23	0.10	13,000	44.26	21,000
J24	0.10	13,000	44.07	21,000
J25	0.10	13,000	43.92	21,000
J26	0.10	13,000	44.10	21,000
J27	0.10	13,000	44.59	22,000
J28	0.10	13,000	44.71	22,000
J29	0.10	13,000	45.04	22,000
J30	0.07	13,000	45.65	22,000
J31	0.07	13,000	46.08	23,000
J32	0.07	13,000	46.61	23,000
J33	0.07	13,000	47.37	24,000
J34	0.07	13,000	48.16	24,000
J35	0.07	13,000	48.80	25,000
J36	0.07	13,000	50.10	26,000
J37	0.08	13,000	43.11	21,000
J38	0.08	13,000	42.37	20,000
J39	0.08	13,000	41.67	20,000
J40	0.11	13,000	52.60	31,000
J41	0.11	13,000	40.08	19,000
J42	0.11	13,000	39.51	19,000
J43	0.11	13,000	37.87	18,000
J44	0.11	13,000	36.34	18,000
J45	0.11	13,000	33.18	17,000
J46	0.11	13,000	52.29	30,000
J48	0.11	13,000	51.93	30,000
J50	0.11	13,000	44.52	22,000
J51	0.11	13,000	44.20	21,000
J52	0.11	13,000	41.04	20,000
J53	0.11	13,000	40.35	19,000
J55	0.11	13,000	42.46	20,000
J56	0.11	13,000	37.12	18,000
J57	0.11	13,000	39.08	19,000
J58	0.11	13,000	37.63	18,000
J59	0.11	13,000	33.31	16,000
J60	0.11	13,000	29.37	15,000
J61	0.11	13,000	28.96	15,000
J62	0.11	13,000	33.07	16,000
J63	0.11	13,000	32.97	16,000
J64	0.11	13,000	29.49	15,000
J65	0.11	13,000	29.56	15,000
J66	0.11	13,000	33.75	17,000
J67	0.11	13,000	33.92	17,000
J68	0.11	13,000	29.43	15,000
J69	0.11	13,000	29.46	15,000
J70	0.11	13,000	32.92	16,000
J71	0.11	13,000	32.99	16,000
J72	0.11	13,000	29.36	15,000
J73	0.11	13,000	29.53	15,000
J74	0.11	13,000	32.78	16,000
J75	0.10	13,000	35.75	17,000
J76	0.10	13,000	40.43	19,000
J77	0.10	13,000	35.66	17,000
J78	0.07	13,000	41.59	20,000
J79	0.07	13,000	38.58	18,000
J80	0.15	13,000	31.52	16,000
J81	0.15	13,000	41.51	20,000
J82	0.15	13,000	41.49	20,000
J83	0.15	13,000	42.73	21,000
J84	0.15	13,000	43.17	21,000
J85	0.15	13,000	26.43	15,000
J86	0.15	13,000	22.78	14,000
J87	0.15	13,000	22.80	14,000
J88	0.15	13,000	26.90	15,000
J89	0.15	13,000	44.16	21,000
J90	0.15	13,000	35.62	17,000
J91	0.15	13,000	42.23	20,000
J92	0.15	13,000	31.56	16,000
J93	0.15	13,000	28.36	15,000
J94	0.08	13,000	40.36	19,000
J95	0.08	13,000	39.95	19,000
J96	0.08	13,000	39.97	19,000
J97	0.08	13,000	31.59	16,000
J98	0.00	13,000	38.70	18,000

BARRHAVEN CONSERVANCY EAST (PHASES 2, 3, 4 & JOCK RIVER): WATER DISTRIBUTION SYSTEM ANALYSIS

June 2, 2022

Appendix E WATER AGE CALCULATIONS



163401660 - Barrhaven Conservancy

Water Age Assessment

Sizing Option A		Pipe Dimensions			Average Day Demand		Water Age		
Phase	Pipe Diameter	Pipe Length	Pipe Volume		(L/s)	(m ³ /s)	(s)	(h)	(d)
	(mm)	(m)	(m ³)						
2	406	2,609	337.81		3.50	0.004	96,464	26.80	1.12
2	305	1,442	105.38		3.50	0.004	30,091	8.36	0.35
2	203	742	24.00		3.50	0.004	6,853	1.90	0.08
2	152	305	5.54		3.50	0.004	1,581	0.44	0.02
2	Total		472.73		3.50	0.004	134,989	37.50	1.56
3	406	2,609	337.81		6.64	0.007	50,906	14.14	0.59
3	305	2,326	169.96		6.64	0.007	25,612	7.11	0.30
3	203	2,001	64.75		6.64	0.007	9,757	2.71	0.11
3	152	427	7.75		6.64	0.007	1,167	0.32	0.01
3	Total		580.27		6.64	0.007	87,442	24.29	1.01
4	406	2,977	385.42		10.30	0.010	37,402	10.39	0.43
4	305	2,982	217.89		10.30	0.010	21,145	5.87	0.24
4	203	3,747	121.28		10.30	0.010	11,769	3.27	0.14
4	152	474	8.60		10.30	0.010	835	0.23	0.01
4	Total		733.20		10.30	0.010	71,151	19.76	0.82
Ultimate	406	2,977	385.42		13.92	0.014	27,691	7.69	0.32
Ultimate	305	2,982	217.89		13.92	0.014	15,654	4.35	0.18
Ultimate	203	6,024	194.98		13.92	0.014	14,008	3.89	0.16
Ultimate	152	555	10.08		13.92	0.014	724	0.20	0.01
Ultimate	Total		808.38		13.92	0.014	58,078	16.13	0.67

Sizing Option B		Pipe Dimensions			Average Day Demand		Water Age		
Phase	Pipe Diameter	Pipe Length	Pipe Volume		(L/s)	(m ³ /s)	(s)	(h)	(d)
	(mm)	(m)	(m ³)						
2	406	0	0.00		3.50	0.004	0	0.00	0.00
2	305	3,933	287.37		3.50	0.004	82,059	22.79	0.95
2	203	742	24.00		3.50	0.004	6,853	1.90	0.08
2	152	305	5.54		3.50	0.004	1,581	0.44	0.02
2	Total		316.90		3.50	0.004	90,493	25.14	1.05
3	406	0	0.00		6.64	0.007	0	0.00	0.00
3	305	4,817	351.95		6.64	0.007	53,036	14.73	0.61
3	203	2,001	64.75		6.64	0.007	9,757	2.71	0.11
3	152	427	7.75		6.64	0.007	1,167	0.32	0.01
3	Total		424.45		6.64	0.007	63,960	17.77	0.74
4	406	0	0.00		10.30	0.010	0	0.00	0.00
4	305	5,841	426.75		10.30	0.010	41,413	11.50	0.48
4	203	3,747	121.28		10.30	0.010	11,769	3.27	0.14
4	152	474	8.60		10.30	0.010	835	0.23	0.01
4	Total		556.64		10.30	0.010	54,017	15.00	0.63
Ultimate	406	0	0.00		13.92	0.014	0	0.00	0.00
Ultimate	305	6,619	483.60		13.92	0.014	34,744	9.65	0.40
Ultimate	203	6,024	194.98		13.92	0.014	14,008	3.89	0.16
Ultimate	152	555	10.08		13.92	0.014	724	0.20	0.01
Ultimate	Total		688.66		13.92	0.014	49,477	13.74	0.57

**Kennedy-Burnett Potable
Water Master Servicing Study**



Prepared for:
City of Ottawa
100 Constellation Crescent
Ottawa, ON K2G 6G8

Prepared by:
Stantec Consulting Ltd.
400-1331 Clyde Avenue
Ottawa, ON K2C 3G4

File No. 1634-01221

April 29, 2014

KENNEDY-BURNETT POTABLE WATER MASTER SERVICING STUDY

Hydraulic Assessment
April 29, 2014

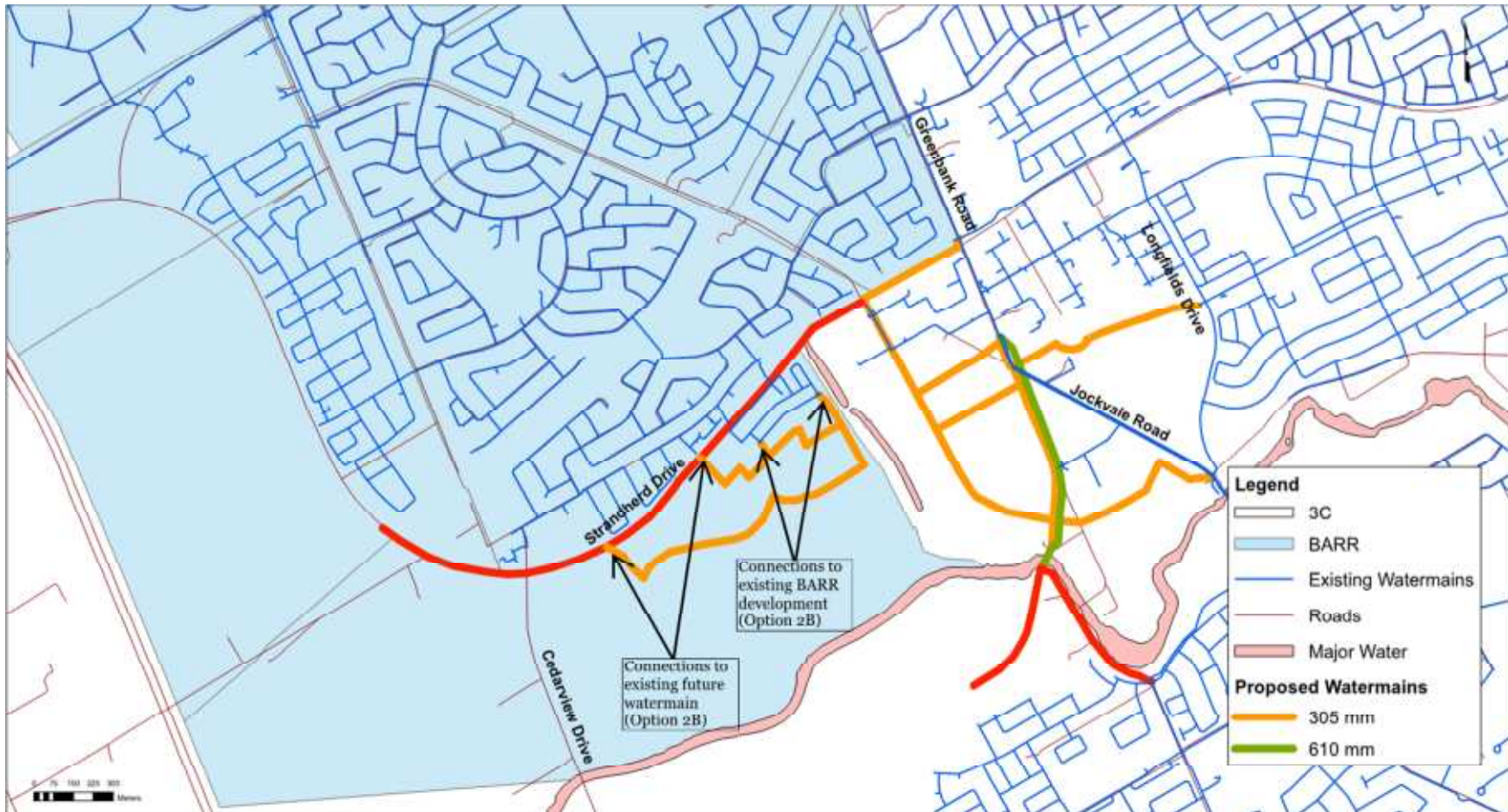


Figure 2-6: Proposed Pipe Layout Post Zone Reconfiguration – Scenario 2B

APPENDIX C

SANITARY

MEMORANDUM

DATE: MAY 30, 2019
TO: JOSÉE VALLEE – CITY OF OTTAWA
FROM: CONRAD STANG – NOVATECH
RE: STRANDHERD DRIVE WIDENING PROJECT
SOUTH NEPEAN COLLECTOR PHASE 3: SANITARY FLOW CALCULATIONS
CC: EDSON DONNELLY – NOVATECH

1.0 PURPOSE

This memorandum provides the sanitary sewer flow calculations and design sheet for Phase 3 of the proposed South Nepean Collector (SNC), as part of the Strandherd Drive Widening Project. Sanitary design flows have been estimated for both current-day operational flows and future development peak design flows. They are based on the latest available planning information for the vacant lands within the SNC sewershed.

2.0 BACKGROUND

In January 1998, the Master Servicing Study for the South Nepean Urban Area provided a conceptual plan for water, wastewater and stormwater infrastructure. The preferred alternative for wastewater servicing was an east/west trunk sewer alignment that was to be completed in several phases. The proposed sanitary trunk sewer was initially called the Jock River Collector, but was renamed the South Nepean Collector during the original functional design study completed in 2003.

Phase 1 of the South Nepean Collector was completed in 2005 and currently terminates at a 2400mm maintenance hole located east of Longfields Drive, north of Bren-Maur Road. Phase 2 was completed in 2016 and currently terminates at a 2400mm maintenance hole located at the intersection of Strandherd Drive and Fraser Fields Way.

Phase 3 will extend the trunk sewer along Strandherd Drive to the intersection of Kennevale Drive. Here it will connect with the existing sanitary trunk sewer that was constructed as part of the 2014 works to improve Strandherd Drive and develop the CitiGate Lands.

The sanitary sewer flows were previously documented in the *South Nepean Collector – Functional Design Report and Update* (Dillon, 2012). Novatech (2016) completed a *Hydraulics Review / Assessment* of the sanitary flows presented in the Dillon Report (attached). This was based on the latest planning information for the vacant lands within the SNC sewershed. The results of the *Hydraulics Review / Assessment* (Novatech, 2016) were similar to the results from the Dillon (2012) analysis.

3.0 DESIGN PARAMETERS AND POPULATION ESTIMATES

3.1 Design Parameters

The sanitary design flow were calculated using the parameters from the City of Ottawa Sewer Design Guidelines (October 2012), revised per Technical Bulletin ISTB-2018-01 (March 2018). These parameters are summarized in **Table 1** and **Table 2**.

Table 1: Peak Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	280 L/cap/day	Harmon Equation, K=0.8 (1.6 min – 3.2 max)	0.33 L/s/ha
Commercial	28,000 L/ha/day	1.0 – 1.5*	
Institutional	28,000 L/ha/day	1.0 – 1.5*	
Other†	0 L/ha/day	N/A	

*Peak Factor = 1.5 if contributing area is >20%; Peak Factor = 1.0 if contributing area is <20%

†Open Space, Arterial ROW, SWM Blocks, etc. with no sanitary flow contribution (extraneous flow only)

Table 2: Operational Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	200 L/cap/day	Harmon Equation, K=0.6 (1.2 min – 2.4 max)	0.30 L/s/ha
Commercial	17,000 L/ha/day	1.0 (non-coincident peak)	
Institutional	17,000 L/ha/day	1.0 (non-coincident peak)	

*There are no industrial areas identified within the tributary area.

$$\text{Harmon Equation} = 1 + \frac{14}{4 + \left(\frac{P}{1000}\right)^{\frac{1}{2}}} \times K$$

Where:

P = Population

K = Correction Factor:

- Peak Flow = 0.8
- Operational = 0.6

3.2 Land Use Designations & Population Estimates

Population densities and unit counts for future residential development are based on the Novatech (2016) Hydraulics Review / Assessment; refer to **Table 3**. They are based on the concept plans provided by the developers of the future residential areas.

Table 3: Residential Land Use Population Densities

Residential Land Use	Units per ha	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

The land use designations shown in **Table 4** have been applied for the areas within Phases 2 & 3 of the SNC (Node 70 to 130). The Hydraulics Analysis / Review delineated the sewershed areas and land use designations using aerial photos (existing development) and conceptual site plans (future development).

Table 4: Land Use Designations

Land Use Designation	
Secondary Plan	SNC Design
Residential	Residential (Low / Medium / High Density)
Institutional / Office	Institutional
Commercial	Commercial
Recreational	
Business Park	
Prestige Business Park	
Park/Open Space Area	Other*
Ex. Snow Disposal Facility (future commercial)	
Stormwater Management Facility	
Conservation Lands	
Arterial Right-of-Ways	

* No sanitary flow contribution - extraneous flows (inflow/infiltration) only.

The overall residential population estimate and sewershed area for Phases 2 and 3 of the SNC is provided in **Table 5** below. It is assumed that the snow dump facility at the Stranderd Drive and McKenna Casey Drive will ultimately be re-zoned for commercial development.

Table 5: Population Estimates and Areas

Existing / Future	Estimated Population / Area	Novatech (2015)
Existing	Estimated Population	6,944 persons
	Gross Residential Area	60.09 ha
	Gross Commercial / Institutional Area	64.37 ha
	<i>Total Sewershed Area</i>	124.5 ha
Future (full service)	Estimated Population	27,312 persons
	Gross Residential Area	248.48 ha
	Gross Commercial / Institutional Area	228.82 ha
	<i>Total Sewershed Area</i>	477.3 ha

4.0 SANITARY DESIGN FLOWS

The sanitary flow allocations for Phases 2 and 3 of the SNC are provided in **Table 6**. The corresponding sanitary drainage area plan is provided as **Figure 1**. Sanitary sewer flow calculations for Phases 2 and 3 and detailed sanitary sewer design sheets for Phase 3 are attached to this memorandum.

The estimated sanitary design flows from Phase 3 of the SNC (entering Node 90) are as follows:

- Present-Day Operational Design Flows (Theoretical) = 55.1 L/s
- Future Peak Design Flows = 282.5 L/s

The outlet for Phase 3 of the SNC is the existing 900mm outlet pipe at the 2400mm maintenance hole (Node 90) located at the intersection of Strandherd Drive and Fraser Fields Way. Given a minimum design slope of 0.10%, this 900mm sanitary trunk sewer would have a full flow capacity of 597.2 L/s. Therefore, the downstream sanitary trunk sewer would be at 64% capacity, based on the future peak design flow being 282.5 L/s.

ATTACHMENTS:

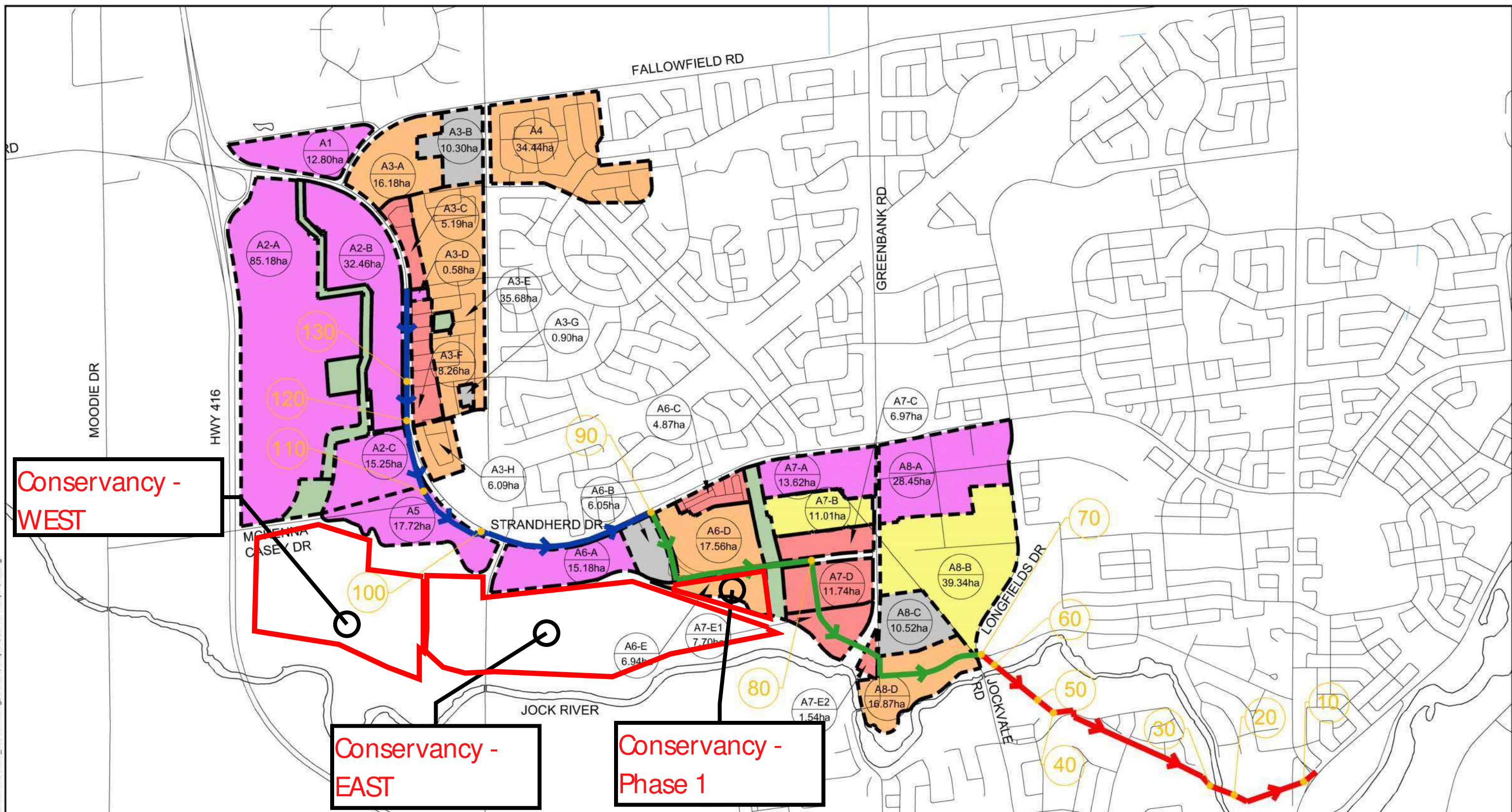
- Figure 1: Sanitary Drainage Areas and Land Use
- Sanitary Sewer Flow Calculations
- Sanitary Sewer Design Sheets (Phase 3)
- South Nepean Collector Phase 2: Hydraulics Review / Assessment (Novatech, 2016)
- Excerpts from Dillion (2012)



Table 6: Updated Allocation of Commercial, Institutional and Residential Demands to Phases 2 & 3 (Nodes 70 – 130) of the SNC by Collection Area

Collection Area	Upstream Node	Existing / Proposed Development	Existing / Proposed Land Use	Area (ha)	Estimated Number of Residential Units	Population Density (persons / ha)	Comment	Reference
A1	130	Proposed	Commercial	12.80	-	-	O'Keefe Court – Conceptual site plan shows proposed commercial.	Conceptual Plans for O'Keefe Court
A2-A	130	Proposed	Commercial	85.18	-	-	CitiGate – Analysis uses same approach as the design for CitiGate.	Detailed Servicing and SWM Report (Phase 1) (Novatech, 2014)
A2-B	130	Proposed	Commercial	32.46	-	-		
A2-C	120	Proposed	Commercial (ex. Snow dump)	15.25	-	-	Existing snow dump facility assumed to be future commercial.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)
A3-A	130	Proposed	Low Density Residential	16.18	461	95.2	Havencrest – Existing single family units.	Havencrest Design Report (IBI, 2013)
A3-B	130	Existing	Institutional	10.30	-	-	Cedarview Middle School and Cedarview Alliance Church.	Aerial Photos / Site Visits
A3-C	130	Existing	Medium Density Residential	5.19	311	162	Existing townhouse units.	
A3-D	130	Existing	Commercial	0.58	-	-	Existing commercial buildings.	
A3-E	130	Existing	Low Density Residential	35.68	999	95.2	Existing single family units.	
A3-F	130	Existing	Medium Density Residential	8.26	496	162.0	Existing townhouse units.	
A3-G	130	Existing	Institutional	0.90	-	-	Ottawa Torah Centre Chibad.	
A3-H	120	Existing	Low Density Residential	6.09	171	95.2	Existing single family units.	
A4	130	Existing	Low Density Residential	34.44	964	95.2	Existing single family units currently serviced by Jockvale pump station; to be redirected to SNC.	
A5	110	Proposed	Commercial	17.72	-	-	Proposed commercial south of McKenna Casey Drive.	Site Visits
A6-A	100	Proposed	Commercial	15.18	-	-	Proposed commercial south of Srandherd Drive; east of Borrisokane Road.	Conceptual Plan for Lands Adjacent the Kennedy-Burnett SWMF provided by Minto (2015)
A6-B	100	Proposed	Institutional	6.05	-	-	Proposed school site on Minto property.	
A6-C	90	Existing	Medium Density Residential	4.87	292	162.0	Existing townhouse units.	Aerial Photos / Site Visits
A6-D	90	Proposed	Low Density Residential	17.56	492	95.2	Proposed single family units on lands owned by Minto / Mion.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A6-E	90	Proposed	Low Density Residential	6.94	203	95.2	Proposed single family units on lands owned by Pavic / Braovac.	
A7-A	80	Existing	Commercial	13.62	-	-	Existing large retail stores (commercial).	Aerial Photos
A7-B	80	Proposed	High Density Residential	11.01	826	135.0	Proposed high density units on lands owned by Richcraft / Trinity.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A7-C	80	Proposed	Medium Density Residential	6.97	418	162.0	Proposed Medium density units on lands owned by Mion.	
A7-D	80	Proposed	Medium Density Residential	11.74	704	162.0	Proposed Medium density units on lands owned by Caivan.	
A7-E1/E2	80	Proposed	Medium Density Residential	9.24	554	162.0	Proposed Medium density units on lands owned by Claridge.	
A8-A	80	Existing	Commercial	28.45	-	-	Existing Barrhaven Market Place (commercial).	Aerial Photos / Site Visits
A8-B	80	Proposed	High Density Residential	39.34	2951	135.0	Future development similar to Ampersands development.	Site Visits
A8-C	80	Existing	Institutional	10.52	-	-	Existing St. Joseph High School.	Aerial Photos / Site Visits
A8-D	80	Proposed	Low Density Residential	16.87	1012	162.0	Proposed 600 low density residential units.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)

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LEGEND

	EXISTING / PROPOSED HIGH DENSITY RESIDENTIAL
	EXISTING / PROPOSED MEDIUM DENSITY RESIDENTIAL
	EXISTING / PROPOSED LOW DENSITY RESIDENTIAL
	EXISTING / PROPOSED COMMERCIAL
	EXISTING / PROPOSED INSTITUTIONAL

	OTHER LANDS (OPEN SPACE, PARKS, AND SWMFS)
	SOUTH NEPEAN COLLECTOR PHASE 1
	SOUTH NEPEAN COLLECTOR PHASE 2
	SOUTH NEPEAN COLLECTOR PHASE 3
	SOUTH NEPEAN COLLECTOR NODE ID

NOVATECH
 Engineers, Planners & Landscape Architects
 Suite 200, 240 Michael Cowpland Drive
 Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
 Facsimile (613) 254-5867
 Website www.novatech-eng.com

SOUTH NEPEAN COLLECTOR SEWER

SANITARY DRAINAGE AREAS AND LAND USE

SCALE 1:20 000

DATE MAY 2019 JOB 117190 FIGURE FIG. 1

PROJECT #: 117190
DESIGNED BY: CMS
CHECKED BY: RJD
DATE: December 5, 2018

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Current Operational Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (17,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (17,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.3 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (200 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-B	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-B	Institutional	130		10.30		10.30					0.0	2.0	3.1	0.0	2.0	3.1	0.0	5.1
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	841	2.40	0.0	0.0	1.6	0.0	2.0	4.6	4.7	11.3
A3-D	Commercial	130	0.58			0.58		841	841	2.40	0.1	0.0	0.2	0.1	2.0	4.8	4.7	11.6
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	4238	2.39	0.0	0.0	10.7	0.1	2.0	15.5	23.4	41.1
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	5576	2.32	0.0	0.0	2.5	0.1	2.0	18.0	29.9	50.1
A3-G	Institutional	130		0.90		0.90			5576	2.32	0.0	0.2	0.3	0.1	2.2	18.3	29.9	50.5
A4	Low Density Residential*	130				0.00			5576	2.32	0.0	0.0	0.0	0.1	2.2	18.3	29.9	50.5
A2-C	Snow Dump Facility	120				0.00			5576	2.32	0.0	0.0	0.0	0.1	2.2	18.3	29.9	50.5
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	6155	2.30	0.0	0.0	1.8	0.1	2.2	20.1	32.7	55.1
A5	Open Space	110				0.00			6155	2.30	0.0	0.0	0.0	0.1	2.2	20.1	32.7	55.1
A6-A	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	2.2	20.1	32.7	55.1
A6-B	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	2.2	20.1	32.7	55.1
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	6944	2.27	0.0	0.0	1.5	0.1	2.2	21.6	36.4	60.3
A6-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	2.2	21.6	36.4	60.3
A6-E	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	2.2	21.6	36.4	60.3
A7-A	Commercial	90	13.62			13.62			6944	2.27	2.7	0.0	4.1	2.8	2.2	25.6	36.4	67.1
A7-B	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A7-C	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A7-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A7-E1/E2	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	2.2	25.6	36.4	67.1
A8-A	Commercial	80	28.45			28.45			6944	2.27	5.6	0.0	8.5	8.4	2.2	34.2	36.4	81.2
A8-B	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	2.2	34.2	36.4	81.2
A8-C	Institutional	80		10.52		10.52			6944	2.27	0.0	2.1	3.2	8.4	4.3	37.3	36.4	86.4
A8-D	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	4.3	37.3	36.4	86.4
ROW Along SNC Sewer Alignment	-	80				14.34			6944	2.27	0.0	0.0	4.3	8.4	4.3	41.6	36.4	90.7
TOTAL		80	42.65	21.72	60.09	138.80	-	6944	6944	2.27	8.4	4.3	41.6	8.4	4.3	41.6	36.4	90.7

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
Where: P = population; K = correction factor = 0.6
- Institutional / Commercial Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station; currently not directed to SNC

PROJECT #: 117190
 DESIGNED BY: CMS
 CHECKED BY: RJD
 DATE: December 5, 2018

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Future Full Service Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing / Proposed Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (28,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (28,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.33 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (280 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Commercial	130	12.80			12.80					6.2	0.0	4.2	6.2	0.0	4.2	0.0	10.4
A2-A	Commercial	130	85.18			85.18					41.4	0.0	28.1	47.6	0.0	32.3	0.0	80.0
A2-B	Commercial	130	32.46			32.46					15.8	0.0	10.7	63.4	0.0	43.0	0.0	106.5
A3-A	Low Density Residential	130			16.18	16.18	95.2	1540	1540	3.14	0.0	0.0	5.3	63.4	0.0	48.4	15.7	127.5
A3-B	Institutional	130		10.30		10.30				3.14	0.0	3.3	3.4	63.4	3.3	51.8	15.7	134.2
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	2381	3.02	0.0	0.0	1.7	63.4	3.3	53.5	23.3	143.6
A3-D	Commercial	130	0.58			0.58				3.02	0.3	0.0	0.2	63.7	3.3	53.7	23.3	144.0
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	5778	2.75	0.0	0.0	11.8	63.7	3.3	65.5	51.5	184.0
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	7116	2.68	0.0	0.0	2.7	63.7	3.3	68.2	61.8	197.0
A3-G	Institutional	130		0.90		0.90				2.68	0.0	0.3	0.3	63.7	3.6	68.5	61.8	197.6
A4	Low Density Residential	130			34.44	34.44	95.2	3279	10395	2.55	0.0	0.0	11.4	63.7	3.6	79.9	85.9	233.1
A2-C	Commercial (ex. snow dump)	120	15.25			15.25				2.55	7.4	0.0	5.0	71.1	3.6	84.9	85.9	245.5
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	10974	2.53	0.0	0.0	2.0	71.1	3.6	86.9	90.0	251.7
A5	Commercial	110	17.72			17.72				2.53	8.6	0.0	5.8	79.7	3.6	92.7	90.0	266.1
A6-A	Commercial	100	15.18			15.18				2.53	7.4	0.0	5.0	87.1	3.6	97.7	90.0	278.5
A6-B	Institutional	100		6.05		6.05				2.53	0.0	2.0	2.0	87.1	5.6	99.7	90.0	282.5
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	11763	2.51	0.0	0.0	1.6	87.1	5.6	101.4	95.6	289.6
A6-D	Low Density Residential	90			17.56	17.56	95.2	1672	13435	2.46	0.0	0.0	5.8	87.1	5.6	107.1	107.2	307.0
A6-E	Low Density Residential	90			6.94	6.94	95.2	661	14096	2.44	0.0	0.0	2.3	87.1	5.6	109.4	111.7	313.8
A7-A	Commercial	90	13.62			13.62				2.44	6.6	0.0	4.5	93.7	5.6	113.9	111.7	324.9
A7-B	High Density Residential	90			11.01	11.01	135.0	1486	15582	2.41	0.0	0.0	3.6	93.7	5.6	117.6	121.7	338.5
A7-C	Medium Density Residential	90			6.97	6.97	162.0	1129	16711	2.38	0.0	0.0	2.3	93.7	5.6	119.9	129.2	348.3
A7-D	Medium Density Residential	90			11.74	11.74	162.0	1902	18613	2.35	0.0	0.0	3.9	93.7	5.6	123.7	141.6	364.6
A7-E1/E2	Medium Density Residential	90			9.24	9.24	162.0	1497	20110	2.32	0.0	0.0	3.0	93.7	5.6	126.8	151.2	377.3
A8-A	Commercial	80	28.45			28.45				2.32	13.8	0.0	9.4	107.5	5.6	136.2	151.2	400.5
A8-B	High Density Residential	80			39.34	39.34	135.0	5311	25421	2.24	0.0	0.0	13.0	107.5	5.6	149.2	184.4	446.7
A8-C	Institutional	80		10.52		10.52				2.24	0.0	3.4	3.5	107.5	9.0	152.6	184.4	453.6
A8-D	Low Density Residential	80			16.87	16.87	120.9	2040	27461	2.21	0.0	0.0	5.6	107.5	9.0	158.2	196.9	471.6
ROW Along SNC Sewer Alignment	-	80				14.34				2.21	0.0	0.0	4.7	107.5	9.0	162.9	196.9	476.3
TOTAL		80	221.24	27.77	230.38	493.73	-	27461	27461	2.21	107.5	9.0	162.9	107.5	9.0	162.9	196.9	476.3

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 - 28 (28 used)	2.7 - 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 - 60 (60 used)	2.7	162.0
High Density (apartments)	60 - 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
 Where: P = population; K = correction factor = 0.8
- Commercial Peaking Factor = 1.5; Institutional Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units

See Note (2) in the DSEL "Barrhaven Conservancy - Evaluation of SNC Flows" design sheet

THE PRIOR NOVATECH SNC DESIGN SHEET HAD FLOWS AT 423.6 L/s AFTER AREA ID "A6-E". THIS UPDATED NOVATECH 'PHASE 3' EVALUATION HAS A FLOW OF 313.8 L/s. THE DSEL EVALUATION OF SANITARY FLOWS WITH THE NEW CITY DESIGN PARAMETERS AT THIS SAME NODE (WITH CONSERVANCY WEST AND EAST INCLUDED) IS ~401.58 L/s (WHICH IS LESS THAN THE PRIOR 423.6 L/s NOTED ABOVE)

SOUTH NEPEAN COLLECTOR (PHASE 3)
SANITARY SEWER DESIGN SHEET



DECEMBER 5 2018
JOB# 117190

LOCATION			Area				Population		Cumulative Design Flows					PROPOSED SEWER						
From MH	To MH	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (280 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)	Length (m)	Pipe Size (mm)	Type	Slope %	Capacity (L/s)	Full Flow Velocity (m/s)	Ratio (Q/Qfull)
SA 22	SA 21	120	146.27	11.20	105.84	263.31	10974	2.53	71.1	3.6	86.9	90.0	251.7	131.9	750	CONC	0.10	367.3	0.81	69%
SA 21	SA 20	120											251.7	90.6	750	CONC	0.10	367.3	0.81	69%
SA 20	SA 19	120											251.7	90.0	750	CONC	0.10	367.3	0.81	69%
SA 19	SA 18	120											251.7	72.1	750	CONC	0.10	367.3	0.81	69%
SA 18	SA 17	120											251.7	71.9	750	CONC	0.10	367.3	0.81	69%
SA 17	SA 16	120											251.7	71.4	750	CONC	0.10	367.3	0.81	69%
SA 16	SA 15	110	163.99	11.20	105.84	281.03	10974	2.53	79.7	3.6	92.7	90.0	266.1	73.2	750	CONC	0.10	367.3	0.81	72%
SA 15	SA 14	110											266.1	67.5	750	CONC	0.10	367.3	0.81	72%
SA 14	SA 13	110											266.1	56.6	750	CONC	0.10	367.3	0.81	72%
SA 13	SA 12	110											266.1	133.5	750	CONC	0.10	367.3	0.81	72%
SA 12	SA 11	110											266.1	150.0	750	CONC	0.10	367.3	0.81	72%
SA 11	SA 10	100	179.17	17.25	105.84	302.26	10974	2.53	87.1	5.6	99.7	90.0	282.5	97.8	750	CONC	0.10	367.3	0.81	77%
SA 10	SA 9	100											282.5	76.7	750	CONC	0.10	367.3	0.81	77%
SA 9	SA 8	100											282.5	79.7	750	CONC	0.10	367.3	0.81	77%
SA 8	SA 7	100											282.5	75.3	750	CONC	0.10	367.3	0.81	77%
SA 7	SA 6	100											282.5	84.9	750	CONC	0.10	367.3	0.81	77%
SA 6	SA 5	100											282.5	77.1	750	CONC	0.10	367.3	0.81	77%
SA 5	SA 4	100											282.5	78.9	750	CONC	0.10	367.3	0.81	77%
SA 4	SA 3	100											282.5	80.5	750	CONC	0.10	367.3	0.81	77%
SA 3	SA 2	100											282.5	150.0	750	CONC	0.10	367.3	0.81	77%
SA 2	SA 1	100											282.5	114.6	750	CONC	0.10	367.3	0.81	77%
SA 1	EX 80	100											282.5	12.4	750	CONC	0.10	367.3	0.81	77%

Design Parameters:

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles / semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row / townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
Where: P = population; K = correction factor = 0.8
- Commercial Peaking Factor = 1.5; Institutional Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units



MEMORANDUM

DATE: MAY 26, 2016
TO: JONATHAN KNOYLE – CITY OF OTTAWA
FROM: CONRAD STANG – NOVATECH
RE: SOUTH NEPEAN COLLECTOR PHASE 2: SANITARY FLOW CALCULATIONS
CC: EDSON DONNELLY – NOVATECH

1.0 PURPOSE

This memorandum provides the sanitary sewer flow calculations and design sheet for Phase 2 of the proposed South Nepean Collector (SNC). Sanitary design flows have been estimated for both current-day operational flows and future development peak design flows, based on the latest available planning information for the vacant lands within the SNC sewershed.

2.0 BACKGROUND

In January 1998, the Master Servicing Study for the South Nepean Urban Area provided a conceptual plan for water, wastewater and stormwater infrastructure. The preferred alternative for wastewater servicing was an east/west trunk sewer alignment that was to be completed in several phases. The proposed sanitary trunk sewer was initially called the Jock River Collector, but was renamed the South Nepean Collector during the original functional design study completed in 2003.

Phase 1 of the South Nepean Collector was completed in 2005 and currently terminates at a 2400mm maintenance hole located east of Longfields Drive, north of Bren-Maur Road. Phase 2 will extend the trunk sewer to Strandherd Drive at the intersection of the proposed transitway along the proposed extension to Chapman Mills Drive. Phase 3 will extend the trunk sewer along Strandherd Drive to the intersection of Maravista Drive.

The sanitary sewer flows were previously documented in the *South Nepean Collector – Functional Design Report and Update* (Dillon, 2012). A review of the sanitary flows provided in the Dillon Report based on the latest planning information for the vacant lands within the SNC sewershed was documented in the technical memorandum titled *South Nepean Collector Phase 2: Hydraulics Review / Assessment* (Novatech, 2015), which is attached to this memorandum. The results of the *Hydraulics Review / Assessment* (Novatech, 2015) were very similar to the results from the Dillon (2012) analysis.

3.0 DESIGN PARAMETERS AND POPULATION ESTIMATES

3.1 Design Parameters

The sanitary design flow were calculated using the parameters from the City of Ottawa Sewer Design Guidelines (October 2012), and are summarized in **Table 1** and **Table 2**.

Table 1: Peak Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	350 L/cap/day	Harmon Equation, K=1 (2.0 min – 4.0 max)	0.28 L/s/ha
Commercial	50,000 L/ha/day	1.5	
Institutional	50,000 L/ha/day	1.5	
Other*	0 L/ha/day	N/A	

*Open Space, Arterial ROW, SWM Blocks, etc. with no sanitary flow contribution (extraneous flow only)

Table 2: Operational Design Flow Parameters

Land Use	Average Daily Flow	Peaking Factor	Peak Extraneous Flows
Residential	300 L/cap/day	Harmon Equation, K=0.6 (1.2 min – 2.4 max)	<u>Dry weather</u> 0.05-0.08 L/s/ha
Commercial	17,000 L/ha/day	1.0 (non-coincident peak)	<u>Wet Weather</u> 0.15 - 0.20 L/s/ha (typical events) 0.28 L/s/ha (large/annual events) 0.30 - 0.50 L/s/ha (extreme events)
Institutional	10,000 L/ha/day	1.0 (non-coincident peak)	

*There are no industrial areas identified within the tributary area.

$$\text{Harmon Equation} = 1 + \frac{14}{4 + \left(\frac{P}{1000}\right)^{\frac{1}{2}}} \times K$$

Where:

P = Population

K = Correction Factor:

- Peak Flow = 1
- Operational = between 0.4 to 0.6 (0.6 used)

3.2 Land Use Designations & Population Estimates

Population densities and unit counts for future residential development are based on the current concept plans for these areas, and are presented in **Table 3**.

Table 3: Residential Land Use Population Densities

Residential Land Use	Units per ha	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

The land use designations shown in **Table 4** have been applied for the areas within Phases 2 and 3 of the SNC (Node 70 to 130). The sewershed areas and land use designations were delineated using aerial photos (existing development) and conceptual site plans (future development).

Table 4: Land Use Designations

Land Use Designation	
Secondary Plan	SNC Design
Residential	Residential (Low / Medium / High Density)
Institutional / Office	Institutional
Commercial	Commercial
Recreational	
Business Park	
Prestige Business Park	
Park/Open Space Area	Other*
Ex. Snow Disposal Facility (future commercial)	
Stormwater Management Facility	
Conservation Lands	
Arterial Right-of-Ways	

* No sanitary flow contribution - extraneous flows (inflow/infiltration) only.

The overall residential population estimate and sewershed area for Phases 2 and 3 of the SNC is provided in **Table 5** below. It is assumed that the snow dump facility at the Stranderd Drive and McKenna Casey Drive will ultimately be re-zoned for commercial development.

Table 5: Population Estimates and Areas

Existing / Future	Estimated Population / Area	Novatech (2015)
Existing	Estimated Population	6,944 persons
	Gross Residential Area	60.09 ha
	Gross Commercial / Institutional Area	64.37 ha
	<i>Total Sewershed Area</i>	124.5 ha
Future (full service)	Estimated Population	27,312 persons
	Gross Residential Area	248.48 ha
	Gross Commercial / Institutional Area	228.82 ha
	<i>Total Sewershed Area</i>	477.3 ha

4.0 SANITARY DESIGN FLOWS

The sanitary flow allocations for Phases 2 and 3 of the SNC are provided in **Table 6**. The corresponding sanitary drainage area plan is provided as **Figure 1**. Sanitary sewer flow calculations for Phases 2 and 3 and detailed sanitary sewer design sheets for Phase 2 are attached to this memorandum.

The estimated sanitary design flows from Phases 2 and 3 of the SNC (entering Node 70) are as follows:

- Present-Day Operational Design Flows (Theoretical) = 72.5 L/s
- Future Peak Design Flows = 634.2 L/s

The outlet for Phase 2 of the SNC is the existing 1050mm outlet pipe at the 2400mm maintenance hole (Node 70) located east of Longfields Drive, north of Bren-Maur Road. Given a minimum design slope of 0.10%, this sanitary trunk sewer would have a full flow capacity of 900.5 L/s. Therefore, the downstream sanitary trunk sewer would be at 70% capacity, based on the future peak design flow being 634.2 L/s.

ATTACHMENTS:

- Figure 1: Sanitary Drainage Areas and Land Use
- Sanitary Sewer Flow Calculations
- Sanitary Sewer Design Sheets (Phase 2)
- South Nepean Collector Phase 2: Hydraulics Review / Assessment (Novatech, 2015)

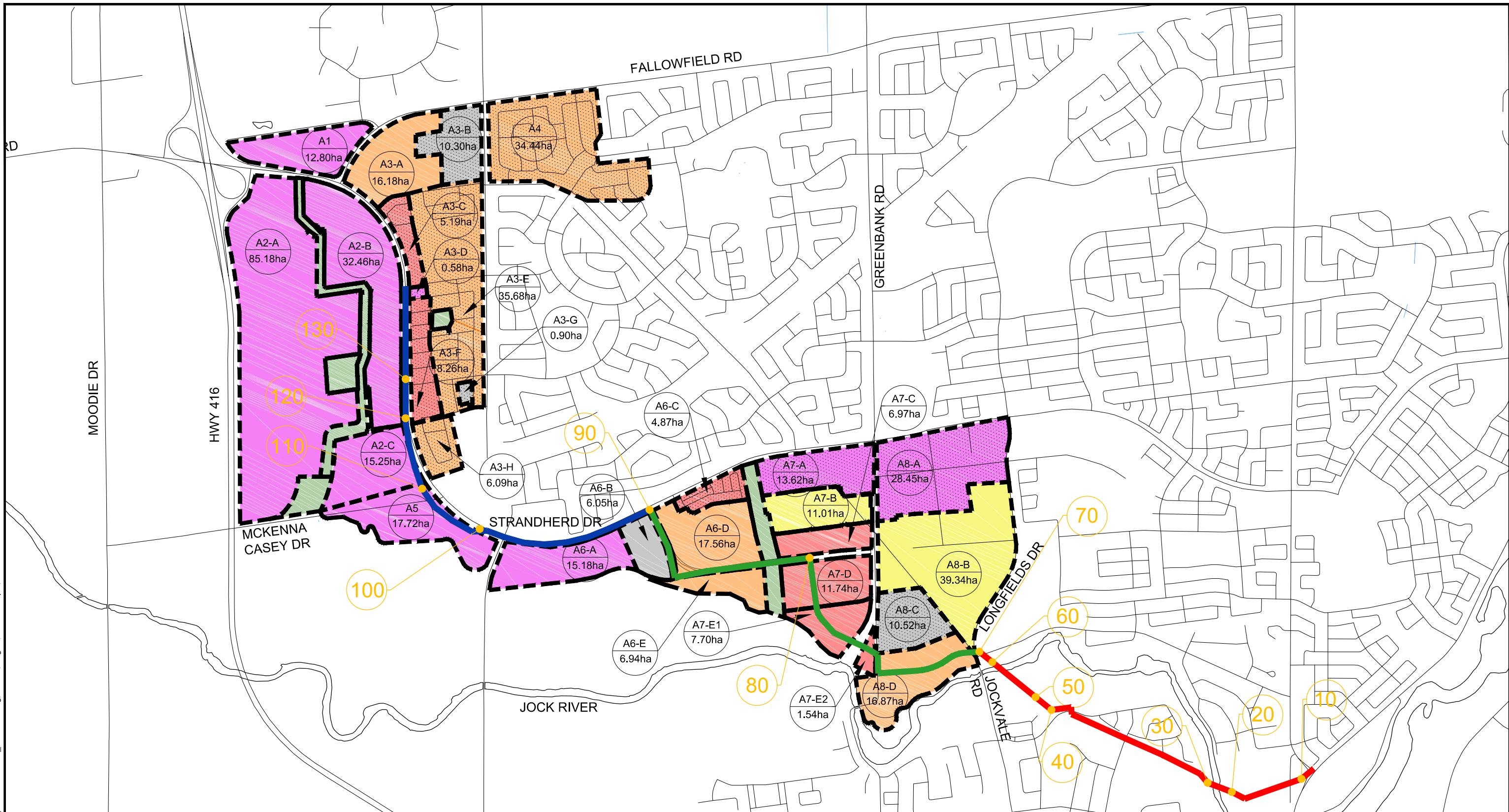


Table 6: Updated Allocation of Commercial, Institutional and Residential Demands to Phases 2 & 3 (Nodes 70 – 130) of the SNC by Collection Area

Collection Area	Upstream Node	Existing / Proposed Development	Existing / Proposed Land Use	Area (ha)	Estimated Number of Residential Units	Population Density (persons / ha)	Comment	Reference
A1	130	Proposed	Commercial	12.80	-	-	O'Keefe Court – Conceptual site plan shows proposed commercial.	Conceptual Plans for O'Keefe Court
A2-A	130	Proposed	Commercial	85.18	-	-	CitiGate – Analysis uses same approach as the design for CitiGate.	Detailed Servicing and SWM Report (Phase 1) (Novatech, 2014)
A2-B	130	Proposed	Commercial	32.46	-	-		
A2-C	120	Proposed	Commercial (ex. Snow dump)	15.25	-	-	Existing snow dump facility assumed to be future commercial.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)
A3-A	130	Proposed	Low Density Residential	16.48	461	95.2	Havencrest – Existing single family units.	Havencrest Design Report (IBI, 2013)
A3-B	130	Existing	Institutional	10.30	-	-	Cedarview Middle School and Cedarview Alliance Church.	Aerial Photos / Site Visits
A3-C	130	Existing	Medium Density Residential	5.19	311	162	Existing townhouse units.	
A3-D	130	Existing	Commercial	0.58	-	-	Existing commercial buildings.	
A3-E	130	Existing	Low Density Residential	35.68	999	95.2	Existing single family units.	
A3-F	130	Existing	Medium Density Residential	8.26	496	162.0	Existing townhouse units.	
A3-G	130	Existing	Institutional	0.90	-	-	Ottawa Torah Centre Chibad.	
A3-H	120	Existing	Low Density Residential	6.09	171	95.2	Existing single family units.	
A4	130	Existing	Low Density Residential	34.44	964	95.2	Existing single family units currently serviced by Jockvale pump station; to be redirected to SNC.	
A5	110	Proposed	Commercial	17.72	-	-	Proposed commercial south of McKenna Casey Drive.	Site Visits
A6-A	100	Proposed	Institutional	20.70	-	-	Proposed school site on Minto property.	Conceptual Plan for Lands Adjacent the Kennedy-Burnett SWMF provided by Minto (2015)
A6-B	90	Existing	Medium Density Residential	4.87	292	162.0	Existing townhouse units.	Aerial Photos / Site Visits
A6-C	90	Proposed	Low Density Residential	10.11	283	95.2	Proposed single family units on lands owned by Minto.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A6-D	90	Proposed	Low Density Residential	5.59	157	95.2	Proposed single family units on lands owned by Mion.	
A6-E	90	Proposed	Low Density Residential	7.24	203	95.2	Proposed single family units on lands owned by Pavic / Braovac.	
A7-A	80	Existing	Commercial	13.62	-	-	Existing large retail stores (commercial).	Aerial Photos
A7-B	80	Proposed	High Density Residential	11.01	826	135.0	Proposed high density units on lands owned by Richcraft / Trinity.	Conceptual Plans for Lands Adjacent the Kennedy-Burnett SWMF provided by land owners.
A7-C	80	Proposed	Medium Density Residential	6.97	418	162.0	Proposed Medium density units on lands owned by Mion.	
A7-D	80	Proposed	Medium Density Residential	11.74	704	162.0	Proposed Medium density units on lands owned by Caivan.	
A7-E1/E2	80	Proposed	Medium Density Residential	9.24	554	162.0	Proposed Medium density units on lands owned by Claridge.	
A8-A	80	Existing	Commercial	28.45	-	-	Existing Barrhaven Market Place (commercial).	Aerial Photos / Site Visits
A8-B	80	Proposed	High Density Residential	39.34	2951	135.0	Future development similar to Ampersands development.	Site Visits
A8-C	80	Existing	Institutional	10.52	-	-	Existing St. Joseph High School.	Aerial Photos / Site Visits
A8-D	80	Proposed	Low Density Residential	16.87	1012	162.0	Proposed 600 low density residential units.	Functional Design Report and Update – SNC Phase 2 and 3 (Dillon, 2012)

Attachment 1
Sanitary Drainage Areas and Land Use

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LEGEND

- EXISTING / PROPOSED HIGH DENSITY RESIDENTIAL
- EXISTING / PROPOSED MEDIUM DENSITY RESIDENTIAL
- EXISTING / PROPOSED LOW DENSITY RESIDENTIAL
- EXISTING / PROPOSED COMMERCIAL
- EXISTING / PROPOSED INSTITUTIONAL
- OTHER LANDS (OPEN SPACE, PARKS, AND SWMFS)
- SOUTH NEPEAN COLLECTOR PHASE 1
- SOUTH NEPEAN COLLECTOR PHASE 2
- SOUTH NEPEAN COLLECTOR PHASE 3
- SOUTH NEPEAN COLLECTOR NODE ID



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SOUTH NEPEAN COLLECTOR SEWER

SANITARY DRAINAGE AREAS AND LAND USE

SCALE 1:20 000

DATE MAY 2016 JOB 115075 FIGURE FIG. 1

Attachment 2
Sewer Flow Calculations

PROJECT #: 115075
DESIGNED BY: CMS
CHECKED BY: MJP
DATE: August 20, 2015

SANITARY SEWER DESIGN SHEET
South Nepean Collector - Phase 2 & 3
Theoretical Current Operational Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (17,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (10,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.05 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (300 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A2-B	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-A	Open Space	130				0.00					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
A3-B	Institutional	130		10.30		10.30					0.0	1.2	0.5	0.0	1.2	0.5	0.0	1.7
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	841	2.71	0.0	0.0	0.3	0.0	1.2	0.8	7.9	9.9
A3-D	Commercial	130	0.58			0.58		841	841	2.71	0.1	0.0	0.0	0.1	1.2	0.8	7.9	10.0
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	4238	2.39	0.0	0.0	1.8	0.1	1.2	2.6	35.1	39.0
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	5576	2.32	0.0	0.0	0.4	0.1	1.2	3.0	44.9	49.2
A3-G	Institutional	130		0.90		0.90			5576	2.32	0.0	0.1	0.0	0.1	1.3	3.0	44.9	49.4
A4	Low Density Residential*	130				0.00			5576	2.32	0.0	0.0	0.0	0.1	1.3	3.0	44.9	49.4
A2-C	Snow Dump Facility	120				0.00			5576	2.32	0.0	0.0	0.0	0.1	1.3	3.0	44.9	49.4
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	6155	2.30	0.0	0.0	0.3	0.1	1.3	3.4	49.1	53.8
A5	Open Space	110				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-A	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-B	Open Space	100				0.00			6155	2.30	0.0	0.0	0.0	0.1	1.3	3.4	49.1	53.8
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	6944	2.27	0.0	0.0	0.2	0.1	1.3	3.6	54.6	59.6
A6-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	1.3	3.6	54.6	59.6
A6-E	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	0.1	1.3	3.6	54.6	59.6
A7-A	Commercial	90	13.62			13.62			6944	2.27	2.7	0.0	0.7	2.8	1.3	4.3	54.6	63.0
A7-B	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-C	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-D	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A7-E1/E2	Open Space	90				0.00			6944	2.27	0.0	0.0	0.0	2.8	1.3	4.3	54.6	63.0
A8-A	Commercial	80	28.45			28.45			6944	2.27	5.6	0.0	1.4	8.4	1.3	5.7	54.6	70.0
A8-B	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	1.3	5.7	54.6	70.0
A8-C	Institutional	80		10.52		10.52			6944	2.27	0.0	1.2	0.5	8.4	2.5	6.2	54.6	71.8
A8-D	Open Space	80				0.00			6944	2.27	0.0	0.0	0.0	8.4	2.5	6.2	54.6	71.8
ROW Along SNC Sewer Alignment	-	80				14.34			6944	2.27	0.0	0.0	0.7	8.4	2.5	6.9	54.6	72.5
TOTAL		80	42.65	21.72	60.09	138.80	-	6944	6944	2.27	8.4	2.5	6.9	8.4	2.5	6.9	54.6	72.5

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
Where: P = population; K = correction factor = 0.6
- Institutional / Commercial Peaking Factor = 1.0

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station; currently not directed to SNC

PROJECT #: 115075
DESIGNED BY: CMS
CHECKED BY: MJP
DATE: August 20, 2015

SANITARY SEWER DESIGN SHEET

South Nepean Collector - Phase 2 & 3

Theoretical Future Full Service Peak Wastewater Flow



Location			Areas				Population				Individual Design Flows			Cumulative Design Flows				
Area I.D.	Existing / Proposed Land Use	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.28 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (350 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)
A1	Commercial	130	12.80			12.80					11.1	0.0	3.6	11.1	0.0	3.6	0.0	14.7
A2-A	Commercial	130	85.18			85.18					73.9	0.0	23.9	85.1	0.0	27.4	0.0	112.5
A2-B	Commercial	130	32.46			32.46					28.2	0.0	9.1	113.2	0.0	36.5	0.0	149.8
A3-A	Low Density Residential	130			16.18	16.18	95.2	1540	1540	3.67	0.0	0.0	4.5	113.2	0.0	41.1	22.9	177.2
A3-B	Institutional	130		10.30		10.30		1540	1540	3.67	0.0	8.9	2.9	113.2	8.9	43.9	22.9	189.0
A3-C	Medium Density Residential	130			5.19	5.19	162.0	841	2381	3.53	0.0	0.0	1.5	113.2	8.9	45.4	34.0	201.6
A3-D	Commercial	130	0.58			0.58		2381	2381	3.53	0.5	0.0	0.2	113.7	8.9	45.6	34.0	202.2
A3-E	Low Density Residential	130			35.68	35.68	95.2	3397	5778	3.19	0.0	0.0	10.0	113.7	8.9	55.5	74.6	252.8
A3-F	Medium Density Residential	130			8.26	8.26	162	1338	7116	3.10	0.0	0.0	2.3	113.7	8.9	57.9	89.4	269.9
A3-G	Institutional	130		0.90		0.90		7116	7116	3.10	0.0	0.8	0.3	113.7	9.7	58.1	89.4	270.9
A4	Low Density Residential	130			34.44	34.44	95.2	3279	10395	2.94	0.0	0.0	9.6	113.7	9.7	67.8	123.7	314.9
A2-C	Commercial (ex. snow dump)	120	15.25			15.25		10395	10395	2.94	13.2	0.0	4.3	127.0	9.7	72.0	123.7	332.4
A3-H	Low Density Residential	120			6.09	6.09	95.2	580	10974	2.91	0.0	0.0	1.7	127.0	9.7	73.7	129.6	340.0
A5	Commercial	110	17.72			17.72		10974	10974	2.91	15.4	0.0	5.0	142.4	9.7	78.7	129.6	360.3
A6-A	Commercial	100	15.18			15.18		10974	10974	2.91	13.2	0.0	4.3	155.5	9.7	82.9	129.6	377.8
A6-B	Institutional	100		6.05		6.05		10974	10974	2.91	0.0	5.3	1.7	155.5	15.0	84.6	129.6	384.7
A6-C	Medium Density Residential	90			4.87	4.87	162.0	789	11763	2.88	0.0	0.0	1.4	155.5	15.0	86.0	137.4	393.9
A6-D	Low Density Residential	90			17.56	17.56	95.2	1672	13435	2.83	0.0	0.0	4.9	155.5	15.0	90.9	153.8	415.2
A6-E	Low Density Residential	90			6.94	6.94	95.2	661	14096	2.81	0.0	0.0	1.9	155.5	15.0	92.9	160.2	423.6
A7-A	Commercial	90	13.62			13.62		14096	14096	2.81	11.8	0.0	3.8	167.4	15.0	96.7	160.2	439.2
A7-B	High Density Residential	90			11.01	11.01	135.0	1486	15582	2.76	0.0	0.0	3.1	167.4	15.0	99.8	174.3	456.4
A7-C	Medium Density Residential	90			6.97	6.97	162.0	1129	16711	2.73	0.0	0.0	2.0	167.4	15.0	101.7	184.9	468.9
A7-D	Medium Density Residential	90			11.74	11.74	162.0	1902	18613	2.68	0.0	0.0	3.3	167.4	15.0	105.0	202.4	489.7
A7-E1/E2	Medium Density Residential	90			9.24	9.24	162.0	1497	20110	2.65	0.0	0.0	2.6	167.4	15.0	107.6	215.9	505.8
A8-A	Commercial	80	28.45			28.45		20110	20110	2.65	24.7	0.0	8.0	192.0	15.0	115.5	215.9	538.5
A8-B	High Density Residential	80			39.34	39.34	135.0	5311	25421	2.55	0.0	0.0	11.0	192.0	15.0	126.6	262.4	596.0
A8-C	Institutional	80		10.52		10.52		25421	25421	2.55	0.0	9.1	2.9	192.0	24.1	129.5	262.4	608.1
A8-D	Low Density Residential	80			16.87	16.87	120.9	2040	27461	2.52	0.0	0.0	4.7	192.0	24.1	134.2	279.8	630.2
ROW Along SNC Sewer Alignment	-	80				14.34			27461	2.52	0.0	0.0	4.0	192.0	24.1	138.2	279.8	634.2
TOTAL		80	221.24	27.77	230.38	493.73	-	27461	27461	2.52	192.0	24.1	134.2	192.0	24.1	138.2	279.8	634.2

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles and semis)	26 – 28 (28 used)	2.7 – 3.4 (3.4 used)	95.2
Medium Density (row/townhouse)	50 – 60 (60 used)	2.7	162.0
High Density (apartments)	60 – 75 (75 used)	1.8	135.0

Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
Where: P = population; K = correction factor = 1.0
- Institutional / Commercial Peaking Factor = 1.5

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units

THIS PRIOR NOVATECH SNC DESIGN SHEET HAD DESIGN FLOWS AT 423.6 L/S AFTER AREA ID "A6-E".

THE DSEL EVALUATION WITH NEW PARAMETERS AT THIS SAME NODE WITH CONSERVANCY WEST AND EAST INCLUDED IS ~401.58 < 423.6 L/S

Attachment 3
Sanitary Sewer Design Sheets (Phase 2)

SOUTH NEPEAN COLLECTOR (PHASE 2)
SANITARY SEWER DESIGN SHEET

MAY 26, 2016
JOB# 115075



LOCATION			Area					Population				Individual Design Flows			Cumulative Design Flows				PROPOSED SEWER							
From MH	To MH	Upstream Node	Gross Commercial Area (ha)	Gross Institutional Area (ha)	Gross Residential Area (ha)	Right-of-Way (ha)	Total Gross Area (ha)	Residential Population Density (people / ha)	Individual Residential Population	Cumulative Residential Population	Residential Peaking Factor (Harmon Eqn ¹)	Commercial Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Institutional Peak Flow Rate ² (50,000 L/ha/d) (L/s)	Infiltration / Inflow Rate (0.28 L/s/ha) (L/s)	Commercial (L/s)	Institutional (L/s)	Infiltration / Inflow (L/s)	Residential Peak Flow Rate (350 L/cap/d) (L/s)	Cumulative Peak Design Flow (L/s)	Length (m)	Pipe Size (mm)	Type	Slope %	Capacity (L/s)	Full Flow Velocity (m/s)	Ratio (Q/Qfull)
MHSA 1	MHSA 2	90	192.79	17.25	174.17	0.00	384.21	1678	20110	20110	2.65	167.352	14.97	107.58	167.4	15.0	107.6	215.9	505.8	57.3	900	CONC	0.10	597.2	0.91	85%
MHSA 2	MHSA 3	90																	505.8	57.3	900	CONC	0.10	597.2	0.91	85%
MHSA 3	MHSA 4	90																	505.8	73.9	900	CONC	0.10	597.2	0.91	85%
MHSA 4	MHSA 5	90																	505.8	34.6	900	CONC	0.10	597.2	0.91	85%
MHSA 5	MHSA 6	90																	505.8	42.8	900	CONC	0.10	597.2	0.91	85%
MHSA 6	MHSA 7	90																	505.8	84.4	900	CONC	0.10	597.2	0.91	85%
MHSA 7	MHSA 8	90																	505.8	16.5	900	CONC	0.10	597.2	0.91	85%
MHSA 8	MHSA 9	90																	505.8	85.4	900	CONC	0.10	597.2	0.91	85%
MHSA 9	MHSA 10	90																	505.8	70.6	900	CONC	0.10	597.2	0.91	85%
MHSA 10	MHSA 11	90																	505.8	70.6	900	CONC	0.10	597.2	0.91	85%
MHSA 11	MHSA 12	90																	505.8	77.8	900	CONC	0.10	597.2	0.91	85%
MHSA 12	MHSA 13	90																	505.8	77.8	900	CONC	0.10	597.2	0.91	85%
MHSA 13	MHSA 14	90																	505.8	77.8	900	CONC	0.10	597.2	0.91	85%
MHSA 14	MHSA 15	90																	505.8	25.4	900	CONC	0.10	597.2	0.91	85%
MHSA 15	MHSA 16	90																	505.8	34.2	900	CONC	0.10	597.2	0.91	85%
MHSA 16	MHSA 17	90																	505.8	86.7	900	CONC	0.10	597.2	0.91	85%
MHSA 17	MHSA 18	90																	505.8	34.3	900	CONC	0.10	597.2	0.91	85%
MHSA 18	MHSA 19	90																	505.8	68.6	900	CONC	0.10	597.2	0.91	85%
MHSA 19	MHSA 20	90																	505.8	65.5	900	CONC	0.10	597.2	0.91	85%
MHSA 20	MHSA 21	80	221.24	27.77	230.38	14.34	493.73	256	7351	27461	2.52	192.049	24.11	138.24	192.0	24.1	138.2	279.8	634.2	18.2	1050	CONC	0.10	900.9	1.01	70%
MHSA 21	MHSA 22	80																	634.2	81.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 22	MHSA 23	80																	634.2	84.7	1050	CONC	0.10	900.9	1.01	70%
MHSA 23	MHSA 24	80																	634.2	77.4	1050	CONC	0.10	900.9	1.01	70%
MHSA 24	MHSA 25	80																	634.2	45.5	1050	CONC	0.10	900.9	1.01	70%
MHSA 25	MHSA 26	80																	634.2	35.8	1050	CONC	0.10	900.9	1.01	70%
MHSA 26	MHSA 27	80																	634.2	83.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 27	MHSA 28	80																	634.2	74.4	1050	CONC	0.10	900.9	1.01	70%
MHSA 28	MHSA 29	80																	634.2	77.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 29	MHSA 30	80																	634.2	83.8	1050	CONC	0.10	900.9	1.01	70%
MHSA 30	MHSA 31	80																	634.2	42.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 31	MHSA 32	80																	634.2	100.6	1050	CONC	0.10	900.9	1.01	70%
MHSA 32	MHSA 33	80																	634.2	13.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 33	MHSA 34	80																	634.2	99.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 34	MHSA 35	80																	634.2	99.9	1050	CONC	0.10	900.9	1.01	70%
MHSA 35	MHSA 36	80																	634.2	88.7	1050	CONC	0.10	900.9	1.01	70%
MHSA 36	MHSA 37	80																	634.2	88.8	1050	CONC	0.10	900.9	1.01	70%
MHSA 37	MHSA 38	80																	634.2	90.3	1050	CONC	0.10	900.9	1.01	70%
MHSA 38	MHSA 39	80																	634.2	87.5	1050	CONC	0.10	900.9	1.01	70%

Design Parameters:

Residential Land Use	Population Density (Units / ha)	Persons per Unit	Persons per ha
Low Density (singles / semis)	26 - 28 (28 used)	2.7 - 3.4 (3.4 used)	95.2
Medium Density (row / townhouse)	50 - 60 (60 used)	2.7	162.0
High Density (apartments)	60 - 75 (75 used)	1.8	135.0

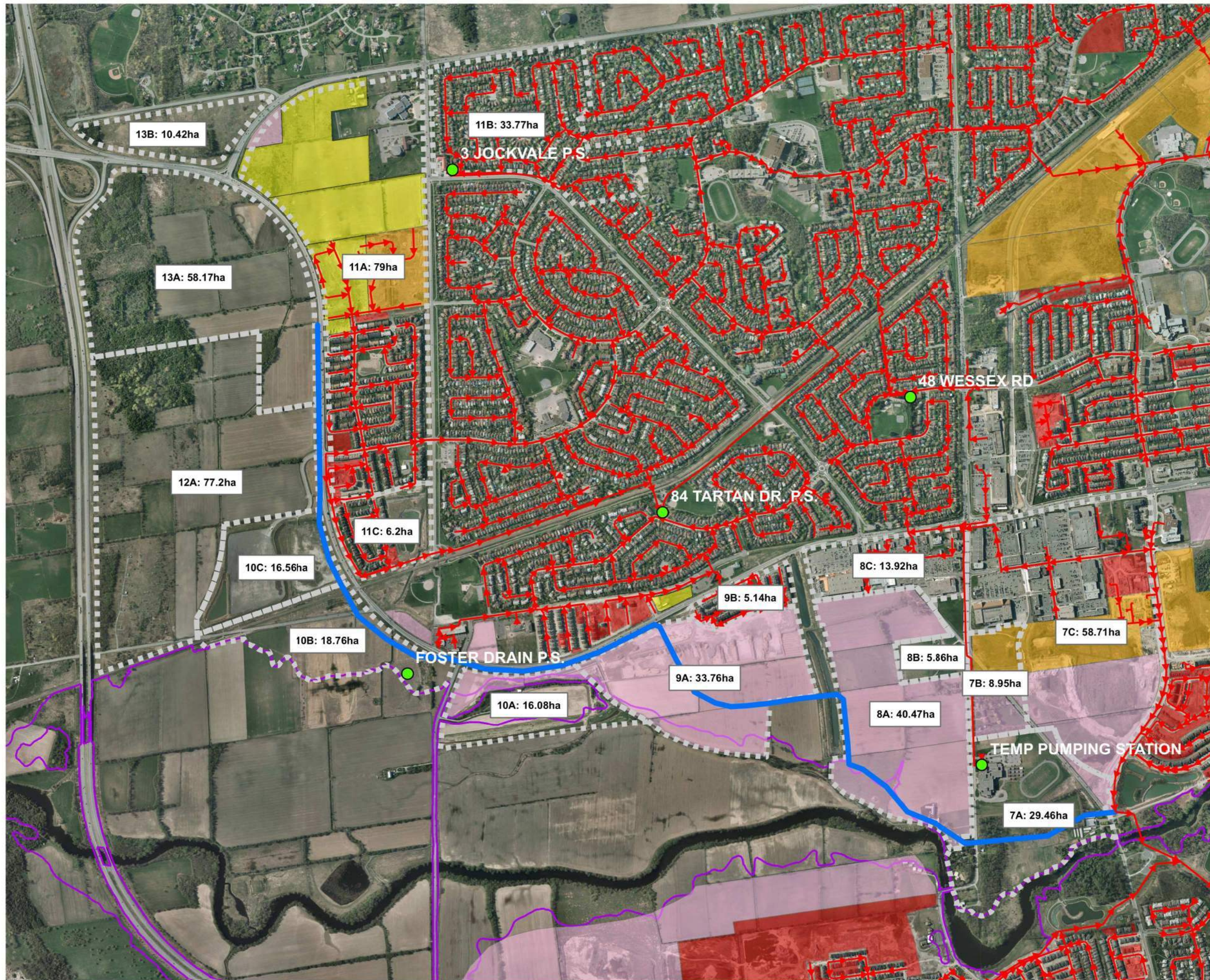
Notes:

- Harmon Equation = $1 + [14 / (4 + (P/1000)^{1/2})] \times K$
Where: P = population; K = correction factor = 1.0
- Institutional / Commercial Peaking Factor = 1.5

Reported Design Flows / Assumptions:

- Area A4: Existing single family units currently serviced by Jockvale pump station to be redirected to SNC
- Area A8-D: proposed 600 medium density residential units

Figure 01
Existing Sanitary Network and Collection Areas



- Pump Station
 - Existing Sanitary Main (With Flow Direction)
 - Proposed Alignment for South Nepean Collector
 - Collection Area
- DEVELOPMENT STATUS**
- Registered
 - Draft Approved
 - Pending
 - No Plan
 - Floodplain

NOT TO SCALE



MAP DRAWING INFORMATION:
DATA PROVIDED BY THE CITY OF OTTAWA

MAP CREATED BY: BC
MAP CHECKED BY: MBM
MAP PROJECTION: NO PROJECTION

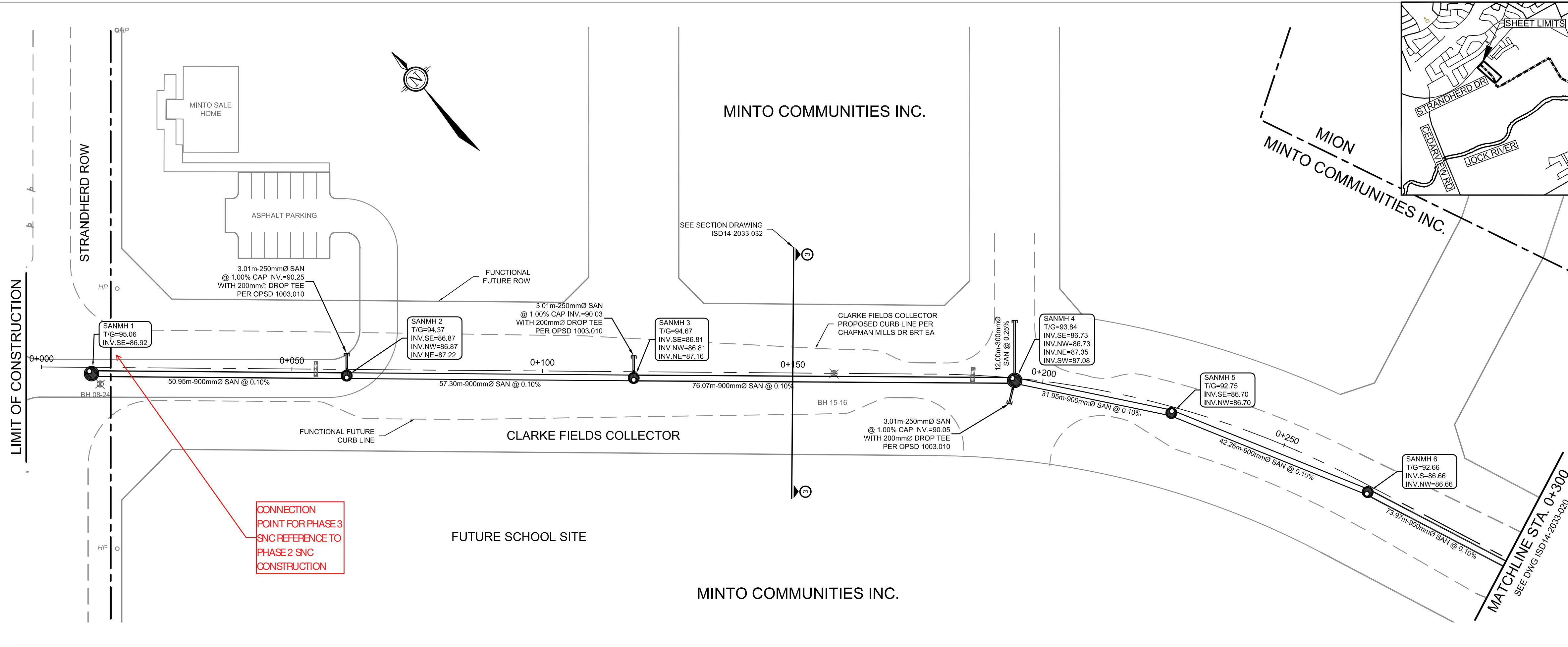
FILE LOCATION: \\Dillon.ca\dillon_dfs\Ottawa\Ottawa_GA\CAD\2011\115681\Design_GIS\MXD\Figure01c_ExistingSanitaryNetwork.mxd

Table 5.1: Allocation of Commercial/Institutional and Residential Demands to SNC by Collection Area

Collection Area	Discharging Node	Estimated from GIS			City of Ottawa VURL Data			Other Space ¹ (ha)	Population (PE)	Residential Density (PE/net ha)	Comments	Additional Source(s)
		Gross Institutional/Commercial (ha)	Gross Residential (ha)	Gross Area (ha)	Net Residential (ha)	Units (#)	Unit Density (#/ha)					
7A	70	13.5	7.4	29.5	4.0	605	0.3	9.1	1637	4.25	Flow calculations include St Joseph H.S. Pump Station firm capacity of 7.0 L/s Additional 600 units (TAC)	3.4ppu (TAC)
7B		0.0	9.24	9.24	6.23	1474	136.7	3.0	3321	638.8	Population from split VURL allocated by area. VURL parcel id 323 - inconsistency between net and gross reported area.	2.7ppu (TAC)
8A		0.0	40.0	40.0	24.1	4462	185.1	15.9	12047.4	499.9		2.7ppu (TAC)
8B		5.9	0.0	5.9	0.0	0	0	0.0			Future Commercial area	
8C		13.9	0.0	13.9	0.0	0	0	0.0			Commercial area includes Home Depot	
9A	80	0.0	33.8	33.8	18.6	635	34.1	15.2	2210	116.2		3.4ppu (TAC)
10A	90	0.0	16.1	16.1	9.7	451	28.0	6.4	1533.4	158.0	Assume net population = 60% gross.	3.4ppu (TAC)
10B	100	18.8	0.0	35.3	0.0	0	0	16.5			Allocated as potential future I/C use as directed by TAC	
10C	110	16.6	0.0	35.3	0.0	0	0	18.7			Area includes current Municipal Snow Dump. Flow allowance is made for potential future I/C use	
11C		0.0	6.2	6.2	Note 2			2.5	306	82.7	This area is south of '11 block' in the existing development	From IBI Apr 2010 Report Figure 1
11A	120	12.5	66.5	79.0				26.6	3923	98.3	Institutional includes 4.38ha church site and 6.89 ha institution at northeast corner, as well and Claridge Commercial (0.56ha) and DCR/Phoenix Commercial (0.64ha)	From IBI Apr2010 Report Figure 1
11B		0.0	37.0	37.0				14.8	1550	69.8	Presently serviced by Jockvale pump station; to be redirected to SNC.	Estimated from 2011 Census Block data
12A		77.2	0.0	77.2				0.0			Allow sanitary peak flow 79.0 L/s	Novatech, Employment Lands Report, Revised Jan 2012
13A	130	58.5	0.0	58.5				0.0			Allow sanitary peak flow 62.8 L/s plus Collection Area 13B, total 82.2 L/s	
13B		12.5	0.0	12.5	0.0			Allow sanitary peak flow 19.4 L/s; gravity discharge to Collection Area 13A	IBI/Novatech			

Notes:

- Other space includes other residential space accounting for the difference between gross area (measured with GIS) and net area (provided in VURL data), such as sidewalks, roads, greenspace, etc.
- Collection Area 11A and 11B population and land use as identified under Additional Source(s). Other space reported as 60% of gross residential area, consistent with VURL average.



CITY OF OTTAWA
SOUTH NEPEAN COLLECTOR (SNC)
SEWER PHASE 2 - STRANDHERD DRIVE
TO JOCKVALE ROAD

PLAN AND PROFILE
STA. 0+000 TO 0+300

Contract No. **ISD14-2033** Dwg. No. **019**
 Sheet 19 of 51

Asset No. _____
 Asset Group **ISD**

Wayne Newell, P.Eng. General Manager
 Jonathan Knoyle, P.Eng. Senior Engineer

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 Kanata, Ontario, Canada, K2M 1P6
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 Facsimile: (613) 254-5867
 Email: novatech@novatech-eng.com

PROFESSIONAL ENGINEER
M.A. BISSETT
 PROVINCE OF ONTARIO

Des. RJD Chk'd. ERD
 Dwn. NCS Chk'd. RJD
 Utility Circ. No. _____ Index No. _____
 Const. Inspector _____

Scale: HORIZONTAL 1:500
 VERTICAL 1:100

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

No.	Description	By	Date (dd/mm/yy)
1.	ISSUED FOR PRELIMINARY DESIGN CIRCULATION	ERD	21/12/15
2.	CHANGES TO ORIENTATION ACROSS KB SWM FACILITY	ERD	16/02/16
3.	ISSUED FOR PRELIMINARY DESIGN REPORT	ERD	02/03/16
4.	ISSUED FOR FINAL DESIGN CIRCULATION	ERD	29/04/16
5.	ISSUED FOR MOECC ECA APPLICATION	ERD	26/05/16
6.	ISSUED FOR TENDER	ERD	20/06/16
7.	ISSUED FOR CONSTRUCTION	ERD	30/08/16
8.	SEWER ALIGNMENT SHIFT ON GREENBANK	ERD	16/09/16
9.	REVISED PER MION SERVICING	ERD	08/12/16

LEGEND

EXISTING ITEMS

- WATERMAIN
- WATERMAIN VALVE
- STORM SEWER
- STORM MH
- CATCH BASIN & LEAD
- CULVERT
- STORM STRUCTURE
- SANITARY SEWER
- SANITARY MH

PROPOSED ITEMS

- SANITARY SEWER
- SANITARY MH & LID
- CULVERT
- FUTURE STRUCTURE T/G ADJUSTMENT

- NOTES:**
- CONCRETE PRESSURE PIPE SHALL BE AWWA C301 (L) CL-16. FITTING SHALL BE DESIGNED TO THE SAME CRITERIA AS THE ADJACENT PIPE.
 - CONTRACTOR TO PROVIDE PIPE CLASS CALCULATIONS, AS PER AWWA C304 (DESIGN OF PRESTRESSED CONCRETE CYLINDER PIPE), BY THE PIPE MANUFACTURER, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
 - PIPE EMBEDMENT SHALL BE AS PER CITY OF OTTAWA DETAIL S6. SAND MAY BE USED AS PIPE COVER MATERIAL ABOVE THE SPRINGLINE.
 - A CLOTH DIAPER APPROVED BY THE PIPE MANUFACTURE SHALL BE PLACED AROUND EACH EXTERIOR JOINT RECESS AND FASTENED IN PLACE WITH EITHER WIRE OR STEEL STRAPPING STITCHED INTO ITS EDGES.
 - THE JOINT SHALL BE FILLED WITH MORTAR IN ONCE CONTINUOUS OPERATION AND PATTED OR MANIPULATED TO SETTLE THE MORTAR AND EXPEL AND ENTRAPPED AIR.
 - INTERIOR JOINTS SHALL BE FILLED WITH MORTAR AFTER BACKFILLING AND FINISHED SMOOTH WITH A TROWEL. CEMENT USED SHALL MEET THE REQUIREMENTS OF TYPE HS CEMENT (HIGH-SULPHATE-RESISTANCE), OR APPROVED EQUIVALENT.
 - THE INTERIOR OF THE JOINTS SHALL BE PROTECTED FROM CORROSION WITH EPOXY AND ZINC COATING APPLIED DURING FABRICATION.
 - THE INTERIOR STRUCTURAL CONCRETE CORE SHALL BE MANUFACTURED WITH TYPE HS CEMENT (HIGH-SULPHATE-RESISTANCE), OR APPROVED EQUIVALENT.
 - SEE MANHOLE DETAIL DRAWINGS ISD14-2033-36 TO ISD14-2033-45 FOR ADDITIONAL DETAILS

MAINTENANCE HOLE DATA

MH ID	STATION	OFFSET	STRUCTURE	COVER	T/G ELEV.	LOW. INV.
1	0+010.00	1.25R	OPSD 701.013	S24/S25	95.06	86.92
2	0+060.95	1.25R	OPSD 701.012	S24/S25	94.37	86.87
3	0+118.25	1.25R	OPSD 701.012	S24/S25	94.67	86.81
4	0+194.38	0.46R	OPSD 701.013	S24/S25	93.84	86.73
5	0+226.51	1.25R	OPSD 701.012	S24/S25	92.75	86.70
6	0+269.14	1.25R	OPSD 701.012	S24/S25	92.66	86.66

SANITARY SEWER PIPE DATA

CONNECTED STRUCTURES & INVERTS	DIA (mm)	LENGTH (m)	MATERIAL
SANMH 1 = 86.92 SANMH 2 = 86.87	900	50.95	AWWA C-301 (L)
SANMH 2 = 86.87 SANMH 3 = 86.81	900	57.30	AWWA C-301 (L)
SANMH 3 = 86.81 SANMH 4 = 86.73	900	76.07	AWWA C-301 (L)
SANMH 4 = 86.73 SANMH 5 = 86.70	900	31.95	AWWA C-301 (L)
SANMH 5 = 86.70 SANMH 6 = 86.66	900	42.26	AWWA C-301 (L)
SANMH 6 = 86.66 SANMH 7 = 86.59	900	73.97	AWWA C-301 (L)

STATION	EXISTING ELEVATION	CHAINAGE	DESCRIPTION	INVERT ELEVATION	DESCRIPTION	INVERT ELEVATION
0+000	83.47	0+000	SANMH 1 (2400mm ²)	86.92	50.95m - 900mm ² SAN AWWA C-301 (L) @ 0.10%	86.92
0+050	82.75	0+050	SANMH 2 (1800mm ²)	86.87	57.30m - 900mm ² SAN AWWA C-301 (L) @ 0.10%	86.87
0+100	82.74	0+100	SANMH 3 (1800mm ²)	86.81	76.07m - 900mm ² SAN AWWA C-301 (L) @ 0.10%	86.81
0+150	82.59	0+150	SANMH 4 (2400mm ²)	86.73	31.95m - 900mm ² SAN AWWA C-301 (L) @ 0.10%	86.73
0+200	82.46	0+200	SANMH 5 (1800mm ²)	86.70	42.26m - 900mm ² SAN AWWA C-301 (L) @ 0.10%	86.70
0+250	82.36	0+250	SANMH 6 (1800mm ²)	86.66	73.97m - 900mm ² SAN AWWA C-301 (L) @ 0.10%	86.66
0+300	82.29	0+300	SANMH 7 (1800mm ²)	86.59		86.59

AS-BUILT

THESE AS-BUILT PLANS HAVE BEEN PREPARED BASED ON INFORMATION PROVIDED BY OTHERS. THE DESIGN PROFESSIONAL HAS NOT VERIFIED THE ACCURACY AND/OR THE COMPLETENESS OF THIS INFORMATION AND SHALL NOT BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS WHICH MAY BE INCORPORATED HEREIN AS A RESULT.



CITY OF OTTAWA
SOUTH NEPEAN COLLECTOR (SNC)
SEWER PHASE 2 - STRANDHERD DRIVE
TO JOCKVALE ROAD

PLAN AND PROFILE
STA. 0+300 TO 0+600

Contract No. **ISD14-2033** Dwg. No. **020**
Sheet 20 of 51

Asset No. _____
Asset Group **ISD**

Wayne Newell, P.Eng. General Manager
Jonathan Knoyle, P.Eng. Senior Engineer

NOVATECH
200, 210 Mihaljev Crapfield Drive
Kanata, Ontario, Canada, K2M 1P6
Tel: (613) 254-9613
Fax: (613) 254-9817
Email: novatech@novatech.ca

Scale: HORIZONTAL 1:500
VERTICAL 1:100

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

No.	Description	By	Date (dd/mm/yyyy)
1	ISSUED FOR PRELIMINARY DESIGN CIRCULATION	FRN	21/12/15
2	CHANGES TO ORIENTATION ACROSS KB SWM FACILITY	ERD	16/02/16
3	ISSUED FOR PRELIMINARY DESIGN REPORT	ERD	02/03/16
4	ISSUED FOR FINAL DESIGN CIRCULATION	ERD	29/04/16
5	ISSUED FOR MOECC ECA APPLICATION	ERD	26/05/16
6	ISSUED FOR TENDER	ERD	20/06/16
7	ISSUED FOR CONSTRUCTION	ERD	30/08/16
8	SEWER ALIGNMENT SHIFT ON GREENBANK	ERD	16/09/16
9	REVISED PER MION SERVICING	ERD	08/12/16
10	MINTO LANDS MANHOLE UPDATE	ERD	24/04/17
11	ISSUED FOR AS-BUILT	ERD	28/09/17

LEGEND

EXISTING ITEMS
WATERMAIN
WATERMAIN VALVE
STORM SEWER
STORM MH
CATCH BASIN & LEAD
CULVERT
STORM STRUCTURE
SANITARY SEWER
SANITARY MH

PROPOSED ITEMS
SANITARY SEWER
SANITARY MH & LID
CULVERT
FUTURE STRUCTURE T/G
ADJUSTMENT

NOTES:

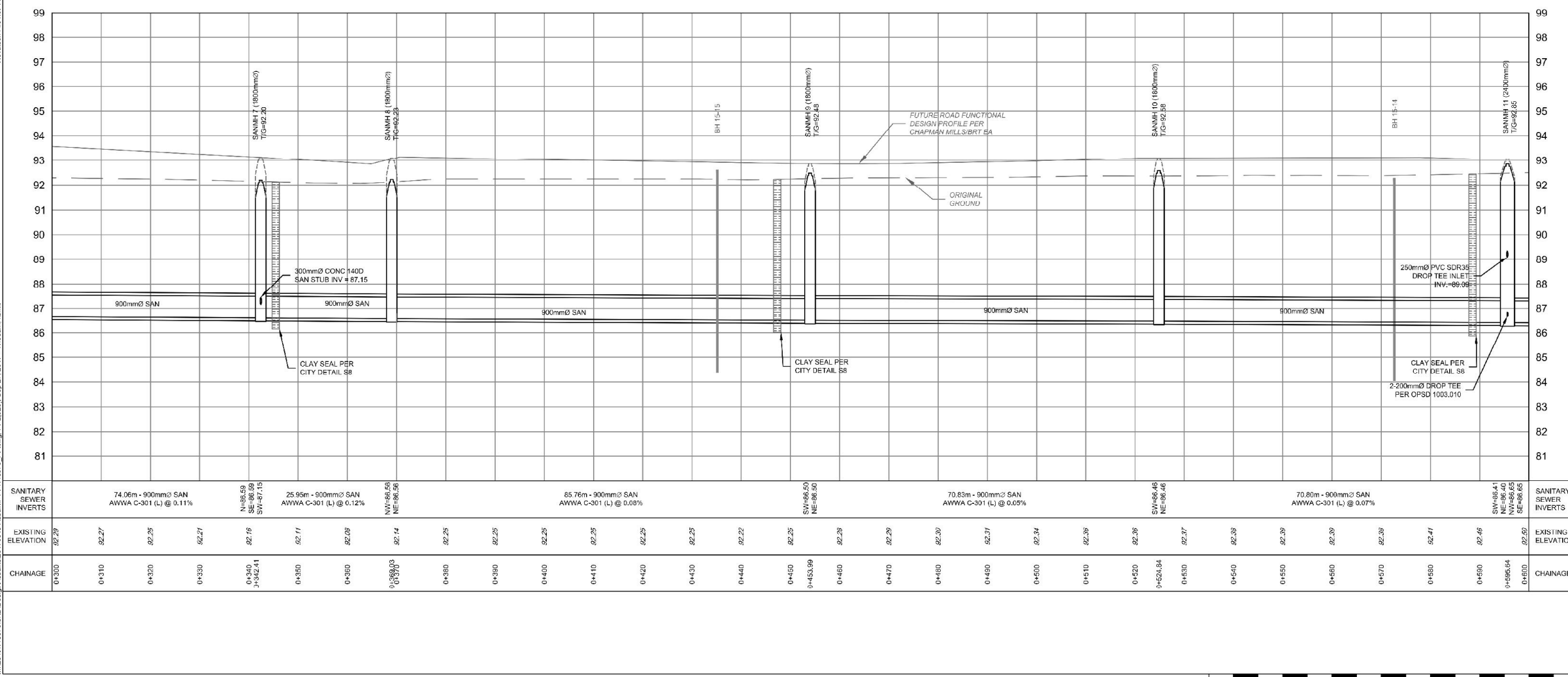
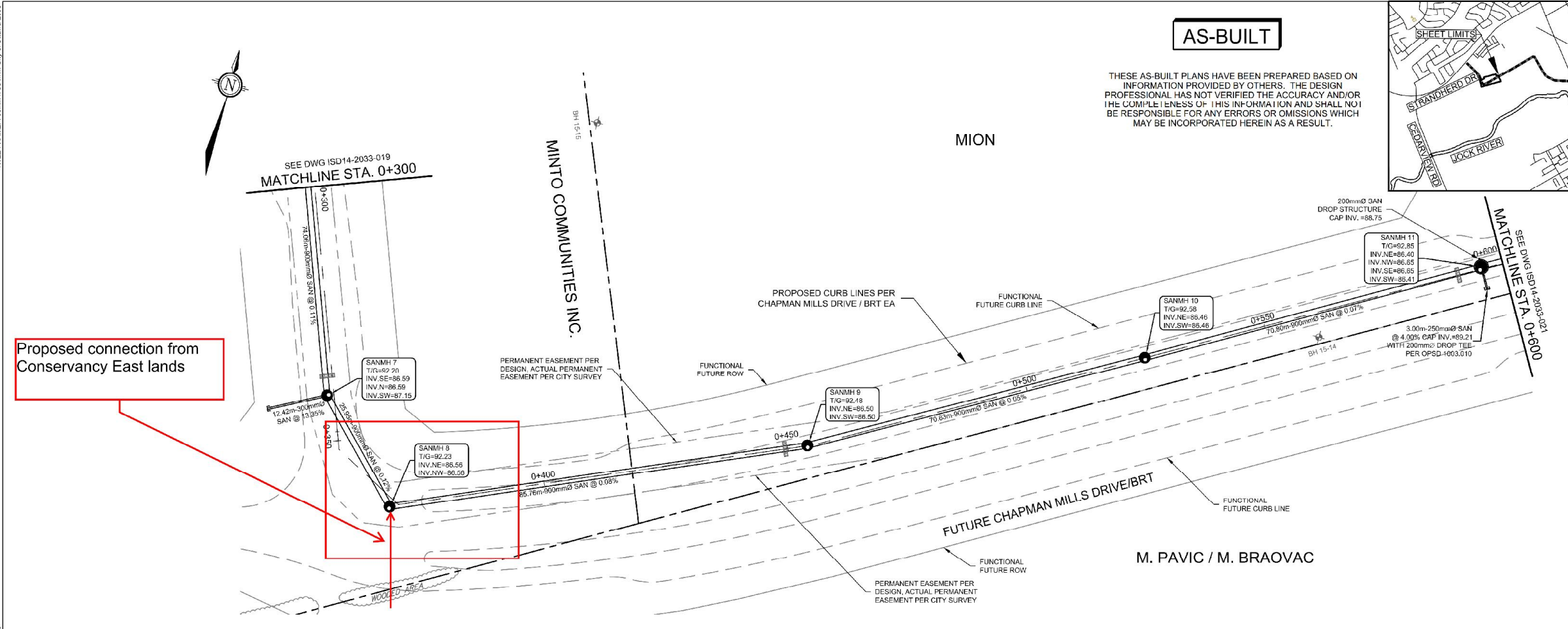
- CONCRETE PRESSURE PIPE SHALL BE AWWA C301 (L) CL-16. FITTING SHALL BE DESIGNED TO THE SAME CRITERIA AS THE ADJACENT PIPE.
- CONTRACTOR TO PROVIDE PIPE CLASS CALCULATIONS, AS PER AWWA C304 (DESIGN OF PRESTRESSED CONCRETE CYLINDER PIPE), BY THE PIPE MANUFACTURER. SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
- PIPE EMBEDMENT SHALL BE AS PER CITY OF OTTAWA DETAIL S6. SAND MAY BE USED AS PIPE COVER MATERIAL ABOVE THE SPRINGLINE.
- A CLOTH DIAPER APPROVED BY THE PIPE MANUFACTURER SHALL BE PLACED AROUND EACH EXTERIOR JOINT RECESS AND FASTENED IN PLACE WITH EITHER WIRE OR STEEL STRAPPING STITCHED INTO ITS EDGES.
- THE JOINT SHALL BE FILLED WITH MORTAR IN ONCE CONTINUOUS OPERATION AND PATTED OR MANIPULATED TO SETTLE THE MORTAR AND EXPEL AND ENTRAPPED AIR.
- INTERIOR JOINTS SHALL BE FILLED WITH MORTAR AFTER BACKFILLING AND FINISHED SMOOTH WITH A TROWEL. CEMENT USED SHALL MEET THE REQUIREMENTS OF TYPE HS CEMENT (HIGH-SULPHATE-RESISTANCE), OR APPROVED EQUIVALENT.
- THE INTERIOR OF THE JOINTS SHALL BE PROTECTED FROM CORROSION WITH EPOXY AND ZINC COATING APPLIED DURING FABRICATION.
- THE INTERIOR STRUCTURAL CONCRETE CORE SHALL BE MANUFACTURED WITH TYPE HS CEMENT (HIGH-SULPHATE-RESITANCE), OR APPROVED EQUIVALENT.
- SEE MANHOLE DETAIL DRAWINGS ISD14-2033-36 TO ISD14-2033-45 FOR ADDITIONAL DETAILS

MAINTENANCE HOLE DATA

MH ID	STATION	OFFSET	STRUCTURE	COVER	T/G ELEV	LOW. INV.
7	0+342.41	1.41R	OPSD 701.012	S24/S25	92.20	86.59
8	0+389.03	1.50R	OPSD 701.012	S24/S25	92.23	86.56
9	0+453.99	0.07L	OPSD 701.012	S24/S25	92.48	86.50
10	0+524.84	0.04L	OPSD 701.012	S24/S25	92.58	86.46
11	0+595.64	0.41L	OPSD 701.013	S24/S25	92.85	86.40

SANITARY SEWER PIPE DATA

CONNECTED STRUCTURES & INVERTS	DIA (mm)	LENGTH (m)	MATERIAL
SANMH 6 = 86.68 SANMH 7 = 86.59	900	74.06	AWWA C-301 (L)
SANMH 7 = 86.59 SANMH 8 = 86.56	900	25.95	AWWA C-301 (L)
SANMH 8 = 86.56 SANMH 9 = 86.50	900	85.76	AWWA C-301 (L)
SANMH 9 = 86.50 SANMH 10 = 86.46	900	70.83	AWWA C 301 (L)
SANMH 10 = 86.46 SANMH 11 = 86.41	900	70.80	AWWA C-301 (L)
SANMH 11 = 86.41 SANMH 12 = 86.32	900	78.15	AWWA C-301 (L)



TITLE FRAME: 700mm x 634mm, City of Ottawa 2008
Novatech File No. 115075
M:\2015\115075\CADD\Design\Adv\115075_Plan.dwg, IP:2020, Sep 27, 2017, 10:08am, nsm

SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

LOCATION		RESIDENTIAL AREA AND POPULATION							COMM		INSTIT		PARK		C+H		INFILTRATION				PIPE						
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	POP.	CUMULATIVE		PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL.	
						AREA (ha)	POP.																			(FULL) (m/s)	(ACT.) (m/s)
Centerline13 - 13																											
	529A	530A	0.15		16	0.15	16	3.7	0.19		0.00	0.00	0.00	0.00	0.00	0.15	0.15	0.04	0.24	20.5	200	0.65	26.44	0.01	0.84	0.26	
			0.19		20	0.34	36				0.00	0.00	0.00	0.00	0.19	0.34											
	530A	534A	0.26		53	0.60	89	3.6	1.04		0.00	0.00	0.00	0.00	0.26	0.60	0.17	1.21	70.5	250	0.25	29.73	0.04	0.61	0.29		
			0.22		23	0.82	112				0.00	0.00	0.00	0.00	0.22	0.82											
	534A	531A	0.26		28	1.08	140	3.6	1.62		0.00	0.00	0.00	0.00	0.26	1.08	0.31	1.92	82.0	250	0.25	29.73	0.06	0.61	0.34		
	To Centerline1 - 01, Pipe 531A - 532A					1.08	140				0.00	0.00	0.00	0.00	1.08												
Centerline9 - 09																											
	525A	527A	0.62		65	0.62	65	3.6	0.77		0.00	0.00	0.00	0.00	0.62	0.62	0.18	0.94	104.0	200	0.65	26.44	0.04	0.84	0.39		
	To Centerline6 - 06, Pipe 527A - 528A					0.62	65				0.00	0.00	0.00	0.00	0.62	0.62											
Centerline6 - 06																											
	526A	527A	0.21		22	0.21	22	3.7	0.26		0.00	0.00	0.64	0.64	0.07	0.85	0.85	0.24	0.58	63.0	200	0.65	26.44	0.02	0.84	0.34	
	Contribution From Centerline9 - 09, Pipe 525A - 527A					0.62	65				0.00	0.00	0.00	0.00	0.62	1.47											
	527A	528A	0.12		12	0.95	99	3.6	1.15		0.00	0.00	0.64	0.64	0.07	1.12	1.59	0.45	1.68	44.5	200	0.35	19.40	0.09	0.62	0.37	
	To Centerline14 - 14, Pipe 528A - 531A					0.95	99				0.00	0.00	0.64	0.64		1.59											
Centerline14 - 14																											
	523A	524A				0.00				1.47	1.47	0.00	0.00	0.48	1.47	1.47	0.42	0.90	26.5	200	0.65	26.44	0.03	0.84	0.39		
			0.25		26	0.25	26				1.47	0.00	0.00	0.00	0.25	1.72											
	524A	528A	0.45		47	0.70	73	3.6	0.86		1.47	0.00	0.00	0.48	0.45	2.17	0.62	1.95	117.5	200	0.35	19.40	0.10	0.62	0.39		
	Contribution From Centerline6 - 06, Pipe 527A - 528A					0.95	99				0.00	0.00	0.64	0.64	1.59	3.76											
	528A	531A	0.17		18	1.82	190	3.5	2.17		1.47	0.00	0.64	0.64	0.55	0.17	3.93	1.12	3.84	48.5	200	0.35	19.40	0.20	0.62	0.48	
	To Centerline1 - 01, Pipe 531A - 532A					1.82	190				1.47	0.00	0.64	0.64		3.93											
Centerline8 - 08																											
	533A	522A	0.16		14	0.16	14	3.7	0.17		0.00	0.00	0.00	0.00	0.16	0.16	0.05	0.21	40.5	200	0.65	26.44	0.01	0.84	0.24		
	522A	520A				0.16	14	3.7	0.17		0.00	0.00	0.00	0.00	0.00	0.16	0.05	0.21	13.0	200	0.35	19.40	0.01	0.62	0.20		
	520A	521A	0.86		72	1.02	86	3.6	1.01		0.00	0.00	0.00	0.00	0.86	1.02	0.29	1.30	137.0	250	0.25	29.73	0.04	0.61	0.30		
	To Centerline1 - 01, Pipe 521A - 531A					1.02	86				0.00	0.00	0.00	0.00	1.02												
Centerline15 - 15																											
	501A	503A	0.68		71	0.68	71	3.6	0.83		0.00	0.00	0.00	0.00	0.68	0.68	0.19	1.03	101.0	200	0.65	26.44	0.04	0.84	0.40		
	To Centerline4 - 04, Pipe 503A - 509A					0.68	71				0.00	0.00	0.00	0.00	0.68	0.68											
	502A	503A	0.49		51	0.49	51	3.7	0.60		0.00	0.00	0.00	0.00	0.49	0.49	0.14	0.74	21.0	200	0.65	26.44	0.03	0.84	0.37		
	To Centerline4 - 04, Pipe 503A - 509A					0.49	51				0.00	0.00	0.00	0.00	0.49	0.49											
Centerline4 - 04																											
	Contribution From Centerline15 - 15, Pipe 501A - 503A					0.68	71				0.00	0.00	0.00	0.00	0.68	0.68											
	Contribution From Centerline15 - 15, Pipe 502A - 503A					0.49	51				0.00	0.00	0.00	0.00	0.49	1.17											
	503A	509A	0.50		52	1.67	174	3.5	1.99		0.00	0.00	0.00	0.00	0.50	1.67	0.48	2.47	108.5	250	0.25	29.73	0.08	0.61	0.36		
	To Centerline11 - 11, Pipe 509A - 510A					1.67	174				0.00	0.00	0.00	0.00	1.67												
Centerline2 - 02																											
	507A	508A	0.41		43	0.41	43	3.7	0.51		0.00	0.00	0.00	0.00	0.41	0.41	0.12	0.63	93.5	200	0.65	26.44	0.02	0.84	0.35		
	To Centerline11 - 11, Pipe 508A - 509A					0.41	43				0.00	0.00	0.00	0.00	0.41	0.41											

Park Flow = 9300 L/ha/da Average Daily Flow = 280 l/p/day Comm/Inst Flow = 28000 L/ha/da Industrial Flow = 35000 L/ha/da Max Res. Peak Factor = 4.00 Commercial/Inst./Park Peak Factor = 1.00 Institutional = 0.32 l/s/ha	DESIGN PARAMETERS 0.10764 l/s/ha Industrial Peak Factor = as per MOE Graph Extraneous Flow = 0.286 L/s/ha Minimum Velocity = 0.600 m/s Manning's n = (Conc) 0.013 (Pvc) 0.013 Townhouse coeff= 2.7 Single house coeff= 3.4												Designed: A.S. Checked: W.L./V.W. Dwg. Reference: Sanitary Drainage Plan, Dwgs. No.	PROJECT: Barrhaven Conservancy East Phase 5 LOCATION: City of Ottawa File Ref:	Date: 01 Dec 2022	Sheet No. 1 of 2
---	---	--	--	--	--	--	--	--	--	--	--	--	---	--	--------------------------	---------------------

SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

Main calculation table with columns for LOCATION, RESIDENTIAL AREA AND POPULATION, COMM, INSTIT, PARK, C+H, INFILTRATION, PIPE, and VEL. Includes data for various centerlines and pipe segments.

Lower flows than what was previously projected (77.81 L/s) in the Phase 2, 3, & 4 sanitary design sheet downstream.

DESIGN PARAMETERS table containing flow rates, factors, and velocity. Includes a sub-table for PROJECT INFORMATION with details for Barrhaven Conservancy East Phase 5.

SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

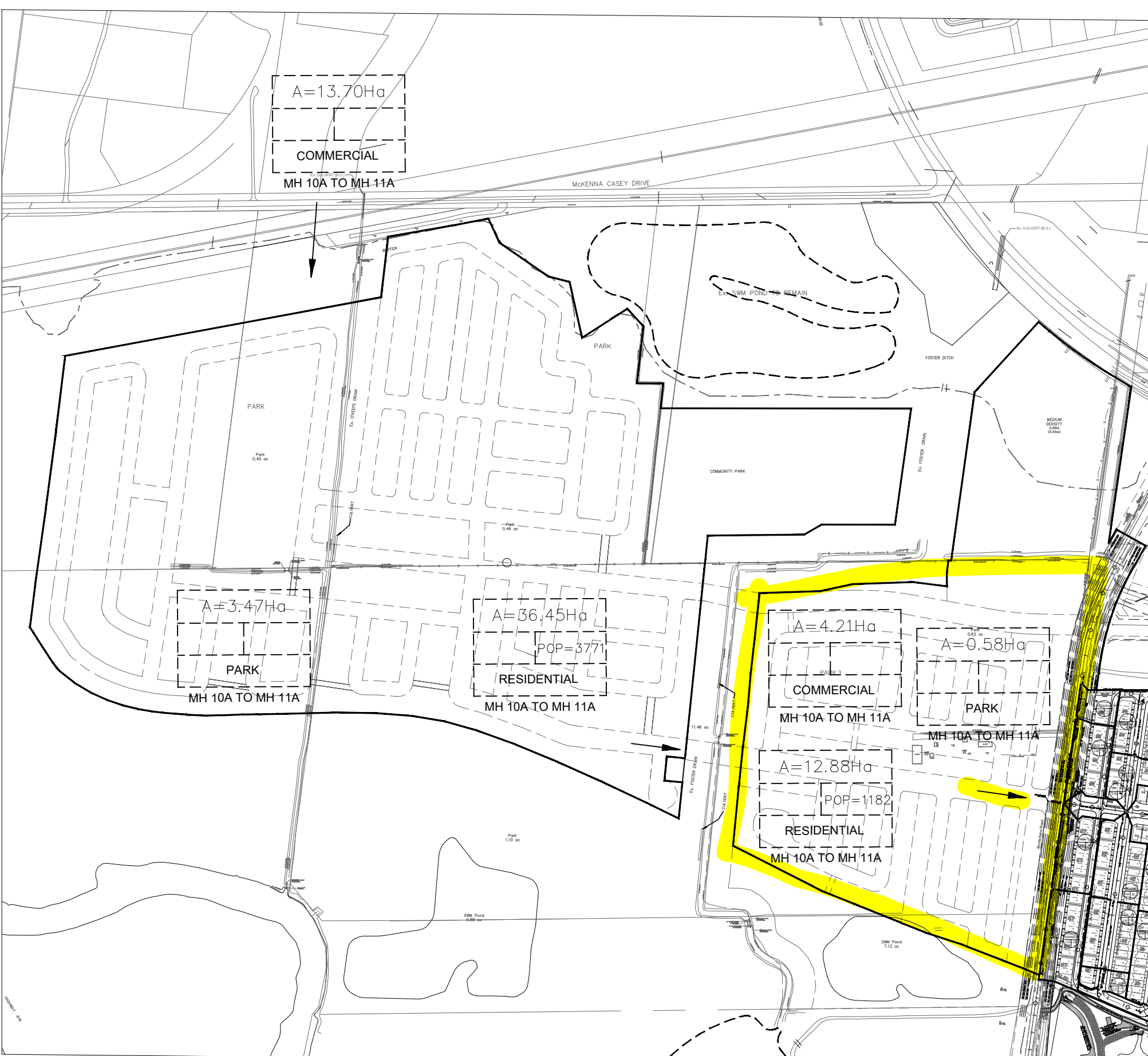
Table with columns: LOCATION, RESIDENTIAL AREA AND POPULATION, COMM, INSTIT, PARK, C+I, INFILTRATION, PIPE, and VEL. Rows include street addresses like 31A-32A, 30A-33A, 250A-26A, etc., with various numerical values.

DESIGN PARAMETERS

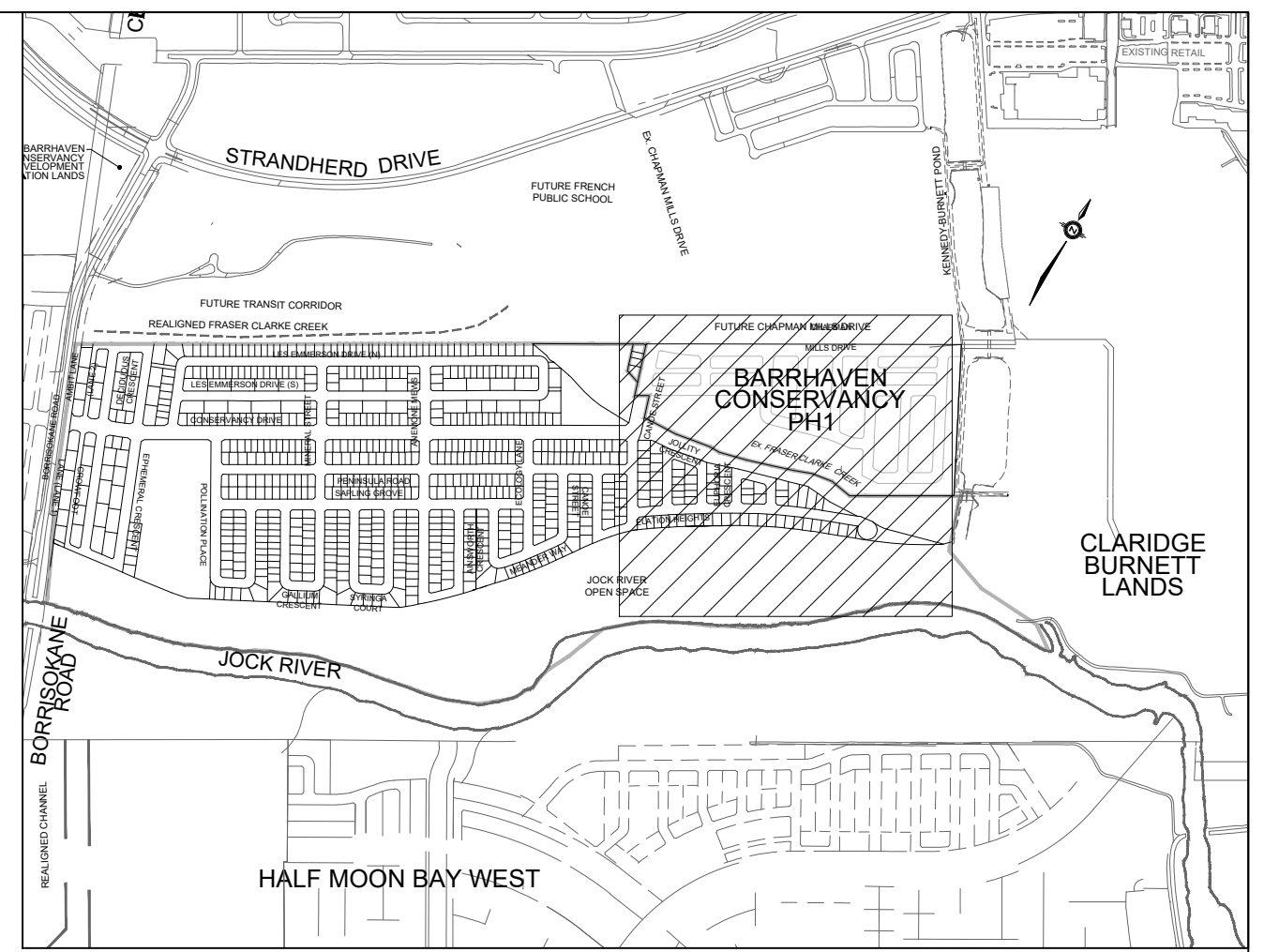
Park Flow = 9300 L/ha/day, Average Daily Flow = 280 l/p/day, Industrial Peak Factor = as per MOE Graph, etc.



Designed: A.K., Checked: W.L., PROJECT: BARRHAVEN CONCERVANCY EAST PH2, 3, AND JOCK RIVER, LOCATION: City of Ottawa, Sheet No. 2 of 6

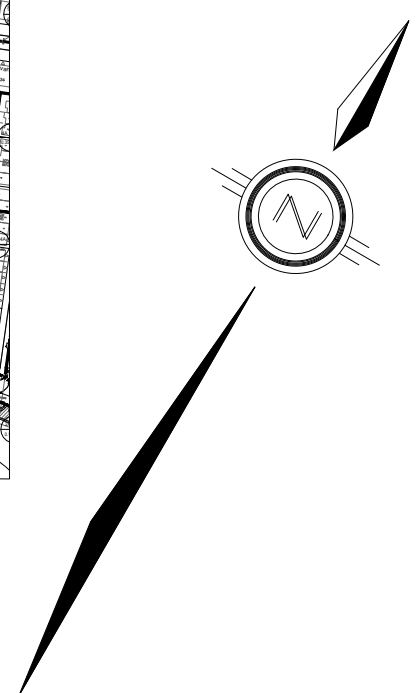


EXTERNAL SANITARY DRAINAGE PLAN
SCALE: 1:5000



LEGEND

SANITARY DRAINAGE BOUNDARY	
SANITARY SUB-DRAINAGE BOUNDARY	
UPSTREAM MH TO DOWNSTREAM MH AREA IN HECTARES	43A - 44A 0.78 61
POPULATION	43A - 44A 0.78 61
UPSTREAM MH TO DOWNSTREAM MH AREA IN OTHER PHASES IN HECTARES	43A - 44A 0.78 61
POPULATION	43A - 44A 0.78 61
EXTERNAL AREA IN HECTARES	A=53.63
EXTERNAL POPULATION	107 POP=5739
DENSITY (PERSONS/HECTARE)	107 POP=5739
EXTERNAL LAND USE	RESIDENTIAL
MAINTENANCE HOLE	MH202A
CAP	
EXISTING SANITARY MAINTENANCE HOLE	



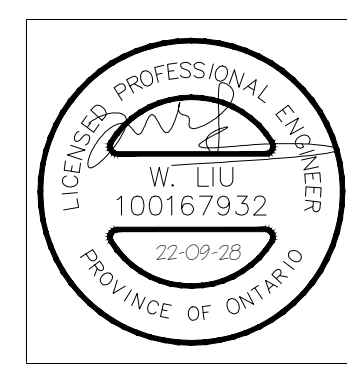
TOPOGRAPHIC INFORMATION
TOPOGRAPHIC INFORMATION PROVIDED BY J.D. BARNES LIMITED,
PROJECT NO. 16-10-127-00, SURVEY DATED APRIL 10, 2018.
LEGAL INFORMATION
M-PLAN PROVIDED BY J.D. BARNES, PROJECT NO.
16-10-127-00, RECEIVED ON AUGUST 2, 2022.
NOT FOR CONSTRUCTION

ELEVATION NOTE
ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM
AND ARE REFERRED TO THE PUBLISHED BENCH MARK NO. D011964U3710
ELEVATION = 71.724m

No.	BY	DATE	DESCRIPTION
7	W.L.	22-09-28	RETAINING WALL UPDATES
6	W.L.	22-09-27	WATERMAIN AND CUP UPDATES
5	W.L.	22-08-31	CITY COMMENTS AND TRANSPORTATION UPDATES
4	W.L.	22-08-10	REVISED STREET NAME & LOT 99 SIGHT TRIANGLE
3	W.L.	22-06-28	3rd SUBMISSION
2	W.L.	22-04-22	2nd SUBMISSION
1	W.L.	21-12-22	1st SUBMISSION

CITY OF OTTAWA

PROJECT No. 20-1180



SANITARY DRAINAGE PLAN

BARRHAVEN
CONSERVANCY
DEVELOPMENT
CORPORATION

BARRHAVEN
CONSERVANCY EAST
PHASE 2, 3 & JOCK RIVER



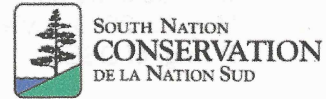
DRAWN BY: A.K./R.A./V.W.	CHECKED BY: W.L.	SHEET NO.
DESIGNED BY: W.L.	CHECKED BY: C.M.	112
SCALE: 1:1000	DATE: DECEMBER 2021	

CITY PLAN No. 18754
CITY FILE No. D07-16-20-0021

APPENDIX D

STORMWATER

Conservation Partners Partenaires en conservation



May 31, 2020

City of Ottawa
110 Laurier Avenue,
Ottawa, ON K1P 1J1

Attention: Doug James

Subject: **Barrhaven Conservancy Development Corporation
Status of As-Built Grading
Related: RVCA Permit # RV5-4419 and RV5-1718)
Vacant land on the north side of the Jock River generally bounded by
Highway 416 and the Fraser Clarke Creek, City of Ottawa**

Dear Mr. James:

The RVCA has reviewed information recently submitted by David Schaeffer Engineering Ltd. including as-built grades in support of works approved by the Rideau Valley Conservation Authority under Section 28 of the Conservation Authorities Act (Permit File Number: RV5-4419 and RV5-1718). The RVCA offers the following comments related to future development proposed for the area within the scope of approved the permits.

The subject lands as identified as part of Lots 11, 12, 13, 14, 15 former geographic Township of Nepean, Concessions 3 & 4, now in the City of Ottawa have been addressed through the general placement of fill and the formal construction of a berm around the perimeter of four blocks within the subject lands. The site specific elevations of the berm have been reviewed by the RVCA and are generally accepted as being appropriate as removing these lands from the floodplain in accordance with the aforementioned approved permits.

The detailed grading plans submitted by David Schaeffer Engineering Ltd. titled "As Constructed plan of Berms and Cut Areas – Barrhaven Conservancy", dated May 27, 2020, prepared by Adam Fobert, P.Eng. of DSEL, DSEL File Number 16891 using the following resources:

- Orthoimagery Survey, dated April 20, 2020, acquired and processed by First Base Solutions a division of JD Barnes Ltd and certified by Chris Fox, O.L.S., A.L.S., P. Eng. of JD Barnes Ltd, file reference number 2037OTTA0001; ·
- Topographic Detail of Part of Lot 13, 14, & 15 Concession 3&4, dated May 6, 2020, certified by Chris Fox, O.L.S., A.L.S., P. Eng. of JD Barnes Ltd, file reference number 16-10-127-00; ·



- Contractor as-built collected by the Tomlinson Group of Companies of Phase 1 dated May 15, 2020, reviewed by Jeremy Chouindard, EIT and certified by Stephen Pichette, P.Eng. of DSEL

The above information indicates that land within the berm have generally been raised to exceed the flood elevation cross sections throughout the project area. However, it is noted that as this is considered an active construction site the presence of lower areas to manage construction, on-site erosion and sediment control show lower elevations. These areas will be addressed through the construction process, as sufficient material is presently stockpiled for this purpose to ensure. For the purposes of the floodplain, these areas are considered removed by virtue of the berm.

Conclusion:

The grade modifications, including construction of the berm and filling behind the berm, as documented in the above noted "as constructed" plans, have been completed in accordance with the plans approved by the RVCA under permits RV5-4419 and RV5-1718.

Please feel free to contact our office with any questions or comments you may have.

Respectfully,

Terry Davidson, P.Eng
Director of Engineering and Regulations
Rideau Valley Conservation Authority
613-692-3571 x1107
terry.davidson@rvca.ca

attach: Technical memorandum by Evelyn Liu, M.Asc., P.Eng. Water Resources
 Engineer, RVCA dated May 29, 2020

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)



Local Roads Return Frequency = 2 years
 Collector Roads Return Frequency = 5 years
 Arterial Roads Return Frequency = 10 years

Manning 0.013

LOCATION			AREA (Ha)																FLOW					SEWER DATA														
			2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO					
Location	From Node	To Node	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full						
Centerline14 - 14																																						
Contribution From Centerline15 - 15, Pipe 526 - 529					2.07					0.00					0.54			0.00		10.95																		
Contribution From Centerline15 - 15, Pipe 527 - 529					0.00					0.00					0.61			0.00		10.82																		
	529	530	0.52	0.80	1.16	3.22			0.00	0.00			0.00	1.16			0.00	0.00	10.95	73.33	99.42	116.52	170.30	371	825	825	CONC	0.11	26.5	476.0801	0.8906	0.4959	0.780					
	530	534			0.00	3.22			0.00	0.00			0.00	1.16			0.00	0.00	11.45	71.66	97.12	113.81	166.33	363	825	825	CONC	0.11	117.0	476.0801	0.8906	2.1895	0.762					
Contribution From Centerline6 - 06, Pipe 533 - 534					2.67					0.00				0.00				0.00		12.72																		
	534	537	0.11	0.80	0.24	6.14			0.00	0.00			0.00	1.16			0.00	0.00	13.64	65.18	88.24	103.36	150.97	520	900	900	CONC	0.15	48.5	701.1305	1.1021	0.7334	0.742					
To Centerline13 - 13, Pipe 537 - 538					6.14					0.00				1.16			0.00		14.37																			
Centerline13 - 13																																						
Contribution From Centerline14 - 14, Pipe 534 - 537					6.14					0.00				1.16			0.00		14.37																			
Contribution From Centerline1 - 01, Pipe 536 - 537					0.00	6.14	0.06	0.67	0.11	0.98			0.00	1.16			0.00	0.00	12.10																			
	537	538	0.28	0.67	0.52	6.66			0.00	0.98			0.00	1.16			0.00	0.00	14.37	63.29	85.65	100.31	146.50	622	975	975	CONC	0.15	83.0	867.9562	1.1625	1.1899	0.716					
	538	543	0.25	0.67	0.47	7.13			0.00	0.98			0.00	1.16			0.00	0.00	15.56	60.48	81.79	95.77	139.84	622	975	975	CONC	0.15	69.0	867.9562	1.1625	0.9892	0.717					
Contribution From Centerline8 - 08, Pipe 542 - 543					1.92					0.00				0.00				0.00		13.56																		
	543	528	0.05	0.67	0.09	9.14			0.00	0.98			0.00	1.16			0.00	0.00	16.55	58.34	78.86	92.33	134.80	717	1050	1050	CONC	0.11	29.0	905.6791	1.0459	0.4621	0.792					
Contribution From Centerline12 - 12, Pipe 548 - 528					3.24					0.09				0.00				0.00		17.17																		
	528	HW9			0.00	12.38			0.00	1.07			0.00	1.16			0.00	0.00	17.17	57.08	77.15	90.32	131.84	894	1050	1050	CONC	0.20	4.5	1221.2174	1.4103	0.0532	0.732					

Definitions:
 Q = 2.78 AIR, where
 Q = Peak Flow in Litres per second (L/s)
 A = Areas in hectares (ha)
 I = Rainfall Intensity (mm/h)
 R = Runoff Coefficient

Notes:
 1) Ottawa Rainfall-Intensity Curve
 2) Min. Velocity = 0.80 m/s

Designed: A.S.	PROJECT: Barrhaven Conservancy East Phase 5
Checked: W.L./V.W.	LOCATION: City of Ottawa
Dwg. Reference:	File Ref:
Date: 01 Dec 2022	Sheet No. SHEET 3 OF 3



**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD
BASED ON A FINE PARTICLE SIZE DISTRIBUTION**



Project Name: 891 Conservancy East	Engineer: DSEL
Location: Ottawa, ON	Contact: K. Murphy
OGS #: 9	Report Date: 20-Oct-22
Area: 7.21 ha	Rainfall Station #: 215
Weighted C: 0.72	Particle Size Distribution: FINE
CDS Model: 4045 (OFFLINE)	CDS Treatment Capacity: 212 l/s

<u>Rainfall Intensity¹</u> <u>(mm/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (l/s)</u>	<u>Treated Flowrate (l/s)</u>	<u>Operating Rate (%)</u>	<u>Removal Efficiency (%)</u>	<u>Incremental Removal (%)</u>
1.0	10.6%	19.8%	14.4	14.4	6.8	96.9	10.3
1.5	9.9%	29.7%	21.6	21.6	10.2	95.9	9.5
2.0	8.4%	38.1%	28.9	28.9	13.6	95.0	8.0
2.5	7.7%	45.8%	36.1	36.1	17.0	94.0	7.2
3.0	5.9%	51.7%	43.3	43.3	20.4	93.0	5.5
3.5	4.4%	56.1%	50.5	50.5	23.8	92.0	4.0
4.0	4.7%	60.7%	57.7	57.7	27.2	91.1	4.2
4.5	3.3%	64.0%	64.9	64.9	30.6	90.1	3.0
5.0	3.0%	67.1%	72.2	72.2	34.0	89.1	2.7
6.0	5.4%	72.4%	86.6	86.6	40.8	87.2	4.7
7.0	4.4%	76.8%	101.0	101.0	47.6	85.2	3.7
8.0	3.5%	80.3%	115.5	115.5	54.4	83.3	2.9
9.0	2.8%	83.2%	129.9	129.9	61.2	81.3	2.3
10.0	2.2%	85.3%	144.3	144.3	67.9	79.4	1.7
15.0	7.0%	92.3%	216.5	212.4	100.0	68.9	4.8
20.0	4.5%	96.9%	288.6	212.4	100.0	51.7	2.3
25.0	1.4%	98.3%	360.8	212.4	100.0	41.3	0.6
30.0	0.7%	99.0%	432.9	212.4	100.0	34.4	0.2
35.0	0.5%	99.5%	505.1	212.4	100.0	29.5	0.1
40.0	0.5%	100.0%	577.3	212.4	100.0	25.8	0.1

87.1

Removal Efficiency Adjustment² = 6.5%
Predicted Net Annual Load Removal Efficiency = 80.6%
Predicted Annual Rainfall Treated = 97.1%

1 - Based on 42 years of hourly rainfall data from Canadian Station 6105976, Ottawa ON
 2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.
 3 - CDS Efficiency based on testing conducted at the University of Central Florida
 4 - CDS design flowrate and scaling based on standard manufacturer model & product specifications



**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD
BASED ON A FINE PARTICLE SIZE DISTRIBUTION**



Project Name: 891 Conservancy East
Location: Ottawa, ON
OGS #: 10

Engineer: DSEL
Contact: K. Murphy
Report Date: 20-Oct-22

Area 6.61 ha
Weighted C 0.70
CDS Model 4045 (OFFLINE)

Rainfall Station # 215
Particle Size Distribution FINE
CDS Treatment Capacity 212 l/s

<u>Rainfall Intensity¹</u> <u>(mm/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (l/s)</u>	<u>Treated Flowrate (l/s)</u>	<u>Operating Rate (%)</u>	<u>Removal Efficiency (%)</u>	<u>Incremental Removal (%)</u>
1.0	10.6%	19.8%	12.9	12.9	6.1	97.1	10.3
1.5	9.9%	29.7%	19.3	19.3	9.1	96.3	9.5
2.0	8.4%	38.1%	25.7	25.7	12.1	95.4	8.0
2.5	7.7%	45.8%	32.2	32.2	15.1	94.5	7.3
3.0	5.9%	51.7%	38.6	38.6	18.2	93.6	5.6
3.5	4.4%	56.1%	45.0	45.0	21.2	92.8	4.0
4.0	4.7%	60.7%	51.5	51.5	24.2	91.9	4.3
4.5	3.3%	64.0%	57.9	57.9	27.3	91.0	3.0
5.0	3.0%	67.1%	64.3	64.3	30.3	90.2	2.7
6.0	5.4%	72.4%	77.2	77.2	36.3	88.4	4.8
7.0	4.4%	76.8%	90.0	90.0	42.4	86.7	3.8
8.0	3.5%	80.3%	102.9	102.9	48.4	85.0	3.0
9.0	2.8%	83.2%	115.8	115.8	54.5	83.2	2.3
10.0	2.2%	85.3%	128.6	128.6	60.6	81.5	1.8
15.0	7.0%	92.3%	192.9	192.9	90.8	72.8	5.1
20.0	4.5%	96.9%	257.3	212.4	100.0	58.0	2.6
25.0	1.4%	98.3%	321.6	212.4	100.0	46.4	0.7
30.0	0.7%	99.0%	385.9	212.4	100.0	38.6	0.3
35.0	0.5%	99.5%	450.2	212.4	100.0	33.1	0.2
40.0	0.5%	100.0%	514.5	212.4	100.0	29.0	0.2

88.4

Removal Efficiency Adjustment² = 6.5%

Predicted Net Annual Load Removal Efficiency = 81.9%

Predicted Annual Rainfall Treated = 97.8%

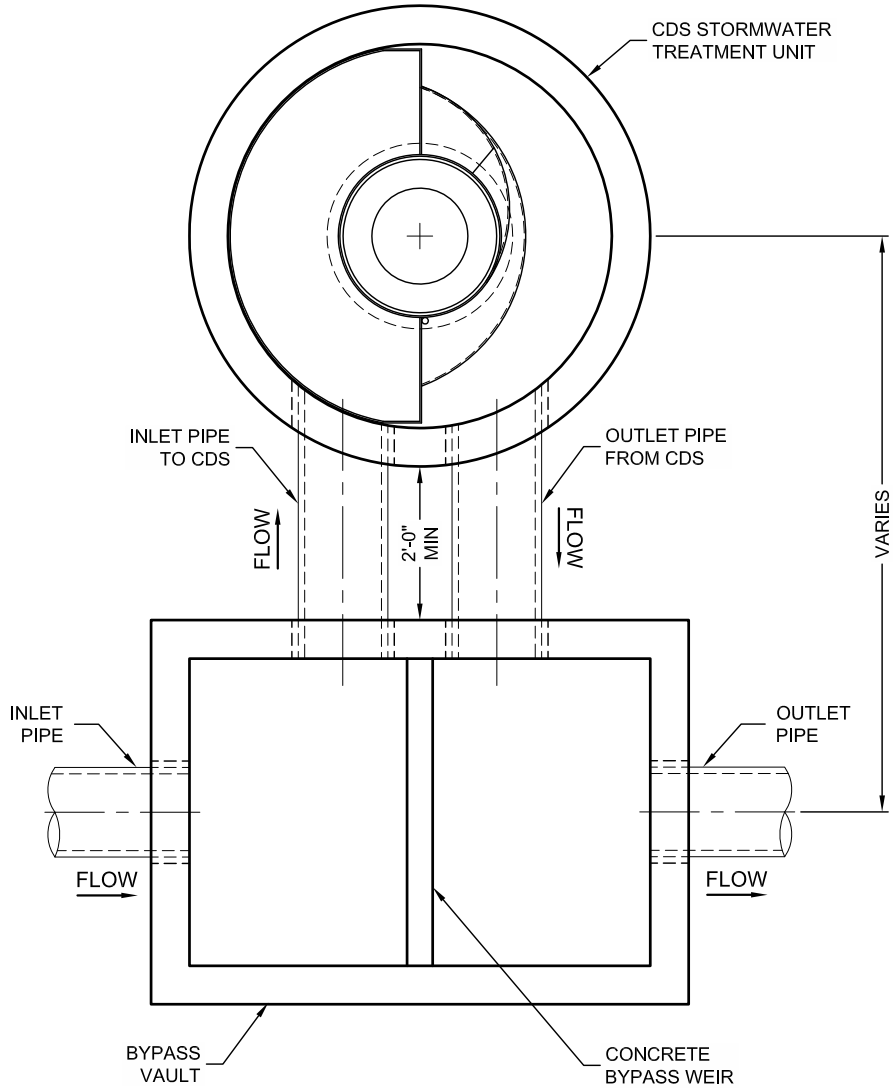
1 - Based on 42 years of hourly rainfall data from Canadian Station 6105976, Ottawa ON

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.

3 - CDS Efficiency based on testing conducted at the University of Central Florida

4 - CDS design flowrate and scaling based on standard manufacturer model & product specifications

I:\STORMWATER\COM\WOPS\22 CDS\40 STANDARD DRAWINGS\OFFLINE LAYOUTS DWG\OFFLINE CDS-C LAYOUT BYPASS VAULT STRUCTURE.DWG 3/12/2013 3:35 PM



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CDS STORMWATER TREATMENT SYSTEM
TYPICAL OFFLINE LAYOUT
WITH BYPASS VAULT STRUCTURE

DATE:03/12/13

SCALE: NONE

PROJECT No.: N/A

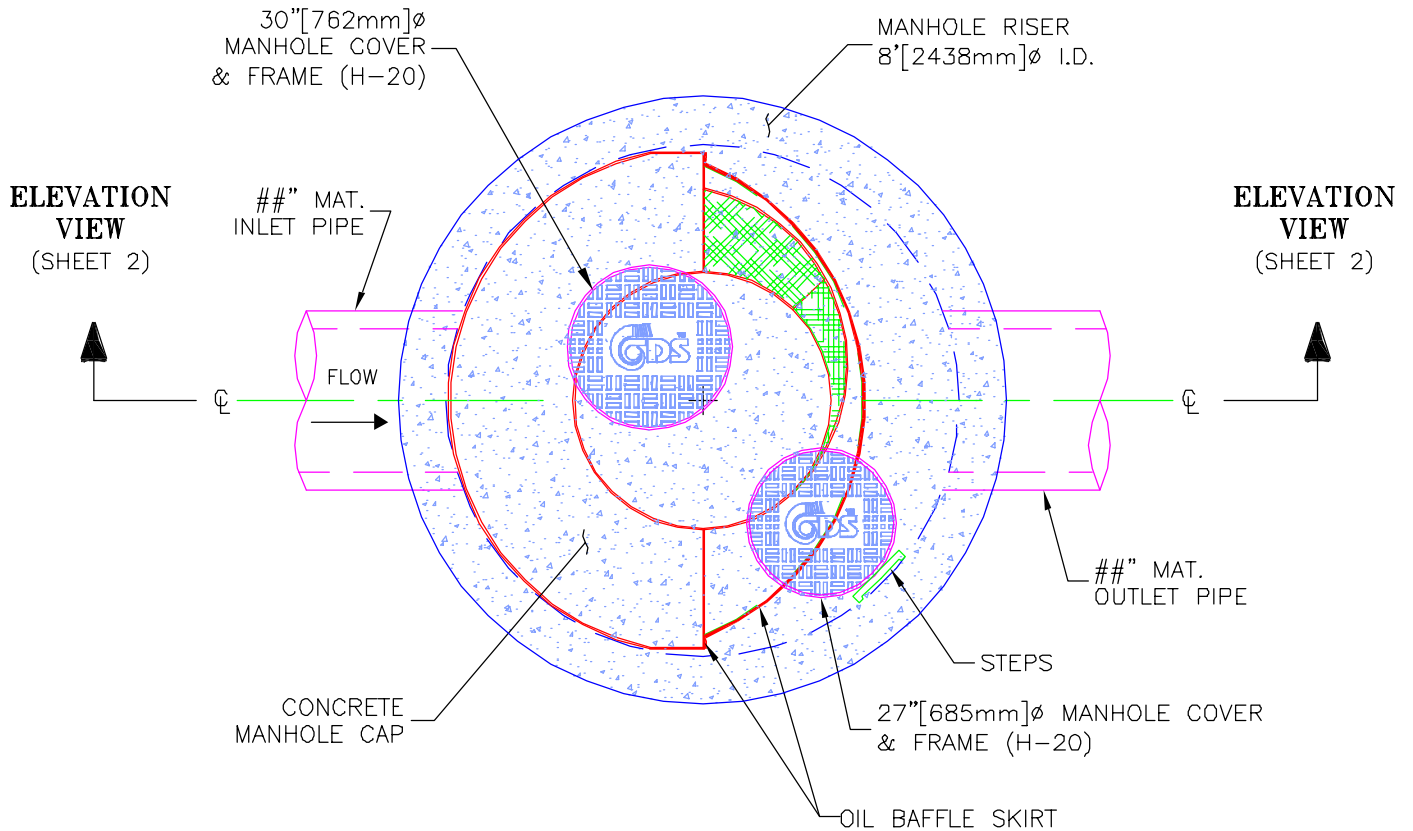
SEQ. No.: N/A

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CHECKED: N/A



PLAN VIEW



CDS MODEL PMSU40_45m, 7.5 CFS TREATMENT CAPACITY STORM WATER TREATMENT UNIT



PROJECT NAME
CITY, STATE

JOB# XX-##-###

DATE ##/##/##

DRAWN INITIALS

APPROV.

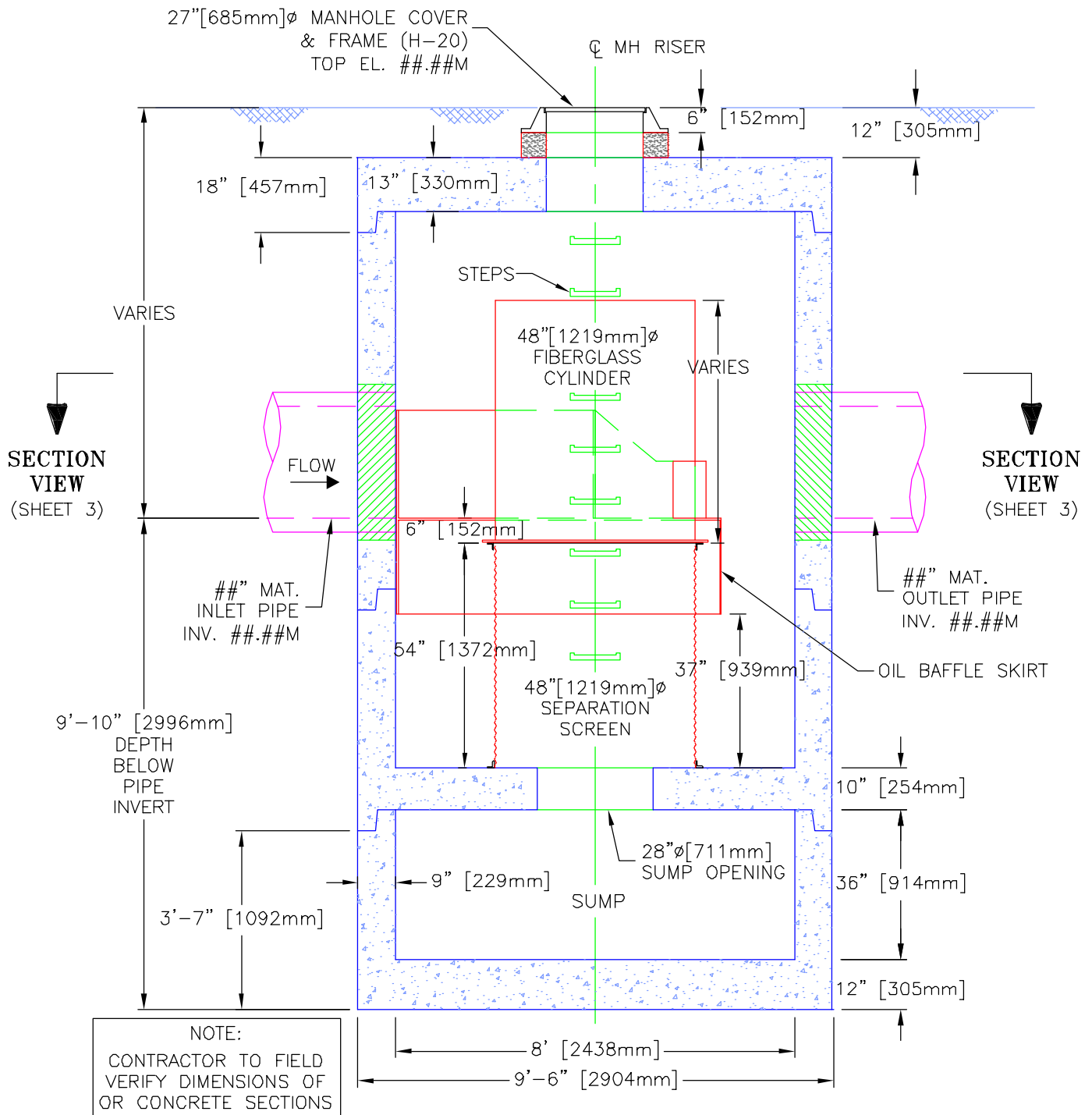
SCALE
1" = 3'

SHEET

1



ELEVATION VIEW

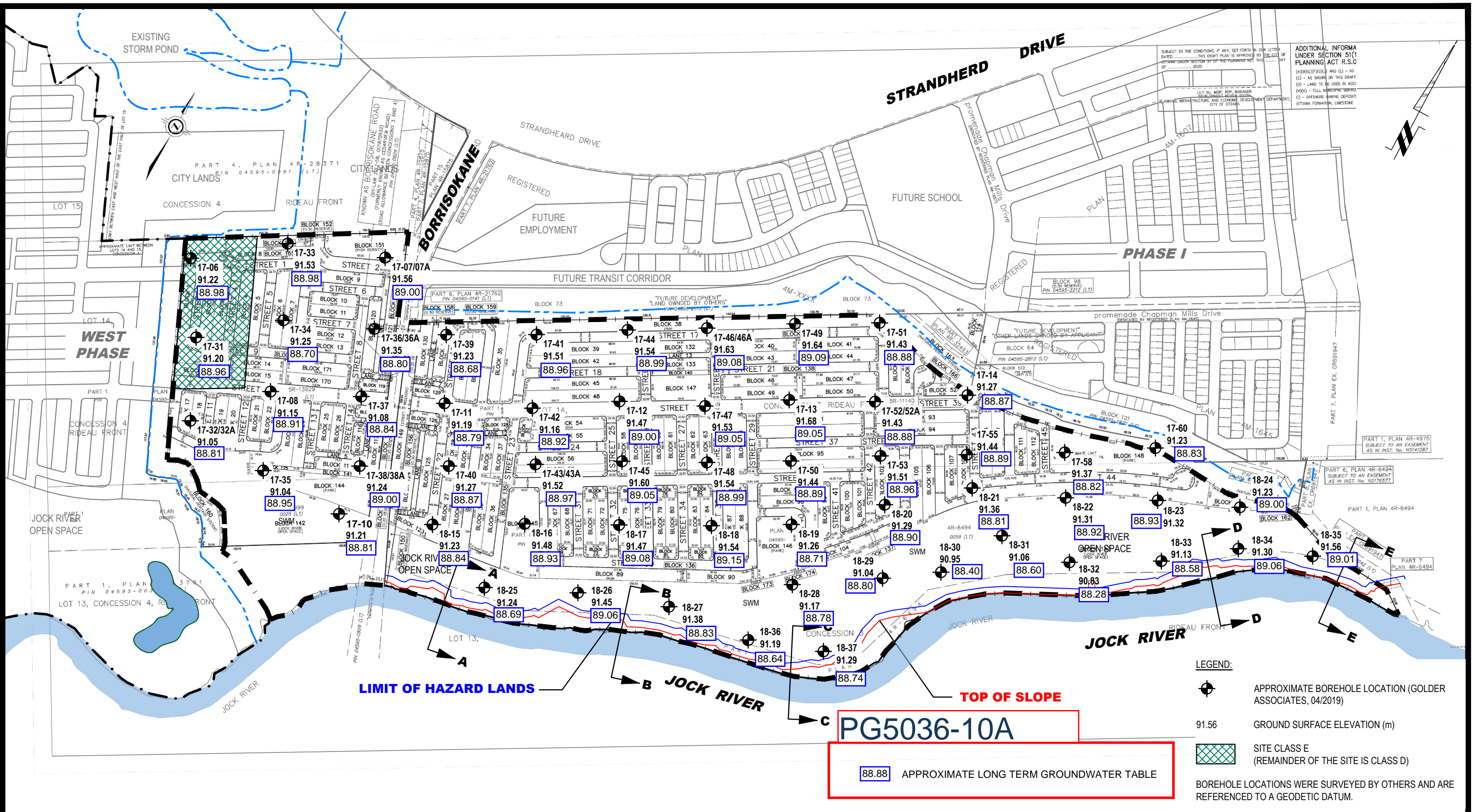


CDS MODEL PMSU40_45m, 7.5 CFS TREATMENT CAPACITY STORM WATER TREATMENT UNIT



PROJECT NAME
CITY, STATE

JOB#	XX-##-###	SCALE	1" = 3'
DATE	##/##/##	SHEET	2
DRAWN	INITIALS		
APPROV.			



SUBJECT TO THE CONDITIONS, IF ANY, SET FORTH IN OUR LETTER DATED: _____ THIS DRAFT PLAN IS APPROVED BY THE CITY OF OTTAWA UNDER SECTION 51(1) OF THE PLANNING ACT R.S.O. (A) (B) (C) (D) (E) (F) (G) (H) (I) (J) (K) (L) (M) (N) (O) (P) (Q) (R) (S) (T) (U) (V) (W) (X) (Y) (Z) (AA) (AB) (AC) (AD) (AE) (AF) (AG) (AH) (AI) (AJ) (AK) (AL) (AM) (AN) (AO) (AP) (AQ) (AR) (AS) (AT) (AU) (AV) (AW) (AX) (AY) (AZ) (BA) (BB) (BC) (BD) (BE) (BF) (BG) (BH) (BI) (BJ) (BK) (BL) (BM) (BN) (BO) (BP) (BQ) (BR) (BS) (BT) (BU) (BV) (BW) (BX) (BY) (BZ) (CA) (CB) (CC) (CD) (CE) (CF) (CG) (CH) (CI) (CJ) (CK) (CL) (CM) (CN) (CO) (CP) (CQ) (CR) (CS) (CT) (CU) (CV) (CW) (CX) (CY) (CZ) (DA) (DB) (DC) (DD) (DE) (DF) (DG) (DH) (DI) (DJ) (DK) (DL) (DM) (DN) (DO) (DP) (DQ) (DR) (DS) (DT) (DU) (DV) (DW) (DX) (DY) (DZ) (EA) (EB) (EC) (ED) (EE) (EF) (EG) (EH) (EI) (EJ) (EK) (EL) (EM) (EN) (EO) (EP) (EQ) (ER) (ES) (ET) (EU) (EV) (EW) (EX) (EY) (EZ) (FA) (FB) (FC) (FD) (FE) (FF) (FG) (FH) (FI) (FJ) (FK) (FL) (FM) (FN) (FO) (FP) (FQ) (FR) (FS) (FT) (FU) (FV) (FW) (FX) (FY) (FZ) (GA) (GB) (GC) (GD) (GE) (GF) (GG) (GH) (GI) (GJ) (GK) (GL) (GM) (GN) (GO) (GP) (GQ) (GR) (GS) (GT) (GU) (GV) (GW) (GX) (GY) (GZ) (HA) (HB) (HC) (HD) (HE) (HF) (HG) (HH) (HI) (HJ) (HK) (HL) (HM) (HN) (HO) (HP) (HQ) (HR) (HS) (HT) (HU) (HV) (HW) (HX) (HY) (HZ) (IA) (IB) (IC) (ID) (IE) (IF) (IG) (IH) (II) (IJ) (IK) (IL) (IM) (IN) (IO) (IP) (IQ) (IR) (IS) (IT) (IU) (IV) (IW) (IX) (IY) (IZ) (JA) (JB) (JC) (JD) (JE) (JF) (JG) (JH) (JI) (JJ) (JK) (JL) (JM) (JN) (JO) (JP) (JQ) (JR) (JS) (JT) (JU) (JV) (JW) (JX) (JY) (JZ) (KA) (KB) (KC) (KD) (KE) (KF) (KG) (KH) (KI) (KJ) (KK) (KL) (KM) (KN) (KO) (KP) (KQ) (KR) (KS) (KT) (KU) (KV) (KW) (KX) (KY) (KZ) (LA) (LB) (LC) (LD) (LE) (LF) (LG) (LH) (LI) (LJ) (LK) (LL) (LM) (LN) (LO) (LP) (LQ) (LR) (LS) (LT) (LU) (LV) (LW) (LX) (LY) (LZ) (MA) (MB) (MC) (MD) (ME) (MF) (MG) (MH) (MI) (MJ) (MK) (ML) (MM) (MN) (MO) (MP) (MQ) (MR) (MS) (MT) (MU) (MV) (MW) (MX) (MY) (MZ) (NA) (NB) (NC) (ND) (NE) (NF) (NG) (NH) (NI) (NJ) (NK) (NL) (NM) (NN) (NO) (NP) (NQ) (NR) (NS) (NT) (NU) (NV) (NW) (NX) (NY) (NZ) (OA) (OB) (OC) (OD) (OE) (OF) (OG) (OH) (OI) (OJ) (OK) (OL) (OM) (ON) (OO) (OP) (OQ) (OR) (OS) (OT) (OU) (OV) (OW) (OX) (OY) (OZ) (PA) (PB) (PC) (PD) (PE) (PF) (PG) (PH) (PI) (PJ) (PK) (PL) (PM) (PN) (PO) (PP) (PQ) (PR) (PS) (PT) (PU) (PV) (PW) (PX) (PY) (PZ) (QA) (QB) (QC) (QD) (QE) (QF) (QG) (QH) (QI) (QJ) (QK) (QL) (QM) (QN) (QO) (QP) (QQ) (QR) (QS) (QT) (QU) (QV) (QW) (QX) (QY) (QZ) (RA) (RB) (RC) (RD) (RE) (RF) (RG) (RH) (RI) (RJ) (RK) (RL) (RM) (RN) (RO) (RP) (RQ) (RR) (RS) (RT) (RU) (RV) (RW) (RX) (RY) (RZ) (SA) (SB) (SC) (SD) (SE) (SF) (SG) (SH) (SI) (SJ) (SK) (SL) (SM) (SN) (SO) (SP) (SQ) (SR) (SS) (ST) (SU) (SV) (SW) (SX) (SY) (SZ) (TA) (TB) (TC) (TD) (TE) (TF) (TG) (TH) (TI) (TJ) (TK) (TL) (TM) (TN) (TO) (TP) (TQ) (TR) (TS) (TT) (TU) (TV) (TW) (TX) (TY) (TZ) (UA) (UB) (UC) (UD) (UE) (UF) (UG) (UH) (UI) (UJ) (UK) (UL) (UM) (UN) (UO) (UP) (UQ) (UR) (US) (UT) (UU) (UV) (UW) (UX) (UY) (UZ) (VA) (VB) (VC) (VD) (VE) (VF) (VG) (VH) (VI) (VJ) (VK) (VL) (VM) (VN) (VO) (VP) (VQ) (VR) (VS) (VT) (VU) (VV) (VW) (VX) (VY) (VZ) (WA) (WB) (WC) (WD) (WE) (WF) (WG) (WH) (WI) (WJ) (WK) (WL) (WM) (WN) (WO) (WP) (WQ) (WR) (WS) (WT) (WU) (WV) (WW) (WX) (WY) (WZ) (XA) (XB) (XC) (XD) (XE) (XF) (XG) (XH) (XI) (XJ) (XK) (XL) (XM) (XN) (XO) (XP) (XQ) (XR) (XS) (XT) (XU) (XV) (XW) (XX) (XY) (XZ) (YA) (YB) (YC) (YD) (YE) (YF) (YG) (YH) (YI) (YJ) (YK) (YL) (YM) (YN) (YO) (YP) (YQ) (YR) (YS) (YT) (YU) (YV) (YW) (YX) (YZ) (ZA) (ZB) (ZC) (ZD) (ZE) (ZF) (ZG) (ZH) (ZI) (ZJ) (ZK) (ZL) (ZM) (ZN) (ZO) (ZP) (ZQ) (ZR) (ZS) (ZT) (ZU) (ZV) (ZW) (ZX) (ZY) (ZZ)

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 Ottawa, Ontario K2E 7J5
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NO.	REVISIONS	DATE	INITIAL
1	CONCEPTUAL PLAN ADDED	09/01/2020	DJG

CAIVAN COMMUNITIES
 GEOTECHNICAL INVESTIGATION
 PROP. RESIDENTIAL DEVELOPMENT - CONSERVANCY LANDS EAST
 OTTAWA, ONTARIO
 Title: **TEST HOLE LOCATION PLAN**

Scale:	1:6000	Date:	09/2019
Drawn by:	MPG	Report No.:	PG5036-1
Checked by:	SD	Dwg. No.:	PG5036-1
Approved by:	DJG	Revision No.:	1

December 01, 2022

Project Number: 1474(03)

David Schaeffer Engineering Ltd
120 Iber Road, Unit 103
Ottawa, Ontario
K2S 1E9

Attention: Kevin Murphy, P.Eng

Subject: BCDC Phase 5 – Preliminary HGL Analysis

Introduction

Phase 5 of the Barrhaven Conservancy Development is located in Barrhaven, Ontario, north of the Jock River, east of the Foster Creek and West of Borrisokane Road. The proposed development is approximately **13.17 ha** that will primarily comprise of single and townhouse residential lots along with a **0.64 ha** park. The following outlines the preliminary hydraulic grade line (HGL) assessment for the site, to ensure that the proposed minor system within the development is adequately sized to safely convey flows to the Jock River under various conditions. As such the following memo outlines the approach taken in assessing the development's HGL and summarises the findings of this analysis.

Analysis Approach

Preliminary hydraulic grade line calculations for the proposed BCDC Phase 5 development were completed using PCSWMM modelling software. Pipe data, trunk storm sewer layout and Rational Method flows in the storm sewer are as provided by DSEL. The Rational Method flows were calculated based on the 2-, 5- or 10-year level of service requirements, and the 100-year flows in the hydraulic grade line calculations were estimated as 14% greater than the Rational Method flows, to account for the additional flows captured by catchbasin grates, lead pipes and/or inlet control devices under the higher surface water depths of the 100-year storm.

The 14% increase in flows for the 100-year event is based on the assumption that the head on a lead pipe/ICD will increase by 35 cm (maximum allowable major system ponding depth) during the 100-Year event. Taking a typical 250 mm lead pipe and assuming that the head on the pipe is just below the top of the grate (assumed at 1.38 m) results in a peak flow of 209 L/s, then assuming that the head is increased by 35 cm during the 100-Year (head of 1.730m) the flow through the lead pipe would increase to 234.5 L/s, which results in a 12% increase in peak flows. It is important to note that a 12% increase is observed when the same calculations are applied to the various lead pipe and ICD sizes. An additional 2% is added as a safety factor to allow some flexibility in the design, as it is likely that not all lead pipes will have a head of 1.38 m (just below the top of MH) for the level of service specified.

The proposed storm sewer infrastructure data was extracted from DSEL's drawings and incorporated into a PCSWMM model, and flows derived by DSEL's Rational Method calculations were then applied to each Maintenance Hole (MH) in the model as steady flows (using the baseflow option). Exit losses were applied to all storm sewer pipes in the system based on the angle of the downstream connection.

As in line with all other works completed for the BCDC development phases, the preliminary HGL analysis was completed under two conditions:

- 100-year rainfall event on the development and a 5-year spring water level on the Jock River
- Level of service (2/5/10-year) rainfall event on the development and a 100-year spring water level on the Jock River

Note that the water level along the Jock River through the length of this development varies, and as such the nearest corresponding upstream water surface elevation calculated by RVCA's HEC-RAS floodplain mapping model of the Jock River was applied at each of the respective storm sewer outlets. Also, note that assuming a 5-year spring water level on the Jock River for a 100-year rainfall event on the development is an inherently conservative assumption, as the critical storm for the proposed development is a summer (intense rainfall) event while the critical storm for the Jock River is a spring (snowmelt + rainfall) event. A preliminary Single Station Flood Frequency analysis was completed by JFSA using only summer flows (from May 15 to October 31) based on historical flow data recorded at the Moodie Drive Water Survey Canada gauge. This analysis found that the 100-year summer flow on the Jock River is around **99 m³/s**, while the 5-year spring flow is around **123 m³/s**, therefore the downstream boundary condition applied is conservative.

Within the proposed development Oil and Grit Separators (OGS) units in conjunction with LID measures will be implemented to ensure the site meets quality control requirements. Preliminary OGS units and associated by-pass weir elevations have also been included in the model, based on similar drainage areas and imperviousness seen in BCDC Phase 2.

Trunk Sewer 9 will have a drainage area of **7.21 ha** at **75%** imperviousness, Trunk Sewer 10 has a drainage area of **6.56 ha** at **80%** imperviousness. In a preliminary consultation with the OGS manufacturer, it was suggested that the closest match to the detailed OGS sizing for BCDC Phase 2 is **OGS 3**, which used a **PMSU 4045-8** OGS unit with a weir height of **0.65m**, this unit had a drainage area of **6.77 ha** at **64%** imperviousness.

Results

The maximum HGL obtained at each MH has been extracted from the level of service (2/5/10-year) event / 100-year Jock River water level scenario and the 100-year event / 5-year Jock River water level scenario, with the results from this analysis provided in Tables 1 & 2, respectively. As all proposed units within this development will have sump pumps, the simulated HGL was compared against the top of MH elevation to ensure that all storm sewers infrastructure is sufficiently sized and is not surcharging to the major system during the assessed events.

From this analysis, it was found that the critical scenario for HGL within the development was the level of service development event and 100-year water level on the Jock River scenario. Based on this scenario, no MHs will have an HGL elevation above the top of MH (minimum freeboard of **0.64 m** at **MH-514** to **MH-516** and **MH-521**), with an average freeboard of **0.74 m** from the top of MH throughout the proposed development.

For the 100-year event and 5-year water level on the Jock River, no MHs will have HGL elevations above the top of MH (minimum freeboard of **1.22 m** at **MH-514** and **MH-515**), with an average freeboard of **1.38 m** from the top of MH throughout the proposed development. As such it can be concluded that the proposed storm sewer infrastructure is sufficiently sized, to safely convey minor system flows from the development under various extreme conditions.

Conclusion

A preliminary HGL analysis for Phase 5 of the Barrhaven Conservancy Development was completed using PCSWMM based on storm sewer and flow details provided by DSEL. From this analysis, it was found that the proposed storm sewer infrastructure is sufficiently sized to convey all minor system flows to the Jock River and will not result in any MHs surcharging to the street under extreme events such as 100-year rainfall events on the development and a 5-year spring water level on the Jock River and a level of service (2/5/10 Year) rainfall event on the development and a 100-year spring water level on the Jock River, with the former being the more critical scenario for the HGL within the development.

Yours truly,
J.F Sabourin and Associates Inc.



Jonathon Burnett, P.Eng
Water Resources Engineer



cc: J.F Sabourin, M.Eng, P.Eng
Director of Water Resources Projects

Figures

Figure 1: PCSWMM Model Overview

Tables

Table 1: HGL Result Tables - Level of service (2/5/10-year) BCDC Development & 100-Year Jock River

Table 2: HGL Result Tables - 100-Year BCDC Development & 5-Year Jock River

Attachments

Attachment A: DSEL Rational Method Calculations

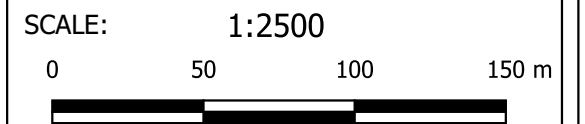
Modelling Files - Provided Electronically

PCSWMM: BCDC-P5_HGL_v01.1-2-5-10-YrDev-100YrJock.inp
BCDC-P5_HGL_v01.1-100-YrDev-5YrJock.inp



Legend

- Junctions
- ▲ Outfalls
- Conduits
- Weirs
- Site Plan



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Barrhaven Conservancy Development
 Phase 5

Figure 1: Preliminary HGL Analysis
 Model Overview

PROJECT	1474(03)-21
DRAWN	JB
DATE	December 2022

Table 1: BCDC Phase 5 - Preliminary HGL Analysis
Level of Service (2/5/10 Year) BCDC Development & 100-Year Jock River

MH-ID	Invert Elevation	Top of MH (m)	Max HGL (m)	Freeboard (m)
MH-507	90.62	93.26	92.54	0.72
MH-513	90.42	93.10	92.43	0.67
MH-514	90.34	93.02	92.38	0.64
MH-515	90.30	93.00	92.36	0.64
MH-516	90.17	92.91	92.27	0.64
MH-521	90.05	92.83	92.19	0.64
MH-524	89.71	92.66	91.99	0.67
MH-525	89.64	92.65	91.93	0.72
MH-525-1	89.63	92.64	91.88	0.76
MH-528	89.63	92.67	91.89	0.78
MH-528-1	89.63	92.67	91.83	0.84
MH-529	90.43	93.01	92.23	0.78
MH-530	90.37	93.01	92.20	0.81
MH-534	90.17	92.99	92.11	0.88
MH-537	90.02	92.91	92.07	0.84
MH-538	89.87	92.91	92.01	0.90
MH-543	89.69	92.65	91.96	0.69
			Min	0.64
			Max	0.90
			Average	0.74

Note: Analysis assumes 100 year spring water level on the Jock River

Model Name:BCDC-P5_HGL_v01.1-2-5-10-YrDev-100YrJock.inp

**Table 2: BCDC Phase 5 - Preliminary HGL Analysis
100-Year BCDC Development & 5-Year Jock River**

MH-ID	Invert Elevation	Top of MH (m)	Max HGL (m)	Freeboard (m)
MH-507	90.62	93.26	92.02	1.24
MH-513	90.42	93.10	91.87	1.23
MH-514	90.34	93.02	91.81	1.22
MH-515	90.30	93.00	91.78	1.22
MH-516	90.17	92.91	91.67	1.24
MH-521	90.05	92.83	91.56	1.27
MH-524	89.71	92.66	91.29	1.37
MH-525	89.64	92.65	91.22	1.43
MH-525-1	89.63	92.64	91.16	1.48
MH-528	89.63	92.67	91.19	1.48
MH-528-1	89.63	92.67	91.12	1.55
MH-529	90.43	93.01	91.63	1.38
MH-530	90.37	93.01	91.59	1.42
MH-534	90.17	92.99	91.47	1.52
MH-537	90.02	92.91	91.42	1.49
MH-538	89.87	92.91	91.34	1.57
MH-543	89.69	92.65	91.27	1.38
			Min	1.22
			Max	1.57
			Average	1.38

Note: Analysis assumes 5 year spring water level on the Jock River

Model Name:BCDC-P5_HGL_v01.1-100-YrDev-5YrJock.inp



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Attachment A

DSEL Rational Method Calculations

March 08, 2021

Project Number: 1474

David Schaeffer Engineering Ltd
120 Iber Road, Unit 103
Ottawa, Ontario
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Attention: Steve Pichette, P.Eng.

Subject: Review of Quantity Control Requirement for Jock River-Reach One

Introduction

Phase 2 of the Barrhaven Conservancy Development (aka Conservancy East) is located in Barrhaven, Ontario, north of the Jock River, south of the Fraser Clarke Creek and east of the Foster Creek. The proposed development is approximately 59.26 ha that will primarily comprise of single and townhouse residential lots. As a part of the City of Ottawa's review of the proposed development draft plan of Phase 2 of the Barrhaven Conservancy Development, submitted in December 2020, it is proposed that flood quantity control measures will not need to be implemented as a part of this development. This assumption is based on the work completed by Stantec in June 2007 in the "Jock River Reach One Subwatershed Study" which concluded that for future developments within Reach 1 of the Jock River: "No quantity control storage is required for flood control purposes as the hydrograph from the subwatershed will peak before the upstream peak in the Jock River" and that "No erosion control storage is required to maintain the predevelopment in-stream erosion condition". Although this study did not consider the future development of the Barrhaven Conservancy Lands, and as such the modelling completed by Stantec has been updated by JFSA to reflect these changes. The following memo outlines data sources, assessed scenarios, assumptions, and conclusions of this independent Jock River Reach One study.

It is noted that RVCA is currently engaging in a formal update/review of the Jock River Reach One Subwatershed Study, with the findings of this study having the potential to affect the above-noted design criteria. While that study is underway J.F. Sabourin and Associates Inc. (JFSA) has completed an independent Jock River - Reach One study to re-assess/confirm that the assumptions presented in the original 2007 study by Stantec are still valid, as any changes to this conclusion could greatly impact the current BCDC Phase 2 development plan.

Background Data

The following outlines all the model and data sources used in this analysis:

- "Jock River Floodplain Mapping Report", (2005 - PSR Group Ltd. & JFSA)
- "Jock River Reach One Subwatershed Study Final Report", (2007 – Stantec)
- "Corrigan Stormwater Management Facility Stormwater Management Report and Design Brief", (2010 - IBI Group)
- "Citi Gate, Highway 416 Employment Lands, Servicing Study and Stormwater Management Report (O'Keefe SWM)", (2012 – Novatech)
- "Foster Stormwater Management Facility, Environmental Study Report", (2013 - CH2MHill)
- "Todd Pond Model Keeper Analysis (Re-Assessment of Existing System Capacity)", (2015 – JFSA)

- “CitiGate 416 Corporate Campus Detailed Servicing and Stormwater Management Report (Phase 1)”, (2015 – Novatech)
- “Kennedy-Burnett Stormwater Management Facility Retrofit, Detailed Design Report”, (2020 – Novatech)
- “Half Moon Bay South / Addendum to April 2015 Todd Pond Model Keeper Analysis, Re-Assessment of Existing System Capacity Report” (2020 - JFSA).

Model Development/Scenarios

The following section outlines the various hydrologic model scenarios developed as a part of this work, with a brief description of the data sources used for each scenario and how they have been incorporated into the existing Jock River subwatershed hydrologic model.

Model 1 - Jock River Floodplain Model – JFSA, 2005

This hydrologic model was developed as a part of the floodplain mapping study of the Jock River completed in 2005. The hydrologic model of the Jock River was developed by JFSA using SWMHYMO, with independent models developed to simulate both summer and spring events. Both models were calibrated to field measured flows, recorded at the Water Survey Canada Flow Gauge at Moodie Drive. These models function as the basis for which all future models (both by JFSA and others) have been built on. Refer to Figure 1 (JFSA, 2005) for an overview of the subcatchments for reach one in this model, with full SWMHYMO input and summary files provided in Attachment A.

Model 2 – Jock River Reach One Model – Stantec, 2007

The hydrologic analysis completed by Stantec in 2007 built upon the JFSA 2005 floodplain mapping modelling. As a part of the Stantec work, the lower reach of the Jock River (3,176 ha) which was represented as a single subcatchment in the 2005 study was subdivided into thirteen (13) subcatchments to better delineate the drainage areas to the various tributaries (O’Keefe, Fraser, Foster, Todd, Corrigan and Clarke) and to also provide a better representation of the existing development areas (Kennedy Burnett, Chapman Mills, Jockvale and Hearts Desire). The remaining natural/undeveloped areas within the Jock River corridor were subdivided into three smaller (3) sub-catchments. Refer to Figure 2 (Stantec, 2007) for an overview of the subcatchments for reach one in this model, with full SWMHYMO input and summary files provided in Attachment B.

As mentioned above from this study, it was concluded that developments located in the lower reaches of the Jock River do not require any quantity control storage for flood control purposes as the hydrograph from the subwatershed will peak before the upstream peak in the Jock River and that no erosion control storage is required to maintain the pre-development in-stream erosion conditions.

Model 3 – Jock River Reach One Model Update – JFSA, 2021

As a part of the study outlined in this report, the 2007 Stantec SWMHYMO model of the Jock River was updated to reflect (as best as possible with the available information) proposed, approved and potential future developments, since the 2007 study.

At the time of the 2007 study, it was assumed that the floodplain of reach one of the Jock River (from Highway 416 to Greenbank Road) would not be filled and developed. Furthermore, the assumptions that were made in 2007 for the total imperviousness of future developments are not reflective of the actual constructed conditions observed in 2021; for example, the total impervious area for the Todd drainage area was assumed to be 43% in the 2007 study, while based on latest aerial photography it appears that the imperviousness for this area is closer to 58.5%). Additionally, SWM quantity controls were implemented in some tributaries within Reach One (e.g., O'Keefe, Foster and Kennedy-Burnett) to respect the hydraulic capacity of the local watercourses or other existing hydraulic constraints.

As outlined in the Background Data section of this memo, data from various reports and studies were collected and used to update Stantec's 2007 model, to best reflect existing conditions and known approved and planned development projects. As such, the thirteen (13) subcatchments of the 2007 Stantec model have been further discretized into one hundred ten (110) subcatchments, with numerous additional major system storage, SWM Ponds, and channel routing commands added. This updated existing condition model is reflective of current 2021 conditions, which assumes that the lower Jock River floodplain is undeveloped. It should also be noted that only subcatchments downstream of Highway 416 have been updated as a part of this analysis. Updates to other catchments of the Jock River further upstream, such as the Monahan Drain, Hobbs Creek, King Creek, and development areas in Richmond, where additional new information may be available, have not been included in the model updates at this time. Refer to Figure 3 for an overview of the subcatchments for reach one in this model, with full SWMHYMO input and summary files been provided in Attachment C, detailed schematics of the subcatchments updated as a part of this study have been provided in Attachment F.

Note that Novatech's PCSWMM model of the Kennedy-Burnett area was used to create a detailed SWMHYMO model of the same area. In creating this SWMHYMO model it was found that the 100-year peak outflows from the Kennedy Burnett facility were 1.4 times higher than that reported in the Novatech PCSWMM model. While it is expected that different modelling software will produce slightly different results, this difference is significant and should be investigated further; although it is unlikely that this difference is expected to change the fundamental conclusions of this analysis.

Model 4A & 4B – Jock River Reach One Future Conditions (without and with quantity SWM controls) – JFSA, 2021

Two additional models (4A and 4B) were created (which built on model 3) to evaluate the impacts of developing portions of the lower Jock River floodplain (from Highway 416 to Greenbank Road). These lands make up approximately 156 ha and would include BCDC and other properties on the south side of the Jock River. Model 4A assumes that these lands would be developed without any SWM quantity controls and Model 4B assumes that the lands would be developed with SWM Post to Pre-development quantity controls. Refer to Figure 4 for an overview of the subcatchments for reach one in these models, with full SWMHYMO input and summary files for scenarios 4A and 4B provided in Attachment D & E, respectively.

Results

All hydrologic models were run using a 24 hours SCS storm for the 2-to-100-year events. Note that this analysis focuses on this particular rainfall event as for developed areas the summer rainfall events are more critical than the spring rainfall plus snowmelt conditions. Hence, only the summer peak flows have been summarized and compared for the various scenarios below, as the flow contributions from the developments in the lower Jock River under the spring rainfall + snowmelt event are negligible compared to the flows upstream from the greater Jock River. It is further noted that the same design storms were used in all models.

Peak flows at key locations along Reach One of the Jock River have been extracted from the various hydrologic models and provided in the following section. As a part of this analysis, 5 key locations on the Jock River have been selected to compare the simulated peak flows and are as follows: Highway 416, Borrisokane Road, Greenbank Road, Jockvale Road and the Jock River's confluence with the Rideau River. Note that for the older models (JFSA 2005 & Stantec 2007) results have only been provided at some locations, as these original models were not discretized to this higher level of detail. Additionally, the Stantec 2007 model did not assess flows on the Jock River for the 10- and 50-year events at any locations.

Table 1: Comparison of Summer Peak Flows (m³/s) at Highway 416 (52483.00 ha)

Scenario	Return Period					
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	45.676	66.292	82.076	104.643	122.469	141.415
Model 2: Stantec 2007 Reach One Analysis	45.789	66.413	n/a	104.834	n/a	141.853
Model 3: Updated Model 2 to current/ approved conditions	46.294	67.222	83.235	106.109	124.249	143.580
Model 4A: Model 3 with BCDC & others w/o SWM	46.294	67.222	83.235	106.109	124.249	143.580
Model 4B: Model 3 with BCDC & others with SWM	46.294	67.222	83.235	106.109	124.249	143.580

From Table 1 above it is seen that the peak flows on the Jock River at Highway 416 for Models 3, 4A and 4B are higher than in Models 1 and 2. This is because the computational time step in the updated models was reduced from 5 - 10 minutes to 1 minute. This change was necessary to provide stable results in the various models, especially with the additional ROUTE CHANNEL commands that have been added to the updated models. There were no other changes made to the models upstream of Highway 416.

Table 2: Comparison of Summer Peak Flows (m³/s) at Borrisokane Road (53577.82 ha)

Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	n/a	n/a	n/a	n/a	n/a	n/a
Model 2: Stantec 2007 Reach One Analysis	46.817	68.124	n/a	107.402	n/a	144.892
Model 3: Updated Model 2 to current/ approved conditions	47.379	69.117	85.613	108.988	127.740	147.849
Model 4A: Model 3 with BCDC & others w/o SWM	47.426	68.998	85.561	109.064	127.650	147.535
Model 4B: Model 3 with BCDC & others with SWM	47.599	69.319	85.870	109.449	128.055	147.939

From Table 2 it is seen that the peak flows on the Jock River at Borrisokane are generally lower in model 4A (developed without SWM controls) than under existing conditions (Model 3). The inclusion of SWM controls for these future developments results in the peak flows increasing from existing conditions for all return periods at this location. Note that the increase in flows at this location between the Stantec model (model 2) and the JFSA existing conditions model (model 3) again is due to the greater discretization of subcatchments at Borrisokane Road in the JFSA model. For example, in the Stantec model, the subcatchment that represents the currently undeveloped lands along the Jock River is represented as a single subcatchment (S-1). Where the JFSA updated model represents these lands as 14 individual subcatchments, all discharging to their respective locations within the Jock River (e.g. O’Keefe Creek, Foster Creek & Borrisokane Road).

Table 3: Comparison of Summer Peak Flows (m³/s) at Greenbank Road (54717.80 ha)

Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	n/a	n/a	n/a	n/a	n/a	n/a
Model 2: Stantec 2007 Reach One Analysis	49.195	71.220	n/a	111.172	n/a	149.236
Model 3: Updated Model 2 to current/ approved conditions	49.055	70.826	86.895	110.282	128.564	147.488
Model 4A: Model 3 with BCDC & others w/o SWM	48.599	69.773	85.389	103.842	126.050	144.531
Model 4B: Model 3 with BCDC & others with SWM	48.982	70.171	85.928	103.651	126.537	144.894

From Table 3 it is seen that peak flows on the Jock River at Greenbank Road are the lowest without SWM controls in place (Model 4A). With SWM controls in place, the peak flows are lower than the existing conditions, but not as low as when SWM controls are not implemented. Note that the JFSA existing conditions model (model 3) is presenting peak flows lower than the Stantec model (model 2) at this location, again this is due to the greater discretization in the JFSA model as discussed above.

Table 4: Comparison of Summer Peak Flows (m³/s) at Jockvale Road (55476.26 ha)

Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	n/a	n/a	n/a	n/a	n/a	n/a
Model 2: Stantec 2007 Reach One Analysis	49.870	72.143	n/a	112.074	n/a	150.033
Model 3: Updated Model 2 to current/ approved conditions	49.619	72.224	88.294	111.989	130.865	149.819
Model 4A: Model 3 with BCDC & others w/o SWM	49.482	71.017	86.165	105.082	128.174	146.840
Model 4B: Model 3 with BCDC & others with SWM	49.606	71.408	86.690	104.765	128.229	147.027

From Table 4 is seen that the peak flows at Jockvale Road are generally at their lowest without SWM controls in place, and that either implementing or not implementing SWM controls for future developments results in peak flows at this location being less than existing conditions.

Table 5: Comparison of Summer Peak Flows (m³/s) at Outlet of Jock River (55579.20 ha)

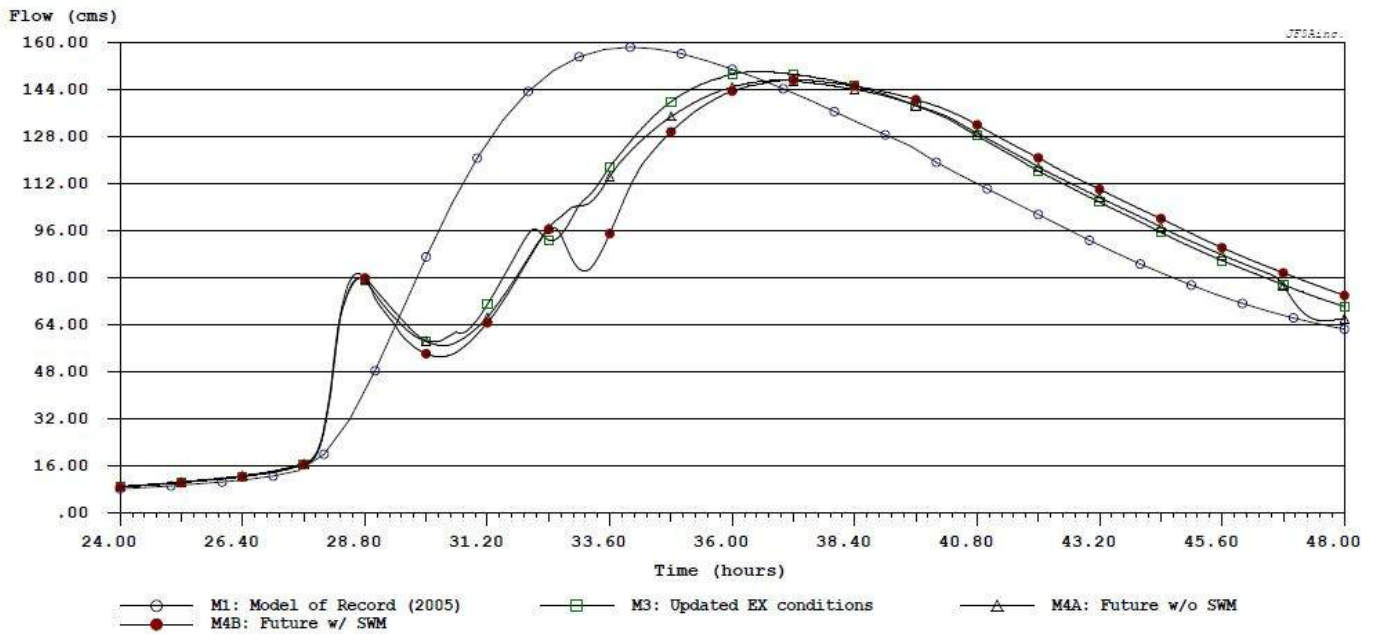
Scenario	Return Period					
	2-Year	5- Year	10-Year	25-Year	50-Year	100-Year
Model 1: 2005 Floodplain Study	49.16	72.08	89.96	115.84	136.46	158.42
Model 2: Stantec 2007 Reach One Analysis	50.78	73.65	n/a	113.97	n/a	157.69
Model 3: Updated Model 2 to current/ approved conditions	49.72	72.36	88.45	112.2	131.12	150.12
Model 4A: Model 3 with BCDC & others w/o SWM	49.58	71.17	86.35	105.27	128.42	147.10
Model 4B: Model 3 with BCDC & others with SWM	49.70	71.54	86.85	104.96	128.45	147.28

From Table 5 it is seen that the peak flows on the Jock River at the confluence with the Rideau River are generally at their lowest without SWM controls in place, and that either implementing or not implementing SWM controls for future developments results in peak flows at this location to be less than existing conditions.

Discussion

Although not initially obvious, the reason that future urban developments within Reach One of the Jock River are decreasing peak flows on the Jock River is because developing land not only affects the peak of the hydrograph but also the overall shape. Figure A below provides a comparison of the simulated hydrographs at the Jock River's confluence with the Rideau River from the various model scenarios. During any rainfall event, the runoff from the existing and future developments within Reach One will have already peaked and decayed before the peak flows arrive at this location from the upstream drainage area. For the 100-Year SCS storm, the peak from the development in Reach One can be seen in the figure below at around 28 hours, while the peak flow on the Jock River from the upstream drainage areas occurs at around 36-37 hours, this is approximately a 9-hour difference in timing. As such, implementing SWM measures for developments in the lower portions of the Jock River will decrease peak flows from the development, but would also prolong the period of time during which they discharge into the Jock River, thus coinciding with flows from the greater Jock River, ultimately resulting in potential increases in peak flows on the Jock River. This is seen in the figure below with the future condition with SWM controls (Model 4B - Red Circles) having a higher flow in the tail than future conditions without SWM controls (Model 4A – Black Triangles). Note that the difference between Model 1 and all other scenarios is simply due to further discretization of subcatchments within Reach One.

Figure A: Comparison of simulated 100 yr Jock River hydrographs at the confluence with the Rideau River



Hydrograph Statistics:

Legend	Filename & Comment	Time Step (min)	Drainage Area (ha)	Peak Flow (cms)	Time to Peak (hrs)	Runoff Volume (mm)	Runoff Volume (cu.m)	Duration of flow (hrs)	Average flow (cms)
○	N1_0100 : M1: Model of Record (2005)	30.00	55659.00	158.420	34.000	14.52	8.082E+06	24.000	93.538
□	SN_N1_0100 : M3: Updated EX conditions	1.00	55579.20	150.120	36.533	14.24	7.914E+06	24.000	91.603
△	SN_N1_0100 : M4A: Future w/o SWM	1.00	55579.20	147.102	36.917	14.12	7.848E+06	24.000	90.831
●	SN_N1_0100 : M4B: Future w/ SWM	1.00	55579.19	147.276	37.250	14.03	7.798E+06	24.000	90.252

Conclusion

The hydrologic model developed as a part of the Jock River Reach One Subwatershed Study (Stantec 2007) has been updated to provide additional refinements in the lower reaches of the Jock River (downstream of Highway 416) and assumes the development of lands that were previously not considered in the 2007 analysis (e.g. Barrhaven Conservancy). Future development condition models were created with and without SWM controls assumed, and the peak flows extracted from these models at key locations along the lower Jock River. From this analysis, it was found that with these additional developments in the lower Jock River peak flows are generally less than existing conditions without SWM controls in place. Implementing SWM controls for these developments has also been found to generally decrease peak flows on the Jock River, but not at all locations and not to the same degree as without SWM controls. Ultimately these findings are consistent with the fundamental conclusions drawn in Stantec's 2007 Jock River Reach One study, which initially determined that for future developments within Reach One of the Jock River "No quantity control storage is required for flood control purposes as the hydrograph from the subwatershed will peak before the upstream peak in the Jock River" and that "No erosion control storage is required to maintain the pre-development in-stream erosion condition". Based on the results of the updated analysis outlined in this memo, it can confirm that the fundamental conclusions drawn in Stantec's 2007 for developments in reach one of the Jock River remain valid.

Yours truly,
J.F Sabourin and Associates Inc.



Jonathon Burnett, P.Eng
Water Resources Engineer

cc: J.F Sabourin, M.Eng, P.Eng
Director of Water Resources Projects



Figures

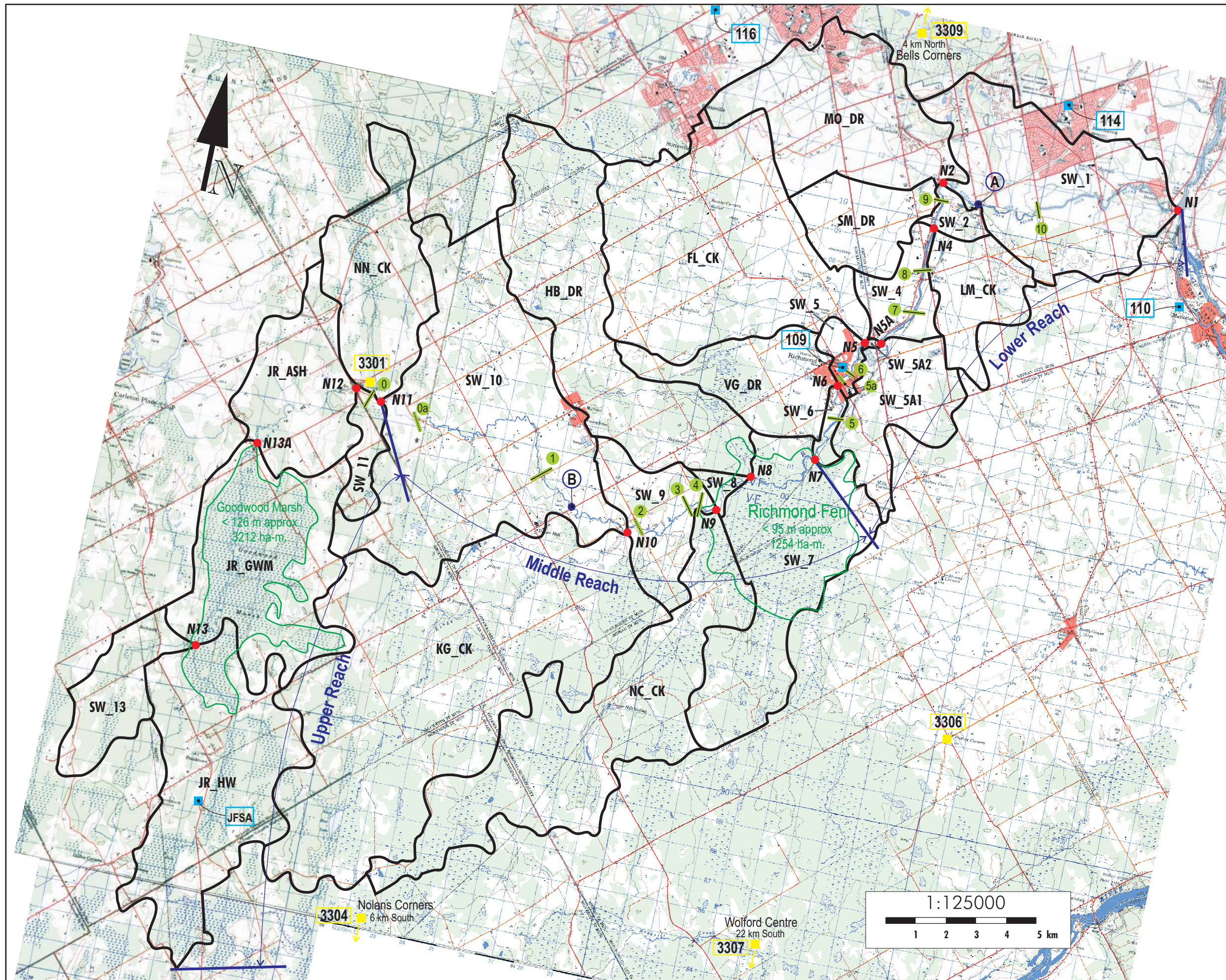
- Figure 1: Model 1 – Jock River Floodplain Model – JFSA, 2005
- Figure 2: Model 2 – Jock River Reach One Model – Stantec, 2007
- Figure 3: Model 3 – Jock River Reach One Model Update - JFSA, 2021
- Figure 4: Model 4A & 4B – Jock River Reach One Future Conditions - JFSA, 2021

Tables

- Table 1: Summer Peak Flows at Highway 416
- Table 2: Summer Peak Flows at Borrisokane Road.
- Table 3: Summer Peak Flows at Greenbank Road
- Table 4: Summer Peak Flows at Jockvale Road
- Table 5: Summer Peak Flows at Outlet of Jock River

Attachments

- Attachment A: Model 1 - SWMHYMO Input & Summary files
- Attachment B: Model 2 - SWMHYMO Input & Summary files
- Attachment C: Model 3 - SWMHYMO Input & Summary files
- Attachment D: Model 4A - SWMHYMO Input & Summary files
- Attachment E: Model 4B - SWMHYMO Input & Summary files
- Attachment F: Updated Subcatchment Schematics & Tables



- Legend:**
- Watershed
 - Bogs (Reservoir)
 - Nodes
 - Flow gauges
 - Rain gauges
 - Snow course stations
 - River Cross-Sections (based on topo. maps)
 - River Cross-Sections adjusted with field data
 - Reaches limits
- Flow gauges ID**
- 02LA007- Jock River near Richmond
 - Jock River at Franktown Rd
- Rain gauges ID**
- Richmond
 - Manotick
 - Barrhaven
 - Maple Grove
 - JFSA Inc, Temporary Rain Gauge 2003
- Snow course stations ID**
- Ashton
 - Nolans Corners
 - Pierces Corners
 - Bells Corners
 - Wolford Centre

Client:

Project:
Jock River Flood Plain Mapping Study

Title:
Watershed Delineation

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Figure 1 Ref. File: Base Map Jock River B.cdr



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Legend

- Existing SWM Facility
- Proposed Stormwater Facility
- Proposed Pond Outlet
- Jock River Tributary (Municipal Drain)
- Jock River Tributary (Non-Municipal Drain)
- Watershed Boundary
- 91.7 Regulatory Flood Level
- W/L 89.50 Normal Water Level
- Woodlot Limits
- Sub-drainage Area Limit
- Desire 24 ha
77 50% hrs
SCS Curve Number
- Application Plans Under Review
- 10 Year Flood Line
- 25 Year Flood Line
- 100 Year Flood Line

1	FINAL REPORT	BCB	N.C.	JUNE 2007
Revision		By	Appd.	YY.MM.DD
File Name:		Own.	Chgd.	Dgn.
				YY.MM.DD

Seal

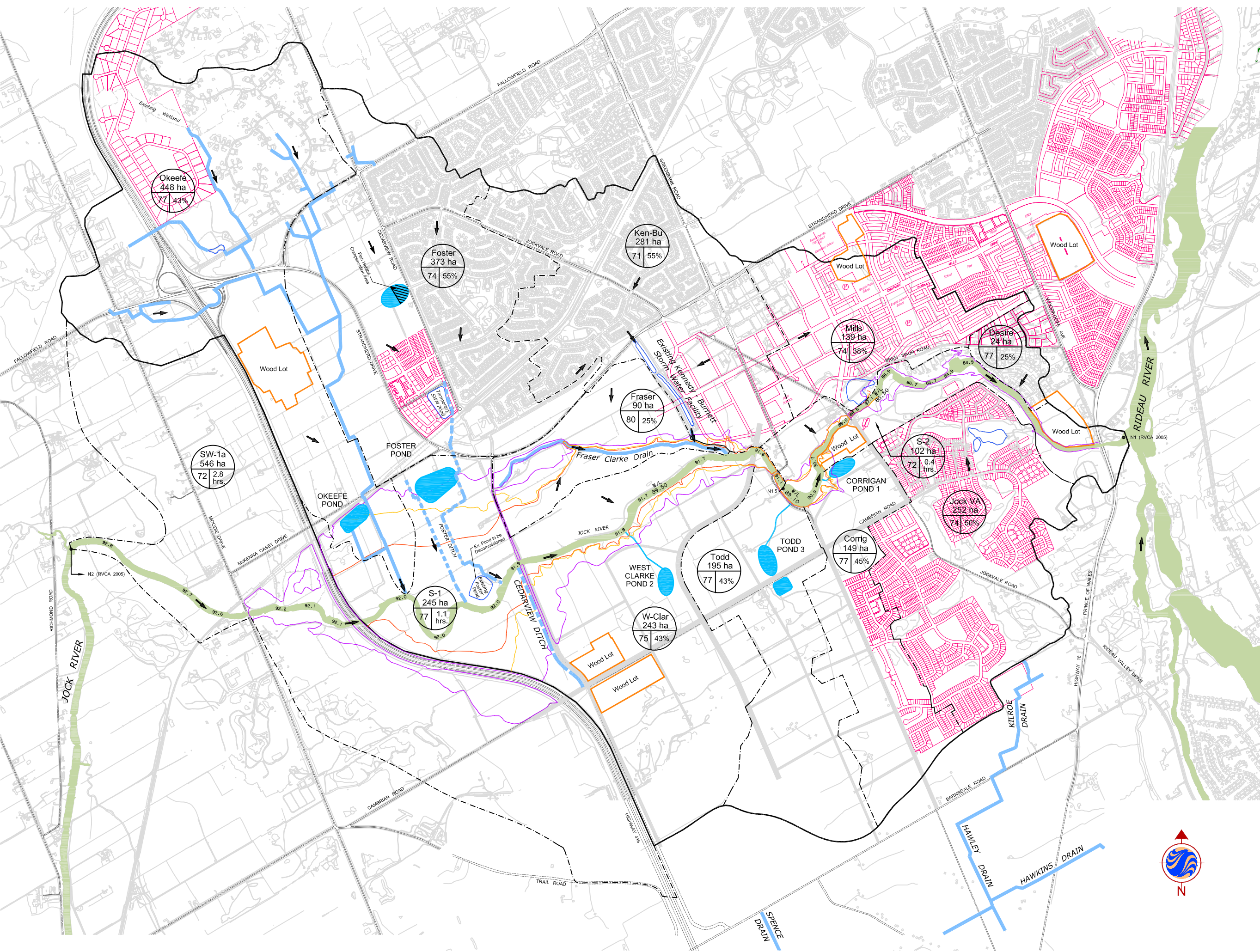
Client/Project

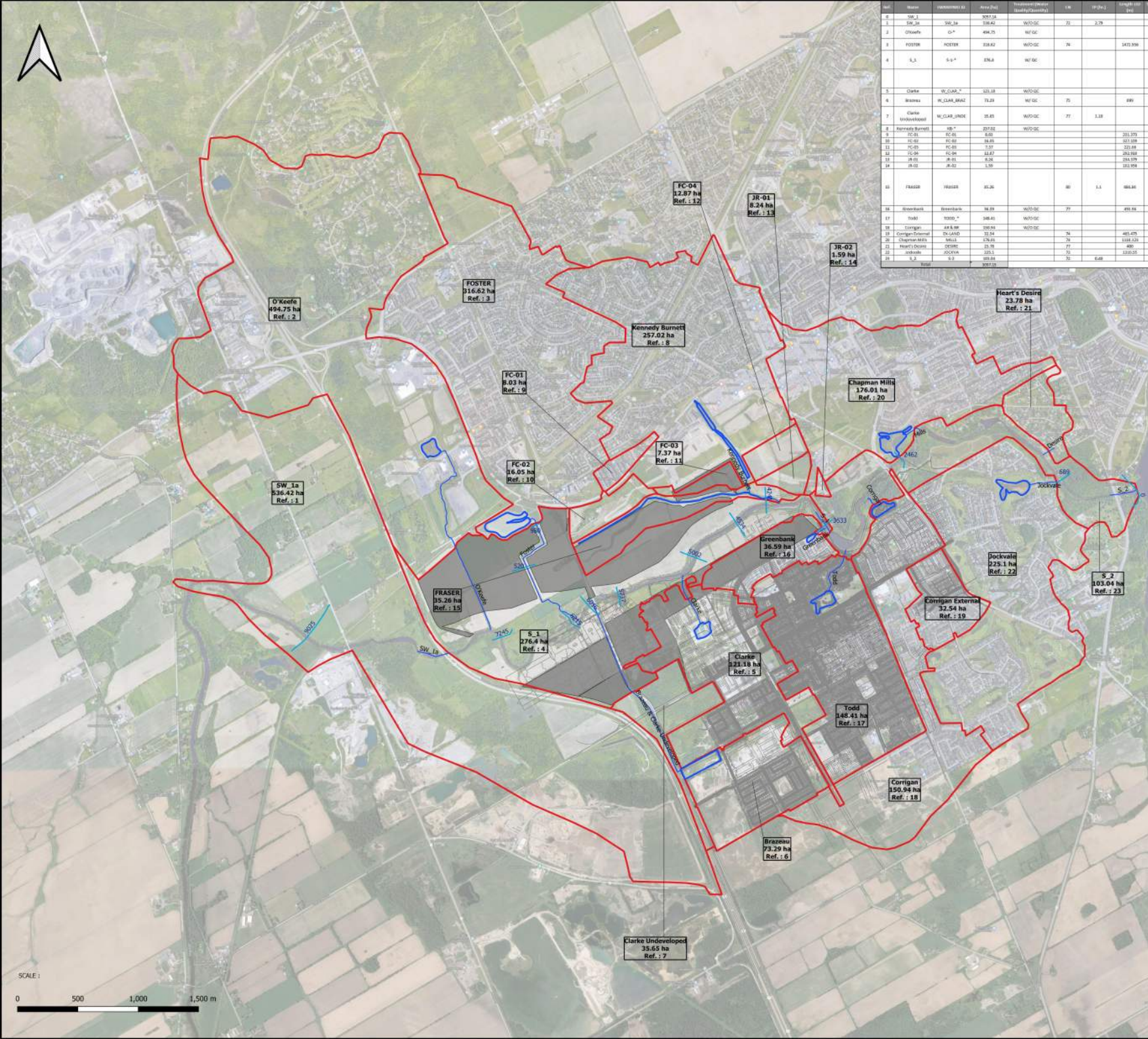
JOCK RIVER REACH ONE
 SUB-WATERSHED STUDY
 Ottawa ON Canada

Title
 PROPOSED CONDITIONS
 HYDROLOGIC MODEL
 DRAINAGE BOUNDARIES

Project No. 60400414 Scale 0 100 300 500m
 Drawing No. 1:10,000 Sheet 1 of 1

Figure 2 1 of 1 1





Ref	Area ID	Area (ha)	Sub-catchment	Ref	Area (ha)	Sub-catchment	Ref	Area (ha)	Sub-catchment	Ref	Area (ha)	Sub-catchment
1	SW_1a	536.42	SW_1a	1	536.42	SW_1a	1	536.42	SW_1a	1	536.42	SW_1a
2	O'Keefe	494.75	O'Keefe	2	494.75	O'Keefe	2	494.75	O'Keefe	2	494.75	O'Keefe
3	Foster	316.62	Foster	3	316.62	Foster	3	316.62	Foster	3	316.62	Foster
4	S_1	276.4	S_1	4	276.4	S_1	4	276.4	S_1	4	276.4	S_1
5	Clarke	121.18	Clarke	5	121.18	Clarke	5	121.18	Clarke	5	121.18	Clarke
6	Brazeau	73.29	Brazeau	6	73.29	Brazeau	6	73.29	Brazeau	6	73.29	Brazeau
7	Clarke Undeveloped	35.65	Clarke Undeveloped	7	35.65	Clarke Undeveloped	7	35.65	Clarke Undeveloped	7	35.65	Clarke Undeveloped
8	Kennedy Burnett	257.02	Kennedy Burnett	8	257.02	Kennedy Burnett	8	257.02	Kennedy Burnett	8	257.02	Kennedy Burnett
9	FC-01	8.03	FC-01	9	8.03	FC-01	9	8.03	FC-01	9	8.03	FC-01
10	FC-02	16.05	FC-02	10	16.05	FC-02	10	16.05	FC-02	10	16.05	FC-02
11	FC-03	7.37	FC-03	11	7.37	FC-03	11	7.37	FC-03	11	7.37	FC-03
12	FC-04	12.87	FC-04	12	12.87	FC-04	12	12.87	FC-04	12	12.87	FC-04
13	JR-01	8.24	JR-01	13	8.24	JR-01	13	8.24	JR-01	13	8.24	JR-01
14	JR-02	1.59	JR-02	14	1.59	JR-02	14	1.59	JR-02	14	1.59	JR-02
15	Fraser	35.26	Fraser	15	35.26	Fraser	15	35.26	Fraser	15	35.26	Fraser
16	Greenbank	36.59	Greenbank	16	36.59	Greenbank	16	36.59	Greenbank	16	36.59	Greenbank
17	Todd	148.41	Todd	17	148.41	Todd	17	148.41	Todd	17	148.41	Todd
18	Corrigan	150.94	Corrigan	18	150.94	Corrigan	18	150.94	Corrigan	18	150.94	Corrigan
19	Corrigan External	32.54	Corrigan External	19	32.54	Corrigan External	19	32.54	Corrigan External	19	32.54	Corrigan External
20	Chagman Mills	176.01	Chagman Mills	20	176.01	Chagman Mills	20	176.01	Chagman Mills	20	176.01	Chagman Mills
21	Heart's Desire	23.78	Heart's Desire	21	23.78	Heart's Desire	21	23.78	Heart's Desire	21	23.78	Heart's Desire
22	Jockvale	225.1	Jockvale	22	225.1	Jockvale	22	225.1	Jockvale	22	225.1	Jockvale
23	S_2	133.04	S_2	23	133.04	S_2	23	133.04	S_2	23	133.04	S_2
24	Total	3331.12	Total	24	3331.12	Total	24	3331.12	Total	24	3331.12	Total

Legend

- Channel Cross Section
- Sub-catchments
- SWMF Drains
- SWMF ponds

File name: Figure 3 - Overall Jock River Lower Reach one Sub-catchments.pdf
 XS 4534 Cross Section at station 4534

SW_1a 536.42 ha Ref. 1
 Area ID
 Area (ha)
 Reference Number

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DSEL
 david schaeffer engineering ltd

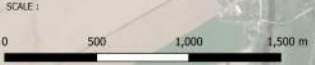
PROJECT : BCDC - Quantity Control Study

TITLE : Figure 3 - Overall Jock River Lower Reach one Sub-catchments
 Table 3 - Overall Jock River Lower Reach one Sub-catchments

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021



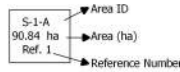


Legend

- Channel Cross Sections
 - S-1 Sub-catchments and Fraser Sub-catchments
 - S-1-A
 - S-1-B
 - S-1-D1
 - S-1-D2
 - S-1-D3
 - S-1-D4
 - S-1-D5
 - S-1-D6
 - S-1-D7
 - S-1-D8
 - S-1-FO-D1
 - S-1-FO-D2
 - S-1-FO-F-D
 - S-1-Okeefe
 - FRASER-DRN
 - FRASER-D
- Google Hybrid

File name:
20210304_S-1_Fraser_Schematic-Model4A.pdf

XS 3633 Cross Section at station 3633



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Station	Channel	Bank	Material	Flow Area (m²)	Velocity (m/s)	Discharge (m³/s)	Energy (m)	Hydraulic Radius (m)	Wetted Perimeter (m)	Channel Slope	Flow Direction	Notes
3633.0	FRASER-D	Left	Gravel	12.5	1.5	18.75	0.5	0.8	1.5	0.001	Down	Channel cross-section at station 3633.0
3633.0	FRASER-D	Right	Gravel	12.5	1.5	18.75	0.5	0.8	1.5	0.001	Down	Channel cross-section at station 3633.0
3633.0	FRASER-D	Center	Gravel	12.5	1.5	18.75	0.5	0.8	1.5	0.001	Down	Channel cross-section at station 3633.0

PROJECT :
BCDC - Quantity Control Study

TITLE :
S-1 Sub-catchment and Fraser Clarke Sub-catchment Schematic

PROJECT NO. : 1474-16

DRAWN : MM

DATE : Mar. 2021



Legend

- Channel Cross Sections
- S-1 Sub-catchments and Fraser Sub-catchments
- S-1-A
- S-1-B
- S-1-D1
- S-1-D2
- S-1-D3
- S-1-D4
- S-1-D5
- S-1-D6
- S-1-D7
- S-1-D8
- S-1-FO-D1
- S-1-FO-D2
- S-1-FO-F-D
- S-1-Okeefe
- FRASER-DRN
- FRASER-D

Google Hybrid

File name: 20210304_S-1_Fraser_Schematic-Model4B.pdf

XS 3633 Cross Section at station 3633

S-1-A	▼ Area ID
90.84 ha	► Area (ha)
Ref. 1	► Reference Number

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PROJECT : BCDC - Quantity Control Study

TITLE : S-1 Sub-catchment and Fraser Clarke Sub-catchment Schematic

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021



Station	Channel	Area (ha)	Ref.	Notes
3633	S-1-A	90.84	1	
3633	S-1-B	55.36	2	
3633	S-1-D1	21.67	3	
3633	S-1-D2	18.67	4	
3633	S-1-D3	6.79	5	
3633	S-1-D4	3.28	6	
3633	S-1-D5	12.84	7	
3633	S-1-D6	1.75	8	
3633	S-1-D7	2.03	9	
3633	S-1-D8	5.27	10	
3633	S-1-FO-D1	5.11	11	
3633	S-1-FO-D2	4.94	12	
3633	S-1-FO-F-D	14.96	13	
3633	S-1-Okeefe	44.93	14	
3633	FRASER-D	21.61	16	
3633	FRASER-DRN	13.65	15	



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Montréal, QC
Québec, QC

Attachment A

Model 1 – Jock River Floodplain Model

JFSA, 2005

SWMHYMO Input & Summary files

```

1  20    Metric units / ID numbers OFF
2  *#*****
3  *# SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *#*****
5  *# Project Name: [Jock River]    Project Number: [411-02]
6  *# Date       : 06-06-2003
7  *# Modeller   : [JoF]
8  *# Company    : JFSAinc.
9  *# License #  : 2549237
10 *#*****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *
16 * Calibrated parameters for Summer 2003 data:  APII=50, APIK=0.85, CN=varies,
17 *                                                SK=0.01, InterEventTime=12,
18 *                                                GWResk=0.96, VHydCond=0.055
19 *
20 *# -----
21 *
22 *START          TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
23 *              ["XAVG0315.STM"] average storm data a 15 minute time step
24 *              The above rainf file is an average of the JFSA gauge data
25 *              with the City of Ottawa rainfall data collected during
26 *              the same period.
27 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
28 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
29 *%              ["C24SC002.stm"] <--storm filename, one per line for NSTORM time
30 *%-----|-----|
31 *%-----|-----|
32 READ STORM     STORM_FILENAME=["storm.001"]
33 *%-----|-----|
34 MODIFY STORM   ICASEms=[1], NSHIFT=[96],
35 *              RedFACT=[1],
36 *%-----|-----|
37 COMPUTE API    APII=[50], APIK=[.85]/day
38 *%-----|-----|
39 *%-----|-----|
40 *#
41 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
42 *# of 1.32
43 *%-----|-----|
44 CONTINUOUS NASHYD  NHYD=["JR_HW"], DT=[30]min, AREA=[3680] (ha),
45 *              DWF=[0] (cms), CN/C=[64], IA=[2.5] (mm),
46 *              N=[3.0], TP=[7.13]hrs,
47 *              Continuous simulation parameters:
48 *              IaRECper=[4] (hrs),
49 *              SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
50 *              InterEventTime=[12] (hrs)
51 *              Baseflow simulation parameters:
52 *              BaseFlowOption=[1] ,
53 *              InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
54 *              VHydCond=[0.055] (mm/hr), END=-1
55 *%-----|-----|
56 *#
57 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
58 *# of 1.32
59 *%-----|-----|
60 CONTINUOUS NASHYD  NHYD=["SW_13"], DT=[30]min, AREA=[971] (ha),
61 *              DWF=[0] (cms), CN/C=[61], IA=[2.5] (mm),
62 *              N=[3.0], TP=[3.76]hrs,
63 *              Continuous simulation parameters:
64 *              IaRECper=[4] (hrs),
65 *              SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
66 *              InterEventTime=[12] (hrs)

```

```

67         Baseflow simulation parameters:
68         BaseFlowOption=[1] ,
69         InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
70         VHydCond=[0.055] (mm/hr),   END=-1
71     *%-----|-----
72     *#
73     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
74     *# of 1.80
75     *%-----|-----
76     CONTINUOUS NASHYD  NHYD=["JR_GWM"], DT=[30]min, AREA=[3074] (ha),
77                       DWF=[0] (cms),   CN/C=[55], IA=[2.5] (mm),
78                       N=[3], TP=[11.33]hrs,
79                       Continuous simulation parameters:
80                       IaRECper=[4] (hrs),
81                       SMIN=[-1] (mm),   SMAX=[-1] (mm), SK=[0.010]/(mm),
82                       InterEventTime=[12] (hrs)
83                       Baseflow simulation parameters:
84                       BaseFlowOption=[1] ,
85                       InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
86                       VHydCond=[0.055] (mm/hr),   END=-1
87     *%-----|-----
88     CONTINUOUS NASHYD  NHYD=["JR_ASH"], DT=[30]min, AREA=[1781] (ha),
89                       DWF=[0] (cms),   CN/C=[72], IA=[2.5] (mm),
90                       N=[3.0], TP=[3.91]hrs,
91                       Continuous simulation parameters:
92                       IaRECper=[4] (hrs),
93                       SMIN=[-1] (mm),   SMAX=[-1] (mm), SK=[0.010]/(mm),
94                       InterEventTime=[12] (hrs)
95                       Baseflow simulation parameters:
96                       BaseFlowOption=[1] ,
97                       InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
98                       VHydCond=[0.055] (mm/hr),   END=-1
99     *%-----|-----
100    CONTINUOUS NASHYD  NHYD=["SW_11"], DT=[30]min, AREA=[500] (ha),
101                      DWF=[0] (cms),   CN/C=[66], IA=[2.5] (mm),
102                      N=[3.0], TP=[1.24]hrs,
103                      Continuous simulation parameters:
104                      IaRECper=[4] (hrs),
105                      SMIN=[-1] (mm),   SMAX=[-1] (mm), SK=[0.010]/(mm),
106                      InterEventTime=[12] (hrs)
107                      Baseflow simulation parameters:
108                      BaseFlowOption=[1] ,
109                      InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
110                      VHydCond=[0.055] (mm/hr),   END=-1
111    *%-----|-----
112    *#
113    *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
114    *# of 1.80
115    *%-----|-----
116    CONTINUOUS NASHYD  NHYD=["NN_CK"], DT=[30]min, AREA=[1917] (ha),
117                      DWF=[0] (cms),   CN/C=[66], IA=[2.5] (mm),
118                      N=[3.0], TP=[5.29]hrs,
119                      Continuous simulation parameters:
120                      IaRECper=[4] (hrs),
121                      SMIN=[-1] (mm),   SMAX=[-1] (mm), SK=[0.010]/(mm),
122                      InterEventTime=[12] (hrs)
123                      Baseflow simulation parameters:
124                      BaseFlowOption=[1] ,
125                      InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
126                      VHydCond=[0.055] (mm/hr),   END=-1
127    *%-----|-----
128    *#
129    *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
130    *# of 1.52
131    *%-----|-----
132    CONTINUOUS NASHYD  NHYD=["SW_10"], DT=[30]min, AREA=[5666] (ha),

```

```

133 DWF=[0] (cms), CN/C=[72], IA=[2.5] (mm),
134 N=[3.0], TP=[8.00]hrs,
135 Continuous simulation parameters:
136 IaREcper=[4] (hrs),
137 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
138 InterEventTime=[12] (hrs)
139 Baseflow simulation parameters:
140 BaseFlowOption=[1] ,
141 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
142 VHydCond=[0.055] (mm/hr), END=-1
143 *%-----|-----
144 *#
145 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
146 *# of 1.75
147 *%-----|-----
148 CONTINUOUS NASHYD NHYD=["KG_CK"], DT=[30]min, AREA=[8376] (ha),
149 DWF=[0] (cms), CN/C=[66], IA=[2.5] (mm),
150 N=[3.0], TP=[11.66]hrs,
151 Continuous simulation parameters:
152 IaREcper=[4] (hrs),
153 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
154 InterEventTime=[12] (hrs)
155 Baseflow simulation parameters:
156 BaseFlowOption=[1] ,
157 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
158 VHydCond=[0.055] (mm/hr), END=-1
159 *%-----|-----
160 *#
161 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
162 *# of 1.68
163 *%-----|-----
164 CONTINUOUS NASHYD NHYD=["SW_9"], DT=[30]min, AREA=[1132] (ha),
165 DWF=[0] (cms), CN/C=[70], IA=[2.5] (mm),
166 N=[3.0], TP=[2.51]hrs,
167 Continuous simulation parameters:
168 IaREcper=[4] (hrs),
169 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
170 InterEventTime=[12] (hrs)
171 Baseflow simulation parameters:
172 BaseFlowOption=[1] ,
173 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
174 VHydCond=[0.055] (mm/hr), END=-1
175 *%-----|-----
176 *#
177 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
178 *# of 1.82
179 *%-----|-----
180 CONTINUOUS NASHYD NHYD=["NC_CK"], DT=[30]min, AREA=[4464] (ha),
181 DWF=[0] (cms), CN/C=[62], IA=[2.5] (mm),
182 N=[3.0], TP=[11.32]hrs,
183 Continuous simulation parameters:
184 IaREcper=[4] (hrs),
185 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
186 InterEventTime=[12] (hrs)
187 Baseflow simulation parameters:
188 BaseFlowOption=[1] ,
189 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
190 VHydCond=[0.055] (mm/hr), END=-1
191 *%-----|-----
192 *#
193 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
194 *# of 1.80
195 *%-----|-----
196 CONTINUOUS NASHYD NHYD=["SW_8"], DT=[30]min, AREA=[131] (ha),
197 DWF=[0] (cms), CN/C=[63], IA=[2.5] (mm),
198 N=[3.0], TP=[0.90]hrs,

```

```

199         Continuous simulation parameters:
200         IaREcper=[4] (hrs),
201         SMIN=[-1] (mm),  SMAX=[-1] (mm), SK=[0.010]/(mm),
202         InterEventTime=[12] (hrs)
203         Baseflow simulation parameters:
204         BaseFlowOption=[1] ,
205         InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
206         VHydCond=[0.055] (mm/hr),  END=-1
207     *%-----|-----
208     *#
209     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
210     *# of 1.65
211     *%-----|-----
212     CONTINUOUS NASHYD  NHYD=["HB_DR"], DT=[30]min, AREA=[3854] (ha),
213         DWF=[0] (cms),  CN/C=[66], IA=[2.5] (mm),
214         N=[3.0], TP=[8.42]hrs,
215         Continuous simulation parameters:
216         IaREcper=[4] (hrs),
217         SMIN=[-1] (mm),  SMAX=[-1] (mm), SK=[0.010]/(mm),
218         InterEventTime=[12] (hrs)
219         Baseflow simulation parameters:
220         BaseFlowOption=[1] ,
221         InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
222         VHydCond=[0.055] (mm/hr),  END=-1
223     *%-----|-----
224     *#
225     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
226     *# of 1.82
227     *%-----|-----
228     CONTINUOUS NASHYD  NHYD=["SW_7"], DT=[30]min, AREA=[3197] (ha),
229         DWF=[0] (cms),  CN/C=[57], IA=[2.5] (mm),
230         N=[3.0], TP=[6.65]hrs,
231         Continuous simulation parameters:
232         IaREcper=[4] (hrs),
233         SMIN=[-1] (mm),  SMAX=[-1] (mm), SK=[0.010]/(mm),
234         InterEventTime=[12] (hrs)
235         Baseflow simulation parameters:
236         BaseFlowOption=[1] ,
237         InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
238         VHydCond=[0.055] (mm/hr),  END=-1
239     *%-----|-----
240     *#
241     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
242     *# of 1.75
243     *%-----|-----
244     CONTINUOUS NASHYD  NHYD=["SW_6"], DT=[30]min, AREA=[165] (ha),
245         DWF=[0] (cms),  CN/C=[67], IA=[2.5] (mm),
246         N=[3.0], TP=[4.18]hrs,
247         Continuous simulation parameters:
248         IaREcper=[4] (hrs),
249         SMIN=[-1] (mm),  SMAX=[-1] (mm), SK=[0.010]/(mm),
250         InterEventTime=[12] (hrs)
251         Baseflow simulation parameters:
252         BaseFlowOption=[1] ,
253         InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
254         VHydCond=[0.055] (mm/hr),  END=-1
255     *%-----|-----
256     *#
257     *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
258     *# of 1.67
259     *%-----|-----
260     CONTINUOUS NASHYD  NHYD=["VG_DR"], DT=[30]min, AREA=[1332] (ha),
261         DWF=[0] (cms),  CN/C=[72], IA=[2.5] (mm),
262         N=[3.0], TP=[5.95]hrs,
263         Continuous simulation parameters:
264         IaREcper=[4] (hrs),

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265 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
266 InterEventTime=[12] (hrs)
267 Baseflow simulation parameters:
268 BaseFlowOption=[1] ,
269 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
270 VHydCond=[0.055] (mm/hr), END=-1
271 *%-----|
272 CONTINUOUS NASHYD NHYD=["SW_5"], DT=[30]min, AREA=[224] (ha),
273 DWF=[0] (cms), CN/C=[77], IA=[2.5] (mm),
274 N=[3.0], TP=[0.75]hrs,
275 Continuous simulation parameters:
276 IaRECper=[4] (hrs),
277 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
278 InterEventTime=[12] (hrs)
279 Baseflow simulation parameters:
280 BaseFlowOption=[1] ,
281 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
282 VHydCond=[0.055] (mm/hr), END=-1
283 *%-----|
284 *#
285 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
286 *# of 1.20
287 *%-----|
288 CONTINUOUS NASHYD NHYD=["FL_CK"], DT=[30]min, AREA=[4945] (ha),
289 DWF=[0] (cms), CN/C=[74], IA=[2.5] (mm),
290 N=[3.0], TP=[4.45]hrs,
291 Continuous simulation parameters:
292 IaRECper=[4] (hrs),
293 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
294 InterEventTime=[12] (hrs)
295 Baseflow simulation parameters:
296 BaseFlowOption=[1] ,
297 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
298 VHydCond=[0.055] (mm/hr), END=-1
299 *%-----|
300 CONTINUOUS NASHYD NHYD=["SW_5A2"], DT=[30]min, AREA=[20] (ha),
301 DWF=[0] (cms), CN/C=[81], IA=[2.5] (mm),
302 N=[3.0], TP=[0.62]hrs,
303 Continuous simulation parameters:
304 IaRECper=[4] (hrs),
305 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
306 InterEventTime=[12] (hrs)
307 Baseflow simulation parameters:
308 BaseFlowOption=[1] ,
309 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
310 VHydCond=[0.055] (mm/hr), END=-1
311 *%-----|
312 *#
313 *# The Tp was modified according to a Peak Reduction factor (MTO-Chart B2-4)
314 *# of 1.61
315 *%-----|
316 CONTINUOUS NASHYD NHYD=["SW_5A1"], DT=[30]min, AREA=[1412] (ha),
317 DWF=[0] (cms), CN/C=[75], IA=[2.5] (mm),
318 N=[3.0], TP=[8.00]hrs,
319 Continuous simulation parameters:
320 IaRECper=[4] (hrs),
321 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
322 InterEventTime=[12] (hrs)
323 Baseflow simulation parameters:
324 BaseFlowOption=[1] ,
325 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
326 VHydCond=[0.055] (mm/hr), END=-1
327 *%-----|
328 CONTINUOUS NASHYD NHYD=["SW_4"], DT=[30]min, AREA=[585] (ha),
329 DWF=[0] (cms), CN/C=[81], IA=[2.5] (mm),
330 N=[3.0], TP=[1.75]hrs,

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331 Continuous simulation parameters:
332 IaREcper=[4] (hrs),
333 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
334 InterEventTime=[12] (hrs)
335 Baseflow simulation parameters:
336 BaseFlowOption=[1] ,
337 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
338 VHydCond=[0.055] (mm/hr), END=-1
339 *%-----|
340 CONTINUOUS NASHYD NHYD=["LM_CK"], DT=[30]min, AREA=[1021] (ha),
341 DWF=[0] (cms), CN/C=[80], IA=[2.5] (mm),
342 N=[3.0], TP=[2.46]hrs,
343 Continuous simulation parameters:
344 IaREcper=[4] (hrs),
345 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
346 InterEventTime=[12] (hrs)
347 Baseflow simulation parameters:
348 BaseFlowOption=[1] ,
349 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
350 VHydCond=[0.055] (mm/hr), END=-1
351 *%-----|
352 CONTINUOUS NASHYD NHYD=["SW_2"], DT=[30]min, AREA=[177] (ha),
353 DWF=[0] (cms), CN/C=[77], IA=[2.5] (mm),
354 N=[3.0], TP=[0.75]hrs,
355 Continuous simulation parameters:
356 IaREcper=[4] (hrs),
357 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
358 InterEventTime=[12] (hrs)
359 Baseflow simulation parameters:
360 BaseFlowOption=[1] ,
361 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
362 VHydCond=[0.055] (mm/hr), END=-1
363 *%-----|
364 CONTINUOUS NASHYD NHYD=["SM_DR"], DT=[30]min, AREA=[1122] (ha),
365 DWF=[0] (cms), CN/C=[81], IA=[2.5] (mm),
366 N=[3.0], TP=[3.25]hrs,
367 Continuous simulation parameters:
368 IaREcper=[4] (hrs),
369 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
370 InterEventTime=[12] (hrs)
371 Baseflow simulation parameters:
372 BaseFlowOption=[1] ,
373 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
374 VHydCond=[0.055] (mm/hr), END=-1
375 *%-----|
376 CONTINUOUS NASHYD NHYD=["MO_DR"], DT=[30]min, AREA=[2737] (ha),
377 DWF=[0] (cms), CN/C=[76], IA=[2.5] (mm),
378 N=[3.0], TP=[3.03]hrs,
379 Continuous simulation parameters:
380 IaREcper=[4] (hrs),
381 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
382 InterEventTime=[12] (hrs)
383 Baseflow simulation parameters:
384 BaseFlowOption=[1] ,
385 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
386 VHydCond=[0.055] (mm/hr), END=-1
387 *%-----|
388 CONTINUOUS NASHYD NHYD=["SW_1"], DT=[30]min, AREA=[3176] (ha),
389 DWF=[0] (cms), CN/C=[78], IA=[2.5] (mm),
390 N=[3.0], TP=[3.56]hrs,
391 Continuous simulation parameters:
392 IaREcper=[4] (hrs),
393 SMIN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
394 InterEventTime=[12] (hrs)
395 Baseflow simulation parameters:
396 BaseFlowOption=[1] ,

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```

397          InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
398          VHydCond=[0.055] (mm/hr),   END=-1
399  *%-----|-----
400  *#
401  *# Routing hydrographs
402  *#
403  *# Starting with the addition of Jock River Headwater and Subwatershed 13
404  *#
405  ADD HYD          NHYDsum=["S_N13"], NHYDs to add=["JR_HW"+"SW_13"]
406  *%-----|-----
407  *#
408  *# Sum of hydrographs from Node 13 routed to Node 13A
409  *# (Approximated cross-section - see cross-section 258)
410  *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
411  *#
412  ROUTE CHANNEL    NHYDout=["N13A"] ,NHYDin=["S_N13"],
413                      RDT=[30] (min),
414                      CHLGTH=[9074] (m),   CHSLOPE=[0.0220] (%),
415                      FPSLOPE=[0.0220] (%),
416                      SECNUM=[1.0],       NSEG=[1]
417                      ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
418                      ( DISTANCE (m), ELEVATION (m))=
419                      [-40, 132.5]
420                      [-30, 132]
421                      [-25, 131.5]
422                      [-13, 130]
423                      [-8, 127.00]
424                      [-7, 126.50]
425                      [-6, 126]
426                      [-5.5, 125.50]
427                      [0, 123.75]
428                      [4.5, 125.50]
429                      [6, 126]
430                      [7.5, 126.5]
431                      [9, 127]
432                      [10, 127.5]
433                      [11.5, 128.0]
434                      [15.5, 129.5]
435  *%-----|-----
436  *#
437  *# Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
438  *#
439  ADD HYD          NHYDsum=["SN13A"], NHYDs to add=["N13A"+"JR_GWM"]
440  *%-----|-----
441  *#
442  *# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
443  *#
444  ROUTE RESERVOIR  NHYDout=["RES_GM"] ,NHYDin=["SN13A"],
445                      RDT=[30] (min),
446                      TABLE of ( OUTFLOW-STORAGE ) values
447                      (cms) - (ha-m)
448                      [ 0.0 , 0.0 ]
449                      [1.991, 2.144 ]
450                      [2.693, 39.826 ]
451                      [3.509, 81.697 ]
452                      [4.578, 318.774 ]
453                      [5.647, 594.947 ]
454                      [7.109, 910.219 ]
455                      [8.616, 1264.589 ]
456                      [10.371, 1658.057 ]
457                      [12.402, 2090.622 ]
458                      [22.056, 3462.487 ]
459                      [ -1 , -1 ] (max twenty pts)
460                      NHYDovf=[" " ] ,
461  *%-----|-----
462  *#

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```

463 SAVE HYD          NHYD=["RES_GM"], # OF PCYCLES=[-1], ICASEsh=[-1]
464                   HYD_FILENAME=["H_RESGM"]
465                   HYD_COMMENT=["Outflow from Res GM"]
466 *%-----|-----
467 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
468 *# (Approximated cross-section - see cross-section 258)
469 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
470 ROUTE CHANNEL    NHYDout=["N12"] ,NHYDin=["RES_GM"] ,
471                   RDT=[30] (min),
472                   CHLGTH=[5926] (m),  CHSLOPE=[0.0759] (%),
473                                           FPSLOPE=[0.0759] (%),
474                   SECNUM=[1.0],        NSEG=[1]
475                   ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
476                   ( DISTANCE (m), ELEVATION (m))=
477                       [-40, 132.5]
478                       [-30, 132]
479                       [-25, 131.5]
480                       [-13, 130]
481                       [-8, 127.00]
482                       [-7, 126.50]
483                       [-6, 126]
484                       [-5.5, 125.50]
485                       [0, 123.75]
486                       [4.5, 125.50]
487                       [6, 126]
488                       [7.5, 126.5]
489                       [9, 127]
490                       [10, 127.5]
491                       [11.5, 128.00]
492                       [15.5, 129.5]
493 *%-----|-----
494 *#
495 *# Addition of Subwatershed Jock River at Ashton to Node 12
496 *#
497 ADD HYD          NHYDsum=["S_N12"], NHYDs to add=["N12"+"JR_ASH"]
498 SAVE HYD          NHYD=["S_N12"], # OF PCYCLES=[-1], ICASEsh=[-1]
499                   HYD_FILENAME=["H_SN12"]
500                   HYD_COMMENT=["flow at S_N12 near Ashton"]
501 *%-----|-----
502 *#
503 *# Sum of hydrographs from Node 12 routed to Node 11
504 *# (Approximated cross-section - see cross-section 258)
505 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
506 ROUTE CHANNEL    NHYDout=["N11"] ,NHYDin=["S_N12"] ,
507                   RDT=[30] (min),
508                   CHLGTH=[972] (m),  CHSLOPE=[0.0514] (%),
509                                           FPSLOPE=[0.0514] (%),
510                   SECNUM=[1.0],        NSEG=[1]
511                   ( SEGROUGH, SEGDIST (m))=[0.04,15.5] NSEG times
512                   ( DISTANCE (m), ELEVATION (m))=
513                       [-40, 132.5]
514                       [-30, 132]
515                       [-25, 131.5]
516                       [-13, 130]
517                       [-8, 127.00]
518                       [-7, 126.50]
519                       [-6, 126]
520                       [-5.5, 125.50]
521                       [0, 123.75]
522                       [4.5, 125.50]
523                       [6, 126]
524                       [7.5, 126.5]
525                       [9, 127]
526                       [10, 127.5]
527                       [11.5, 128.00]
528                       [15.5, 129.5]

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595  *#
596  *# Addition of Subwatershed 10 to Node 10
597  *#
598  ADD HYD          NHYDsum=["S_N10"], NHYDs to add=["N10"+"SW_10"]
599  *%-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
600  SAVE HYD        NHYD=["S_N10"], # OF PCYCLES=[-1], ICASEsh=[-1]
601                  HYD_FILENAME=["H_SN10"]
602                  HYD_COMMENT=["flow at S_N10: N10 + SW_10"]
603  *%-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
604  *# Addition of Kings Creek to S_N10
605  *#
606  ADD HYD          NHYDsum=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
607  *%-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
608  *#
609  *# Sum of hydrographs from Node 10 routed to Node 9
610  *# Section 2
611  *#
612  ROUTE CHANNEL   NHYDout=["N9"] ,NHYDin=["S_N10A"] ,
613                  RDT=[30] (min),
614                  CHLGTH=[3982] (m),  CHSLOPE=[0.0753] (%),
615                                      FPSLOPE=[0.0753] (%),
616                  SECNUM=[1.0],      NSEG=[4]
617                  ( SEGROUGH, SEGDIST (m))=
618                    [0.04,-30.27
619                     0.05,-18.42
620                    -0.05,18.42
621                     0.04,131.58] NSEG times
622                  ( DISTANCE (m), ELEVATION (m))=
623                    [-446.74, 106.00]
624                    [-415.68, 105.50]
625                    [-285.40, 105.00]
626                    [-173.77, 104.50]
627                    [-144.95, 104.00]
628                    [-111.18, 103.50]
629                    [-94.06, 103.00]
630                    [-71.02, 102.50]
631                    [-30.27, 102.00]
632                    [-19.33, 100.00]
633                    [-18.42, 99.50]
634                    [18.42, 99.50]
635                    [20.77, 100.00]
636                    [27.93, 101.00]
637                    [52.29, 101.00]
638                    [68.80, 101.50]
639                    [79.66, 103.00]
640                    [91.50, 103.50]
641                    [131.58, 104.00]
642  *%-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
643  *#
644  *# Addition of Subwatershed 9 and Nichols Creek to Node 9
645  *#
646  ADD HYD          NHYDsum=["S_N9"], NHYDs to add=["N9"+"SW_9"+"NC_CK"]
647  *%-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
648  *#
649  *# Sum of hydrographs from Node 9 routed to Node 8
650  *# Section 3
651  *#
652  ROUTE CHANNEL   NHYDout=["N8"] ,NHYDin=["S_N9"] ,
653                  RDT=[30] (min),
654                  CHLGTH=[2269] (m),  CHSLOPE=[0.0882] (%),
655                                      FPSLOPE=[0.0882] (%),
656                  SECNUM=[1.0],      NSEG=[3]
657                  ( SEGROUGH, SEGDIST (m))=
658                    [0.1,-17.99
659                     -0.045,17.31
660                    0.1,456.58] NSEG times

```

```

661      ( DISTANCE (m), ELEVATION (m))=
662          [-201.19,100.50]
663          [-135.21, 100.00]
664          [-94.83, 99.50]
665          [-67.05, 99.00]
666          [-17.99, 98.50]
667          [-16.02, 98.00]
668          [-13.95, 97.50]
669          [13.95, 97.50]
670          [15.64, 98.00]
671          [17.31, 98.50]
672          [162.02, 98.50]
673          [172.89 ,99.00]
674          [314.38, 99.00]
675          [343.78, 99.50]
676          [365.67, 100.00]
677          [376.68, 100.00 ]
678          [393.11, 99.50]
679          [404.97, 99.50]
680          [431.70, 100.00]
681          [456.58, 100.50 ]
682 *%-----|-----
683 *#
684 *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
685 *#
686 ADD HYD          NHYDsum=["S_N8"], NHYDs to add=["N8"+"SW_8"+"HB_DR"]
687 *%-----|-----
688 *#
689 *# Sum of hydrographs from Node 8 routed to Node 7
690 *# Section 4
691 *#
692 ROUTE CHANNEL   NHYDout=["N7"] ,NHYDin=["S_N8"],
693                   RDT=[30] (min) ,
694                   CHLGTH=[3750] (m),   CHSLOPE=[0.0533] (%),
695                                           FPSLOPE=[0.0533] (%),
696                   SECNUM=[1.0],       NSEG=[3]
697                   ( SEGROUGH, SEGDIST (m))=
698                       [0.12,-18.11
699                       -0.07,17.22
700                       0.12,590.05] NSEG times
701                   ( DISTANCE (m), ELEVATION (m))=
702                       [-433.21, 102.00]
703                       [-425.34, 101.50]
704                       [-377.56, 101.50]
705                       [-366.23, 101.00]
706                       [-202.60, 100.50]
707                       [-96.25, 99.50]
708                       [-68.36 99.00]
709                       [-18.11, 98.50]
710                       [-13.81, 97.50]
711                       [13.81, 97.50]
712                       [17.22, 98.50]
713                       [161.95, 98.50]
714                       [173.11, 99.00]
715                       [314.05, 99.00]
716                       [365.52, 100.00]
717                       [404.70, 99.50]
718                       [476.74, 100.50]
719                       [502.31, 101.00]
720                       [584.69, 101.00]
721                       [585.79, 101.00]
722                       [590.05, 102.00]
723 *%-----|-----
724 *#
725 *# Addition of Subwatershed 7 to Node 7
726 *#

```

```

727 ADD HYD          NHYDsum=["S_N7"], NHYDs to add=["N7"+"SW_7"]
728 *%-----|-----|
729 SAVE HYD        NHYD=["S_N7"], # OF PCYCLES=[-1], ICASEsh=[-1]
730                HYD_FILENAME=["H_SN7"]
731                HYD_COMMENT=["flow at S_N7: N7 + SW_7"]
732 *%-----|-----|
733 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
734 *# Storage area and volumes were estimated from available topo maps.
735 *# Release rate from fen was assumed to be controlled by the downstream
736 *# river cross-section for summer conditions. It is was assumed that for up to
737 *# 0.75 m of water, the main channel of the river provided the storage. Above
738 *# this depth, the wetland starts to signigicantly store water.
739 *#
740 ROUTE RESERVOIR NHYDout=["RES_RF"] ,NHYDin=["S_N7"] ,
741                RDT=[30] (min),
742                TABLE of ( OUTFLOW-STORAGE ) values
743                (cms) - (ha-m)
744                TABLE of ( OUTFLOW-STORAGE ) values
745                (cms) - (ha-m)
746                [ 0.0 , 0.0 ]
747                [0.9051, 2.40]
748                [2.907, 4.13]
749                [9.744, 9.18]
750                [20.304, 14.96]
751                [34.167, 310.21]
752                [74.993, 605.46]
753                [104.876, 900.71]
754                [140.56, 2892.00]
755                [225.00, 3615.63]
756                [ -1 , -1 ] (max twenty pts)
757                NHYDovf=[" " ] ,
758 *%-----|-----|
759 SAVE HYD        NHYD=["RES_RF"], # OF PCYCLES=[-1], ICASEsh=[-1]
760                HYD_FILENAME=["H_ResRF"]
761                HYD_COMMENT=["outflow of Richmond Fen"]
762 *%-----|-----|
763 *#
764 *# Sum of hydrographs from Node 7 routed to Node 6
765 *# Section 5
766 *#
767 ROUTE CHANNEL   NHYDout=["N6"] ,NHYDin=["RES_RF"] ,
768                RDT=[30] (min),
769                CHLGTH=[3056] (m), CHSLOPE=[0.0818] (%),
770                FPSLOPE=[0.0818] (%),
771                SECNUM=[1.0], NSEG=[5]
772                ( SEGROUGH, SEGDIST (m))=
773                [0.025,-70.8
774                0.1,-23.9
775                -0.05,23.9
776                0.06,39.8
777                0.05,96.3] NSEG times
778                ( DISTANCE (m), ELEVATION (m))=
779                [-100.8, 97.00]
780                [-70.8, 96.50]
781                [-52.0, 96.00]
782                [-35.1, 95.50]
783                [-30.6, 95.00]
784                [-23.9, 94.54]
785                [23.9, 94.54]
786                [39.8, 95.00]
787                [50.4, 95.50]
788                [93.5, 96.00]
789                [94.9, 96.50]
790                [96.3, 97.00]
791 *%-----|-----|
792 *#

```

```

793  *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
794  *#
795  ADD HYD          NHYDsum=["S_N6"], NHYDs to add=["N6"+"SW_6"+"VG_DR"]
796  *%-----|-----
797  *#
798  *# Sum of hydrographs from Node 6 routed to Node 5
799  *# Section 6
800  *#
801  ROUTE CHANNEL    NHYDout=["N5"] ,NHYDin=["S_N6"] ,
802                  RDT=[30] (min),
803                  CHLGTH=[1852] (m),  CHSLOPE=[0.0540] (%),
804                                          FPSLOPE=[0.0540] (%),
805                  SECNUM=[1.0],      NSEG=[3]
806                  ( SEGROUGH, SEGDIST (m))=
807                    [0.035,-131.59
808                    -0.045,48.96
809                    0.1,239.04] NSEG times
810                  ( DISTANCE (m), ELEVATION (m))=
811                    [-686.30, 94.50]
812                    [-675.70, 94.00]
813                    [-492.52, 93.00]
814                    [-467.28, 94.00]
815                    [-131.59, 94.00]
816                    [-92.79, 92.50]
817                    [-18.06, 91.00]
818                    [18.06, 91.00]
819                    [43.47, 92.50]
820                    [48.96, 94.00]
821                    [177.43, 94.00]
822                    [239.04,94.50]
823  *%-----|-----
824  *#
825  *# Addition of Subwatershed 5 and Flowing Creek to Node 5
826  *#
827  ADD HYD          NHYDsum=["S_N5"], NHYDs to add=["N5"+"SW_5"+"FL_CK"]
828  *%-----|-----
829  *#
830  *# Sum of hydrographs from Node 5 routed to Node 5A
831  *# Section 7
832  *#
833  ROUTE CHANNEL    NHYDout=["N5A"] ,NHYDin=["S_N5"] ,
834                  RDT=[30] (min),
835                  CHLGTH=[556] (m),  CHSLOPE=[0.0900] (%),
836                                          FPSLOPE=[0.0900] (%),
837                  SECNUM=[1.0],      NSEG=[4]
838                  ( SEGROUGH, SEGDIST (m))=
839                    [0.04,-41.5
840                    0.1,-14.0
841                    -0.045,14.0
842                    0.1,41.1] NSEG times
843                  ( DISTANCE (m), ELEVATION (m))=
844                    [-275.8, 93.00]
845                    [-248.6, 92.50]
846                    [-237.0, 92.00]
847                    [-219.3, 91.50]
848                    [-202.1, 91.50]
849                    [-186.0, 92.00]
850                    [-129.2, 92.00]
851                    [-117.6, 91.50]
852                    [-100.6, 91.00]
853                    [-41.5, 91.00]
854                    [-20.0, 91.00]
855                    [-14.0, 90.54]
856                    [14.0, 90.54]
857                    [15.3, 91.00]
858                    [17.3, 91.50]

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859                                     [38.4, 92.00]
860                                     [39.8, 92.50]
861                                     [41.1, 93.00]
862 *%-----|-----
863 *#
864 *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
865 *#
866 ADD HYD                NHYDsum=["S_N5A"], NHYDs to add=["N5A"+"SW_5A2"+"SW_5A1"]
867 *%-----|-----
868 *#
869 *# Sum of hydrographs from Node 5A routed to Node 4
870 *# Section 8
871 *#
872 ROUTE CHANNEL        NHYDout=["N4"] ,NHYDin=["S_N5A"] ,
873                          RDT=[30] (min),
874                          CHLGTH=[4630] (m),  CHSLOPE=[0.0432] (%),
875                          FPSLOPE=[0.0432] (%),
876                          SECNUM=[1.0],      NSEG=[3]
877                          ( SEGROUGH, SEGDIST (m))=
878                          [0.05,-28.2
879                          -0.035,28.2
880                          0.05,173.1] NSEG times
881                          ( DISTANCE (m), ELEVATION (m))=
882                                     [-38.9, 92.00]
883                                     [-35.8, 91.50]
884                                     [-33.3, 91.00]
885                                     [-28.2, 90.50]
886                                     [-15.0, 87.48]
887                                     [-5.0, 88.34]
888                                     [5.0, 86.20]
889                                     [15.0, 88.55]
890                                     [28.2, 90.50]
891                                     [29.7, 91.00]
892                                     [46.5, 91.00]
893                                     [127.8, 91.00]
894                                     [148.7, 91.50]
895                                     [173.1, 92.00]
896 *%-----|-----
897 *#
898 *# Addition of Subwatershed 4 and Leamy Creek to Node 4
899 *#
900 ADD HYD                NHYDsum=["S_N4"], NHYDs to add=["N4"+"SW_4"+"LM_CK"]
901 SAVE HYD              NHYD=["S_N4"], # OF PCYCLES=[-1], ICASEsh=[1]
902                          HYD_COMMENT=["flow at S_N4"]
903 *%-----|-----
904 *#
905 *# Sum of hydrographs from Node 4 routed to Node 2
906 *# Section 9
907 *#
908 ROUTE CHANNEL        NHYDout=["N2"] ,NHYDin=["S_N4"] ,
909                          RDT=[30] (min),
910                          CHLGTH=[1667] (m),  CHSLOPE=[0.0600] (%),
911                          FPSLOPE=[0.0600] (%),
912                          SECNUM=[1.0],      NSEG=[4]
913                          ( SEGROUGH, SEGDIST (m))=
914                          [0.1,-28.0
915                          -0.04,28.4
916                          0.06,31.7
917                          0.04,80.2] NSEG times
918                          ( DISTANCE (m), ELEVATION (m))=
919                                     [-36.3, 92.00]
920                                     [-32.6, 91.50]
921                                     [-30.2, 91.00]
922                                     [-28.0, 90.45]
923                                     [-15.0, 87.48]
924                                     [-5.0, 88.34]

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925             [5.0, 86.20]
926             [15.0, 88.55]
927             [28.0, 90.45]
928             [28.4, 90.50]
929             [30.4, 91.00]
930             [31.7, 91.50]
931             [80.2, 92.00]
932 *%-----|-----
933 *#
934 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
935 *#
936 ADD HYD           NHYDsum=["S_N2"], NHYDs to add=["N2"+"SW_2"+"SM_DR"+"MO_DR"]
937 *%-----|-----
938 SAVE HYD         NHYD=["S_N2"], # OF PCYCLES=[-1], ICASEsh=[-1]
939                   HYD_FILENAME=["H_SN2"]
940                   HYD_COMMENT=["flow at S_N2 Jock River Gauge at Moodie Dr."]
941 *%-----|-----
942 *#
943 *# Sum of hydrographs from Node 2 routed to Node 1
944 *# Section 10
945 *#
946 ROUTE CHANNEL   NHYDout=["N1"] ,NHYDin=["S_N2"] ,
947                   RDТ=[30] (min),
948                   CHLGTH=[10046] (m),  CHSLOPE=[0.0498] (%),
949                                     FPSLOPE=[0.0498] (%),
950                   SECNUM=[1.0],        NSEG=[5]
951                   ( SEGRROUGH, SEGDIST (m))=
952                     [0.04,-27.6
953                     0.06,-15.0
954                     -0.045,15.0
955                     0.06,25.4
956                     0.04,122.6] NSEG times
957                   ( DISTANCE (m), ELEVATION (m))=
958                                     [-87.0, 91.50]
959                                     [-32.4, 91.00]
960                                     [-27.6, 90.50]
961                                     [-25.0, 90.00]
962                                     [-22.9, 89.57]
963                                     [-15.0, 86.20]
964                                     [-5.0, 84.83]
965                                     [5.0, 84.83]
966                                     [15.0, 88.11]
967                                     [22.9, 89.57]
968                                     [25.4, 90.00]
969                                     [27.9, 90.50]
970                                     [38.0, 91.00]
971                                     [112.5, 91.00]
972                                     [114.3, 90.50]
973                                     [115.1, 90.26]
974                                     [116.3, 90.50]
975                                     [119.0, 91.00]
976                                     [121.0, 91.50]
977                                     [122.6, 92.00]
978 *%-----|-----
979 *#
980 *# Addition of Subwatershed 1 to Node 1
981 *#
982 ADD HYD           NHYDsum=["N1"], NHYDs to add=["N1"+"SW_1"]
983 SAVE HYD         NHYD=["N1"], # OF PCYCLES=[-1], ICASEsh=[1]
984                   HYD_COMMENT=["total outflow of Jock River"]
985 *%-----|-----
986 *#####
987 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
988 START           TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[5]
989 *%               ["C24SC005.stm"] <--storm filename, one per line for NSTORM time
990 *%-----|-----

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991  *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
992  START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[10]
993  *%              ["C24SC010.stm"] <--storm filename, one per line for NSTORM time
994  *%-----|-----|
995  *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
996  START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[25]
997  *%              ["C24SC025.stm"] <--storm filename, one per line for NSTORM time
998  *%-----|-----|
999  *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
1000 START         TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[50]
1001 *%              ["C24SC050.stm"] <--storm filename, one per line for NSTORM time
1002 *%-----|-----|
1003 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
1004 START         TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
1005 *%              ["C24SC100.stm"] <--storm filename, one per line for NSTORM time
1006 FINISH
1007
```


003755 + 30.0 02:SW10 5666.00 10.936 No_date 38:00 13.91 n/a .000
003760 ROUTE CHANL > 30.0 02:SN 17589.00 19.098 No_date 38:30 12.16 n/a .000
003770 R0002:CD0043 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
003780 SAVE HYD >> 30.0 01:SN 17589.00 19.098 No_date 38:30 12.16 n/a .000
003790 # name :H_SND
003800 # remark:flow at S_N0: N0 + SW10
003810 # Addition of Subwatershed 4 and Nichols Creek to Node 9
003820 #
003830 R0002:CD0044 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
003840 ADD HYD + 30.0 02:SN 17589.00 19.098 No_date 38:30 12.16 n/a .000
003850 + 30.0 02:SW2 8376.00 10.656 No_date 39:30 11.98 n/a .000
003860 SUM 30.0 01:SN 25965.00 29.622 No_date 39:30 12.09 n/a .000
003870 #
003880 # Sum of hydrographs from Node 10 routed to Node 9
003890 # Section 2
003900 #
003910 R0002:CD0045 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
003920 ROUTE CHANL > 30.0 02:SN 25965.00 29.622 No_date 39:30 12.09 n/a .000
003930 [RFS=30.00] out > 30.0 01:SN 25965.00 29.622 No_date 39:30 12.09 n/a .000
003940 [L/S=3982. / 0751/040]
003950 [Vmax =591;Dmax= 1.193]
003960 #
003970 # Addition of Subwatershed 9 and Nichols Creek to Node 9
003980 #
003990 R0002:CD0046 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004000 ADD HYD + 30.0 02:SN 25965.00 29.622 No_date 39:30 12.09 n/a .000
004010 + 30.0 02:SW9 1132.00 4.365 No_date 30:30 13.32 n/a .000
004020 + 30.0 02:NC CK 4464.00 5.212 No_date 39:30 10.96 n/a .000
004030 SUM 30.0 01:SN 31561.00 35.488 No_date 39:30 11.98 n/a .000
004040 #
004050 # Sum of hydrographs from Node 9 routed to Node 8
004060 # Section 3
004070 #
004080 R0002:CD0047 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004090 ROUTE CHANL > 30.0 02:SN 31561.00 33.301 No_date 40:00 11.98 n/a .000
004100 [RFS=30.00] out > 30.0 01:SN 31561.00 33.301 No_date 40:00 11.98 n/a .000
004110 [L/S=2269. /0887/045]
004120 [Vmax =420;Dmax= 1.290]
004130 #
004140 # Addition of Subwatershed 8 and Hibbs' Drain to Node 8
004150 #
004160 R0002:CD0048 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004170 ADD HYD + 30.0 02:SN 31561.00 33.301 No_date 40:00 11.98 n/a .000
004180 + 30.0 02:SW 131.00 7.70 No_date 28:30 11.20 n/a .000
004190 + 30.0 02:IBR 3854.00 6.083 No_date 38:30 11.95 n/a .000
004200 SUM 30.0 01:SN 35546.00 39.356 No_date 39:30 11.97 n/a .000
004210 #
004220 # Sum of hydrographs from Node 8 routed to Node 7
004230 # Section 4
004240 #
004250 R0002:CD0049 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004260 ROUTE CHANL > 30.0 01:SN 35546.00 39.356 No_date 39:30 11.97 n/a .000
004270 [RFS=30.00] out > 30.0 01:SN 35546.00 39.356 No_date 44:00 11.97 n/a .000
004280 [L/S=3056. / 0822/025]
004290 [Vmax =209;Dmax= 1.635]
004300 #
004310 # Addition of Subwatershed 7 to Node 7
004320 #
004330 R0002:CD0050 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004340 ADD HYD + 30.0 02:SN 35546.00 39.356 No_date 44:00 11.97 n/a .000
004350 + 30.0 02:SW 3197.00 4.557 No_date 26:30 9.83 n/a .000
004360 + 30.0 01:SN 38743.00 34.345 No_date 43:00 11.79 n/a .000
004370 R0002:CD0051 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004380 SAVE HYD >> 30.0 01:SN 38743.00 34.345 No_date 43:00 11.79 n/a .000
004390 # name :H_SND
004400 # remark:flow at S_N7: N7 + SW7
004410 # Insertion of a reservoir to simulate the effects of the Richmond Fen.
004420 # Storage area and volumes were estimated from available topog.
004430 # Release rate from fen was assumed to be controlled by the downstream
004440 # river cross-section for summer conditions. It is assumed that for up to
004450 # 0.75 m of water of the river provided the storage. Above
004460 # this depth, the wetland starts to significantly store water.
004470 #
004480 R0002:CD0052 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004490 ROUTE RESERVOIR >> 30.0 02:SN 38743.00 34.345 No_date 43:00 11.79 n/a .000
004500 + 30.0 01:RES_RF 38743.00 23.075 No_date 54:30 11.79 n/a .000
004510 [MS=0.04e6;T399902=0]
004520 R0002:CD0053 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004530 SAVE HYD >> 30.0 01:RES_RF 38743.00 23.075 No_date 54:30 11.79 n/a .000
004540 # name :H_ResRF
004550 # remark:outflow of Richmond Fen
004560 #
004570 # Sum of hydrographs from Node 7 routed to Node 6
004580 # Section 5
004590 #
004600 R0002:CD0054 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004610 ROUTE CHANL > 30.0 02:RES_RF 38743.00 23.075 No_date 54:30 11.79 n/a .000
004620 [RFS=30.00] out > 30.0 01:RES_RF 38743.00 23.075 No_date 56:00 11.79 n/a .000
004630 [L/S=3056. / 0822/025]
004640 [Vmax =43;Dmax= 0.05]
004650 #
004660 # Addition of Subwatershed 6 and Van Gal Drain to Node 6
004670 #
004680 R0002:CD0055 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004690 ADD HYD + 30.0 02:SN 40240.01 23.171 No_date 55:00 11.87 n/a .000
004700 + 30.0 02:SW 165.00 4.07 No_date 33:00 12.21 n/a .000
004710 + 30.0 02:IBR 1132.00 2.083 No_date 38:30 11.87 n/a .000
004720 SUM 30.0 01:SN 40240.01 23.225 No_date 39:30 11.87 n/a .000
004730 #
004740 # Sum of hydrographs from Node 6 routed to Node 5
004750 # Section 6
004760 #
004770 R0002:CD0056 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004780 ROUTE CHANL > 30.0 02:SN 40240.01 23.171 No_date 55:00 11.87 n/a .000
004790 [RFS=30.00] out > 30.0 01:SN 40240.01 23.171 No_date 55:00 11.87 n/a .000
004800 [L/S=1852. /0547/035]
004810 [Vmax =378;Dmax= 0.95]
004820 #
004830 # Addition of Subwatershed 5 and Flowing Creek to Node 5
004840 #
004850 R0002:CD0057 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004860 ADD HYD + 30.0 02:SN 40240.01 23.171 No_date 55:00 11.87 n/a .000
004870 + 30.0 02:SW 24.00 2.527 No_date 28:30 15.88 n/a .000
004880 + 30.0 02:IBR 4945.00 14.54 No_date 39:30 17.76 n/a .000
004890 SUM 30.0 01:SN 52509.01 32.974 No_date 37:00 12.18 n/a .000
004900 #
004910 # Sum of hydrographs from Node 5 routed to Node 5A
004920 # Section 7
004930 #
004940 R0002:CD0058 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
004950 ROUTE CHANL > 30.0 01:SN 45409.01 32.974 No_date 37:00 12.18 n/a .000
004960 [RFS=30.00] out > 30.0 01:SN 45409.01 32.974 No_date 37:00 12.18 n/a .000
004970 [L/S=556. /0905/040]
004980 [Vmax =443;Dmax= .935]
004990 #
005000 # Addition of Subwatershed SA1 and Subwatershed 5A2 to Node 5A
005010 #
005020 R0002:CD0059 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005030 ADD HYD + 30.0 02:SN 45409.01 32.974 No_date 37:00 12.18 n/a .000
005040 + 30.0 02:SW 287.00 2.87 No_date 28:30 17.76 n/a .000
005050 + 30.0 02:SW SA1 4412.00 3.007 No_date 38:00 15.19 n/a .000
005060 SUM 30.0 01:SN 46841.01 35.939 No_date 37:00 12.27 n/a .000
005070 #
005080 # Sum of hydrographs from Node 5A routed to Node 4
005090 # Section 8
005100 #
005110 R0002:CD0060 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005120 ROUTE CHANL > 30.0 01:SN 46841.01 35.939 No_date 37:00 12.27 n/a .000
005130 [RFS=30.00] out > 30.0 01:SN 46841.01 35.939 No_date 39:00 12.27 n/a .000
005140 [L/S=4930. /0471/035]
005150 [Vmax =.693;Dmax= 2.836]
005160 #
005170 # Addition of Subwatershed 4 and Leamy Creek to Node 4
005180 #
005190 R0002:CD0061 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005200 ADD HYD + 30.0 02:SN 46841.01 35.939 No_date 39:00 12.27 n/a .000
005210 + 30.0 02:SW 585.00 4.232 No_date 29:00 15.24 n/a .000
005220 + 30.0 02:IBR 1021.00 5.667 No_date 30:30 17.36 n/a .000
005230 SUM 30.0 01:SN 48447.00 37.399 No_date 38:30 12.44 n/a .000
005240 R0002:CD0062 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005250 SAVE HYD >> 30.0 01:SN 48447.00 37.399 No_date 38:30 12.44 n/a .000
005260 # name :S_N 0002
005270 # remark:flow at S_N
005280 #
005290 # Sum of hydrographs from Node 4 routed to Node 2
005300 # Section 9
005310 #
005320 R0002:CD0063 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005330 ROUTE CHANL > 30.0 01:SN 48447.00 37.399 No_date 39:00 12.44 n/a .000
005340 [RFS=30.00] out > 30.0 01:SN 48447.00 37.399 No_date 39:00 12.44 n/a .000
005350 [L/S=1067. /0610/040]
005360 [Vmax =714;Dmax= 2.841]
005370 #
005380 # Addition of Subwatershed 2 with Mbohan Drain and Smith Drain to Node 2
005390 #
005400 R0002:CD0064 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005410 ADD HYD + 30.0 02:SN 48447.00 37.399 No_date 39:00 12.44 n/a .000
005420 + 30.0 02:SW 277.00 1.996 No_date 28:30 15.88 n/a .000
005430 + 30.0 02:IBR 1122.00 5.257 No_date 31:30 17.76 n/a .000
005440 + 30.0 02:MO DR 1737.00 11.338 No_date 31:30 15.83 n/a .000
005450 SUM 30.0 01:SN 52483.00 45.676 No_date 33:30 12.73 n/a .000
005460 R0002:CD0065 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005470 SAVE HYD >> 30.0 01:SN 52483.00 45.676 No_date 33:30 12.73 n/a .000
005480 # name :H_SND
005490 # remark:flow at S_N Jock River Gauge at Meodie Dr.
005500 #
005510 # Sum of hydrographs from Node 2 routed to Node 1
005520 # Section 9
005530 #
005540 R0002:CD0066 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005550 ROUTE CHANL > 30.0 02:SN 52483.00 45.676 No_date 33:30 12.73 n/a .000
005560 [RFS=30.00] out > 30.0 01:SN 52483.00 42.605 No_date 39:30 12.73 n/a .000
005570 [L/S=10406. /0501/040]
005580 [Vmax =.767;Dmax= 2.662]
005590 #
005600 # Addition of Subwatershed 1 to Node 1
005610 #

005620 R0002:CD0067 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005630 ADD HYD + 30.0 02:SN 52483.00 42.605 No_date 39:30 12.73 n/a .000
005640 + 30.0 02:SW 1 3176.00 12.490 No_date 32:00 16.23 n/a .000
005650 SUM 52483.00 49.164 No_date 36:30 12.93 n/a .000
005660 R0002:CD0068 >> Dfm-n-1D-NDD >>> AREAh-AQEAgm-TpeakDte-hh-mm--Rvmm-R.C--Dfwm
005670 SAVE HYD >> 30.0 01:SN 52483.00 49.164 No_date 36:30 12.93 n/a .000
005680 # name :N0 0002
005690 # remark:total outflow of Jock River
005700 # *****
005710 ** END OF RUN : 4
005720 *****
005730 *****
005740 *****
005750 *****
005760 *****
005770 *****
005780 *****
005790 *****
005800 *****
005810 *****
005820 *****
005830 *****
005840 *****
005850 *****
005860 *****
005870 *****
005880 *****
005890 *****
005900 *****
005910 *****
005920 *****
005930 *****
005940 *****
005950 *****
005960 *****
005970 *****
005980 *****
005990 *****
006000 *****
006010 *****
006020 *****
006030 *****
006040 *****
006050 *****
006060 *****
006070 *****
006080 *****
006090 *****
006100 *****
006110 *****
006120 *****
006130 *****
006140 *****
006150 *****
006160 *****
006170 *****
006180 *****
006190 *****
006200 *****
006210 *****
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Table with columns for ID, description, and numerical values. Includes entries like 012125, 012126, 012127, etc., with detailed descriptions of peak reduction factors and reservoir insertions.

Table with columns for ID, description, and numerical values. Includes entries like 013105, 013106, 013107, etc., with detailed descriptions of reservoir insertions and flow routing.

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014977 # SUM= 30.0 01: NS 45409.01 50.940 No.date 34:30 21.53 n/a .000
014988 #
014989 # Sum of hydrographs from Node 5 routed to Node 5A
015000 # Section 7
015010 #
015020 # R010: C00058..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015030 ROUTE CHANNEL -> 30.0 02: S_N 45409.01 50.940 No.date 34:30 21.53 n/a .000
015040 [RFD=30.00] out.c 30.0 01: NS 45409.01 50.883 No.date 35:00 21.53 n/a .000
015050 [L/S= 1667. / 0.017 0.05]
015060 [Vmax = 484; Dmax= 1.127]
015070 #
015080 # Addition of Subwatershed SAI and Subwatershed 5A2 to Node 5A
015090 #
015100 # R010: C00059..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015110 AID HYD + 30.0 02: NS 45409.01 50.883 No.date 35:00 21.53 n/a .000
015120 + 30.0 02: SWA 20.00 5.69 No.date 28:30 31.34 n/a .000
015130 + 30.0 02: SWA 1412.00 5.651 No.date 37:30 27.03 n/a .000
015140 SUM= 30.0 01: NS 46484.01 56.195 No.date 35:00 21.70 n/a .000
015150 #
015160 # Sum of hydrographs from Node 5A routed to Node 4
015170 # Section 8
015180 #
015190 # R010: C00060..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015200 ROUTE CHANNEL -> 30.0 02: S_NA 46484.01 56.195 No.date 35:00 21.70 n/a .000
015210 [RFD=30.00] out.c 30.0 01: NS 46484.01 54.050 No.date 36:30 21.70 n/a .000
015220 [L/S= 4630. / 0.017 0.05]
015230 [Vmax = 790; Dmax= 3.283]
015240 #
015250 # Addition of Subwatershed 4 and Leamy Creek to Node 4
015260 #
015270 # R010: C00061..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015280 AID HYD + 30.0 02: NS 46484.01 54.050 No.date 36:30 21.70 n/a .000
015290 + 30.0 02: SWA 585.00 8.289 No.date 29:30 31.34 n/a .000
015300 + 30.0 02: LMCK 1021.00 11.041 No.date 30:30 30.69 n/a .000
015310 SUM= 30.0 01: NS 48447.00 52.883 No.date 36:00 22.01 n/a .000
015320 R010: C00062..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015330 SAVE HYD 30.0 01: S_N 48447.00 59.486 No.date 36:00 22.01 n/a .000
015340 frame : S_N 0010
015350 remark: flow at S_N
015360 #
015370 # Sum of hydrographs from Node 4 routed to Node 2
015380 # Section 9
015390 #
015400 # R010: C00063..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015410 ROUTE CHANNEL -> 30.0 01: NS 48447.00 59.486 No.date 36:00 22.01 n/a .000
015420 [RFD=30.00] out.c 30.0 01: NS 48447.00 59.258 No.date 36:00 22.01 n/a .000
015430 [L/S= 1667. / 0.017 0.05]
015440 [Vmax = 822; Dmax= 3.316]
015450 #
015460 # Addition of Subwatershed 2 with Maohan Drain and Smith Drain to Node 2
015470 #
015480 # R010: C00064..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015490 AID HYD + 30.0 02: NS 48447.00 59.258 No.date 36:00 22.01 n/a .000
015500 + 30.0 02: SWA 3176.00 24.273 No.date 28:30 28.21 n/a .000
015510 + 30.0 02: SMIR 1122.00 10.121 No.date 31:30 31.34 n/a .000
015520 + 30.0 02: MDLR 2737.00 22.263 No.date 34:30 27.61 n/a .000
015530 SUM= 30.0 01: NS 52483.00 82.076 No.date 33:00 22.52 n/a .000
015540 R010: C00065..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015550 SAVE HYD 30.0 01: S_N 52483.00 82.076 No.date 33:00 22.52 n/a .000
015560 frame : H_SND
015570 remark: flow at S_N2 Jock River Gauge at Middle R.
015580 #
015590 # Sum of hydrographs from Node 2 routed to Node 1
015600 # Section
015610 #
015620 # R010: C00066..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015630 ROUTE CHANNEL -> 30.0 02: S_N 52483.00 82.076 No.date 33:00 22.52 n/a .000
015640 [RFD=30.00] out.c 30.0 01: NS 52483.00 72.984 No.date 36:30 22.52 n/a .000
015650 [L/S= 10464. / 0.017 0.05]
015660 [Vmax = 924; Dmax= 3.539]
015670 #
015680 # Addition of Subwatershed 1 to Node 1
015690 #
015700 # R010: C00067..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015710 AID HYD + 30.0 02: NS 52483.00 72.984 No.date 36:30 22.52 n/a .000
015720 + 30.0 02: SWA 3176.00 24.273 No.date 28:30 28.21 n/a .000
015730 SUM= 30.0 01: NS 55659.00 89.955 No.date 34:30 22.88 n/a .000
015740 R010: C00068..... Dfm=1D NND..... AREAh-QPEAGm-TpeakDte-hh:mm--- Rvfm R.C... DfWcm
015750 SAVE HYD 30.0 01: NS 55659.00 89.955 No.date 34:30 22.88 n/a .000
015760 frame : 0010
015770 remark: outflow of Jock River
015780 #####
015790 # END OF RUN 21.000
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018173 # 30.0 02: NS CK 1917.00 10.139 No.date 34.00 26.99 n/a .000
018178 SIM 30.0 02: S_N1 11923.00 27.440 No.date 33.00 25.40 n/a .000
018179 # Sum of hydrographs from Node 11 routed to Node 10
018175 # Section 1
018176 #
018177 R0025: C00041 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
018178 ROUTE CHANNEL -> 30.0 02: S_N1 11923.00 27.440 No.date 33.00 25.40 n/a .000
018179 [RDR-30.00] out c 30.0 01: N1 11923.00 17.756 No.date 40.00 25.40 n/a .000
018180 [L/S=104028 / 1577.040]
018181 [Vmax =.463;Dmax=1.320]
018182 #
018183 # Addition of Subwatershed 10 to Node 10
018184 #
018185 R0025: C00042 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
018186 ADD HYD + 30.0 02: NS 31561.00 44.045 No.date 38.30 27.35 n/a .000
018187 + 30.0 02: SW10 5666.00 26.665 No.date 37.30 31.47 n/a .000
018188 SIM 30.0 01: S_N1 17589.00 24.045 No.date 38.30 27.35 n/a .000
018189 R0025: C00043 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
018190 SAVE HYD 30.0 01: S_N1 17589.00 44.045 No.date 38.30 27.35 n/a .000
018191 frame -H_SND
018192 remark:flow at S_N1: N0 +SW10
018193 # Addition of Kings Creek to S_N1
018194 #
018195 R0025: C00044 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
018196 ADD HYD + 30.0 02: NS 8376.00 25.107 No.date 39.30 26.99 n/a .000
018197 SIM 30.0 01: S_N1A 25965.00 68.824 No.date 39.30 27.24 n/a .000
018198 #
018199 # Sum of hydrographs from Node 10 routed to Node 9
019001 # Section 2
019002 #
019003 R0025: C00045 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019004 ROUTE CHANNEL -> 30.0 02: S_N1A 25965.00 68.824 No.date 39.30 27.24 n/a .000
019005 [RDR-30.00] out c 30.0 01: N1 31561.00 25.965 No.date 39.30 27.24 n/a .000
019006 [L/S=3982 / 0757.040]
019007 [Vmax =.713;Dmax=1.864]
019008 #
019009 # Addition of Subwatershed 9 and Nichols Creek to Node 9
019010 #
019011 R0025: C00046 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019012 ADD HYD + 30.0 02: SW9 1132.00 11.574 No.date 30.30 30.15 n/a .000
019013 + 30.0 02: NS CK 4464.00 12.525 No.date 39.30 24.58 n/a .000
019014 SIM 30.0 01: S_N1 31561.00 82.190 No.date 39.30 26.97 n/a .000
019015 #
019016 # Sum of hydrographs from Node 9 routed to Node 8
019017 # Section 3
019018 #
019019 R0025: C00047 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019020 ROUTE CHANNEL -> 30.0 02: S_N1 31561.00 82.190 No.date 39.30 26.97 n/a .000
019021 [RDR-30.00] out c 30.0 01: N1 31561.00 78.196 No.date 40.00 26.97 n/a .000
019022 [L/S=2269 / 0887.045]
019023 [Vmax =.362;Dmax=1.727]
019024 #
019025 # Addition of Subwatershed 8 and Hobbs' Drain to Node 8
019026 #
019027 R0025: C00048 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019028 ADD HYD + 30.0 02: SW8 35546.00 78.196 No.date 45.00 26.96 n/a .000
019029 + 30.0 02: SW3 131.00 2.156 No.date 28.30 25.17 n/a .000
019030 SIM 30.0 01: S_N1 35546.00 91.271 No.date 39.30 26.96 n/a .000
019031 #
019032 # Sum of hydrographs from Node 8 routed to Node 7
019033 # Section 4
019034 #
019035 R0025: C00049 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019036 ROUTE CHANNEL -> 30.0 02: S_N1 35546.00 91.271 No.date 39.30 26.96 n/a .000
019037 [RDR-30.00] out c 30.0 01: N1 35546.00 78.196 No.date 45.00 26.96 n/a .000
019038 [L/S=3750 / 0537.070]
019039 [Vmax =.225;Dmax =1.864]
019040 #
019041 # Addition of Subwatershed 7 to Node 7
019042 #
019043 R0025: C00050 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019044 ADD HYD + 30.0 02: SW7 3197.00 11.391 No.date 36.00 21.73 n/a .000
019045 SIM 30.0 01: S_N1 38743.00 84.011 No.date 44.00 26.53 n/a .000
019046 R0025: C00051 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019047 SAVE HYD 30.0 01: S_N1 38743.00 84.011 No.date 44.00 26.53 n/a .000
019048 frame -H_SNF
019049 remark:flow at S_N1: N7 +SW7
019050 # Insertion of a reservoir to simulate the effects of the Richmond Fen
019051 # Storage area and volumes were estimated from available top mps.
019052 # Release rate from fen assumed to be controlled by the downstream
019053 # river cross-section for summer conditions. It was assumed that for up
019054 # to 0.75 m of water, the main channel of the river provided the storage. Above
019055 # this depth, the wetland starts to significantly store water.
019056 #
019057 R0025: C00052 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019058 ROUTE RESERVOIR -> 30.0 02: S_N1 38743.00 84.011 No.date 44.00 26.53 n/a .000
019059 [RDR-30.00] out c 30.0 01: RES_RF 38743.00 40.725 No.date 60.30 26.53 n/a .000
019060 [MSI=0.000;S=377E+03;]
019061 R0025: C00053 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019062 SAVE HYD 30.0 01: S_N1 38743.00 84.011 No.date 44.00 26.53 n/a .000
019063 frame -H_RES_RF
019064 remark:flow at S_N1: N7 +RES_RF
019065 # Sum of hydrographs from Node 7 routed to Node 6
019066 # Section 5
019067 #
019068 R0025: C00054 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019069 ROUTE CHANNEL -> 30.0 02: RES_RF 38743.00 40.725 No.date 60.30 26.53 n/a .000
019070 [RDR-30.00] out c 30.0 01: N6 38743.00 40.549 No.date 61.30 26.53 n/a .000
019071 [L/S=3056 / 0827.025]
019072 [Vmax =.510;Dmax=1.101]
019073 #
019074 # Addition of Subwatershed 6 and Van Gaal Drain to Node 6
019075 #
019076 R0025: C00055 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019077 ADD HYD + 30.0 02: SW6 38743.00 40.549 No.date 61.30 26.53 n/a .000
019078 + 30.0 02: SW6 165.00 1.056 No.date 33.00 27.61 n/a .000
019079 SIM 30.0 02: NS CK 1332.00 7.707 No.date 35.00 31.47 n/a .000
019080 SIM 30.0 01: S_N1 40240.00 61.804 No.date 61.30 26.70 n/a .000
019081 #
019082 # Sum of hydrographs from Node 6 routed to Node 5
019083 # Section 6
019084 #
019085 R0025: C00056 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019086 ROUTE CHANNEL -> 30.0 02: NS CK 40240.00 61.804 No.date 61.30 26.70 n/a .000
019087 [RDR-30.00] out c 30.0 01: N5 40240.00 40.523 No.date 62.30 26.70 n/a .000
019088 [L/S=1852 / 0547.035]
019089 [Vmax =.440;Dmax=1.203]
019090 #
019091 # Addition of Subwatershed 5 and Flowing Creek to Node 5
019092 #
019093 R0025: C00057 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019094 ADD HYD + 30.0 02: SW5 45409.00 61.906 No.date 34.00 27.41 n/a .000
019095 + 30.0 02: SW5 224.00 6.682 No.date 28.30 35.63 n/a .000
019096 SIM 30.0 01: S_N1 45409.00 61.906 No.date 34.00 27.41 n/a .000
019097 #
019098 # Sum of hydrographs from Node 5 routed to Node 4
019099 # Section 7
019100 #
019101 R0025: C00058 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019102 ROUTE CHANNEL -> 30.0 02: NS CK 45409.00 61.906 No.date 34.00 27.41 n/a .000
019103 [RDR-30.00] out c 30.0 01: N4 45409.00 61.906 No.date 34.00 27.41 n/a .000
019104 [L/S=556 / 0907.040]
019105 [Vmax =.510;Dmax=1.217]
019106 #
019107 # Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
019108 #
019109 R0025: C00059 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019110 ADD HYD + 30.0 02: NS CK 46841.01 68.494 No.date 34.30 27.62 n/a .000
019111 + 30.0 02: SWA2 20.00 7.319 No.date 28.30 39.33 n/a .000
019112 SIM 30.0 01: S_N1A 46841.01 68.494 No.date 34.30 27.62 n/a .000
019113 #
019114 # Sum of hydrographs from Node 5A routed to Node 4
019115 # Section 8
019116 #
019117 R0025: C00060 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019118 ROUTE CHANNEL -> 30.0 02: NS CK 46841.01 68.494 No.date 34.30 27.62 n/a .000
019119 [RDR-30.00] out c 30.0 01: N4 46841.01 65.794 No.date 36.30 27.62 n/a .000
019120 [L/S=4630 / 0471.015]
019121 [Vmax =.838;Dmax=3.516]
019122 #
019123 # Addition of Subwatershed 4 and Learys Creek to Node 4
019124 #
019125 R0025: C00061 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019126 ADD HYD + 30.0 02: NS CK 46841.01 65.794 No.date 36.30 27.62 n/a .000
019127 + 30.0 02: SWA 585.00 10.733 No.date 29.30 39.33 n/a .000
019128 SIM 30.0 01: S_N1 CK 46841.01 65.794 No.date 36.30 27.62 n/a .000
019129 #
019130 R0025: C00062 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019131 SAVE HYD 30.0 01: S_N1 46841.01 65.794 No.date 36.30 27.62 n/a .000
019132 frame -S_N1.0025
019133 remark:flow at S_N1
019134 # Sum of hydrographs from Node 4 routed to Node 2
019135 # Section 9
019136 #
019137 R0025: C00063 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019138 ROUTE CHANNEL -> 30.0 02: NS CK 46841.01 65.794 No.date 36.30 27.62 n/a .000
019139 [RDR-30.00] out c 30.0 01: N2 46841.01 72.927 No.date 35.30 27.99 n/a .000
019140 [L/S=1867 / 0607.040]
019141 [Vmax =.971;Dmax=3.588]
019142 #
019143 # Addition of Subwatershed 2 with Mnohan Drain and Smith Drain to Node 2
019144 # Section 10
019145 #
019146 R0025: C00064 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
019147 ADD HYD + 30.0 02: SW2 177.00 5.280 No.date 28.30 35.63 n/a .000
019148 + 30.0 02: SW2 1122.00 13.030 No.date 31.30 39.33 n/a .000
019149 SIM 30.0 02: NS CK 2737.00 28.975 No.date 31.00 34.91 n/a .000
019150 SIM 30.0 01: S_N1 52483.00 104.643 No.date 33.00 28.62 n/a .000

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020158 R0025: C00065 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
020159 SAVE HYD 30.0 01: S_N2 52483.00 104.643 No.date 33.00 28.62 n/a .000
020160 frame -H_SND
020161 remark:flow at S_N2 Jock River Gauge at Modoc Dr.
020162 #
020163 # Sum of hydrographs from Node 2 routed to Node 1
020164 # Section 10
020165 #
020166 R0025: C00066 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
020167 ROUTE CHANNEL -> 30.0 02: S_N2 52483.00 104.643 No.date 33.00 28.62 n/a .000
020168 [RDR-30.00] out c 30.0 01: N1 52483.00 92.450 No.date 36.00 28.62 n/a .000
020169 [L/S=10046 / 0507.040]
020170 [Vmax =.998;Dmax=3.955]
020171 #
020172 # Addition of Subwatershed 1 to Node 1
020173 #
020174 R0025: C00067 ----- DfIn-ID NND ----- AREAh-QPEAKm-TpeakDate-hh:mm --- RvIm R.C. --- DfWcm
020175 ADD HYD + 30.0 02: NS 52483.00 92.450 No.date 36.00 28.62 n/a .000
020176 SIM 30.0 01: N1 52483.00 31.429 No.date 35.00 36.35 n/a .000
020177 R0025: C00068 ----- SIM 30.0 01: N1 52483.00 115.838 No.date 34.30 29.06 n/a .000
020178 SAVE HYD 30.0 01: N1 52483.00 115.838 No.date 34.30 29.06 n/a .000
020179 frame -N1.0025
020180 remark:total outflow of Jock River
020181 #
020182 #
020183 #
020184 #
020185 #
020186 #
020187 #
020188 #
020189 #
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020244 #

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02993 #
02984 # Sum of hydrographs from Node 6 routed to Node 5
02985 # Section 6
02986 #
02997 R0100: C00056..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
02998 ROUTE CHANNEL > 30.0 02: S_N 40240.01 60.383 No_date 59:30 36.31 n/a .000
02999 [RDF=30.00] out< 30.0 01: NSA 40240.01 60.383 No_date 60:30 36.31 n/a .000
03000 [L/S/n= 1852. / 054/ 035]
03001 [Vmax= .490; Dmax= 1.346]
03002 #
03003 # Addition of Subwatershed 5 and Flowing Creek to Node 5
03004 #
03005 R0100: C00057..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03006 ADD HYD + 30.0 02: S_N 45409.01 79.891 No_date 34:00 37.22 n/a .000
03007 + 30.0 02: SW_S 224.00 9.294 No_date 28:30 47.59 n/a .000
03008 + 30.0 02: FL_CK 4845.00 51.121 No_date 33:00 44.15 n/a .000
03009 SUM 30.0 01: S_N 45409.01 79.891 No_date 34:00 37.22 n/a .000
03010 #
03011 # Sum of hydrographs from Node 5 routed to Node 5A
03012 # Section 7
03013 #
03014 R0100: C00058..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03015 ROUTE CHANNEL > 30.0 02: S_N 45409.01 79.891 No_date 34:00 37.22 n/a .000
03016 [RDF=30.00] out< 30.0 01: NSA 45409.01 79.815 No_date 34:00 37.22 n/a .000
03017 [L/S/n= 556. 090/ 040]
03018 [Vmax= .544; Dmax= 1.346]
03019 #
03020 # Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
03021 #
03022 R0100: C00059..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03023 ADD HYD + 30.0 02: NS_A 45409.01 79.815 No_date 34:00 37.22 n/a .000
03024 + 30.0 02: SW_S 224.00 9.294 No_date 28:30 47.59 n/a .000
03025 + 30.0 02: SW_S 1412.00 9.884 No_date 37:30 48.85 n/a .000
03026 SUM 30.0 01: S_NSA 46841.01 88.619 No_date 34:30 37.48 n/a .000
03027 #
03028 # Sum of hydrographs from Node 5A routed to Node 4
03029 # Section 8
03030 #
03031 R0100: C00060..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03032 ROUTE CHANNEL > 30.0 02: S_NSA 46841.01 88.619 No_date 34:30 37.48 n/a .000
03033 [RDF=30.00] out< 30.0 01: NI 46841.01 84.955 No_date 36:00 37.48 n/a .000
03034 [L/S/n= 4630. / 043/ 051]
03035 [Vmax= .901; Dmax= 3.849]
03036 #
03037 # Addition of Subwatershed 4 and Leamy Creek to Node 4
03038 #
03039 R0100: C00061..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03040 ADD HYD + 30.0 02: NI 46841.01 84.955 No_date 36:00 37.48 n/a .000
03041 + 30.0 02: SW_C 585.00 14.684 No_date 29:30 52.03 n/a .000
03042 + 30.0 02: FL_CK 1021.00 19.515 No_date 30:30 51.13 n/a .000
03043 SUM 30.0 01: S_N 48447.00 95.694 No_date 34:30 37.95 n/a .000
03044 R0100: C00062..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03045 SAVE HYD + 30.0 01: S_N 48447.00 95.694 No_date 34:30 37.95 n/a .000
03046 fname= S_N1.0100
03047 remark: flow at S_N1
03048 #
03049 # Sum of hydrographs from Node 4 routed to Node 2
03050 # Section 9
03051 #
03052 R0100: C00063..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03053 ROUTE CHANNEL > 30.0 02: S_N 48447.00 95.694 No_date 34:30 37.95 n/a .000
03054 [RDF=30.00] out< 30.0 01: NI 48447.00 95.342 No_date 35:00 37.95 n/a .000
03055 [L/S/n= 1667. / 060/ 040]
03056 [Vmax= .942; Dmax= 3.915]
03057 #
03058 # Addition of Subwatershed 2 with Mnohan Drain and Smith Drain to Node 2
03059 #
03060 R0100: C00064..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03061 ADD HYD + 30.0 02: NI 48447.00 95.342 No_date 35:00 37.95 n/a .000
03062 + 30.0 02: SW_C 177.00 7.344 No_date 28:30 47.59 n/a .000
03063 + 30.0 02: MD_DR 1122.00 17.710 No_date 31:30 52.03 n/a .000
03064 + 30.0 02: MD_DR 2737.00 40.026 No_date 31:00 46.72 n/a .000
03065 SUM 30.0 01: S_N 52483.00 141.415 No_date 32:30 38.74 n/a .000
03066 R0100: C00065..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03067 SAVE HYD + 30.0 01: S_N 52483.00 141.415 No_date 32:30 38.74 n/a .000
03068 fname= S_N2.0100
03069 remark: flow at S_N2 Jock River Gauge at Moudie Dr.
03070 #
03071 # Sum of hydrographs from Node 2 routed to Node 1
03072 # Section 10
03073 #
03074 R0100: C00066..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03075 ROUTE CHANNEL > 30.0 02: S_N 52483.00 141.415 No_date 32:30 38.74 n/a .000
03076 [RDF=30.00] out< 30.0 01: NI 52483.00 124.304 No_date 35:00 38.74 n/a .000
03077 [L/S/n=1046. / 050/ 040]
03078 [Vmax= 1.091; Dmax= 4.553]
03079 #
03080 # Addition of Subwatershed 1 to Node 1
03081 #
03082 R0100: C00067..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03083 ADD HYD + 30.0 02: NI 52483.00 124.304 No_date 35:00 38.74 n/a .000
03084 + 30.0 02: SW_C 3176.00 43.079 No_date 32:00 48.46 n/a .000
03085 SUM 30.0 01: NI 55659.00 158.420 No_date 34:00 39.29 n/a .000
03086 R0100: C00068..... Dfm n-1 D NMYD ..... ARE:AbA-QPEAGcm-TpeakDtte_hh:mm..... Rvmm R.C..... DfWcm
03087 SAVE HYD + 30.0 01: NI 55659.00 158.420 No_date 34:00 39.29 n/a .000
03088 fname= NI.0100
03089 remark: initial outflow of Jock River
03090 *****
03091 R0100: C0002.....
03092 FLSN1
03093 *****
03094 *****
03095 *****
03096 *****
03097 R0002: C00015 CONTINUES NASHID
03098 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03099 R0002: C00020 CONTINUES NASHID
03100 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03101 R0002: C00022 CONTINUES NASHID
03102 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03103 R0002: C00026 CONTINUES NASHID
03104 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03105 R0005: C00015 CONTINUES NASHID
03106 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03107 R0005: C00020 CONTINUES NASHID
03108 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03109 R0005: C00022 CONTINUES NASHID
03110 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03111 R0005: C00026 CONTINUES NASHID
03112 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03113 R0010: C00015 CONTINUES NASHID
03114 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03115 R0010: C00020 CONTINUES NASHID
03116 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03117 R0010: C00022 CONTINUES NASHID
03118 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03119 R0010: C00026 CONTINUES NASHID
03120 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03121 R0025: C00015 CONTINUES NASHID
03122 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03123 R0025: C00020 CONTINUES NASHID
03124 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03125 R0025: C00022 CONTINUES NASHID
03126 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03127 R0025: C00026 CONTINUES NASHID
03128 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03129 R0050: C00015 CONTINUES NASHID
03130 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03131 R0050: C00020 CONTINUES NASHID
03132 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03133 R0050: C00022 CONTINUES NASHID
03134 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03135 R0050: C00026 CONTINUES NASHID
03136 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03137 R0100: C00015 CONTINUES NASHID
03138 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03139 R0100: C00020 CONTINUES NASHID
03140 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03141 R0100: C00022 CONTINUES NASHID
03142 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03143 R0100: C00026 CONTINUES NASHID
03144 *** WARN NG: Tim step is too large for value of TP. RV may be ok. Peak flow could be off.
03145 Simulation ended on 2021-02-22 at 15:43:22
03146 *****
03147 *****
03148 *****

```

Attachment B

Model 2 – Jock River Reach One Model

Stantec, 2007

SWMHYMO Input & Summary files

```

1  2  Metric units
2  ****
3  *# Project Name: [Jock River Reach 1 SubWatershed Study]Project #: [160400414]
4  *# Date       : October 2006
5  *# Mdeller    : [Navin Gautam/ Original by Ana M Paerez]
6  *# Company    : Stantec.
7  *# License #  : 3824306
8  ****
9  START          TZERO=[ 0.0],  METOUT=[ 2],  NSTORM=[ 1],  NRUN=[ 002]
10 *%             ["C24SC002.stm"] <--storm filename, one per line for NSTORMtime
11 *#-----|-----|
12 READ STORM     STORM_FILENAME=[ "st or m 001"]
13 *%-----|-----|
14 MODIFY STORM   I CASEms=[ 1],  NSHIFT=[ 96],
15               RedFACT=[ 1],
16 *%-----|-----|
17 DEFAULT VALUES I CASEdv=[ 1],  read and print values
18               DEFVAL_FILENAME=[ "MODIFIED.VAL"]
19 COMPUTE API    API I=[ 50],  API K=[. 85]/day
20 ****
21 *#
22 *#           JOCK RIVER REACH 1 SUBWATERSHED STUDY DISCRETIZED MODEL
23 *#           PROPOSED CONDITIONS DESIGN STORM MODEL (SUMMER)
24 *#
25 *# Version: Draft Final Report, October 2006
26 *# Revision History
27 *# -Draft Interim Condition Report, Nov. 2005
28 ****
29 *# Assumptions
30 *# - All catchments are assumed to be developed except S-1, S-2, and SW1a
31 *# - SWM facilities are modeled
32 *# - Rating curves were estimated based on existing reports and modeling for the
33 *#   proposed SWM facilities
34 *# - The rating curve for the existing Kennedy Burnett SWM Facility was obtained from
35 *#   the Urban Runoff Treatment in the Kennedy Burnett Settling Pond (URTKBP)- Regional
36 *#   Municipality of Ottawa Carleton, March 1983
37 *# - River routing modeled
38 *# - River cross sections obtained from RVCA's HEC-RAS hydraulic model
39 *#-----|-----|
40 *# Parameters
41 *# - Design Storms: 2, 5, 10, 25, 50 & 100yr events: 24hr SCS (DT=10min)-model comparison
42 *# - Impervious area weighted based on: rural subdivision @0% urban @5%
43 *# - NRCS(SCS) CN based on landuse (airphoto) and soil type (base mapping)
44 *# - Time to peak using Uplands Method
45 *#-----|-----|
46 ****
47 *#Read hydrograph upstream of N2 from RVCA Jock R. floodrisk watershed modeling
48 ****
49 READ HYD       ID=[ 1 ],  NHYD=[ "S_N2" ],
50               HYD_FILENAME=[ "H-S_N2" ]
51 *%-----|-----|
52 *#
53 *# Hydrograph from Node 2 routed to Node 416
54 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
55 *#
56 ROUTE CHANNEL  IDout=[ 4],  NHYD=[ "N_416" ] , IDin=[ 1] ,
57               RDT=[ 10] (min),
58               CHLGTH=[ 2327] (m),  CHSLOPE=[ 0.0498] (%),
59               FPSLOPE=[ 0.0498] (%),
60               SECNUM=[ 1.0],  NSEG=[ 3]
61               ( SEGROUGH, SEGDIST (m) )=
62               [ 0.075, -23.96
63               -0.055, 23.96
64               0.075, 157.38]  NSEG times
65               ( DISTANCE (m), ELEVATION (m) )=
66               [-336.97, 93.5]

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67 [- 318. 85, 93]
68 [- 259, 92. 5]
69 [- 133. 18, 92]
70 [- 33. 17, 92]
71 [- 27. 21, 92]
72 [- 26. 14, 91. 5]
73 [- 24. 99, 91]
74 [- 23. 96, 90. 5]
75 [- 14. 33, 88. 26]
76 [- 0. 68, 88. 12]
77 [14. 33, 88. 26]
78 [23. 96, 90. 5]
79 [32. 12, 91]
80 [43. 74, 91. 5]
81 [57. 09, 92]
82 [73. 53, 92. 5]
83 [108. 27, 93]
84 [125. 88, 93. 5]
85 [144. 81, 94]
86 [157. 38, 94. 5]
87 *%-----|-----|
88 *#*****|
89 *# Catchment SW1a
90 *# - Portion of RVCA catchment SW1 outside of Reach 1 subwatershed
91 *# - Undeveloped agricultural land
92 *#*****|
93 CONTINUOUS NASHYD ID=[ 2], NHYD=[ "SW_1a"], DT=[ 5] mi n, AREA=[ 546] ( ha),
94 DWF=[ 0] ( cms), CN C=[ 72], IA=[ 4. 67] ( mm),
95 N=[ 3], TP=[ 2. 79] hr s,
96 Continuous simulation parameters:
97 IaRECper=[ 4] ( hr s),
98 SM N=[ - 1] ( mm), SMAX=[ - 1] ( mm), SK=[ 0. 010] / ( mm),
99 InterEvent Time=[ 12] ( hr s)
100 Baseflow simulation parameters:
101 BaseFlowOption=[ 1],
102 In it GWRes Vol=[ 50] ( mm), GWRes K=[ 0. 96] ( mm/ day/ mm)
103 VHydCond=[ 0. 055] ( mm/ hr), END=- 1
104 *%-----|-----|
105 ADD HYD IDsum=[ 3 ], NHYD=[ "SN_416"], IDsto add=[ 4, 2]
106 *%-----|-----|
107 SAVE HYD ID=[ 3 ], # OF PCYCLES=[ - 1], ICASEsh=[ 1]
108 HYD_COMMENT=[ "Total Flows at Highway 416"]
109 *%-----|-----|
110 *#
111 *# Hydrograph from Node 416 routed to Node at Okeefe drain
112 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245
113 *#
114 ROUTE CHANNEL IDout =[ 1], NHYD=[ "N_OK" ], IDin =[ 3],
115 RDT=[ 5] ( mi n),
116 CHLGTH=[ 497] ( m), CHSLOPE=[ 0. 3018] ( %),
117 FPSLOPE=[ 0. 3018] ( %),
118 SECNUM=[ 1. 0], NSEG=[ 3]
119 ( SEGROUGH, SEGDI ST ( m)) =
120 [ 0. 075, - 19. 40
121 - 0. 055, 19. 40
122 0. 075, 377. 02] NSEG times
123 ( DI STANCE ( m), ELEVATI ON ( m)) =
124 [- 1062. 81, 93. 00]
125 [- 1061. 41, 92. 50]
126 [- 945. 91, 92. 00]
127 [- 783. 64, 91. 50]
128 [- 136. 74, 91. 00]
129 [- 134. 06, 91. 00]
130 [- 128. 97, 91. 00]
131 [- 86. 04, 91. 00]
132 [- 20. 86, 91. 00]

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133      [- 20.18, 90.50]
134      [- 19.40, 90.00]
135      [- 11.68, 86.89]
136      [ 0.00, 86.10]
137      [ 12.09, 86.81]
138      [ 19.40, 90.00]
139      [ 34.68, 90.50]
140      [ 60.56, 91.00]
141      [ 170.14, 91.00]
142      [ 175.05, 90.50]
143      [ 180.29, 90.00]
144      [ 193.41, 90.00]
145      [ 195.98, 90.50]
146      [ 377.02, 92.50]
147  *%-----|-----|
148  *#*****|
149  *#      Catchment OKEEFE
150  *#      - To O Keefe drain (north of the Jock)
151  *#      - Developed with assumed 43% imp.
152  *#*****|
153  CONTINUOUS STANDHYD ID=[ 2], NHYD=["OKEEFE"], DT=[ 5](mi n), AREA=[ 448](ha),
154  XI MP=[ 0.43], TI MP=[ 0.43], DWF=[ 0](cms), LOSS=[ 2],
155  SCS curve number CN=[ 77],
156  Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 0.5](%),
157  LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
158  Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 0.5](%),
159  LGI=[ 1728](m), MNI=[ 0.013], SCI=[ 0](mi n),
160  Continuous simulation parameters:
161  I a RE C per=[ 4](hrs), I a RE C i mp=[ 4](hrs),
162  SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
163  Inter Event Ti me=[ 18](hrs), END=- 1
164
165  *#*****|
166  *#      Okeefe Pond
167  *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
168  *#      and a ratio of the catchment area to the West Clarke pond rating curve
169  *#      from the MS for the next coordinates
170  *#*****|
171  ROUTE RESERVOIR I D out=[ 4], NHYD=["P_OKE"], I D i n=[ 2],
172  RDT=[ 5](mi n),
173  TABLE of ( OUTFLOW STORAGE ) values
174  ( cms ) - ( ha - m)
175  [ 0.0 , 0.0 ]
176  [ 0.20 , 1.72]
177  [ -1 , -1 ] (max twenty pts)
178  I D o v f=[ 9], NHY D o v f=["ok-OVF"]
179
180  *%-----|-----|
181  ADD HYD I D s um=[ 3 ], NHYD=["SN_OK"], I D s t o a dd=[ 1,4,9]
182  *%-----|-----|
183  SAVE HYD I D=[ 3 ], # OF PCYCLES=[ -1], I C A S E s h=[ 1]
184  HYD_COMMENT=["Total Flows at Okeefe Drain"]
185  *%-----|-----|
186  *#
187  *# Hydrograph from Node Okeefe routed to Node at Foster Drain
188  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6215
189  *#
190  ROUTE CHANNEL I D out=[ 1], NHYD=["N_FO"], I D i n=[ 3 ],
191  RDT=[ 5](mi n),
192  CHLGTH=[ 1183](m), CHSLOPE=[ 0.0761](%),
193  FPSLOPE=[ 0.0761](%),
194  SECNUM=[ 1.0], NSEG=[ 3]
195  ( SEGROUGH, SEGDI ST (m))=
196  [ 0.050, -33.89
197  -0.035, 31.59
198  0.050, 854.54] NSEG times

```

```

199      ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
200      [ - 1075. 50 , 93. 00 ]
201      [ - 1070. 59 , 92. 50 ]
202      [ - 1003. 21 , 92. 00 ]
203      [ - 1001. 67 , 92. 00 ]
204      [ - 986. 64 , 92. 00 ]
205      [ - 816. 61 , 91. 50 ]
206      [ - 797. 29 , 91. 00 ]
207      [ - 794. 18 , 91. 00 ]
208      [ - 775. 41 , 91. 50 ]
209      [ - 702. 63 , 91. 50 ]
210      [ - 546. 19 , 91. 50 ]
211      [ - 529. 54 , 91. 50 ]
212      [ - 323. 44 , 91. 00 ]
213      [ - 320. 71 , 91. 00 ]
214      [ - 183. 59 , 91. 00 ]
215      [ - 182. 54 , 90. 50 ]
216      [ - 181. 36 , 90. 00 ]
217      [ - 177. 37 , 90. 00 ]
218      [ - 87. 70 , 90. 00 ]
219      [ - 33. 89 , 90. 00 ]
220      [ - 18. 52 , 86. 88 ]
221      [ 0. 00 , 85. 20 ]
222      [ 16. 20 , 86. 83 ]
223      [ 31. 59 , 90. 00 ]
224      [ 33. 03 , 90. 50 ]
225      [ 34. 41 , 91. 00 ]
226      [ 34. 99 , 91. 00 ]
227      [ 72. 19 , 91. 00 ]
228      [ 208. 76 , 91. 50 ]
229      [ 846. 25 , 92. 00 ]
230      [ 854. 54 , 94. 00 ]
231      *%-----|-----|
232      *#*****|
233      *#      Catchment FOSTER
234      *#      - To Foster ditch (north of the Jock)
235      *#      - Partially developed (medium density); remaining agricultural
236      *#*****|
237      CONTINUOUS STANDHYD ID=[ 2 ] , NHYD=[ "FOSTER" ] , DT=[ 5 ] mi n , AREA=[ 373 ] ( ha ) ,
238      XI MP=[ 0. 55 ] , TI MP=[ 0. 55 ] , DWF=[ 0 ] ( cms ) , LOSS=[ 2 ] ,
239      SCS curve number CN=[ 74 ] ,
240      Per vi ous surfaces: I A per=[ 4. 67 ] ( mm ) , SLPP=[ 0. 5 ] ( % ) ,
241      LGP=[ 40 ] ( m ) , MNP=[ 0. 25 ] , SCP=[ 0 ] ( mi n ) ,
242      Imper vi ous surfaces: I A i mp=[ 1. 57 ] ( mm ) , SLPI=[ 0. 5 ] ( % ) ,
243      LGI=[ 1577 ] ( m ) , MNI=[ 0. 013 ] , SCI=[ 0 ] ( mi n ) ,
244      Continuous simulation parameters:
245      I a REC per=[ 4 ] ( hr s ) , I a REC i mp=[ 4 ] ( hr s ) ,
246      SM N=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
247      Inter Event Ti me=[ 18 ] ( hr s ) , END=- 1
248
249      *#*****|
250      *#      Foster Pond
251      *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
252      *#      and a ratio of the catchment area to the West Clarke pond rating curve
253      *#      from the MS for the next coordinates
254      *#*****|
255      ROUTE RESERVOIR I D out =[ 4 ] , NHYD=[ "P_FOS" ] , I D i n =[ 2 ] ,
256      RDT=[ 5 ] ( mi n ) ,
257      TABLE of ( OUTFLOW STORAGE ) values
258      ( cms ) - ( ha - m )
259      [ 0. 0 , 0. 0 ]
260      [ 0. 20 , 1. 72 ]
261      [ - 1 , - 1 ] ( max twenty pts )
262      I D ovf =[ 9 ] , NHYD ovf =[ "FO- OVF" ]
263      *%-----|-----|
264      ADD HYD I D s um =[ 3 ] , NHYD=[ "SN_FO" ] , I D s to add =[ 1 , 4 , 9 ]

```



```

265  *%-----|-----|
266  SAVE HYD          ID=[ 3 ], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
267                  HYD_COMMENT=["Total Flows at Foster Drain"]
268  *%-----|-----|
269  *#
270  *# Hydrograph from Node Foster routed to Node at Cedarview Road
271  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
272  *#
273  ROUTE CHANNEL    IDout=[ 1], NHYD=["N_CE"], IDin=[ 3],
274                  RDT=[ 5](min),
275                  CHLGTH=[ 159](m), CHSLOPE=[ 0.0818](%),
276                  FPSLOPE=[ 0.0818](%),
277                  SECNUM=[ 1.0], NSEG=[ 3]
278                  ( SEGROUGH, SEGDI ST (m))=
279                    [ 0.050, -15.46
280                    -0.035, 26.55
281                    0.050, 1299.52] NSEG times
282                  ( DI STANCE (m), ELEVATI ON (m))=
283                    [- 891.38, 93.00]
284                    [- 882.49, 93.00]
285                    [- 880.92, 92.50]
286                    [- 879.37, 92.00]
287                    [- 877.72, 91.50]
288                    [- 876.10, 91.00]
289                    [- 873.23, 91.00]
290                    [- 871.82, 91.50]
291                    [- 870.40, 92.00]
292                    [- 803.44, 92.00]
293                    [- 645.23, 91.50]
294                    [- 391.20, 91.50]
295                    [- 91.00, 91.50]
296                    [- 85.52, 91.50]
297                    [- 15.46, 89.40]
298                    [- 9.79, 89.31]
299                    [- 3.22, 86.24]
300                    [ 3.22, 85.07]
301                    [ 10.96, 85.79]
302                    [ 16.44, 86.49]
303                    [ 26.55, 89.45]
304                    [ 29.03, 90.27]
305                    [ 35.76, 90.67]
306                    [ 36.67, 91.00]
307                    [ 108.08, 91.00]
308                    [ 109.82, 90.50]
309                    [ 112.04, 90.50]
310                    [ 114.62, 91.00]
311                    [ 116.76, 91.50]
312                    [ 118.42, 92.00]
313                    [ 449.53, 92.50]
314                    [ 571.98, 92.50]
315                    [ 1093.81, 93.50]
316                    [ 1150.48, 94.00]
317                    [ 1299.52, 95.00]
318  *%-----|-----|
319  *#*****|*****|
320  *# Catchment S-1
321  *# - To Jock River (north and south of Jock)
322  *# - Primarily agricultural fields; portion of sand quarry
323  *#*****|*****|
324  CONTINUOUS NASHYD ID=[ 2], NHYD=["S-1"], DT=[ 5]min, AREA=[ 245](ha),
325                  DWF=[ 0](cms), CNV C=[ 77], IA=[ 4.67](mm),
326                  N=[ 3], TP=[ 1.10]hrs,
327                  Continuous simulation parameters:
328                  IaRECper=[ 4](hrs),
329                  SMN=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0.010]/(mm),
330                  Inter Event Time=[ 12](hrs)

```

```

331 Baseflow simulation parameters:
332 BaseFlowOption=[ 1] ,
333 InitGWResVol=[ 50] ( mm) , GWResK=[ 0.96] ( mm/ day/ mm)
334 VHydCond=[ 0.055] ( mm/ hr) , END=- 1
335
336 *%-----|-----|
337 ADD HYD IDsum=[ 3] , NHYD=[ "SN_CE" ] , IDsto add=[ 1,2]
338 *%-----|-----|
339 SAVE HYD ID=[ 3] , # OF PCYCLES=[ -1] , ICASEsh=[ 1]
340 HYD_COMMENT=[ "Total Flows at Cedarview Road" ]
341 *%-----|-----|
342 *#
343 *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
344 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5002
345 *#
346 ROUTE CHANNEL IDout=[ 1] , NHYD=[ "N_WC" ] , IDin=[ 3] ,
347 RDT=[ 5] ( min) ,
348 CHLGTH=[ 825] ( m) , CHSLOPE=[ 0.01] ( % ) ,
349 FPSLOPE=[ 0.01] ( % ) ,
350 SECNUM=[ 1.0] , NSEG=[ 3]
351 ( SEGROUGH, SEGDI ST ( m) ) =
352 [ 0.050, -37.5
353 -0.035, 37.50
354 0.050, 1367.08] NSEG times
355 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
356 [- 1095.18, 94.00]
357 [- 1091.79, 93.50]
358 [- 1088.95, 93.00]
359 [- 1086.77, 93.00]
360 [- 1069.38, 93.00]
361 [- 1063.14, 93.00]
362 [- 1017.52, 93.00]
363 [- 899.70, 93.00]
364 [- 877.78, 93.00]
365 [- 859.62, 92.50]
366 [- 803.18, 93.00]
367 [- 789.92, 92.00]
368 [- 37.50, 90.00]
369 [- 19.61, 87.04]
370 [ 0.00, 85.70]
371 [ 14.87, 86.93]
372 [ 37.50, 90.00]
373 [ 38.54, 90.50]
374 [ 42.23, 91]
375 [ 157.05, 91.50]
376 [ 161.44, 91.50]
377 [ 236.48, 93.00]
378 [ 385.47, 92.50]
379 [ 390.78, 92.50]
380 [ 863.80, 93.00]
381 [ 866.13, 93.00]
382 [ 990.85, 92.50]
383 [ 991.82, 92.50]
384 [ 993.04, 93.00]
385 [ 994.81, 93.50]
386 [ 1005.36, 93.00]
387 [ 1190.52, 93.00]
388 [ 1267.97, 93.50]
389 [ 1318.99, 94.00]
390 [ 1367.08, 94.50]
391 *%-----|-----|
392 *#*****
393 *# Catchment W_CLAR
394 *# - To West Clarke Drain (south of the Jock)
395 *# - Subdivision with 43% imp. as per Barrhaven South MSS
396 *#*****

```

```

397 CONTINUOUS STANDHYD ID=[ 2], NHYD=["W_CLAR"], DT=[ 5] mi n, AREA=[ 243] (ha),
398 XI MP=[ 0.43], TI MP=[ 0.43], DWF=[ 0] (cms), LOSS=[ 2],
399 SCS curve number CN=[ 75],
400 Pervious surfaces: I A per=[ 4.67] (mm), SLPP=[ 1] (%),
401 LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (mi n),
402 Impervious surfaces: I A i mp=[ 1.57] (mm), SLPI=[ 1] (%),
403 LGI=[ 1273] (m), MNI=[ 0.013], SCI=[ 0] (mi n),
404 Continuous simulation parameters:
405 I a REC per=[ 4] (hr s), I a REC i mp=[ 4] (hr s),
406 SM N=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
407 Inter Event Ti me=[ 18] (hr s), END=- 1
408
409 *%-----|-----|
410 *#*****|
411 *# West Clarke Pond 2
412 *# - Rating curve obtained from Barrhaven South M&S modeling
413 *# - Tributary Drainage Area to M&S Pond 2 = 241 ha
414 *#*****|
415 ROUTE RESERVOIR I D out=[ 8], NHYD=["M&S_P2"], I D i n=[ 2],
416 RDT=[ 5] (mi n),
417 TABLE of ( OUTFLOW STORAGE ) values
418 ( cms ) - ( ha - m)
419 [ 0.0 , 0.0 ]
420 [ 0.11 , 0.96]
421 [ -1 , -1 ] (max twenty pts)
422 I D ovf=[ 9], NHYD ovf=["P2- OVF"]
423 *%-----|-----|
424 ADD HYD I D s um=[ 4 ], NHYD=["SN_WC"], I D s to add=[ 8,9,1]
425 *%-----|-----|
426 SAVE HYD I D=[ 4], # OF PCYCLES=[ -1], I C A S E s h=[ 1]
427 HYD_COMMENT=["Total Flows at West Clarke Pond Outlet"]
428 *%-----|-----|
429 *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
430 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
431 *#
432 ROUTE CHANNEL I D out=[ 1], NHYD=["N_KB"], I D i n=[ 4],
433 RDT=[ 5] (mi n),
434 CHLGTH=[ 1020] (m), CHSLOPE=[ 0.0498] (%),
435 FPSLOPE=[ 0.0498] (%),
436 SECNUM=[ 1.0], NSEG=[ 3]
437 ( SEGROUGH, SEGDI ST (m)) =
438 [ 0.050, -23.63
439 -0.035, 23.63
440 0.050, 728.3] NSEG times
441 ( DI STANCE (m), ELEVATI ON (m)) =
442 [-1082.01, 94]
443 [-1028.17, 92.5]
444 [-992.3, 93.5]
445 [-279.34, 90]
446 [-23.63, 90]
447 [-13.45, 87.13]
448 [-0.07, 86.24]
449 [10.54, 87.15]
450 [23.63, 90]
451 [24.86, 90.5]
452 [26.72, 91]
453 [45.07, 91.5]
454 [128.17, 91.5]
455 [270.7, 92.5]
456 [728.3, 95]
457
458 *%-----|-----|
459 *#*****|
460 *# Catchment KEN_BU
461 *# - To Kennedy-Burnett SWM Facility
462 *# - Outlets to Fraser-Clarke drain (north of the Jock)

```

```

463  *# - Medium density residential subdivision
464  *#*****
465  CONTINUOUS STANDHYD ID=[ 2], NHYD=["KEN_BU"], DT=[ 5] mi n, AREA=[ 281] (ha),
466  XI MP=[ 0.55], TI MP=[ 0.55], DWF=[ 0] (cms), LOSS=[ 2],
467  SCS curve number CN=[ 71],
468  Pervious surfaces: IAper=[ 4.67] (mm), SLPP=[ 1] (%),
469  LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (mi n),
470  Impervious surfaces: IAi mp=[ 1.57] (mm), SLPI =[ 1] (%),
471  LGI =[ 1369] (m), MNI =[ 0.013], SCI =[ 0] (mi n),
472  Continuous simulation parameters:
473  IaRECper=[ 4] (hrs), IaRECI mp=[ 4] (hrs),
474  SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
475  InterEvent Time=[ 18] (hrs), END=- 1
476  *%-----|-----|
477  *#*****
478  *# Existing Kennedy-Burnett SWM Facility
479  *# - Rating curve obtained from URTKBP
480  *# - Tributary Drainage Area to Pond = 160 ha
481  *#*****
482  ROUTE RESERVOIR IDout=[ 5], NHYD=["KEN_P"], IDi n=[ 2],
483  RDT=[ 5] (mi n),
484  TABLE of ( OUTFLOW STORAGE ) values
485  ( cms ) - ( ha-m)
486  [ 0.0 , 0.0 ]
487  [ 0.13 , 0.26]
488  [ 0.43 , 0.56]
489  [ 0.67 , 0.90]
490  [ 0.86 , 1.32]
491  [ 1.01 , 1.79]
492  [ 1.15 , 2.33]
493  [ -1 , -1 ] (max twenty pts)
494  IDovf=[ 6], NHYDovf=["KEN_OV"]
495  *%-----|-----|
496  *#*****
497  *# Catchment FRASER
498  *# - To Fraser-Clarke drain (north of the Jock)
499  *# - Developed land with assumed 43% imp.
500  *#*****
501  CONTINUOUS STANDHYD ID=[ 7], NHYD=["FRASER"], DT=[ 5] mi n, AREA=[ 90] (ha),
502  XI MP=[ 0.25], TI MP=[ 0.25], DWF=[ 0] (cms), LOSS=[ 2],
503  SCS curve number CN=[ 80],
504  Pervious surfaces: IAper=[ 4.67] (mm), SLPP=[ 1] (%),
505  LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (mi n),
506  Impervious surfaces: IAi mp=[ 1.57] (mm), SLPI =[ 1] (%),
507  LGI =[ 775] (m), MNI =[ 0.013], SCI =[ 0] (mi n),
508  Continuous simulation parameters:
509  IaRECper=[ 4] (hrs), IaRECI mp=[ 4] (hrs),
510  SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
511  InterEvent Time=[ 18] (hrs), END=- 1
512
513  *%-----|-----|
514  ROUTE RESERVOIR IDout=[ 8], NHYD=["MS_P2"], IDi n=[ 7],
515  RDT=[ 5] (mi n),
516  TABLE of ( OUTFLOW STORAGE ) values
517  ( cms ) - ( ha-m)
518  [ 0.0 , 0.0 ]
519  [ 0.04 , 0.36]
520  [ -1 , -1 ] (max twenty pts)
521  IDovf=[ 9], NHYDovf=["P2-OVF"]
522  *%-----|-----|
523  ADD HYD IDsum=[ 4 ], NHYD=["SN_KB"], IDst o add=[ 5, 6, 8, 9, 1]
524  *%-----|-----|
525  SAVE HYD ID=[ 4], # OF PCYCLES=[ -1], ICASEsh=[ 1]
526  HYD_COMMENT=["Total Flows at Ken-Burnett Outlet"]
527  *%-----|-----|
528  *# Hydrograph from Node Kennedy - Burnett Drain to Node Todd Drain

```

```

529  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
530  *#
531  ROUTE CHANNEL      I Dout =[ 1], NHYD=[ "N_TO" ], I Di n=[ 4] ,
532                    RDT=[ 5] (mi n),
533                    CHLGTH=[ 650] (m),   CHSLOPE=[ 0. 0498] ( %),
534                    FPSLOPE=[ 0. 0498] ( %),
535                    SECNUM=[ 1. 0],      NSEG=[ 3]
536                    ( SEGROUGH, SEGDI ST ( m) )=
537                    [ 0. 050, - 23. 74
538                    - 0. 035, 23. 74
539                    0. 050, 74. 7] NSEG ti mes
540                    ( DI STANCE ( m), ELEVATI ON ( m) )=
541                    [- 74. 18, 92. 5]
542                    [- 65. 96, 92]
543                    [- 54. 17, 91. 5]
544                    [- 29. 24, 91]
545                    [- 27. 41, 90. 5]
546                    [- 25. 64, 90]
547                    [- 23. 74, 89. 5]
548                    [- 22, 89. 26]
549                    [- 20, 88. 51]
550                    [- 19, 88. 32]
551                    [- 15, 88. 1]
552                    [- 10, 88. 11]
553                    [- 5, 88. 17]
554                    [ 0, 88. 27]
555                    [ 5, 88. 19]
556                    [ 10, 88. 06]
557                    [ 15, 88. 48]
558                    [ 16, 88. 7]
559                    [ 23. 74, 89. 5]
560                    [ 24. 68, 90]
561                    [ 25. 57, 90. 5]
562                    [ 26. 5, 91]
563                    [ 47. 55, 91]
564                    [ 74. 7, 92. 5]
565  *%-----|-----|
566
567  *#*****
568  *#      Catchment TODD
569  *#      - To Todd Drain (south of the Jock)
570  *#      - Subdivision with 43% imp. as per Barrhaven South M&S
571  *#*****
572  CONTINUOUS STANDHYD I D=[ 3], NHYD=[ "TODD" ], DT=[ 5] mi n, AREA=[ 195] (ha),
573                    XI MP=[ 0. 43], TI MP=[ 0. 43], DWF=[ 0] (cms), LOSS=[ 2],
574                    SCS curve number CN=[ 77],
575                    Pervious surfaces: I Aper=[ 4. 67] (mm), SLPP=[ 1] ( %),
576                    LGP=[ 40] (m), MNP=[ 0. 25], SCP=[ 0] (mi n),
577                    Impervious surfaces: I Ai mp=[ 1. 57] (mm), SLPI =[ 1] ( %),
578                    LGI =[ 1140] (m), MNI =[ 0. 013], SCI =[ 0] (mi n),
579                    Continuous simulation parameters:
580                    I aREcper=[ 4] (hr s), I aRECi mp=[ 4] (hr s),
581                    SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
582                    Inter Event Ti me=[ 18] (hr s), END=- 1
583
584  *#*****
585  *#      Todd Pond 3
586  *#      - Rating curve obtained from Barrhaven South M&S modeling
587  *#      - Tributary Drainage Area to M&S Pond 3 = 193 ha
588  *#*****
589  ROUTE RESERVOIR    I Dout =[ 2], NHYD=[ "M&S_P3" ], I Di n=[ 3],
590                    RDT=[ 5] (mi n),
591                    TABLE of ( OUTFLOW STORAGE ) values
592                    ( cms ) - ( ha - m)
593                    [ 0. 0 , 0. 0 ]
594                    [ 0. 08 , 0. 78]

```

```

595                                     [ -1 , -1 ] (max twenty pts)
596                               IDovf=[9], NHYDovf=["P3-OVF"]
597 *%-----|-----|
598 ADD HYD                               IDsum=[10], NHYD=["SN_TO"], IDst o add=[1,2,9]
599 *%-----|-----|
600 SAVE HYD                               ID=[10], # OF PCYCLES=[-1], ICASEsh=[1]
601                               HYD_COMMENT=["Total Flows at Todd Drain"]
602 *%-----|-----|
603 *#
604 *# Hydrograph from Todd Drain routed to Corrigan Drain
605 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
606 *#
607 ROUTE CHANNEL                         IDout=[1], NHYD=["N_TO"], IDin=[10],
608                               RDT=[5] (min),
609                               CHLGTH=[280] (m), CHSLOPE=[0.033] (%),
610                               FPSLOPE=[0.033] (%),
611                               SECNUM=[1.0], NSEG=[3]
612                               ( SEGROUGH, SEGDI ST (m)) =
613                               [0.075, -17.72
614                               -0.045, 17.72
615                               0.075, 80.62] NSEG times
616                               ( DI STANCE (m), ELEVATI ON (m)) =
617                               [-83.32, 90.00]
618                               [-81.36, 89.50]
619                               [-79.12, 89.00]
620                               [-76.13, 88.50]
621                               [-20.46, 88.00]
622                               [-19.36, 87.50]
623                               [-18.51, 87.00]
624                               [-17.72, 86.50]
625                               [-11.95, 85.24]
626                               [-0.11, 85.12]
627                               [11.49, 85.20]
628                               [17.72, 86.50]
629                               [19.74, 87.00]
630                               [21.22, 87.50]
631                               [22.68, 88.00]
632                               [24.28, 88.50]
633                               [26.79, 89.00]
634                               [71.98, 90.00]
635                               [80.62, 90.50]
636 *%-----|-----|
637 *#*****|*****|
638 *# Catchment CORRIG
639 *# - To Corrigan Drain (south of the Jock)
640 *# - Primarily Developed (medium density)
641 *#*****|*****|
642 CONTINUOUS STANDHYD ID=[2], NHYD=["CORRIG"], DT=[5] min, AREA=[149] (ha),
643                               XI MP=[0.45], TI MP=[0.45], DWF=[0] (cms), LOSS=[2],
644                               SCS curve number CN=[77],
645                               Pervious surfaces: IAper=[4.67] (mm), SLPP=[1] (%),
646                               LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
647                               Impervious surfaces: IAimp=[1.57] (mm), SLPI=[1] (%),
648                               LGI=[997] (m), MNI=[0.013], SCI=[0] (min),
649                               Continuous simulation parameters:
650                               IaRECper=[4] (hrs), IaRECI mp=[4] (hrs),
651                               SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010] / (mm),
652                               InterEvent Time=[18] (hrs), END=-1
653
654 *%-----|-----|
655 *#*****|*****|
656 *# Corrigan Pond 1
657 *# - Rating curve obtained from Barrhaven South MSS modeling
658 *# - Tributary Drainage Area to MSS Pond 1 = 145 ha
659 *#*****|*****|
660 ROUTE RESERVOIR IDout=[5], NHYD=["MS_P1"], IDin=[2],

```

```

661 RDT=[ 5] ( mi n),
662 TABLE of ( OUTFLOW STORAGE ) values
663 ( cms ) - ( ha- m)
664 [ 0. 0 , 0. 0 ]
665 [ 0. 06 , 0. 58]
666 [ -1 , -1 ] (max twenty pts)
667 I Dovf=[ 4], NHYDovf=[" P1- OVF" ]
668 *%-----|-----|
669 ADD HYD I Dsum=[ 3 ], NHYD=[" SN_CO" ], I Ds to add=[ 1, 4, 5]
670 *%-----|-----|
671 SAVE HYD I D=[ 3 ], # OF PCYCLES=[ -1], I CASEsh=[ 1]
672 HYD_COMMENT=[" Total Flows at Corrigan Drain" ]
673 *%-----|-----|
674 *#
675 *# Hydrograph from Corrigan Drain routed to Jockvale Road
676 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
677 *#
678 ROUTE CHANNEL I Dout =[ 1], NHYD=[" N_M" ], I Din =[ 3] ,
679 RDT=[ 5] ( mi n),
680 CHLGTH=[ 580] ( m), CHSLOPE=[ 0. 4448] ( %),
681 FPSLOPE=[ 0. 4448] ( %),
682 SECNUM=[ 1. 0], NSEG=[ 3]
683 ( SEGROUGH, SEGDI ST ( m) ) =
684 [ 0. 075, -17. 72
685 -0. 045, 17. 72
686 0. 075, 80. 62] NSEG times
687 ( DI STANCE ( m), ELEVATI ON ( m) ) =
688 [- 83. 32, 90. 00]
689 [- 81. 36, 89. 50]
690 [- 79. 12, 89. 00]
691 [- 76. 13, 88. 50]
692 [- 20. 46, 88. 00]
693 [- 19. 36, 87. 50]
694 [- 18. 51, 87. 00]
695 [- 17. 72, 86. 50]
696 [- 11. 95, 85. 24]
697 [- 0. 11, 85. 12]
698 [ 11. 49, 85. 20]
699 [ 17. 72, 86. 50]
700 [ 19. 74, 87. 00]
701 [ 21. 22, 87. 50]
702 [ 22. 68, 88. 00]
703 [ 24. 28, 88. 50]
704 [ 26. 79, 89. 00]
705 [ 71. 98, 90. 00]
706 [ 80. 62, 90. 50]
707 *%-----|-----|
708 *#*****|-----|
709 *# Catchment MLLS
710 *# - To SWM Facility north of the Jock
711 *# - Primarily residential development
712 *#*****|-----|
713 CONTINUOUS STANDHYD I D=[ 2], NHYD=[" MLLS" ], DT=[ 5] mi n, AREA=[ 139] ( ha),
714 XI MP=[ 0. 38], TI MP=[ 0. 38], DWF=[ 0] ( cms), LOSS=[ 2],
715 SCS curve number CN=[ 74],
716 Pervious surfaces: I Aper=[ 4. 67] ( mm), SLPP=[ 1] ( %),
717 LGP=[ 40] ( m), MNP=[ 0. 25], SCP=[ 0] ( mi n),
718 Impervious surfaces: I Ai mp=[ 1. 57] ( mm), SLPI=[ 1] ( %),
719 LGI=[ 963] ( m), MNI=[ 0. 013], SCI=[ 0] ( mi n),
720 Continuous simulation parameters:
721 I aRECper=[ 4] ( hrs), I aRECI mp=[ 4] ( hrs),
722 SM N=[ -1] ( mm), SMAX=[ -1] ( mm), SK=[ 0. 010] / ( mm),
723 I nter Event Ti me=[ 18] ( hrs), END=- 1
724
725 *%-----|-----|
726 *#*****|-----|

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727  *#      Chapman Mills SWM Pond
728  *#      - Rating curve obtained from CCL hydraulic modeling
729  *#*****
730  ROUTE RESERVOIR      I Dout =[ 5],      NHYD=[ "M LL_P" ],      I Di n=[ 2],
731                      RDT=[ 5] (mi n),
732                      TABLE of ( OUTFLOW STORAGE ) values
733                      ( cms ) - ( ha- m)
734                      [ 0.0 , 0.0 ]
735                      [ 0.01 , 0.01]
736                      [ 0.05 , 0.06]
737                      [ 0.09 , 0.11]
738                      [ 0.13 , 0.15]
739                      [ 0.18 , 0.19]
740                      [ 0.28 , 0.28]
741                      [ 0.37 , 0.34]
742                      [ 0.45 , 0.40]
743                      [ 0.51 , 0.44]
744                      [ 0.56 , 0.47]
745                      [ 0.64 , 0.52]
746                      [ 0.76 , 0.59]
747                      [ 0.86 , 0.65]
748                      [ 1.09 , 0.78]
749                      [ 1.44 , 0.96]
750                      [ 3.18 , 1.84]
751                      [ 4.05 , 2.31]
752                      [ -1 , -1 ] (max twenty pts)
753                      I Dovf =[ 4],      NHYDovf=[ "M L- OV" ]
754  *%-----|-----|
755  ADD HYD              I Dsum=[ 3 ],      NHYD=[ "SN_M" ],      I Ds to add=[ 1,4,5]
756  *%-----|-----|
757  SAVE HYD             I D=[ 3 ],      # OF PCYCLES=[ -1],      I CASEsh=[ 1]
758                      HYD_COMMENT=[ "Tot al Fl ows at Jockvale Road" ]
759  *%-----|-----|
760  *#
761  *# Hydrograph from Jockvale Road routed to Heart's Desire
762  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689
763  *#
764  ROUTE CHANNEL        I Dout =[ 1],      NHYD=[ "N_DE" ] , I Di n=[ 3] ,
765                      RDT=[ 5] (mi n),
766                      CHLGTH=[ 1962] ( m),      CHSLOPE=[ 0.2227] ( %),
767                      FPSLOPE=[ 0.2227] ( %),
768                      SECNUM=[ 1.0],      NSEG=[ 3]
769                      ( SEGROUGH, SEGDI ST ( m) ) =
770                      [ 0.075, -17.56
771                      -0.045, 18.27
772                      0.075, 67.59] NSEG times
773                      ( DI STANCE ( m), ELEVATI ON ( m) ) =
774                      [- 111.59, 88.00]
775                      [- 102.58, 87.50]
776                      [- 96.20, 87.00]
777                      [- 90.04, 86.50]
778                      [- 84.02, 86.00]
779                      [- 77.54, 85.50]
780                      [- 54.07, 85.00]
781                      [- 39.43, 84.50]
782                      [- 28.30, 84.00]
783                      [- 24.12, 83.50]
784                      [- 22.30, 83.00]
785                      [- 20.55, 82.50]
786                      [- 17.56, 82.00]
787                      [- 12.63, 81.22]
788                      [- 0.11, 80.75]
789                      [ 11.55, 81.22]
790                      [ 18.27, 82.00]
791                      [ 19.82, 82.50]
792                      [ 22.48, 83.00]

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793 [27. 90, 83. 50]
794 [29. 31, 84. 00]
795 [30. 81, 84. 50]
796 [32. 51, 85. 00]
797 [34. 24, 85. 50]
798 [36. 34, 86. 00]
799 [41. 65, 86. 50]
800 [62. 64, 87. 00]
801 [65. 14, 87. 50]
802 [67. 59, 88. 00]

803 *%-----|-----|
804 *#*****|

805 *# Catchment DESIRE
806 *# - To Jock River (north of the Jock)
807 *# - Rural-estate subdivision (Heart's Desire Community)
808 *#*****|

809 CONTINUOUS STANDHYD ID=[2], NHYD=["DESIRE"], DT=[5] min, AREA=[24] (ha),
810 XI MP=[0. 25], TI MP=[0. 25], DWF=[0] (cms), LOSS=[2],
811 SCS curve number CN=[77],
812 Pervious surfaces: IAper=[4. 67] (mm), SLPP=[1] (%),
813 LGP=[40] (m), MNP=[0. 25], SCP=[0] (min),
814 Impervious surfaces: IAimp=[1. 57] (mm), SLPI=[1] (%),
815 LGI=[400] (m), MNI=[0. 013], SCI=[0] (min),
816 Continuous simulation parameters:
817 IaRECper=[4] (hrs), IaRECImp=[4] (hrs),
818 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
819 InterEventTime=[18] (hrs), END=- 1

821 *%-----|-----|
822 *#*****|

823 *# Catchment JOCKVA
824 *# - To Jockvale SWM Facility
825 *# - Residential development & golf course
826 *#*****|

827 CONTINUOUS STANDHYD ID=[3], NHYD=["JOCKVA"], DT=[5] min, AREA=[252] (ha),
828 XI MP=[0. 50], TI MP=[0. 50], DWF=[0] (cms), LOSS=[2],
829 SCS curve number CN=[74],
830 Pervious surfaces: IAper=[4. 67] (mm), SLPP=[1] (%),
831 LGP=[40] (m), MNP=[0. 25], SCP=[0] (min),
832 Impervious surfaces: IAimp=[1. 57] (mm), SLPI=[1] (%),
833 LGI=[1296] (m), MNI=[0. 013], SCI=[0] (min),
834 Continuous simulation parameters:
835 IaRECper=[4] (hrs), IaRECImp=[4] (hrs),
836 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
837 InterEventTime=[18] (hrs), END=- 1

839 *%-----|-----|
840 *#*****|

841 *# Jockvale SWM Facility
842 *# - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
843 *#*****|

844 ROUTE RESERVOIR IDout=[5], NHYD=["JOCK_P"], IDin=[3],
845 RDT=[5] (min),
846 TABLE of (OUTFLOW STORAGE) values

847 (cms) - (ha-m)
848 [0. 0 , 0. 0]
849 [0. 27 , 0. 03]
850 [0. 28 , 0. 55]
851 [0. 29 , 1. 14]
852 [0. 30 , 1. 80]
853 [0. 31 , 2. 32]
854 [1. 12 , 2. 87]
855 [2. 92 , 3. 45]
856 [4. 64 , 4. 07]
857 [6. 69 , 4. 72]
858 [9. 02 , 5. 39]

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859             [ 11.62 , 6.10]
860             [ 14.42 , 6.85]
861             [ 17.45 , 7.62]
862             [ 20.69 , 8.44]
863             [ 24.08 , 9.28]
864             [ 27.68 , 10.17]
865             [ -1 , -1 ] (max twenty pts)
866             IDovf=[4], NHYDovf=["JO-OVF"]
867 *%-----|-----|
868 ADD HYD           IDsum=[ 3 ], NHYD=["SN_DE"], IDs to add=[1,2,4,5]
869 *%-----|-----|
870 SAVE HYD         ID=[ 3 ], # OF PCYCLES=[-1], ICASEsh=[1]
871                 HYD_COMMENT=["Total Flows at Heart's Desire"]
872 *%-----|-----|
873 *#
874 *# Hydrograph from Heart's Desire routed to Rideau River
875 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 0
876 *#
877 ROUTE CHANNEL    IDout=[1], NHYD=["NI"], IDin=[3],
878                 RDT=[5](min),
879                 CHLGTH=[563](m), CHSLOPE=[0.9668](%),
880                                     FPSLOPE=[0.9668](%),
881                 SECNUM=[1.0], NSEG=[3]
882                 ( SEGROUGH, SEGDIST (m) )=
883                 [0.075, -30.20
884                 -0.045, 30.20
885                 0.075, 168.81] NSEG times
886                 ( DISTANCE (m), ELEVATION (m) )=
887                 [-170.17, 86.00]
888                 [-164.75, 85.50]
889                 [-158.08, 85.00]
890                 [-113.12, 82.00]
891                 [-98.46, 81.50]
892                 [-92.24, 81.00]
893                 [-86.88, 80.50]
894                 [-81.54, 80.00]
895                 [-74.36, 79.50]
896                 [-63.54, 79.00]
897                 [-39.23, 78.50]
898                 [-34.51, 78.00]
899                 [-33.01, 77.50]
900                 [-30.20, 77.00]
901                 [-13.42, 76.18]
902                 [-1.14, 76.09]
903                 [17.06, 76.18]
904                 [30.20, 77.00]
905                 [32.95, 77.50]
906                 [34.06, 78.00]
907                 [35.11, 78.50]
908                 [36.32, 79.00]
909                 [37.74, 79.50]
910                 [48.48, 81.50]
911                 [49.25, 82.00]
912                 [55.61, 84.50]
913                 [57.09, 85.00]
914                 [59.51, 85.50]
915                 [64.34, 86.00]
916                 [66.30, 86.00]
917                 [76.71, 86.50]
918                 [101.83, 86.50]
919                 [119.73, 87.00]
920                 [142.04, 87.50]
921                 [168.81, 88.00]
922 *%-----|-----|
923 *#*****
924 *# Cat chment S-2

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```

925 *# - To Jock River (north and south)
926 *# - Undeveloped floodplain and river
927 *#*****
928 CONTINUOUS NASHYD ID=[ 2], NHYD=["S-2"], DT=[ 5] min, AREA=[ 102] (ha),
929 DWF=[ 0] (cms), CNVC=[ 72], IA=[ 4.67] (mm),
930 N=[ 3], TP=[ 0.40] hrs,
931 Continuous simulation parameters:
932 IaRECper=[ 4] (hrs),
933 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
934 InterEventTime=[ 12] (hrs)
935 Baseflow simulation parameters:
936 BaseFlowOption=[ 1],
937 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/day/mm)
938 VHydCond=[ 0.055] (mm/hr), END=- 1
939
940 *%-----|-----
941 ADD HYD IDsum=[ 3 ], NHYD=["SN_NI"], IDs to add=[ 1,2]
942 *%-----|-----
943 SAVE HYD ID=[ 3 ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
944 HYD_COMMENT=["Total Flows at Rideau River"]
945 *%-----|-----
946 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
947 START TZERO=[ 0.0], METOUT=[ 2], NSTORM=[ 1], NRUN=[ 005]
948 *% ["C24SC005.stm"] <--storm filename, one per line for NSTORMtime
949 *%-----|-----
950 *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
951 *%START TZERO=[ 0.0], METOUT=[ 2], NSTORM=[ 1], NRUN=[ 010]
952 *% ["C24SC010.stm"] <--storm filename, one per line for NSTORMtime
953 *%-----|-----
954 *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
955 START TZERO=[ 0.0], METOUT=[ 2], NSTORM=[ 1], NRUN=[ 025]
956 *% ["C24SC025.stm"] <--storm filename, one per line for NSTORMtime
957 *%-----|-----
958 *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
959 *%START TZERO=[ 0.0], METOUT=[ 2], NSTORM=[ 1], NRUN=[ 050]
960 *% ["C24SC050.stm"] <--storm filename, one per line for NSTORMtime
961 *%-----|-----
962 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
963 START TZERO=[ 0.0], METOUT=[ 2], NSTORM=[ 1], NRUN=[ 100]
964 *% ["C24SC100.stm"] <--storm filename, one per line for NSTORMtime
965 *%-----|-----
966
967 *#####
968 FINISH
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00749 # 09:04:07 359.57 18.63 N6_date 28:20 34.02 m/a
 00750 [ID# 5.00] SLM# 03:SN OK 53477.01 68.150 N6_date 34:05 18.44 m/a
 00751 [RDE# 5.00] out< 01:SN PO 53477.01 68.150 N6_date 34:05 18.44 m/a
 00752 [L/S# 181.8 / 076.035]
 00753 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 00754 remark:Total Flow at Keefer Drain
 00755 #
 00756 # Hydrograph from Node Keefer routed to Node at Foster Drain
 00757 # Channel X-Section obtained from RCVA Hydraulic Model - Station 6215
 00758 #
 00759 005:0016ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00760 ROUTE CHANNEL -> 03:SN OK 53477.01 68.150 N6_date 34:05 18.44 m/a
 00761 [RDE# 5.00] out< 01:SN PO 53477.01 68.124 N6_date 34:20 18.44 m/a
 00762 [L/S# 181.8 / 076.035]
 00763 [Vmax=1.826 Dmax=.015]
 00764 # Catchment FOSTER
 00765 # - To Foster ditch (north of the Jack)
 00766 # - Partially developed (medium density); remaining agricultural
 00767 #
 00768 005:0017ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00769 CONTINUOUS STANDHDD2:FOSTER 373.00 20.654 N6_date 28:10 37.71.660
 00770 [L/S# 55.71/M# 25.51]
 00771 [L/S# 2 C# 74.0]
 00772 [Pervious area: IPer=4.67;SLP=0.00;LCP= 40.0;MNP=250;SCP= 0]
 00773 [Impervious area: IImp=1.57;SLP=1.00;LGI=140.0;MNI=013;SCL= 0]
 00774 [IARECmp= 4.00; IAREPer= 4.00]
 00775 [SMN= 36.67; SMM=244.49; SK= 010]
 00776 # Foster Pond
 00777 # - Rating curve obtained assuming 40m/ha in 24 hours for quality control
 00778 # and a ratio of the catchment area to the West Clarke pond rating curve
 00779 #
 00780 005:0018ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00781 ROUTE RESERVOIR -> 02:FOSTER 373.00 20.654 N6_date 28:10 37.71 m/a
 00782 [RDE# 5.00] out< 04:FOF 79.61 200.00 N6_date 26:45 37.71 m/a
 00783 [RDE# 5.00] out< 09:FOF 293.39 20.155 N6_date 28:15 37.71 m/a
 00784 [MSL0Lde=1720E+01, TotOfVol=1106E+01, NOf= 2, TotDurOf= 14 hrs]
 00785 005:0019ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00786 ADD HFD 01:N TO 53477.01 68.124 N6_date 34:20 18.44 m/a
 00787 [RDE# 5.00] out< 04:FOF 79.61 200.00 N6_date 26:45 37.71 m/a
 00788 + 09:FOF 293.39 20.155 N6_date 28:15 37.71 m/a
 00789 [ID# 5.00] SLM# 03:SN PO 53477.01 68.977 N6_date 34:20 18.57 m/a
 00790 005:0020ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00791 SAVED HFD 04:SN PO 53850.01 69.977 N6_date 34:20 18.57 m/a
 00792 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_PO.005
 00793 remark:Total Flow at Foster Drain
 00794 # Hydrograph from Node Foster routed to Node at Cedevard Road
 00795 # Channel X-Section obtained from RCVA Hydraulic Model - Station 6016
 00800 #
 00801 005:0021ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00802 CONTINUOUS STANDHDD2:FOSTER 373.00 20.654 N6_date 28:10 37.71 m/a
 00803 [RDE# 5.00] out< 01:SN OK 53850.01 69.977 N6_date 34:20 18.57 m/a
 00804 [L/S# 159.7 / 082.035]
 00805 [Vmax= 3.197 Dmax= 0.124]
 00806 # Catchment S-1
 00807 # - To Jack River (north and south of Jack)
 00808 # - Primarily agricultural fields; portion of sand quarry
 00809 #
 00810 005:0022ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00811 CONTINUOUS STANDHDD2:S-1 245.00 2.163 N6_date 29:10 21.69.380
 00812 [L/S# 77.0 / 38.000]
 00813 [L/S# 10.0 / 5.000]
 00814 [IAREC 4.00; SMM=207.66; SK= 010]
 00815 [InterVenTm= 12.00]
 00816 005:0023ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00817 ADD HFD 01:SN OK 53850.01 69.977 N6_date 34:20 18.57 m/a
 00818 [RDE# 5.00] out< 01:SN OK 53850.01 69.977 N6_date 34:20 18.57 m/a
 00819 [ID# 5.00] SLM# 03:SN OK 54095.01 69.514 N6_date 34:15 18.58 m/a
 00820 005:0024ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00821 SAVED HFD 03:SN PO 54095.01 69.514 N6_date 34:15 18.58 m/a
 00822 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 00823 remark:Total Flow at Cedar
 00824 # Hydrograph from Node Cedevard Road routed to Node at West Clarke Drain
 00825 # Channel X-Section obtained from RCVA Hydraulic Model - Station 5002
 00828 #
 00829 005:0025ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00830 ROUTE CHANNEL -> 03:SN OK 54095.01 69.514 N6_date 34:15 18.58 m/a
 00831 [RDE# 5.00] out< 01:SN OK 54095.01 69.485 N6_date 34:25 18.58 m/a
 00832 [L/S# 825.7 / 010.035]
 00833 [Vmax= 2.82 Dmax= .001]
 00834 # Catchment WCLAR
 00835 # - To West Clarke Drain (south of the Jack)
 00836 # - Subdivision with 43% imp. as per Barhaven South MS
 00837 #
 00838 005:0026ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00839 CONTINUOUS STANDHDD2:WCLAR 243.00 13.629 N6_date 28:05 33.48.386
 00840 [L/S# 43.71/M# 43.71]
 00841 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 00842 [Impervious area: IImp=1.57;SLP=1.00;LGI=127.3;MNI=013;SCL= 0]
 00843 [IARECmp= 4.00; IAREPer= 4.00]
 00844 [SMN= 33.81; SMM=225.45; SK= 010]
 00845 # West Clarke Pond
 00846 # - Rating curve obtained from Barhaven South MS modeling
 00847 # - Tributary Drainage Area to MS Pond = 243 ha
 00848 #
 00849 005:0027ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00850 ROUTE RESERVOIR -> 02:WCLAR 243.00 13.629 N6_date 28:05 33.48 m/a
 00851 [RDE# 5.00] out< 08:MLP 49.32 110.00 N6_date 26:50 33.48 m/a
 00852 [RDE# 5.00] out< 13:FOF 193.68 13.346 N6_date 28:10 33.48 m/a
 00853 [MSL0Lde=9598E+00, TotOfVol=644E+01, NOf= 1, TotDurOf= 14 hrs]
 00854 005:0028ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00855 ADD HFD 01:N TO 54095.01 69.514 N6_date 34:25 18.58 m/a
 00856 [RDE# 5.00] out< 08:MLP 49.32 110.00 N6_date 26:50 33.48 m/a
 00857 + 13:FOF 193.68 13.346 N6_date 28:10 33.48 m/a
 00858 [ID# 5.00] SLM# 04:SN OK 54338.01 70.010 N6_date 34:25 18.65 m/a
 00859 005:0029ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00860 SAVED HFD 04:SN OK 54338.01 70.010 N6_date 34:25 18.65 m/a
 00861 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 00862 remark:Total Flow at West Clarke Pond Outlet
 00863 # Hydrograph from Node West Clarke Pond routed to Node at Kennedy - Burnett Drain
 00864 # Channel X-Section obtained from RCVA Hydraulic Model - Station 4534
 00865 #
 00866 005:0030ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00867 ROUTE CHANNEL -> 04:SN OK 54338.01 70.010 N6_date 34:25 18.65 m/a
 00868 [RDE# 5.00] out< 01:SN OK 54338.01 69.990 N6_date 34:30 18.65 m/a
 00869 [L/S# 1020.7 / 050.035]
 00870 [Vmax= 946 Dmax= 2.601]
 00871 # Catchment KEN-BU
 00872 # - To Kennedy-Burnett SWM Facility
 00873 # - Outlets to Fraser-Clerke Drain (north of the Jack)
 00874 # - Medium density residential
 00875 #
 00876 005:0031ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00877 CONTINUOUS STANDHDD2:KEN-BU 281.00 18.799 N6_date 28:05 37.13.650
 00878 [L/S# 25.71/M# 55.71]
 00879 [L/S# 2 C# 71.0]
 00880 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 00881 [Impervious area: IImp=1.57;SLP=1.00;LGI=169.3;MNI=013;SCL= 0]
 00882 [IARECmp= 4.00; IAREPer= 4.00]
 00883 [SMN= 41.38; SMM=275.84; SK= 010]
 00884 # Existing Kennedy-Burnett SWM Facility
 00885 # - Rating curve obtained from ERREP
 00886 # - Tributary Drainage Area to Pond = 160 ha
 00887 #
 00888 005:0032ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00889 ROUTE RESERVOIR -> 02:KEN-BU 281.00 18.799 N6_date 28:05 37.13 m/a
 00890 [RDE# 5.00] out< 05:KEN-P 180.29 1.150 N6_date 28:05 37.13 m/a
 00891 [RDE# 5.00] out< 06:KEN-O 100.71 17.309 N6_date 28:10 37.13 m/a
 00892 [MSL0Lde=2330E+01, TotOfVol=3739E+01, NOf= 2, TotDurOf= 14 hrs]
 00893 005:0033ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00894 SAVED HFD 03:SN OK 54338.01 70.010 N6_date 34:25 18.65 m/a
 00895 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 00896 remark:Total Flow at Kennedy - Burnett Drain to Node Todd Drain
 00897 # Channel X-Section obtained from RCVA Hydraulic Model - Station 3633
 00898 #
 00899 005:0034ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00900 CONTINUOUS STANDHDD2:FRASER 90.00 4.131 N6_date 28:05 28.74.503
 00901 [L/S# 25.71/M# 25.71]
 00902 [L/S# 2 C# 80.0]
 00903 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 00904 [Impervious area: IImp=1.57;SLP=1.00;LGI=775.3;MNI=013;SCL= 0]
 00905 [IARECmp= 4.00; IAREPer= 4.00]
 00906 [SMN= 26.32; SMM=175.56; SK= 010]
 00907 # Fraser Pond
 00908 # - Rating curve obtained from ERREP
 00909 # - Tributary Drainage Area to Pond = 90 ha
 00910 #
 00911 005:0035ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00912 ROUTE RESERVOIR -> 07:FRASER 90.00 4.131 N6_date 28:05 28.74 m/a
 00913 [RDE# 5.00] out< 08:MLP 20.53 0.040 N6_date 27:45 28.74 m/a
 00914 [RDE# 5.00] out< 09:FOF 69.47 4.131 N6_date 28:05 28.74 m/a
 00915 [MSL0Lde=3600E+00, TotOfVol=1997E+01, NOf= 2, TotDurOf= 13 hrs]
 00916 005:0036ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00917 ADD HFD 01:N TO 54338.01 70.010 N6_date 34:30 18.76 m/a
 00918 [RDE# 5.00] out< 08:MLP 20.53 0.040 N6_date 27:45 28.74 m/a
 00919 + 09:FOF 69.47 4.131 N6_date 28:05 28.74 m/a
 00920 [ID# 5.00] SLM# 04:SN OK 54709.01 71.220 N6_date 34:30 18.76 m/a
 00921 005:0037ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00922 SAVED HFD 04:SN OK 54709.01 71.220 N6_date 34:30 18.76 m/a
 00923 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 00924 remark:Total Flow at Ken-Burnett Outlet
 00925 # Hydrograph from Node Kennedy - Burnett Drain to Node Todd Drain
 00926 # Channel X-Section obtained from RCVA Hydraulic Model - Station 3633
 00927 #
 00928 005:0038ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00929 CONTINUOUS STANDHDD2:FRASER 90.00 4.131 N6_date 28:05 28.74.503
 00930 [L/S# 25.71/M# 25.71]
 00931 [L/S# 2 C# 80.0]
 00932 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 00933 [Impervious area: IImp=1.57;SLP=1.00;LGI=775.3;MNI=013;SCL= 0]
 00934 [IARECmp= 4.00; IAREPer= 4.00]
 00935 [SMN= 26.32; SMM=175.56; SK= 010]
 00936 # Fraser Pond
 00937 # - Rating curve obtained from ERREP
 00938 # - Tributary Drainage Area to Pond = 90 ha
 00939 #
 00940 005:0039ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00941 ROUTE CHANNEL -> 04:SN OK 54709.01 71.220 N6_date 34:30 18.76 m/a
 00942 [RDE# 5.00] out< 01:N TO 54709.01 70.805 N6_date 35:15 18.76 m/a
 00943 [L/S# 650.7 / 050.035]
 00944 [Vmax= 281 Dmax= .999]
 00945 # Catchment TODD
 00946 # - To Todd Drain (south of the Jack)
 00947 # - Subdivision with 43% imp. as per Barhaven South MS
 00948 #
 00949 005:0040ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00950 CONTINUOUS STANDHDD2:TODD 195.00 11.301 N6_date 28:05 34.02.596
 00951 [L/S# 43.71/M# 43.71]
 00952 [L/S# 2 C# 77.0]
 00953 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 00954 [Impervious area: IImp=1.57;SLP=1.00;LGI=140.0;MNI=013;SCL= 0]
 00955 [IARECmp= 4.00; IAREPer= 4.00]
 00956 [SMN= 31.15; SMM=207.66; SK= 010]
 00957 # Todd Pond 3
 00958 # - Rating curve obtained from Barhaven South MS modeling
 00959 # - Tributary Drainage Area to MS Pond 3 = 193 ha
 00960 #
 00961 005:0041ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00962 ROUTE RESERVOIR -> 03:TODD 195.00 11.301 N6_date 28:05 34.02 m/a
 00963 [RDE# 5.00] out< 02:MLP 37.74 0.000 N6_date 26:50 34.02 m/a
 00964 [RDE# 5.00] out< 09:FOF 157.26 11.047 N6_date 28:10 34.02 m/a
 00965 [MSL0Lde=7800E+00, TotOfVol=5350E+01, NOf= 2, TotDurOf= 14 hrs]
 00966 005:0042ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00967 ADD HFD 01:N TO 54709.01 70.805 N6_date 35:15 18.76 m/a
 00968 [RDE# 5.00] out< 02:MLP 37.74 0.000 N6_date 26:50 34.02 m/a
 00969 + 09:FOF 157.26 11.047 N6_date 28:10 34.02 m/a
 00970 [ID# 5.00] SLM# 10:SN TO 54904.01 71.226 N6_date 35:15 18.82 m/a
 00971 005:0043ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00972 SAVED HFD 10:SN TO 54904.01 71.226 N6_date 35:15 18.82 m/a
 00973 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_TO.005
 00974 remark:Total Flow at Todd Drain
 00975 # Hydrograph from Todd Drain routed to Corrigan Drain
 00976 # Channel X-Section obtained from RCVA Hydraulic Model - Station 2462
 00979 #
 00980 005:0044ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00981 ROUTE CHANNEL -> 10:NND TO 54904.01 71.226 N6_date 35:15 18.82 m/a
 00982 [RDE# 5.00] out< 01:N TO 54904.01 71.106 N6_date 35:40 18.82 m/a
 00983 [L/S# 280.7 / 033.045]
 00984 [Vmax= 771 Dmax= 2.533]
 00985 # Catchment CORRIG
 00986 # - To Corrigan Drain (south of the Jack)
 00987 # - Primarily Developed (medium density)
 00988 #
 00989 005:0045ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 00990 CONTINUOUS STANDHDD2:CORRIG 149.00 9.766 N6_date 28:05 34.78.609
 00991 [L/S# 45.71/M# 45.71]
 00992 [L/S# 2 C# 70.0]
 00993 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 00994 [Impervious area: IImp=1.57;SLP=1.00;LGI=99.7;MNI=013;SCL= 0]
 00995 [IARECmp= 4.00; IAREPer= 4.00]
 00996 [SMN= 31.15; SMM=207.66; SK= 010]
 00997 # Corrigan Pond 1
 00998 # - Rating curve obtained from Barhaven South MS modeling
 00999 # - Tributary Drainage Area to MS Pond = 145 ha
 01000 #
 01001 005:0046ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01002 ROUTE RESERVOIR -> 02:CORRIG 149.00 9.766 N6_date 28:05 34.78 m/a
 01003 [RDE# 5.00] out< 05:MLP 27.65 0.000 N6_date 26:30 34.78 m/a
 01004 [RDE# 5.00] out< 04:FOF 121.35 9.684 N6_date 28:05 34.78 m/a
 01005 [MSL0Lde=5799E+00, TotOfVol=4220E+01, NOf= 3, TotDurOf= 14 hrs]
 01006 005:0047ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01007 ADD HFD 01:N TO 54904.01 71.106 N6_date 35:40 18.82 m/a
 01008 [RDE# 5.00] out< 04:FOF 121.35 9.684 N6_date 28:05 34.78 m/a
 01009 + 05:MLP 27.65 0.000 N6_date 26:30 34.78 m/a
 01010 [ID# 5.00] SLM# 03:SN OK 55053.01 71.431 N6_date 35:40 18.86 m/a
 01011 005:0048ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01012 SAVED HFD 03:SN OK 55053.01 71.431 N6_date 35:40 18.86 m/a
 01013 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 01014 remark:Total Flow at Corrigan Drain
 01015 # Hydrograph from Corrigan Drain routed to Jockvale Road
 01016 # Channel X-Section obtained from RCVA Hydraulic Model - Station 2462
 01017 #
 01018 005:0049ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01019 ROUTE CHANNEL -> 03:SN OK 55053.01 71.427 N6_date 35:45 18.86 m/a
 01020 [RDE# 5.00] out< 01:N TO 55053.01 71.427 N6_date 35:45 18.86 m/a
 01021 [L/S# 580.7 / 045.045]
 01022 [Vmax= 1.654 Dmax= 1.485]
 01023 # Catchment MLLS
 01024 # - To SWM Facility north of the Jack
 01025 # - Primarily developed
 01026 #
 01027 005:0050ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01028 CONTINUOUS STANDHDD2:MLLS 139.00 7.873 N6_date 28:05 30.98.242
 01029 [L/S# 38.71/M# 38.71]
 01030 [L/S# 2 C# 74.0]
 01031 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 01032 [Impervious area: IImp=1.57;SLP=1.00;LGI=96.3;MNI=013;SCL= 0]
 01033 [IARECmp= 4.00; IAREPer= 4.00]
 01034 [SMN= 36.67; SMM=244.49; SK= 010]
 01035 # Chapman Mills SWM Pond
 01036 # - Rating curve obtained from CCL hydraulic modeling
 01037 # - Tributary Drainage Area to MS Pond = 139 ha
 01038 #
 01039 005:0051ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01040 ROUTE RESERVOIR -> 02:MLLS 139.00 7.873 N6_date 28:05 30.98 m/a
 01041 [RDE# 5.00] out< 05:MLP 139.00 2.516 N6_date 28:40 30.98 m/a
 01042 [RDE# 5.00] out< 04:FOF 0.000 N6_date 0:00 0.00 m/a
 01043 [MSL0Lde=1500E+01, TotOfVol=0.000E+00, NOf= 0, TotDurOf= 0 hrs]
 01044 005:0052ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01045 ADD HFD 01:N TO 55053.01 71.427 N6_date 35:45 18.86 m/a
 01046 [RDE# 5.00] out< 04:FOF 0.000 N6_date 0:00 0.00 m/a
 01047 + 05:MLP 139.00 2.516 N6_date 28:40 30.98 m/a
 01048 [ID# 5.00] SLM# 03:SN OK 55192.01 71.786 N6_date 35:45 18.89 m/a
 01049 005:0053ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01050 SAVED HFD 03:SN OK 55192.01 71.786 N6_date 35:45 18.89 m/a
 01051 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 01052 remark:Total Flow at Jockvale Road
 01053 # Hydrograph from Jockvale Road routed to Heart's Desire
 01054 # Channel X-Section obtained from RCVA Hydraulic Model - Station 689
 01055 #
 01056 005:0054ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01057 CONTINUOUS STANDHDD2:HEART'S DESIRE 252.00 15.852 N6_date 28:05 35.73.626
 01058 [L/S# 50.71/M# 50.71]
 01059 [L/S# 2 C# 74.0]
 01060 [Pervious area: IPer=4.67;SLP=1.00;LCP= 40.0;MNP=250;SCP= 0]
 01061 [Impervious area: IImp=1.57;SLP=1.00;LGI=1296.3;MNI=013;SCL= 0]
 01062 [IARECmp= 4.00; IAREPer= 4.00]
 01063 [SMN= 36.67; SMM=244.49; SK= 010]
 01064 # Heart's Desire SWM Facility
 01065 # - Rating curve obtained from CCL hydraulic modeling
 01066 # - Tributary Drainage Area to MS Pond = 252 ha
 01067 #
 01068 005:0055ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01069 ROUTE RESERVOIR -> 03:HEART'S DESIRE 252.00 15.852 N6_date 28:05 35.73 m/a
 01070 [RDE# 5.00] out< 05:FOCK 252.00 4.332 N6_date 28:50 35.73 m/a
 01071 [RDE# 5.00] out< 04:FOF 0.000 N6_date 0:00 0.00 m/a
 01072 [MSL0Lde=3962E+01, TotOfVol=0.000E+00, NOf= 0, TotDurOf= 0 hrs]
 01073 005:0056ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01074 ADD HFD 01:N TO 55192.01 71.488 N6_date 36:20 18.89 m/a
 01075 [RDE# 5.00] out< 04:FOF 0.000 N6_date 0:00 0.00 m/a
 01076 + 05:FOCK 252.00 4.332 N6_date 28:50 35.73 m/a
 01077 [ID# 5.00] SLM# 03:SN OK 55468.00 72.143 N6_date 36:15 18.97 m/a
 01078 005:0057ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01079 SAVED HFD 03:SN OK 55468.00 72.143 N6_date 36:15 18.97 m/a
 01080 frame: C:\Navit\OCTOBE-1\CONTIN-1\SMR_POSTH-H-SN_OK.005
 01081 remark:Total Flow at Heart's Desire routed to Riddan River
 01082 # Channel X-Section obtained from RCVA Hydraulic Model - Station 0
 01083 #
 01084 005:0058ID NNDAREAQPEAK TpeakDate:hh:mmR.V.R.C.
 01085 ROUTE CHANNEL -> 03:JOCKVA 252.00 15.852 N6_date 28:0

012125 ADD HYD 01:00 55468.00 72.141 No_date 36:20 18.97 m/a
012126 [DW=5.00] SUM 03:SN SN 55570.00 72.311 No_date 36:20 18.97 m/a
012127 005:0001: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012128 SAVE HYD 03:SN SN 55570.00 72.311 No_date 36:20 18.97 m/a
012129 fname: C:\Navi\OCTOBE-1\CONTIN-1\SMR_POST\H_SN_005
012130 remark:Total Flow at Release Road
012131 ** END OF RUN : 24
012132 *****
012133
012134
012135
012136
012137
012138 RUN COMMAND
012139 025:0001: *****
012140 STAR
012141 [TZERO = 0] hrs on [0]
012142 [METOPT = 2] (Imperial, 2=metric output)
012143 [NTRUNC = 1]
012144 [NSUN = 25]
012145 *****
012146 # Project Name: [Lock River Reach 1 SubWatershed Study]Project #: [16004014]
012147 # Date [October 2006]
012148 # Modeler [Navin Gaur and Original by Anil Pareek]
012149 # Company [Stantec]
012150 # License [3824306]
012151 *****
012152 025:0002: *****
012153 READ STORM
012154 P[Name = storm001]
012155 Comment [P[Inlet SCS of 24 hrs 1:25 ans pour Ottawa CDA]
012156 [SD=10.00:SDR= 24.00:P[OT= 74.39]
012157 *****
012158 025:0003: *****
012159 MDRF STORM
012160 [REACT= 1.0:TSHFT= 960.00 min]
012161 [I[Area = 4.67 m^2:LD=0.00 m^2:MM= 250]
012162 [S[Inlet=10.00:SDR= 40.00:P[OT= 74.39]
012163 *****
012164 025:0004: *****
012165 DEFAULT VALUES
012166 filename: C:\Navi\OCTOBE-1\CONTIN-1\SMR_POST\MODIFIED.VAL
012167 filename: (read and print data)
012168 FileTitle: File comment: [2005 City of Ottawa Sewer Design Guideline]
012169 *****
012170 PARAMETERS USED IN THE TESTON STANDARD COMMAND FOR OTTAWA
012171 Horton's infiltration equation parameters:
012172 [Flow 76.20 mm/hr] [Kp=3.20 mm/hr] [KCN= 4.14 1/hr] [P= .00 mm]
012173 Parameters for PERMOX surfaces in STANMOD:
012174 [I[Area = 4.67 m^2:LD=0.00 m^2:MM= 250]
012175 [I[Area = 1.57 mm^2:LD=1.50] [MM= .013]
012176 Parameters used:
012177 [I[Area = 2.50 mm^2:LD= 3]
012178 *****
012179 COMPUTE API
012180 [API=50.00: APIkey= 8500: APIdate= 9998]
012181 [API=06.00: APIkey= 8517: APIdate= 9998]
012182 *****
012183 # JOCK RIVER REACH 1 SUBWATERSHED STUDY D SCHEMATIC MODEL
012184 # PROPOSED CATCHMENT DESIGN STORM MODEL (SUMMER)
012185 *****
012186 # Version: Draft Final Report, October 2006
012187 # Revision: 1.0
012188 # -Draft Interim Condition Report, Nov, 2005
012189 *****
012190 # Assumptions
012191 # All catchments are assumed to be developed except S-1, S-2, and SW1a
012192 # SMI facilities are modeled
012193 # Rating curves were estimated based on existing reports and modeling for the
012194 # proposed SMI
012195 # The rating curve for the existing Kennedy Burnett SMI facility was obtained
012196 # from the staff Treatment in the Kennedy Burnett Settling Pond (URBPP)
012197 # Municipality of Ottawa Carleton, March 1983
012198 # River routing modeled
012199 # River curve sections obtained from RWCA's HEC-RAS hydraulic model
012200 *****
012201 # Parameters
012202 # Design Storm: 2, 5, 10, 25, 50 & 100 year: 24hr SCS (ID=10min)-model compa
012203 # Imperious area weighted based on rural subdivision @20%, urban @55%
012204 # NCS SCS (C) based on landuse (agricultural and soil type (base mapping))
012205 # Time to peak using Uplands Method
012206 *****
012207 # Head hydrograph upstream of NS from RWCA Lock R. Floodrisk watershed modeling
012208 *****
012209 025:0006: *****
012210 READ HYD 01:5:00 52483.01 104.814 No_date 33:00 28.63 m/a
012211 filename: C:\Navi\OCTOBE-1\CONTIN-1\SMR_POST\H_SN_025
012212 *****
012213 # Hydrograph from Node 2 routed to Node 416
012214 # Channel X Section obtained from RWCA Hydraulic Model - Station 9025
012215 *****
012216 025:0007: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012217 ROUTE CHANNEL -> 01:SN_NK 52483.01 104.814 No_date 33:00 28.63 m/a
012218 [RDF=5.00] out: 01:SN_NK 52483.01 104.143 No_date 33:00 28.63 m/a
012219 [L/S[Area = 497.00] 302.05]
012220 [Vmax = 752.00: Dmax = 3.287]
012221 *****
012222 025:0008: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012223 CONTINUOUS STANDSTOD: SW1a 546.00 5.203 No_date 31:15 30.12 4.05
012224 [Cn= 72.00: Cn= 3.00]
012225 [P= 2.79: DT= 5.00]
012226 [I[Area= 4.00: SMI= 19.75: SMM=264.99: Sks= 010]
012227 [InterEvent Time = 12.00]
012228 *****
012229 025:0009: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012230 ADD HYD 04:SN_NK 52483.01 102.143 No_date 33:50 28.63 m/a
012231 [DW=5.00] SUM 02:SN_NK 546.00 5.203 No_date 31:15 30.12 m/a
012232 03:SN_NK 53029.01 106.064 No_date 33:40 28.65 m/a
012233 *****
012234 025:0010: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012235 SAVE HYD 01:SN_NK 53029.01 106.064 No_date 33:40 28.65 m/a
012236 fname: C:\Navi\OCTOBE-1\CONTIN-1\SMR_POST\H_SN_416.025
012237 remark:Total Flow at Highway 416
012238 *****
012239 025:0011: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012240 Hydrograph from Node 416 routed to Node at Keefer drain
012241 # Channel X Section obtained from RWCA Hydraulic Model - Station 7245
012242 *****
012243 025:0012: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012244 ROUTE CHANNEL -> 03:SN_NK 53029.01 106.064 No_date 33:40 28.65 m/a
012245 [RDF=5.00] out: 01:SN_NK 53029.01 106.047 No_date 33:45 28.65 m/a
012246 [L/S[Area = 497.00] 302.05]
012247 [Vmax = 1.563: Dmax = 2.795]
012248 *****
012249 025:0013: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012250 CATCHMENT FORSTER
012251 # To Keefer drain (north of the Jock)
012252 # Developed with assumed 43% imp
012253 *****
012254 025:0014: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012255 CONTINUOUS STANDSTOD: KEEFE 448.00 29.427 No_date 28:10 48.46 6.51
012256 [Cn= 42.75]
012257 [L[Area = 2.00]
012258 [L[Area = 4.00: I[Area = 4.00]
012259 [L[Area = 4.00: I[Area = 4.00]
012260 [L[Area = 4.00: I[Area = 4.00]
012261 [L[Area = 4.00: I[Area = 4.00]
012262 [L[Area = 4.00: I[Area = 4.00]
012263 *****
012264 025:0015: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012265 # Rating curve obtained assuming 40m/h in 24 hours for quality control
012266 # and a ratio of the catchment area to the West Clarke pond rating curve
012267 # from the MS for the next coordinates
012268 *****
012269 025:0016: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012270 ROUTE RESERVOIR -> 02:OKEEFE 448.00 29.427 No_date 28:10 48.46 m/a
012271 [RDF=5.00] out: 01:SN_NK 53029.01 106.047 No_date 33:45 28.65 m/a
012272 [L/S[Area = 497.00] 302.05]
012273 [Vmax = 1.563: Dmax = 2.795]
012274 *****
012275 025:0017: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012276 ADD HYD 01:SN_NK 53029.01 106.047 No_date 33:45 28.65 m/a
012277 [DW=5.00] SUM 01:SN_NK 53029.01 106.047 No_date 33:45 28.65 m/a
012278 02:SN_NK 53477.01 107.487 No_date 33:45 28.82 m/a
012279 *****
012280 025:0018: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012281 SAVE HYD 01:SN_NK 53477.01 107.487 No_date 33:45 28.82 m/a
012282 fname: C:\Navi\OCTOBE-1\CONTIN-1\SMR_POST\H_SN_025
012283 remark:Total Flow at Keefer Drain
012284 *****
012285 025:0019: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012286 Hydrograph from Node Keefer routed to Node at Foster Drain
012287 # Channel X Section obtained from RWCA Hydraulic Model - Station 6215
012288 *****
012289 025:0020: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012290 ROUTE CHANNEL -> 03:SN_NK 53477.01 107.487 No_date 33:45 28.82 m/a
012291 [RDF=5.00] out: 01:SN_NK 53477.01 107.402 No_date 33:50 28.82 m/a
012292 [L/S[Area = 1183.00] 076.015]
012293 [Vmax = 1.826: Dmax = 3.195]
012294 *****
012295 025:0021: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012296 CATCHMENT FOSTER
012297 # To Foster ditch (north of the Jock)
012298 # Partially developed (medium density) remaining agricultural
012299 *****
012300 025:0022: ID NRD AREA QPEAK TpeakDate:hh:mm R.V.R.C.
012301 CONTINUOUS STANDSTOD: FOSTER 373.00 29.559 No_date 28:10 52.35 7.04
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Attachment C

Model 3 – Jock River Reach One Update

JFSA, 2021

SWMHYMO Input & Summary files

```

1  20    Metric units / ID numbers OFF
2  *****
3  *# SWHYMO Ver: 5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *****
5  *# Project Name: [Jock River]    Project Number: [1474-16]
6  *# Date       : 04-03-2021
7  *# Modeller   : [MM]
8  *# Company    : JFSA Inc.
9  *# License #  : 2549237
10 *****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *# 2020-11-30 change TMSTO in COMPUTE DUALHYD (TMSTO = 0.1 instead of 0.0001)
16 *# 2020-12-01 correct pond curve values
17 *# 2020-12-01 change WCLAR_BRAZ_XIMP to 0.55, SLPI=[0.5](%) (impervious slope), and
18 LGI up to 700m
19 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NHYDout=["N_TO"]
20 ,NHYDin=["SN_TO"]) from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model
21 will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELS
22 aren't well suited to really flat slopes.
23 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 5002 (NHYDout=["N_WC"]
24 ,NHYDin=["SN_CE"]) from 0.01 % (as per Stantec Report 2007) to 0.0255 % so the model
25 will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELS
26 aren't well suited to really flat slopes.
27 *
28 * Calibrated parameters for Summer 2003 data: APII=50, APIK=0.85, CN=varies,
29 *                                               SK=0.01, InterEventTime=12,
30 *                                               GWResk=0.96, VHydCond=0.055
31 *
32 *# -----
33 *
34 *START          TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
35 *              ["XAVG0315.STM"] average storm data a 15 minute time step
36 *              The above rainf file is an average of the JFSA gauge data
37 *              with the City of Ottawa rainfall data collected during
38 *              the same period.
39 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
40 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
41 ["C24SC002.stm"] <-- storm filename, one per line for NSTORM time
42 *%-----|-----|
43 *%-----|-----|
44 READ STORM     STORM_FILENAME=["storm 001"]
45 *%-----|-----|
46 MODIFY STORM   ICASEms=[1], NSHIFT=[96],
47 RedFACT=[1],
48 *%-----|-----|
49 DEFAULT VALUES ICASEdef=[1], read and print values
50 DEFVAL_FILENAME=["CitiGate.DEF"]
51 *%-----|-----|
52 *%-----|-----|
53 COMPUTE API     APII=[50], APIK=[.85]/day
54 *%-----|-----|
55 *%-----|-----|
56 *#
57 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
58 *# of 1.32
59 *%-----|-----|
60 CONTINUOUS NASHYD NHYD=["JR_HW"], DT=[1]min, AREA=[3680](ha),
61 DWF=[0](cms), CNVC=[64], IA=[2.5](mm),
62 N=[3.0], TP=[7.13]hrs,
63 Continuous simulation parameters:
64 IARECper=[4](hrs),
65 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
66 InterEventTime=[12](hrs)
67 Baseflow simulation parameters:

```

```

60 BaseFlowOption=[ 1] ,
61 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
62 VHydCond=[ 0.055](mm/hr), END=- 1
63 *%-----|-----
64 *#
65 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
66 *# of 1.32
67 *%-----|-----
68 CONTINUOUS NASHYD NHYD=[ "SW_13"], DT=[ 1]min, AREA=[ 971](ha),
69 DWF=[ 0](cms), CNVC=[ 61], IA=[ 2.5](mm),
70 N=[ 3.0], TP=[ 3.76]hrs,
71 Continuous simulation parameters:
72 IaRECper=[ 4](hrs),
73 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
74 InterEventTime=[ 12](hrs)
75 Baseflow simulation parameters:
76 BaseFlowOption=[ 1] ,
77 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
78 VHydCond=[ 0.055](mm/hr), END=- 1
79 *%-----|-----
80 *#
81 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
82 *# of 1.80
83 *%-----|-----
84 CONTINUOUS NASHYD NHYD=[ "JR_GWM"], DT=[ 1]min, AREA=[ 3074](ha),
85 DWF=[ 0](cms), CNVC=[ 55], IA=[ 2.5](mm),
86 N=[ 3], TP=[ 11.33]hrs,
87 Continuous simulation parameters:
88 IaRECper=[ 4](hrs),
89 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
90 InterEventTime=[ 12](hrs)
91 Baseflow simulation parameters:
92 BaseFlowOption=[ 1] ,
93 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
94 VHydCond=[ 0.055](mm/hr), END=- 1
95 *%-----|-----
96 CONTINUOUS NASHYD NHYD=[ "JR_ASH"], DT=[ 1]min, AREA=[ 1781](ha),
97 DWF=[ 0](cms), CNVC=[ 72], IA=[ 2.5](mm),
98 N=[ 3.0], TP=[ 3.91]hrs,
99 Continuous simulation parameters:
100 IaRECper=[ 4](hrs),
101 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
102 InterEventTime=[ 12](hrs)
103 Baseflow simulation parameters:
104 BaseFlowOption=[ 1] ,
105 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
106 VHydCond=[ 0.055](mm/hr), END=- 1
107 *%-----|-----
108 CONTINUOUS NASHYD NHYD=[ "SW_11"], DT=[ 1]min, AREA=[ 500](ha),
109 DWF=[ 0](cms), CNVC=[ 66], IA=[ 2.5](mm),
110 N=[ 3.0], TP=[ 1.24]hrs,
111 Continuous simulation parameters:
112 IaRECper=[ 4](hrs),
113 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
114 InterEventTime=[ 12](hrs)
115 Baseflow simulation parameters:
116 BaseFlowOption=[ 1] ,
117 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
118 VHydCond=[ 0.055](mm/hr), END=- 1
119 *%-----|-----
120 *#
121 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
122 *# of 1.80
123 *%-----|-----
124 CONTINUOUS NASHYD NHYD=[ "NN_CK"], DT=[ 1]min, AREA=[ 1917](ha),
125 DWF=[ 0](cms), CNVC=[ 66], IA=[ 2.5](mm),

```

```

126 N=[ 3.0], TP=[ 5.29] hrs,
127 Continuous simulation parameters:
128 IaRECPper=[ 4] (hrs),
129 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
130 InterEventTime=[ 12] (hrs)
131 Baseflow simulation parameters:
132 BaseFlowOption=[ 1] ,
133 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
134 VHydCond=[ 0.055] (mm/ hr), END=- 1
135 *%-----|-----
136 *#
137 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
138 *# of 1.52
139 *%-----|-----
140 CONTINUOUS NASHYD NHYD=[ "SW_10"], DT=[ 1] min, AREA=[ 5666] (ha),
141 DWF=[ 0] (cms), CNVC=[ 72], IA=[ 2.5] (mm),
142 N=[ 3.0], TP=[ 8.00] hrs,
143 Continuous simulation parameters:
144 IaRECPper=[ 4] (hrs),
145 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
146 InterEventTime=[ 12] (hrs)
147 Baseflow simulation parameters:
148 BaseFlowOption=[ 1] ,
149 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
150 VHydCond=[ 0.055] (mm/ hr), END=- 1
151 *%-----|-----
152 *#
153 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
154 *# of 1.75
155 *%-----|-----
156 CONTINUOUS NASHYD NHYD=[ "KG_CK"], DT=[ 1] min, AREA=[ 8376] (ha),
157 DWF=[ 0] (cms), CNVC=[ 66], IA=[ 2.5] (mm),
158 N=[ 3.0], TP=[ 11.66] hrs,
159 Continuous simulation parameters:
160 IaRECPper=[ 4] (hrs),
161 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
162 InterEventTime=[ 12] (hrs)
163 Baseflow simulation parameters:
164 BaseFlowOption=[ 1] ,
165 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
166 VHydCond=[ 0.055] (mm/ hr), END=- 1
167 *%-----|-----
168 *#
169 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
170 *# of 1.68
171 *%-----|-----
172 CONTINUOUS NASHYD NHYD=[ "SW_9"], DT=[ 1] min, AREA=[ 1132] (ha),
173 DWF=[ 0] (cms), CNVC=[ 70], IA=[ 2.5] (mm),
174 N=[ 3.0], TP=[ 2.51] hrs,
175 Continuous simulation parameters:
176 IaRECPper=[ 4] (hrs),
177 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
178 InterEventTime=[ 12] (hrs)
179 Baseflow simulation parameters:
180 BaseFlowOption=[ 1] ,
181 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
182 VHydCond=[ 0.055] (mm/ hr), END=- 1
183 *%-----|-----
184 *#
185 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
186 *# of 1.82
187 *%-----|-----
188 CONTINUOUS NASHYD NHYD=[ "NC_CK"], DT=[ 1] min, AREA=[ 4464] (ha),
189 DWF=[ 0] (cms), CNVC=[ 62], IA=[ 2.5] (mm),
190 N=[ 3.0], TP=[ 11.32] hrs,
191 Continuous simulation parameters:

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```

192 IaREcper=[ 4] (hr s),
193 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
194 InterEventTime=[ 12] (hr s)
195 Baseflow simulation parameters:
196 BaseFlowOption=[ 1] ,
197 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
198 VHydCond=[ 0. 055] (mm/ hr), END=- 1
199 *%-----|-----
200 *#
201 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
202 *# of 1.80
203 *%-----|-----
204 CONTINUOUS NASHYD NHYD=[ "SW_8" ], DT=[ 1] mi n, AREA=[ 131] (ha),
205 DWF=[ 0] (cms), CN C=[ 63], IA=[ 2. 5] (mm),
206 N=[ 3. 0], TP=[ 0. 90] hr s,
207 Continuous simulation parameters:
208 IaREcper=[ 4] (hr s),
209 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
210 InterEventTime=[ 12] (hr s)
211 Baseflow simulation parameters:
212 BaseFlowOption=[ 1] ,
213 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
214 VHydCond=[ 0. 055] (mm/ hr), END=- 1
215 *%-----|-----
216 *#
217 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
218 *# of 1.65
219 *%-----|-----
220 CONTINUOUS NASHYD NHYD=[ "HB_DR" ], DT=[ 1] mi n, AREA=[ 3854] (ha),
221 DWF=[ 0] (cms), CN C=[ 66], IA=[ 2. 5] (mm),
222 N=[ 3. 0], TP=[ 8. 42] hr s,
223 Continuous simulation parameters:
224 IaREcper=[ 4] (hr s),
225 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
226 InterEventTime=[ 12] (hr s)
227 Baseflow simulation parameters:
228 BaseFlowOption=[ 1] ,
229 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
230 VHydCond=[ 0. 055] (mm/ hr), END=- 1
231 *%-----|-----
232 *#
233 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
234 *# of 1.82
235 *%-----|-----
236 CONTINUOUS NASHYD NHYD=[ "SW_7" ], DT=[ 1] mi n, AREA=[ 3197] (ha),
237 DWF=[ 0] (cms), CN C=[ 57], IA=[ 2. 5] (mm),
238 N=[ 3. 0], TP=[ 6. 65] hr s,
239 Continuous simulation parameters:
240 IaREcper=[ 4] (hr s),
241 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
242 InterEventTime=[ 12] (hr s)
243 Baseflow simulation parameters:
244 BaseFlowOption=[ 1] ,
245 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
246 VHydCond=[ 0. 055] (mm/ hr), END=- 1
247 *%-----|-----
248 *#
249 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
250 *# of 1.75
251 *%-----|-----
252 CONTINUOUS NASHYD NHYD=[ "SW_6" ], DT=[ 1] mi n, AREA=[ 165] (ha),
253 DWF=[ 0] (cms), CN C=[ 67], IA=[ 2. 5] (mm),
254 N=[ 3. 0], TP=[ 4. 18] hr s,
255 Continuous simulation parameters:
256 IaREcper=[ 4] (hr s),
257 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),

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258 InterEventTime=[ 12] (hrs)
259 Baseflow simulation parameters:
260 BaseFlowOption=[ 1] ,
261 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
262 VHydCond=[ 0.055] (mm/ hr), END=- 1
263 *%-----|-----
264 *#
265 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
266 *# of 1.67
267 *%-----|-----
268 CONTINUOUS NASHYD NHYD=[ "VG_DR" ], DT=[ 1] min, AREA=[ 1332] (ha),
269 DWF=[ 0] (cms), CNVC=[ 72], IA=[ 2.5] (mm),
270 N=[ 3.0], TP=[ 5.95] hrs,
271 Continuous simulation parameters:
272 IaRECper=[ 4] (hrs),
273 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
274 InterEventTime=[ 12] (hrs)
275 Baseflow simulation parameters:
276 BaseFlowOption=[ 1] ,
277 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
278 VHydCond=[ 0.055] (mm/ hr), END=- 1
279 *%-----|-----
280 CONTINUOUS NASHYD NHYD=[ "SW_5" ], DT=[ 1] min, AREA=[ 224] (ha),
281 DWF=[ 0] (cms), CNVC=[ 77], IA=[ 2.5] (mm),
282 N=[ 3.0], TP=[ 0.75] hrs,
283 Continuous simulation parameters:
284 IaRECper=[ 4] (hrs),
285 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
286 InterEventTime=[ 12] (hrs)
287 Baseflow simulation parameters:
288 BaseFlowOption=[ 1] ,
289 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
290 VHydCond=[ 0.055] (mm/ hr), END=- 1
291 *%-----|-----
292 *#
293 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
294 *# of 1.20
295 *%-----|-----
296 CONTINUOUS NASHYD NHYD=[ "FL_CK" ], DT=[ 1] min, AREA=[ 4945] (ha),
297 DWF=[ 0] (cms), CNVC=[ 74], IA=[ 2.5] (mm),
298 N=[ 3.0], TP=[ 4.45] hrs,
299 Continuous simulation parameters:
300 IaRECper=[ 4] (hrs),
301 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
302 InterEventTime=[ 12] (hrs)
303 Baseflow simulation parameters:
304 BaseFlowOption=[ 1] ,
305 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
306 VHydCond=[ 0.055] (mm/ hr), END=- 1
307 *%-----|-----
308 CONTINUOUS NASHYD NHYD=[ "SW_5A2" ], DT=[ 1] min, AREA=[ 20] (ha),
309 DWF=[ 0] (cms), CNVC=[ 81], IA=[ 2.5] (mm),
310 N=[ 3.0], TP=[ 0.62] hrs,
311 Continuous simulation parameters:
312 IaRECper=[ 4] (hrs),
313 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
314 InterEventTime=[ 12] (hrs)
315 Baseflow simulation parameters:
316 BaseFlowOption=[ 1] ,
317 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
318 VHydCond=[ 0.055] (mm/ hr), END=- 1
319 *%-----|-----
320 *#
321 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
322 *# of 1.61
323 *%-----|-----

```

324 CONTI NUOUS NASHYD NYHD=[" SW_5A1"], DT=[1] mi n, AREA=[1412] (ha),
325 DWF=[0] (cms), CNV C=[75], IA=[2. 5] (mm),
326 N=[3. 0], TP=[8. 00] hr s,
327 Continuous simulation parameters:
328 IaRECPper=[4] (hr s),
329 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
330 Inter Event Time=[12] (hr s)
331 Baseflow simulation parameters:
332 BaseFlowOpt ion=[1] ,
333 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
334 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

336 CONTI NUOUS NASHYD NYHD=[" SW_4"], DT=[1] mi n, AREA=[585] (ha),
337 DWF=[0] (cms), CNV C=[81], IA=[2. 5] (mm),
338 N=[3. 0], TP=[1. 75] hr s,
339 Continuous simulation parameters:
340 IaRECPper=[4] (hr s),
341 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
342 Inter Event Time=[12] (hr s)
343 Baseflow simulation parameters:
344 BaseFlowOpt ion=[1] ,
345 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
346 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

348 CONTI NUOUS NASHYD NYHD=[" LM_CK"], DT=[1] mi n, AREA=[1021] (ha),
349 DWF=[0] (cms), CNV C=[80], IA=[2. 5] (mm),
350 N=[3. 0], TP=[2. 46] hr s,
351 Continuous simulation parameters:
352 IaRECPper=[4] (hr s),
353 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
354 Inter Event Time=[12] (hr s)
355 Baseflow simulation parameters:
356 BaseFlowOpt ion=[1] ,
357 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
358 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

360 CONTI NUOUS NASHYD NYHD=[" SW_2"], DT=[1] mi n, AREA=[177] (ha),
361 DWF=[0] (cms), CNV C=[77], IA=[2. 5] (mm),
362 N=[3. 0], TP=[0. 75] hr s,
363 Continuous simulation parameters:
364 IaRECPper=[4] (hr s),
365 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
366 Inter Event Time=[12] (hr s)
367 Baseflow simulation parameters:
368 BaseFlowOpt ion=[1] ,
369 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
370 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

372 CONTI NUOUS NASHYD NYHD=[" SM_DR"], DT=[1] mi n, AREA=[1122] (ha),
373 DWF=[0] (cms), CNV C=[81], IA=[2. 5] (mm),
374 N=[3. 0], TP=[3. 25] hr s,
375 Continuous simulation parameters:
376 IaRECPper=[4] (hr s),
377 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
378 Inter Event Time=[12] (hr s)
379 Baseflow simulation parameters:
380 BaseFlowOpt ion=[1] ,
381 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
382 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

384 CONTI NUOUS NASHYD NYHD=[" MO_DR"], DT=[1] mi n, AREA=[2737] (ha),
385 DWF=[0] (cms), CNV C=[76], IA=[2. 5] (mm),
386 N=[3. 0], TP=[3. 03] hr s,
387 Continuous simulation parameters:
388 IaRECPper=[4] (hr s),
389 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),


```

390 InterEventTime=[ 12]( hrs)
391 Baseflow simulation parameters:
392 BaseFlowOption=[ 1] ,
393 InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
394 VHydCond=[ 0.055]( mm/ hr) , END=- 1
395 *%-----|-----|
396 * -JFSA 2020 replaced SW_1 with a detailed model from Stantec Report 2007
397 *CONTINUOUS NASHYD NHYD=["SW_1"], DT=[ 1]mi n, AREA=[ 3176]( ha),
398 * DWF=[ 0]( cms) , CN/C=[ 78] , IA=[ 2.5]( mm),
399 * N=[ 3.0] , TP=[ 3.56]hrs,
400 * Continuous simulation parameters:
401 * IaRECper=[ 4]( hrs),
402 * SMN=[ -1]( mm) , SMAX=[ -1]( mm) , SK=[ 0.010]/( mm),
403 * InterEventTime=[ 12]( hrs)
404 * Baseflow simulation parameters:
405 * BaseFlowOption=[ 1] ,
406 * InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
407 * VHydCond=[ 0.055]( mm/ hr) , END=- 1
408 *%-----|-----|
409 *#
410 *# Routing hydrographs
411 *#
412 *# Starting with the addition of Jock River Headwater and Subwatershed 13
413 *#
414 ADD HYD NHYDsum=["S_N13"], NHYDs to add=["JR_HW"+"SW_13"]
415 *%-----|-----|
416 *#
417 *# Sum of hydrographs from Node 13 routed to Node 13A
418 *# (Approximated cross-section - see cross-section 258)
419 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
420 *#
421 ROUTE CHANNEL NHYDout=["N13A"] , NHYDin=["S_N13"],
422 RDT=[ 1]( mi n),
423 CHLGTH=[ 9074]( m) , CHSLOPE=[ 0.0220]( %),
424 FPSLOPE=[ 0.0220]( %),
425 SECNUM=[ 1.0] , NSEG=[ 1]
426 ( SEGROUGH, SEGDI ST ( m))=[ 0.04, 15.5] NSEG times
427 ( DI STANCE ( m) , ELEVATI ON ( m))=
428 [- 40, 132.5]
429 [- 30, 132]
430 [- 25, 131.5]
431 [- 13, 130]
432 [- 8, 127.00]
433 [- 7, 126.50]
434 [- 6, 126]
435 [- 5.5, 125.50]
436 [0, 123.75]
437 [4.5, 125.50]
438 [6, 126]
439 [7.5, 126.5]
440 [9, 127]
441 [10, 127.5]
442 [11.5, 128.0]
443 [15.5, 129.5]
444 *%-----|-----|
445 *#
446 *# Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
447 *#
448 ADD HYD NHYDsum=["SN13A"], NHYDs to add=["N13A"+"JR_GWM"]
449 *%-----|-----|
450 *#
451 *# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
452 *#
453 ROUTE RESERVOIR NHYDout=["RES_GM"] , NHYDin=["SN13A"],
454 RDT=[ 1]( mi n),
455 TABLE of ( OUTFLOW STORAGE ) values

```

```

456 (cms) - (ha-m)
457 [ 0.0 , 0.0 ]
458 [ 1.991, 2.144 ]
459 [ 2.693, 39.826 ]
460 [ 3.509, 81.697 ]
461 [ 4.578, 318.774 ]
462 [ 5.647, 594.947 ]
463 [ 7.109, 910.219 ]
464 [ 8.616, 1264.589 ]
465 [ 10.371, 1658.057 ]
466 [ 12.402, 2090.622 ]
467 [ 22.056, 3462.487 ]
468 [ -1 , -1 ] (max twenty pts)
469
470 NHYDovf=[ " " ] ,
471 *%-----|-----|
472 *#
473 SAVE HYD NHYD=[ "RES_GM" ], # OF PCYCLES=[ -1 ], I CASEs h=[ -1 ]
474 HYD_FILE NAME=[ "H_RES GM" ]
475 HYD_COMMENT=[ "Out flow from Res GM" ]
476 *%-----|-----|
477 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
478 *# (Approximated cross-section - see cross-section 258)
479 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
480 ROUTE CHANNEL NHYDout=[ "N12" ] , NHYDin=[ "RES_GM" ] ,
481 RDT=[ 1 ] (min) ,
482 CHLGTH=[ 5926 ] (m) , CHSLOPE=[ 0.0759 ] ( % ) ,
483 FPSLOPE=[ 0.0759 ] ( % ) ,
484 SECNUM=[ 1.0 ] , NSEG=[ 1 ]
485 ( SEGROUGH, SEGDIST (m) )=[ 0.04, 15.5 ] NSEG times
486 ( DISTANCE (m) , ELEVATION (m) )=
487 [- 40, 132.5]
488 [- 30, 132]
489 [- 25, 131.5]
490 [- 13, 130]
491 [- 8, 127.00]
492 [- 7, 126.50]
493 [- 6, 126]
494 [- 5.5, 125.50]
495 [ 0, 123.75]
496 [ 4.5, 125.50]
497 [ 6, 126]
498 [ 7.5, 126.5]
499 [ 9, 127]
500 [ 10, 127.5]
501 [ 11.5, 128.00]
502 [ 15.5, 129.5]
503 *%-----|-----|
504 *#
505 *# Addition of Subwatershed Jock River at Ashton to Node 12
506 *#
507 ADD HYD NHYDsum=[ "S_N12" ] , NHYDsto add=[ "N12"+"JR_ASH" ]
508 SAVE HYD NHYD=[ "S_N12" ] , # OF PCYCLES=[ -1 ], I CASEs h=[ -1 ]
509 HYD_FILE NAME=[ "H_SN12" ]
510 HYD_COMMENT=[ "flow at S_N12 near Ashton" ]
511 *%-----|-----|
512 *#
513 *# Sum of hydrographs from Node 12 routed to Node 11
514 *# (Approximated cross-section - see cross-section 258)
515 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
516 *ROUTE CHANNEL NHYDout=[ "N11" ] , NHYDin=[ "S_N12" ] ,
517 * RDT=[ 1 ] (min) ,
518 * CHLGTH=[ 972 ] (m) , CHSLOPE=[ 0.0514 ] ( % ) ,
519 * FPSLOPE=[ 0.0514 ] ( % ) ,
520 * SECNUM=[ 1.0 ] , NSEG=[ 1 ]
521 * ( SEGROUGH, SEGDIST (m) )=[ 0.04, 15.5 ] NSEG times
522 * ( DISTANCE (m) , ELEVATION (m) )=

```

```

522 * [- 40, 132.5]
523 * [- 30, 132]
524 * [- 25, 131.5]
525 * [- 13, 130]
526 * [- 8, 127.00]
527 * [- 7, 126.50]
528 * [- 6, 126]
529 * [- 5.5, 125.50]
530 * [0, 123.75]
531 * [4.5, 125.50]
532 * [6, 126]
533 * [7.5, 126.5]
534 * [9, 127]
535 * [10, 127.5]
536 * [11.5, 128.00]
537 * [15.5, 129.5]
538 *%-----|-----
539 *#
540 *# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
541 *#
542 ROUTE CHANNEL NHYDout=["Duml1"], NHYDin=["S_N12"],
543 RDT=[1](min),
544 CHLGTH=[972](m), CHSLOPE=[0.054](%),
545 FPSLOPE=[0.054](%),
546 SECNUM=[1.0], NSEG=[1]
547 ( SEGROUGH, SEGDIST (m))=[0.04, 15.5] NSEG times
548 ( DISTANCE (m), ELEVATION (m))=
549 [- 40, 132.5]
550 [- 30, 132]
551 [- 25, 131.5]
552 [- 13, 130]
553 [- 8, 127.00]
554 [- 7, 126.50]
555 [- 6, 126]
556 [- 5.5, 125.50]
557 [0, 123.75]
558 [4.5, 125.50]
559 [6, 126]
560 [7.5, 126.5]
561 [9, 127]
562 [10, 127.5]
563 [11.5, 128.00]
564 [15.5, 129.5]
565 *%-----|-----
566 *#
567 *# Addition of Subwatershed 11 and No Name Creek to Node 11
568 *#
569 ADD HYD NHYDsum=["S_N11"], NHYDstoadd=["Duml1"+"SW_11"+"NN_CK"]
570 *%-----|-----
571 *#
572 *# Sum of hydrographs from Node 11 routed to Node 10
573 *# Section 1
574 *#
575 ROUTE CHANNEL NHYDout=["N10"], NHYDin=["S_N11"],
576 RDT=[1](min),
577 CHLGTH=[14028](m), CHSLOPE=[0.1568](%),
578 FPSLOPE=[0.1568](%),
579 SECNUM=[1.0], NSEG=[5]
580 ( SEGROUGH, SEGDIST (m))=
581 [0.04, -52.82
582 0.1, -6.47
583 -0.05, 6.47
584 0.1, 45.36
585 0.04, 423.88] NSEG times
586 ( DISTANCE (m), ELEVATION (m))=
587 [- 226.24 , 112.50]

```

```

588             [- 167.50 , 111.50]
589             [- 106.81 , 111.00]
590             [- 92.37 , 110.00]
591             [- 52.82 , 109.00]
592             [- 24.90, 109.00]
593             [- 17.02, 108.50]
594             [- 6.47, 108.00]
595             [ 6.47, 108.00]
596             [ 15.67, 108.50]
597             [ 18.95, 109.00]
598             [ 45.36, 109.50]
599             [ 120.79, 110.00]
600             [ 145.72, 111.00]
601             [ 181.56, 111.50]
602             [ 423.88, 112.50]
603 *%-----|-----
604 *#
605 *# Addition of Subwatershed 10 to Node 10
606 *#
607 ADD HYD          NHYDs um=["S_N10"], NHYDs to add=["N10"+"SW_10"]
608 *%-----|-----
609 SAVE HYD         NHYD=["S_N10"], # OF PCYCLES=[- 1], I CASEs h=[- 1]
610                 HYD_FILENAME=["H_SN10"]
611                 HYD_COMMENT=["flow at S_N10: N10 + SW_10"]
612 *%-----|-----
613 *# Addition of Kings Creek to S_N10
614 *#
615 ADD HYD          NHYDs um=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
616 *%-----|-----
617 *#
618 *# Sum of hydrographs from Node 10 routed to Node 9
619 *# Section 2
620 *#
621 ROUTE CHANNEL    NHYDout=["N9"] , NHYDin=["S_N10A"] ,
622                 RDT=[ 1] ( min),
623                 CHLGTH=[ 3982] ( m), CHSLOPE=[ 0.0753] ( %),
624                                     FPSLOPE=[ 0.0753] ( %),
625                 SECNUM=[ 1.0], NSEG=[ 4]
626                 ( SEGROUGH, SEGDIST ( m) ) =
627                 [ 0.04, - 30.27
628                 0.05, - 18.42
629                 - 0.05, 18.42
630                 0.04, 131.58] NSEG times
631                 ( DISTANCE ( m), ELEVATION ( m) ) =
632                 [- 446.74, 106.00]
633                 [- 415.68, 105.50]
634                 [- 285.40, 105.00]
635                 [- 173.77, 104.50]
636                 [- 144.95, 104.00]
637                 [- 111.18, 103.50]
638                 [- 94.06, 103.00]
639                 [- 71.02, 102.50]
640                 [- 30.27, 102.00]
641                 [- 19.33, 100.00]
642                 [- 18.42, 99.50]
643                 [ 18.42, 99.50]
644                 [ 20.77, 100.00]
645                 [ 27.93, 101.00]
646                 [ 52.29, 101.00]
647                 [ 68.80, 101.50]
648                 [ 79.66, 103.00]
649                 [ 91.50, 103.50]
650                 [ 131.58, 104.00]
651 *%-----|-----
652 *#
653 *# Addition of Subwatershed 9 and Nichols Creek to Node 9

```

```

654  *#
655  ADD HYD          NHYDs um=[ "S_N9" ], NHYDs to add=[ "N9"+"SW_9"+"NC_CK" ]
656  *%-----|-----|
657  *#
658  *# Sum of hydrographs from Node 9 routed to Node 8
659  *# Section 3
660  *#
661  ROUTE CHANNEL    NHYDout =[ "N8" ] , NHYDin =[ "S_N9" ] ,
662                  RDT=[ 1 ] ( mi n ) ,
663                  CHLGTH=[ 2269 ] ( m ) ,   CHSLOPE=[ 0.0882 ] ( % ) ,
664                                                    FPSLOPE=[ 0.0882 ] ( % ) ,
665                  SECNUM=[ 1.0 ] ,          NSEG=[ 3 ]
666                  ( SEGROUGH, SEGDI ST ( m ) ) =
667                    [ 0.1, -17.99
668                      -0.045, 17.31
669                      0.1, 456.58 ] NSEG t i m e s
670                  ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
671                    [ -201.19, 100.50 ]
672                    [ -135.21, 100.00 ]
673                    [ -94.83, 99.50 ]
674                    [ -67.05, 99.00 ]
675                    [ -17.99, 98.50 ]
676                    [ -16.02, 98.00 ]
677                    [ -13.95, 97.50 ]
678                    [ 13.95, 97.50 ]
679                    [ 15.64, 98.00 ]
680                    [ 17.31, 98.50 ]
681                    [ 162.02, 98.50 ]
682                    [ 172.89 , 99.00 ]
683                    [ 314.38, 99.00 ]
684                    [ 343.78, 99.50 ]
685                    [ 365.67, 100.00 ]
686                    [ 376.68, 100.00 ]
687                    [ 393.11, 99.50 ]
688                    [ 404.97, 99.50 ]
689                    [ 431.70, 100.00 ]
690                    [ 456.58, 100.50 ]
691  *%-----|-----|
692  *#
693  *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
694  *#
695  ADD HYD          NHYDs um=[ "S_N8" ], NHYDs to add=[ "N8"+"SW_8"+"HB_DR" ]
696  *%-----|-----|
697  *#
698  *# Sum of hydrographs from Node 8 routed to Node 7
699  *# Section 4
700  *#
701  ROUTE CHANNEL    NHYDout =[ "N7" ] , NHYDin =[ "S_N8" ] ,
702                  RDT=[ 1 ] ( mi n ) ,
703                  CHLGTH=[ 3750 ] ( m ) ,   CHSLOPE=[ 0.0533 ] ( % ) ,
704                                                    FPSLOPE=[ 0.0533 ] ( % ) ,
705                  SECNUM=[ 1.0 ] ,          NSEG=[ 3 ]
706                  ( SEGROUGH, SEGDI ST ( m ) ) =
707                    [ 0.12, -18.11
708                      -0.07, 17.22
709                      0.12, 590.05 ] NSEG t i m e s
710                  ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
711                    [ -433.21, 102.00 ]
712                    [ -425.34, 101.50 ]
713                    [ -377.56, 101.50 ]
714                    [ -366.23, 101.00 ]
715                    [ -202.60, 100.50 ]
716                    [ -96.25, 99.50 ]
717                    [ -68.36 99.00 ]
718                    [ -18.11, 98.50 ]
719                    [ -13.81, 97.50 ]

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720 [ 13. 81, 97. 50]
721 [ 17. 22, 98. 50]
722 [ 161. 95, 98. 50]
723 [ 173. 11, 99. 00]
724 [ 314. 05, 99. 00]
725 [ 365. 52, 100. 00]
726 [ 404. 70, 99. 50]
727 [ 476. 74, 100. 50]
728 [ 502. 31, 101. 00]
729 [ 584. 69, 101. 00]
730 [ 585. 79, 101. 00]
731 [ 590. 05, 102. 00]
732 *%-----|-----
733 *#
734 *# Addition of Subwatershed 7 to Node 7
735 *#
736 ADD HYD          NHYDs um=[ "S_N7" ], NHYDs to add=[ "N7"+"SW_7" ]
737 *%-----|-----
738 SAVE HYD         NHYD=[ "S_N7" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ - 1 ]
739                 HYD_FI LENAME=[ "H_SN7" ]
740                 HYD_COMMENT=[ "flow at S_N7: N7 + SW_7" ]
741 *%-----|-----
742 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
743 *# Storage area and volumes were estimated from available topo maps.
744 *# Release rate from fen was assumed to be controlled by the downstream
745 *# river cross-section for summer conditions. It is was assumed that for up to
746 *# 0.75 m of water, the main channel of the river provided the storage. Above
747 *# this depth, the wetland starts to signigicantly store water.
748 *#
749 ROUTE RESERVOIR NHYDout =[ "RES_RF" ] , NHYDi n=[ "S_N7" ] ,
750                 RDT=[ 1 ] ( mi n ) ,
751                 TABLE of ( OUTFLOW STORAGE ) values
752                         ( cms ) - ( ha- m )
753                 TABLE of ( OUTFLOW STORAGE ) values
754                         ( cms ) - ( ha- m )
755                         [ 0. 0 , 0. 0 ]
756                         [ 0. 9051, 2. 40]
757                         [ 2. 907, 4. 13]
758                         [ 9. 744, 9. 18]
759                         [ 20. 304, 14. 96]
760                         [ 34. 167, 310. 21]
761                         [ 74. 993, 605. 46]
762                         [ 104. 876, 900. 71]
763                         [ 140. 56, 2892. 00]
764                         [ 225. 00, 3615. 63]
765                         [ - 1 , - 1 ] (max t wenty pts)
766                 NHYDovf=[ " " ] ,
767 *%-----|-----
768 SAVE HYD         NHYD=[ "RES_RF" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ - 1 ]
769                 HYD_FI LENAME=[ "H_Res RF" ]
770                 HYD_COMMENT=[ "out flow of Ri chmond Fen" ]
771 *%-----|-----
772 *#
773 *# Sum of hydrographs from Node 7 routed to Node 6
774 *# Section 5
775 *#
776 ROUTE CHANNEL   NHYDout =[ "N6" ] , NHYDi n=[ "RES_RF" ] ,
777                 RDT=[ 1 ] ( mi n ) ,
778                 CHLGTH=[ 3056 ] ( m ) , CHSLOPE=[ 0. 0818 ] ( % ) ,
779                 FPSLOPE=[ 0. 0818 ] ( % ) ,
780                 SECNUM=[ 1. 0 ] , NSEG=[ 5 ]
781                 ( SEGROUGH, SEGDI ST ( m ) ) =
782                 [ 0. 025, - 70. 8
783                 0. 1, - 23. 9
784                 - 0. 05, 23. 9
785                 0. 06, 39. 8

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786           0. 05, 96. 3] NSEG times
787           ( DI STANCE ( m), ELEVATI ON ( m))=
788             [- 100. 8, 97. 00]
789             [- 70. 8, 96. 50]
790             [- 52. 0, 96. 00]
791             [- 35. 1, 95. 50]
792             [- 30. 6, 95. 00]
793             [- 23. 9, 94. 54]
794             [ 23. 9, 94. 54]
795             [ 39. 8, 95. 00]
796             [ 50. 4, 95. 50]
797             [ 93. 5, 96. 00]
798             [ 94. 9, 96. 50]
799             [ 96. 3, 97. 00]
800 *%-----|-----
801 *#
802 *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
803 *#
804 ADD HYD           NHYDs um=[ "S_N6" ], NHYDs to add=[ "N6"+"SW_6"+"VG_DR" ]
805 *%-----|-----
806 *#
807 *# Sum of hydrographs from Node 6 routed to Node 5
808 *# Section 6
809 *#
810 ROUTE CHANNEL     NHYDout =[ "N5" ] , NHYDin =[ "S_N6" ] ,
811                   RDT=[ 1] ( mi n),
812                   CHLGTH=[ 1852] ( m),   CHSLOPE=[ 0. 0540] ( %),
813                                           FPSLOPE=[ 0. 0540] ( %),
814                   SECNUM=[ 1. 0],       NSEG=[ 3]
815                   ( SEGROUGH, SEGDI ST ( m))=
816                     [ 0. 035, - 131. 59
817                     - 0. 045, 48. 96
818                     0. 1, 239. 04] NSEG times
819                   ( DI STANCE ( m), ELEVATI ON ( m))=
820                     [- 686. 30, 94. 50]
821                     [- 675. 70, 94. 00]
822                     [- 492. 52, 93. 00]
823                     [- 467. 28, 94. 00]
824                     [- 131. 59, 94. 00]
825                     [- 92. 79, 92. 50]
826                     [- 18. 06, 91. 00]
827                     [ 18. 06, 91. 00]
828                     [ 43. 47, 92. 50]
829                     [ 48. 96, 94. 00]
830                     [ 177. 43, 94. 00]
831                     [ 239. 04, 94. 50]
832 *%-----|-----
833 *#
834 *# Addition of Subwatershed 5 and Flowing Creek to Node 5
835 *#
836 ADD HYD           NHYDs um=[ "S_N5" ], NHYDs to add=[ "N5"+"SW_5"+"FL_CK" ]
837 *%-----|-----
838 *#
839 *# Sum of hydrographs from Node 5 routed to Node 5A
840 *# Section 7
841 *#
842 ROUTE CHANNEL     NHYDout =[ "N5A" ] , NHYDin =[ "S_N5" ] ,
843                   RDT=[ 1] ( mi n),
844                   CHLGTH=[ 556] ( m),   CHSLOPE=[ 0. 0900] ( %),
845                                           FPSLOPE=[ 0. 0900] ( %),
846                   SECNUM=[ 1. 0],       NSEG=[ 4]
847                   ( SEGROUGH, SEGDI ST ( m))=
848                     [ 0. 04, - 41. 5
849                     0. 1, - 14. 0
850                     - 0. 045, 14. 0
851                     0. 1, 41. 1] NSEG times

```

```

852 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
853 [- 275. 8, 93. 00]
854 [- 248. 6, 92. 50]
855 [- 237. 0, 92. 00]
856 [- 219. 3, 91. 50]
857 [- 202. 1, 91. 50]
858 [- 186. 0, 92. 00]
859 [- 129. 2, 92. 00]
860 [- 117. 6, 91. 50]
861 [- 100. 6, 91. 00]
862 [- 41. 5, 91. 00]
863 [- 20. 0, 91. 00]
864 [- 14. 0, 90. 54]
865 [ 14. 0, 90. 54]
866 [ 15. 3, 91. 00]
867 [ 17. 3, 91. 50]
868 [ 38. 4, 92. 00]
869 [ 39. 8, 92. 50]
870 [ 41. 1, 93. 00]
871 *%-----|-----|
872 *#
873 *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
874 *#
875 ADD HYD NHYDs um=[ "S_N5A" ] , NHYDs t o add=[ "N5A"+"SW_5A2"+"SW_5A1" ]
876 *%-----|-----|
877 *#
878 *# Sum of hydrographs from Node 5A routed to Node 4
879 *# Section 8
880 *#
881 ROUTE CHANNEL NHYDout =[ "N4" ] , NHYDi n=[ "S_N5A" ] ,
882 RDT=[ 1 ] ( mi n) ,
883 CHLGTH=[ 4630 ] ( m) , CHSLOPE=[ 0. 0432 ] ( % ) ,
884 FPSLOPE=[ 0. 0432 ] ( % ) ,
885 SECNUM=[ 1. 0 ] , NSEG=[ 3 ]
886 ( SEGROUGH, SEGDI ST ( m) ) =
887 [ 0. 05, -28. 2
888 -0. 035, 28. 2
889 0. 05, 173. 1 ] NSEG t i mes
890 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
891 [- 38. 9, 92. 00]
892 [- 35. 8, 91. 50]
893 [- 33. 3, 91. 00]
894 [- 28. 2, 90. 50]
895 [- 15. 0, 87. 48]
896 [- 5. 0, 88. 34]
897 [ 5. 0, 86. 20]
898 [ 15. 0, 88. 55]
899 [ 28. 2, 90. 50]
900 [ 29. 7, 91. 00]
901 [ 46. 5, 91. 00]
902 [ 127. 8, 91. 00]
903 [ 148. 7, 91. 50]
904 [ 173. 1, 92. 00]
905 *%-----|-----|
906 *#
907 *# Addition of Subwatershed 4 and Leamy Creek to Node 4
908 *#
909 ADD HYD NHYDs um=[ "S_N4" ] , NHYDs t o add=[ "N4"+"SW_4"+"LM_CK" ]
910 SAVE HYD NHYD=[ "S_N4" ] , # OF PCYCLES=[ - 1 ] , I CASEs h=[ 1 ]
911 HYD_COMMENT=[ "f l ow at S_N4" ]
912 *%-----|-----|
913 *#
914 *# Sum of hydrographs from Node 4 routed to Node 2
915 *# Section 9
916 *#
917 ROUTE CHANNEL NHYDout =[ "N2" ] , NHYDi n=[ "S_N4" ] ,

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918 RDT=[ 1] ( mi n),
919 CHLGTH=[ 1667] ( m), CHSLOPE=[ 0. 0600] ( %),
920 FPSLOPE=[ 0. 0600] ( %),
921 SECNUM=[ 1. 0], NSEG=[ 4]
922 ( SEGROUGH, SEGDI ST ( m))=
923 [ 0. 1, - 28. 0
924 - 0. 04, 28. 4
925 0. 06, 31. 7
926 0. 04, 80. 2] NSEG t i m e s
927 ( DI STANCE ( m), ELEVATI ON ( m))=
928 [- 36. 3, 92. 00]
929 [- 32. 6, 91. 50]
930 [- 30. 2, 91. 00]
931 [- 28. 0, 90. 45]
932 [- 15. 0, 87. 48]
933 [- 5. 0, 88. 34]
934 [ 5. 0, 86. 20]
935 [ 15. 0, 88. 55]
936 [ 28. 0, 90. 45]
937 [ 28. 4, 90. 50]
938 [ 30. 4, 91. 00]
939 [ 31. 7, 91. 50]
940 [ 80. 2, 92. 00]
941 *%-----|-----|
942 *#
943 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
944 *#
945 ADD HYD NHYDs um=[ "S_N2"], NHYDs t o add=[ "N2"+"SW_2"+"SM_DR"+"MO_DR"]
946 *%-----|-----|
947 SAVE HYD NHYD=[ "S_N2"], # OF PCYCLES=[ - 1], I CAS E s h=[ - 1]
948 HYD_FI L E N A M E=[ "H_S N2"]
949 HYD_C O M M E N T=[ "f l o w a t S_N2 J o c k R i v e r G a u g e a t M o d i e D r. "]
950 *%-----|-----|
951 *#
952 *# Sum of hydrographs from Node 2 routed to Node 1
953 *# Section 10
954 *#
955 *#*****
956 *%READ HYD NHYD=[ "S_N2"],
957 *% HYD_FI L E N A M E=[ "H_S_N2"]
958 *%-----|-----|
959 *#
960 *# Hydrograph from Node 2 routed to Node 416
961 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
962 *#
963 ROUTE CHANNEL NHYDout =["N_416"] , NHYDin=["S_N2"] ,
964 RDT=[ 1] ( mi n),
965 CHLGTH=[ 2327] ( m), CHSLOPE=[ 0. 0498] ( %),
966 FPSLOPE=[ 0. 0498] ( %),
967 SECNUM=[ 1. 0], NSEG=[ 3]
968 ( SEGROUGH, SEGDI ST ( m))=
969 [ 0. 075, - 23. 96
970 - 0. 055, 23. 96
971 0. 075, 157. 38] NSEG t i m e s
972 ( DI STANCE ( m), ELEVATI ON ( m))=
973 [- 336. 97, 93. 5]
974 [- 318. 85, 93]
975 [- 259, 92. 5]
976 [- 133. 18, 92]
977 [- 33. 17, 92]
978 [- 27. 21, 92]
979 [- 26. 14, 91. 5]
980 [- 24. 99, 91]
981 [- 23. 96, 90. 5]
982 [- 14. 33, 88. 26]
983 [- 0. 68, 88. 12]

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984 [ 14. 33, 88. 26]
985 [ 23. 96, 90. 5]
986 [ 32. 12, 91]
987 [ 43. 74, 91. 5]
988 [ 57. 09, 92]
989 [ 73. 53, 92. 5]
990 [ 108. 27, 93]
991 [ 125. 88, 93. 5]
992 [ 144. 81, 94]
993 [ 157. 38, 94. 5]
994 *%-----|-----|
995 *#*****|
996 *# Catchment SW1a
997 *# - Portion of RVCA catchment SW1 outside of Reach 1 subwatershed
998 *# - Undeveloped agricultural land
999 *#*****|
1000 CONTINUOUS NASHYD NHYD=[ "SW1a" ], DT=[ 1] mi n, AREA=[ 536. 42] ( ha ),
1001 DWF=[ 0] ( cms ), CN C=[ 72], IA=[ 4. 67] ( mm ),
1002 N=[ 3], TP=[ 2. 79] hr s,
1003 Continuous simulation parameters:
1004 IaRECper=[ 4] ( hr s ),
1005 SM N=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
1006 InterEvent Time=[ 12] ( hr s)
1007 Baseflow simulation parameters:
1008 BaseFlowOption=[ 1] ,
1009 InitGWRes Vol =[ 50] ( mm ), GWRes K=[ 0. 96] ( mm/ day/ mm)
1010 VHydCond=[ 0. 055] ( mm/ hr ), END=- 1
1011 *%-----|-----|
1012 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
1013 *CONTINUOUS STANDHYD NHYD=[ "S-1-Okeefe" ], DT=[ 1] ( mi n), AREA=[ 44. 93] ( ha ), XI MP=[ 0. 65],
TI MP=[ 0. 65], DWF=[ 0] ( cms ),
1014 * LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
I Aper =[ 4. 67] ( mm ), SLPP=[ 2. 0] ( % ),
1015 * LGP=[ 40] ( m ), MNP=[ 0. 25], SCP=[ 0] ( mi n), Impervious surfaces:
I Ai mp=[ 1. 57] ( mm ), SLPI =[ 0. 75] ( % ),
1016 * LGI =[ 547. 296] ( m ), MNI =[ 0. 013], SCI =[ 0] ( mi n),
1017 * Continuous simulation parameters:
1018 * IaRECper =[ 4] ( hr s ), IaRECI mp=[ 4] ( hr s ),
1019 * SM N=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
1020 * InterEvent Time =[ 12] ( hr s ), END=- 1
1021 *%-----|-----|
1022 CONTINUOUS NASHYD NHYD=[ "S-1-Okeefe" ], DT=[ 1] mi n, AREA=[ 44. 93] ( ha ),
1023 DWF=[ 0] ( cms ), CN C=[ 77], IA=[ 4. 67] ( mm ),
1024 N=[ 3], TP=[ 1. 049] hr s,
1025 Continuous simulation parameters:
1026 IaRECper=[ 4] ( hr s ),
1027 SM N=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
1028 InterEvent Time =[ 12] ( hr s)
1029 Baseflow simulation parameters:
1030 BaseFlowOption=[ 1] ,
1031 InitGWRes Vol =[ 50] ( mm ), GWRes K=[ 0. 96] ( mm/ day/ mm)
1032 VHydCond=[ 0. 055] ( mm/ hr ), END=- 1
1033 *%-----|-----|
1034 *COMPUTE DUALHYD NHYDi n=[ "S-1-Okeefe" ], CI NLET=[ 4. 796] ( cms ), NI NLET=[ 1],
1035 * Ma j NHYD=[ "S-1-OkM" ]
1036 * M nNHYD=[ "S-1-OkMN" ]
1037 * TM I STO=[ 9999999] ( cu - m)
1038 *%-----|-----|
1039 *ADD HYD NHYDsum=[ "S-1-OkS" ], NHYDs to add=[ "S-1-OkM" +"S-1-OkMN" ]
1040 *%-----|-----|
1041 *ROUTE RESERVOIR NHYDout=[ "S-1-OkSR" ], NHYDi n=[ "S-1-OkS" ],
1042 * RDT=[ 1] ( mi n),
1043 * TABLE of ( OUTFLOW STORAGE ) values
1044 * ( cms ) - ( ha - m)
1045 * [ 0. 0 , 0. 0 ]

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1046 * [ 0.5370, 1.7917 ]
1047 * [ -1 , -1 ] (max twenty pts)
1048 * NHYDovf=["S-1-OkSovf"]
1049 *%-----|-----|
1050 ADD HYD NHYDsum=["SN_416"], NHYDs to add=["N_416"+"SW_1a"+"S-1-Okeefe"]
1051 *%-----|-----|
1052 SAVE HYD NHYD=["SN_416"], # OF PCYCLES=[-1], I CASEs h=[1]
1053 HYD_COMMENT=["Total Flows at Highway 416"]
1054 *%-----|-----|
1055 *#
1056 *# Hydrograph from Node 416 routed to Node at Okeefe drain
1057 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245
1058 *#
1059 ROUTE CHANNEL NHYDout=["N_OK"], NHYDin=["SN_416"],
1060 RDT=[1] (min),
1061 CHLGTH=[497] (m), CHSLOPE=[0.3018] (%),
1062 FPSLOPE=[0.3018] (%),
1063 SECNUM=[1.0], NSEG=[3]
1064 ( SEGROUGH, SEGDI ST (m))=
1065 [0.075, -19.40
1066 -0.055, 19.40
1067 0.075, 377.02] NSEG times
1068 ( DI STANCE (m), ELEVATI ON (m))=
1069 [-1061.41, 92.50]
1070 [-945.91, 92.00]
1071 [-783.64, 91.50]
1072 [-136.74, 91.00]
1073 [-86.04, 91.00]
1074 [-20.86, 91.00]
1075 [-20.18, 90.50]
1076 [-19.40, 90.00]
1077 [-11.68, 86.89]
1078 [0.00, 86.10]
1079 [12.09, 86.81]
1080 [19.40, 90.00]
1081 [34.68, 90.50]
1082 [60.56, 91.00]
1083 [170.14, 91.00]
1084 [175.05, 90.50]
1085 [180.29, 90.00]
1086 [193.41, 90.00]
1087 [195.98, 90.50]
1088 [377.02, 92.50]
1089 *%-----|-----|
1090 *#*****|*****|
1091 *# Catchment OKEEFE
1092 *# - To O'Keefe drain (north of the Jock)
1093 *# - Developed with assumed 43% imp.
1094 *# - 2020-12-01 add Okeefe model (Area 513.02 HA) instead of current Okeefe (Area
1095 513.02 HA)
1096 *# - 2020-11-20 Okeefe detailed model was added as per the NOVATECH SWWHYMD model
1097 (Citi-Gate 2014).
1098 *%-----|-----|
1099 *#*****|*****|
1100 CONTINUOUS NASHYD NHYD=["O-1"], DT=[1] min, AREA=[63.72] (ha),
1101 DWF=[0] (cms), CNVC=[61], IA=[6.2] (mm), N=[3], TP=[.9] hrs,
1102 Continuous simulation parameters:
1103 I aRECper=[4] (hrs),
1104 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1105 InterEventTime=[12] (hrs)
1106 Baseflow simulation parameters:
1107 BaseFlowOption=[1],
1108 In itGWRes Vol=[50] (mm), GWResK=[0.96] (mm/day/mm)
1109 VHydCond=[0.055] (mm/hr), END=-1

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1110  *%-----|-----
1111  *ROUTE FLOW THROUGH AREA 0-2
1112  ROUTE CHANNEL  NHYDout=["O-1R"], NHYDin=["O-1"], RDT=[1](min),
1113                CHLGTH=[960](m), CHSLOPE=[0.63](%), FPSLOPE=[0.63](%),
1114                SECNUM=[1], NSEG=[3]
1115                (SEGROUGH, SEGDIST(m))=[0.06, 4-.043, 6-0.06, 10] NSEG times
1116                (DISTANCE(m), ELEVATION(m))=[0.00, 2.0]
1117                [0.0, 2.0]
1118                [4.0, 0.0]
1119                [6.0, 0.0]
1120                [10.0, 2.0]
1121  *%-----|-----
1122  CONTINUOUS NASHYD  NHYD=["O-2"], DT=[1]min, AREA=[28.61](ha),
1123                DWF=[0](cms), CNVC=[57], IA=[5.2](mm), N=[3], TP=[1.1]hrs,
1124                Continuous simulation parameters:
1125                IARECper=[4](hrs),
1126                SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1127                InterEventTime=[12](hrs)
1128                Baseflow simulation parameters:
1129                BaseFlowOption=[1],
1130                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1131                VHydCond=[0.055](mm/hr), END=-1
1132  *%-----|-----
1133  CONTINUOUS NASHYD  NHYD=["O-4"], DT=[1]min, AREA=[46.94](ha),
1134                DWF=[0](cms), CNVC=[49], IA=[9.2](mm), N=[3], TP=[0.9]hrs,
1135                Continuous simulation parameters:
1136                IARECper=[4](hrs),
1137                SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1138                InterEventTime=[12](hrs)
1139                Baseflow simulation parameters:
1140                BaseFlowOption=[1],
1141                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1142                VHydCond=[0.055](mm/hr), END=-1
1143  *%-----|-----
1144  *TOTAL EXTERNAL FLOW NORTH OF O'KEEFE CT. CROSSING
1145  ADD HYD  NHYDsum=["OKF-N"], NHYDstoadd=["O-1R"+"O-2"+"O-4"]
1146  *%-----|-----
1147  *ROUTE FLOW THROUGH AREA 0-6
1148  ROUTE CHANNEL  ROUTE CHANNEL NHYDout=["OKF-NR"], NHYDin=["OKF-N"], RDT=[1](min),
1149                CHLGTH=[210](m), CHSLOPE=[.81](%), FPSLOPE=[.81](%),
1150                SECNUM=[1], NSEG=[3]
1151                (SEGROUGH, SEGDIST(m))=[0.043, 22.43-0.043, 25.07
1152                0.043, 45.54] NSEG times
1153                (DISTANCE(m), ELEVATION(m))=[0.00, 3.73]
1154                (14.62, 1.56)
1155                (18.41, 1.44)
1156                (22.43, 0.00)
1157                (25.07, 0.70)
1158                (29.10, 1.79)
1159                (33.73, 2.71)
1160                (45.54, 3.58)
1161  *%-----|-----
1162  CONTINUOUS NASHYD  NHYD=["O-6"], DT=[1]min, AREA=[16.46](ha),
1163                DWF=[0](cms), CNVC=[43], IA=[9.2](mm), N=[3], TP=[0.7]hrs,
1164                Continuous simulation parameters:
1165                IARECper=[4](hrs),
1166                SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1167                InterEventTime=[12](hrs)
1168                Baseflow simulation parameters:
1169                BaseFlowOption=[1],
1170                InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1171                VHydCond=[0.055](mm/hr), END=-1
1172  *%-----|-----
1173  CONTINUOUS STANDHYD  NHYD=["O-3"], DT=[1](min), AREA=[39.67](ha), XI MP=[0.15],
                TIMP=[0.30], DWF=[0](cms),

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1174      LOSS=[ 2], SCS curve number CN=[ 50], Pervious surfaces:
1175      I Aper=[ 4.67] (mm), SLPP=[ 0.32] (%),
1176      LGP=[ 440] (m), MNP=[ 0.035], SCP=[ 0] (min), Impervious surfaces:
1177      I Aimp=[ 1.57] (mm), SLPI=[ 0.32] (%),
1178      LGI=[ 1880] (m), MNI=[ 0.013], SCI=[ 0] (min),
1179      Continuous simulation parameters:
1180      I aRECper=[ 4] (hrs), I aRECimp=[ 4] (hrs),
1181      SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1182      InterEventTime=[ 12] (hrs), END=- 1
1183 *%-----|-----|
1184 CONTINUOUS STANDHYD NHYD=[ "O-5"], DT=[ 1] (min), AREA=[ 60.63] (ha), XI MP=[ 0.13],
1185 TIMP=[ 0.26], DWF=[ 0] (cms),
1186      LOSS=[ 2], SCS curve number CN=[ 61],
1187      Pervious surfaces: I Aper=[ 4.67] (mm), SLPP=[ 1.38] (%),
1188      LGP=[ 550] (m), MNP=[ 0.035], SCP=[ 0] (min), Impervious surfaces:
1189      I Aimp=[ 1.57] (mm), SLPI=[ 1.38] (%),
1190      LGI=[ 1450] (m), MNI=[ 0.013], SCI=[ 0] (min),
1191      Continuous simulation parameters:
1192      I aRECper=[ 4] (hrs), I aRECimp=[ 4] (hrs),
1193      SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1194      InterEventTime=[ 12] (hrs), END=- 1
1195 *%-----|-----|
1196 *TOTAL EXTERNAL FLOWS WEST OF THE SITE AND NORTH OF O'KEEFE CRT
1197 *%-----|-----|
1198 ADD HYD NHYDsum=[ "PT1"], NHYDs to add=[ "OKF-NR"+"O-3"+"O-5"+"O-6"]
1199 *%-----|-----|
1200 CONTINUOUS NASHYD NHYD=[ "O-7"], DT=[ 1] min, AREA=[ 5.28] (ha),
1201 DWF=[ 0] (cms), CN C=[ 54], IA=[ 7.5] (mm), N=[ 3], TP=[ 0.6] hrs,
1202 Continuous simulation parameters:
1203 I aRECper=[ 4] (hrs),
1204 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1205 InterEventTime=[ 12] (hrs)
1206 Baseflow simulation parameters:
1207 BaseFlowOption=[ 1],
1208 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/day/mm)
1209 VHydCond=[ 0.055] (mm/hr), END=- 1
1210 *%-----|-----|
1211 *ANALYSIS POINT 1 - TOTAL FLOW NORTH OF FALLOWFIELD DR. AND O'KEEFE CRT.
1212 ADD HYD NHYDsum=[ "FF"], NHYDs to add=[ "PT1"+"O-7"]
1213 *%-----|-----|
1214 *ROUTE FLOW through O'Keefe Drain 1
1215 ROUTE CHANNEL NHYDout=[ "DRAIN1"], NHYDin=[ "FF"], RDT=[ 1] (min),
1216 CHLGTH=[ 302] {m}, CHSLOPE=[ 1.00] (%), FPSLOPE=[ 1.00] (%),
1217 SECNUM=[ 1], NSEG=[ 3]
1218 ( SEGROUGH, SEGDIST (m))=[ 0.07, 13.45 -0.043, 16.55 0.07, 30.00] NSEG
1219 times
1220 ( DISTANCE (m), ELEVATION (m))=[ 0.00, 1.70]
1221 ( 3.45, 0.60)
1222 ( 13.45, 0.50)
1223 ( 14.45, 0.00)
1224 ( 15.55, 0.00)
1225 ( 16.55, 0.50)
1226 ( 26.55, 0.60)
1227 ( 30.00, 1.70)
1228 *%-----|-----|
1229 CONTINUOUS NASHYD NHYD=[ "DI"], DT=[ 1] min, AREA=[ 1.17] (ha),
1230 DWF=[ 0] (cms), CN C=[ 84], IA=[ 9.0] (mm), N=[ 3], TP=[ 0.28] hrs,
1231 Continuous simulation parameters:
1232 I aRECper=[ 4] (hrs),
1233 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1234 InterEventTime=[ 12] (hrs)
1235 Baseflow simulation parameters:
1236 BaseFlowOption=[ 1],
1237 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/day/mm)
1238 VHydCond=[ 0.055] (mm/hr), END=- 1
1239 *%-----|-----|

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1235 CONTINUOUS STANDHYD NHYD=["A1"], DT=[1] min, AREA=[2.50](ha), XI MP=[0.68], TI MP=[0.85],
DWF=[0](cms), LOSS=[1]:

1236 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),

1237 Pervious areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),

1238 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%),
LGI=[223.607](m), MNI=[0.013], SCI=[0](min),

1239 Continuous simulation parameters:
1240 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=- 1

1241 *%-----|-----|

1242 ROUTE RESERVOIR NHYDout=["A1-STR"], NHYDin=["A1"], RDT=[1](min),
1243 TABLE of (OUTFLOW STORAGE) values
1244 (cms) - (ha-m)
1245 [0.000 , 0.000]
1246 [0.035 , 0.038]
1247 [0.072 , 0.051]
1248 [0.100 , 0.059]
1249 [0.125 , 0.070]
1250 [0.160 , 0.074]
1251 [0.185 , 0.081]
1252 [-1 , -1] (max twenty pts)
1253 NHYDovf=["A1-OVF"]

1254 *%-----|-----|

1255 CONTINUOUS STANDHYD NHYD=["ST-2"], DT=[1] min, AREA=[0.59](ha), XI MP=[0.46],
TI MP=[0.57], DWF=[0](cms), LOSS=[1]:

1256 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),

1257 Pervious areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),

1258 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%),
LGI=[108.628](m), MNI=[0.013], SCI=[0](min),

1259 Continuous simulation parameters:
1260 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=- 1

1261 *%-----|-----|

1262 ROUTE RESERVOIR NHYDout=["ST2STR"], NHYDin=["ST-2"], RDT=[1](min),
1263 TABLE of (OUTFLOW STORAGE) values
1264 (cms) - (ha-m)
1265 [0.000 , 0.0000]
1266 [0.052 , 0.0010]
1267 [0.053 , 0.0080]
1268 [-1 , -1] (max twenty pts)
1269 NHYDovf=["ST2OVF"]

1270 *%-----|-----|

1271 *%-----|-----|

1272 *TOTAL FLOW NORTH OF STRANDHERD DR. (EAST BRANCH) CROSSING

1273 *%-----|-----|

1274 CONTINUOUS NASHYD NHYD=["O-8"], DT=[1] min, AREA=[60.55](ha),
1275 DWF=[0](cms), CNV C=[69], I A=[4.0](mm), N=[3], TP=[1.0]hrs,
1276 Continuous simulation parameters:
1277 I a REC per=[4](hrs),
1278 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1279 I n t e r E v e n t T i m e=[12](hrs)
1280 Baseflow simulation parameters:
1281 BaseFlowOption=[1],
1282 I n i t G W R e s V o l=[50](mm), G W R e s K=[0.96](mm/day/mm)
1283 VHydCond=[0.055](mm/hr), END=- 1

1284 *%-----|-----|

1285 ROUTE PIPE PTYPE=[2]rect, NHYDout=["O8PIPE"], RNUMBER=[1], PW DTH=[1800](mm),
PHEI GHT=[1200](mm), PLNGTH=[335.1](m),
1286 PROUGH=[0.013], PSLOPE=[0.001](m/m), NHYDin=["O-8"], RDT=[1](min)

1287 *%-----|-----|

1288 *%-----|-----|

1289 ADD HYD NHYDs um=["ST2-IN"], NHYDs to

```

add=[ " DRAI N1 " + " D1 " + " A1 - STR" + " A1 - OVF" + " ST2STR" + " ST2OVF" + " O8PI PE" ]
1290 *%-----|-----|
1291 CONTI NUOUS STANDHYD NHYD=[ " A7" ], DT=[ 1 ] mi n, AREA=[ 3. 51 ] ( ha ), XI MP=[ 0. 68 ], TI MP=[ 0. 85 ],
DWF=[ 0 ] ( c ms ), LOSS=[ 1 ] :
1292 Hort on: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
1293 Per vious areas: I Aper =[ 4. 67 ] ( mm ), SLPP=[ 0. 5 ] ( % ), LGP=[ 50 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
1294 Impervious areas: I Ai mp=[ 1. 57 ] ( mm ), SLPI =[ 0. 5 ] ( % ),
LGI=[ 264. 953 ] ( m ), MNI =[ 0. 013 ], SCI =[ 0 ] ( mi n ),
1295 Continuous simulation parameters:
1296 IaRECPer=[ 4 ] ( hr s ), IaRECI mp=[ 4 ] ( hr s ), Int er Event Ti me=[ 12 ] ( hr s ),
END=- 1
1297 *%-----|-----|
1298 ROUTE RESERVOI R NHYDout =[ " A7 - STR" ], NHYDi n=[ " A7" ], RDT=[ 1 ] ( mi n ),
1299 TABLE of ( OUTFLOW STORAGE ) values
1300 ( c ms ) - ( ha - m )
1301 [ 0. 000 , 0. 000 ]
1302 [ 0. 049 , 0. 054 ]
1303 [ 0. 102 , 0. 072 ]
1304 [ 0. 140 , 0. 082 ]
1305 [ 0. 175 , 0. 099 ]
1306 [ 0. 225 , 0. 105 ]
1307 [ 0. 260 , 0. 114 ]
1308 [ - 1 , - 1 ] ( max t went y pt s )
1309 NHYDovf=[ " A7 - OVF" ]
1310 *%-----|-----|
1311 CONTI NUOUS STANDHYD NHYD=[ " ST - 3" ], DT=[ 1 ] mi n, AREA=[ 0. 71 ] ( ha ), XI MP=[ 0. 46 ],
TI MP=[ 0. 57 ], DWF=[ 0 ] ( c ms ), LOSS=[ 1 ] :
1312 Hort on: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
1313 Per vious areas: I Aper =[ 4. 67 ] ( mm ), SLPP=[ 0. 5 ] ( % ), LGP=[ 50 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
1314 Impervious areas: I Ai mp=[ 1. 57 ] ( mm ), SLPI =[ 0. 5 ] ( % ),
LGI=[ 119. 164 ] ( m ), MNI =[ 0. 013 ], SCI =[ 0 ] ( mi n ),
1315 Continuous simulation parameters:
1316 IaRECPer=[ 4 ] ( hr s ), IaRECI mp=[ 4 ] ( hr s ), Int er Event Ti me=[ 12 ] ( hr s ),
END=- 1
1317 *%-----|-----|
1318 ROUTE RESERVOI R NHYDout =[ " ST3STR" ], NHYDi n=[ " ST - 3" ], RDT=[ 1 ] ( mi n ),
1319 TABLE of ( OUTFLOW STORAGE ) values
1320 ( c ms ) - ( ha - m )
1321 [ 0. 000 , 0. 0000 ]
1322 [ 0. 063 , 0. 0010 ]
1323 [ 0. 064 , 0. 0094 ]
1324 [ - 1 , - 1 ] ( max t went y pt s )
1325 NHYDovf=[ " ST3OVF" ]
1326 *%-----|-----|
1327 *ANALYSIS POINT 2 - TOTAL FLOW AT OUTLET OF STREET 2/ 3 INTERSECTION
1328 *%-----|-----|
1329 ADD HYD NHYDsum=[ " PT2ST3" ], NHYDs to
add=[ " ST2 - I N" + " A7 - STR" + " A7 - OVF" + " ST3STR" + " ST3OVF" ]
1330 *%-----|-----|
1331 *ROUTE FLOW through O Keefe Drain 2
1332 ROUTE CHANNEL NHYDout =[ " DRAI N2" ], NHYDi n=[ " PT2ST3" ], RDT=[ 1 ] ( mi n ),
1333 CHLGTH=[ 592 ] { m }, CHSLOPE=[ . 23 ] ( % ), FPSLOPE=[ . 23 ] ( % ),
1334 SECNUM=[ 1 ], NSEG=[ 3 ]
1335 ( SEGROUGH, SEGDI ST ( m ) )=[ 0. 07, 12. 60 - 0. 043, 17. 40 0. 07, 30. 00 ] NSEG
ti mes
1336 ( DI STANCE ( m ), ELEVATI ON ( m ) )=[ 0. 00, 1. 70 ]
1337 ( 2. 60, 0. 95 )
1338 ( 12. 60, 0. 75 )
1339 ( 14. 10, 0. 00 )
1340 ( 15. 90, 0. 00 )
1341 ( 17. 40, 0. 75 )
1342 ( 27. 40, 0. 95 )

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1343         (30.00, 1.70)
1344 *%-----|-----|
1345 CONTINUOUS NASHYD NHYD=["D2"], DT=[1] min, AREA=[2.28](ha), DWF=[0](cms), CNVC=[84],
IA=[9.0](mm),
1346 N=[3], TP=[0.99] hrs,
1347 Continuous simulation parameters:
1348 IARECper=[4](hrs),
1349 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1350 InterEventTime=[12](hrs)
1351 Baseflow simulation parameters:
1352 BaseFlowOption=[1],
1353 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1354 VHydCond=[0.055](mm/hr), END=-1
1355 *%-----|-----|
1356 CONTINUOUS STANDHYD NHYD=["A17"], DT=[1] min, AREA=[12.04](ha), XI MP=[0.68],
TI MP=[0.85], DWF=[0](cms), LOSS=[1]:
1357 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1358 Pervious areas: IAPER=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1359 Impervious areas: IAIMP=[1.57](mm), SLPI=[0.5](%),
LGI=[490.714](m), MNI=[0.013], SCI=[0](min),
1360 Continuous simulation parameters:
1361 IARECper=[4](hrs), IARECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1362 *%-----|-----|
1363 ROUTE RESERVOIR NHYDout=["A17STR"], NHYDin=["A17"], RDT=[1](min),
1364 TABLE of ( OUTFLOW STORAGE ) values
1365 (cms) - (ha-m)
1366 [ 0.000 , 0.000 ]
1367 [ 0.169 , 0.185 ]
1368 [ 0.349 , 0.248 ]
1369 [ 0.482 , 0.283 ]
1370 [ 0.602 , 0.338 ]
1371 [ 0.771 , 0.359 ]
1372 [ 0.891 , 0.391 ]
1373 [ -1 , -1 ] (max twenty pts)
1374 NHYDovf=["A17OVF"]
1375 *%-----|-----|
1376 CONTINUOUS STANDHYD NHYD=["ST-4"], DT=[1] min, AREA=[0.35](ha), XI MP=[0.46],
TI MP=[0.57], DWF=[0](cms), LOSS=[1]:
1377 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1378 Pervious areas: IAPER=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1379 Impervious areas: IAIMP=[1.57](mm), SLPI=[0.5](%), LGI=[83.666](m),
MNI=[0.013], SCI=[0](min),
1380 Continuous simulation parameters:
1381 IARECper=[4](hrs), IARECimp=[4](hrs), InterEventTime=[12](hrs),
END=-1
1382 *%-----|-----|
1383 ROUTE RESERVOIR NHYDout=["ST4STR"], NHYDin=["ST-4"], RDT=[1](min),
1384 TABLE of ( OUTFLOW STORAGE ) values
1385 (cms) - (ha-m)
1386 [ 0.000 , 0.0000 ]
1387 [ 0.031 , 0.0010 ]
1388 [ 0.032 , 0.0050 ]
1389 [ -1 , -1 ] (max twenty pts)
1390 NHYDovf=["ST4OVF"]
1391 *%-----|-----|
1392 CONTINUOUS STANDHYD NHYD=["A18"], DT=[1] min, AREA=[5.30](ha), XI MP=[0.68], TI MP=[0.85],
DWF=[0](cms), LOSS=[1]:
1393 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1394 Pervious areas: IAPER=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),

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1395      Impervious areas: I A i mp=[ 1.57] (mm), SLPI=[ 0.5] (%),
1396      LGI=[ 325.576] (m), MNI=[ 0.013], SCI=[ 0] (mi n),
1397      Continuous simulation parameters:
1397      I a RE C per=[ 4] (hr s), I a RE C i mp=[ 4] (hr s), I n t e r E v e n t T i m e=[ 12] (hr s),
1397      END=- 1
1398 *%-----|-----|
1399 ROUTE RESERVOIR NHYDout=[" A18STR"], NHYDin=[" A18"], RDT=[ 1] (mi n),
1400      TABLE of ( OUTFLOW STORAGE ) values
1401      ( cms ) - ( ha - m)
1402      [ 0.000 , 0.000 ]
1403      [ 0.074 , 0.082 ]
1404      [ 0.154 , 0.109 ]
1405      [ 0.212 , 0.125 ]
1406      [ 0.265 , 0.149 ]
1407      [ 0.339 , 0.158 ]
1408      [ 0.392 , 0.172 ]
1409      [ -1 , -1 ] (max twenty pts)
1410      NHYDovf=[" A18OVF"]
1411 *%-----|-----|
1412 *ANALYSIS POINT 3 - TOTAL FLOW AT OUTLET OF STREET 4
1413 *%-----|-----|
1414 ADD HYD NHYDs um=[" PT3ST4"], NHYDs to
1414 add=[" DRAIN2" + "D2" + "A17STR" + "A17OVF" + "ST4STR" + "ST4OVF" + "A18STR" + "A18OVF"]
1415 *%-----|-----|
1416 *ROUTE FLOW through O Keefe Drain 3
1417 ROUTE CHANNEL NHYDout=[" DRAIN3"], NHYDin=[" PT3ST4"], RDT=[ 1] (mi n),
1418      CHLGTH=[ 525] {m}, CHSLOPE=[ .23] (%), FPSLOPE=[ .23] (%),
1419      SECNUM=[ 1], NSEG=[ 3]
1420      ( SEGROUGH, SEGDI ST (m))=[ 0.07, 12.50 -0.043, 17.50 0.07, 30.00] NSEG
1421      t i m e s
1421      ( DI STANCE (m), ELEVATI ON (m))=[ 0.00, 1.70]
1422      ( 2.50, 1.00)
1423      ( 12.50, 0.80)
1424      ( 14.10, 0.00)
1425      ( 15.90, 0.00)
1426      ( 17.50, 0.80)
1427      ( 27.50, 1.00)
1428      ( 30.00, 1.70)
1429 *%-----|-----|
1430 CONTINUOUS NASHYD NHYD=[" D3"], DT=[ 1] mi n, AREA=[ 2.51] (ha),
1431      DWF=[ 0] (cms), CNVC=[ 86], IA=[ 8.7] (mm), N=[ 3], TP=[ 0.73] hr s,
1432      Continuous simulation parameters:
1433      I a RE C per=[ 4] (hr s),
1434      SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1435      I n t e r E v e n t T i m e=[ 12] (hr s)
1436      Baseflow simulation parameters:
1437      BaseFlowOpt ion=[ 1] ,
1438      I n i t G W R e s V o l =[ 50] (mm), G W R e s K=[ 0.96] (mm/ day/ mm)
1439      VHydCond=[ 0.055] (mm/ hr), END=- 1
1440 *%-----|-----|
1441 CONTINUOUS STANDHYD NHYD=[" C1"], DT=[ 1] mi n, AREA=[ 3.41] (ha), XI MP=[ 0.68], TI MP=[ 0.85],
1441      DWF=[ 0] (cms), LOSS=[ 1]:
1442      Hort on: Fo=[ 76.20] (mm/ hr), Fc=[ 13.20] (mm/ hr), DCAY=[ 4.14] (/ hr),
1442      F=[ 0.00] (mm),
1443      P e r v i o u s a r e a s: I A p e r =[ 4.67] (mm), SLPP=[ 0.5] (%), LGP=[ 50] (m),
1443      MNP=[ 0.250], SCP=[ 0] (mi n),
1444      I m p e r v i o u s a r e a s: I A i m p =[ 1.57] (mm), SLPI=[ 0.5] (%),
1444      LGI=[ 261.151] (m), MNI=[ 0.013], SCI=[ 0] (mi n),
1445      Continuous simulation parameters:
1446      I a RE C per=[ 4] (hr s), I a RE C i mp=[ 4] (hr s), I n t e r E v e n t T i m e=[ 12] (hr s),
1446      END=- 1
1447 *%-----|-----|
1448 ROUTE RESERVOIR NHYDout=[" C1- STR"], NHYDin=[" C1"], RDT=[ 1] (mi n),
1449      TABLE of ( OUTFLOW STORAGE ) values
1450      ( cms ) - ( ha - m)
1451      [ 0.000 , 0.000 ]

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1452         [ 0.048 , 0.052 ]
1453         [ 0.099 , 0.070 ]
1454         [ 0.136 , 0.080 ]
1455         [ 0.170 , 0.096 ]
1456         [ 0.218 , 0.102 ]
1457         [ 0.252 , 0.111 ]
1458         [ -1 , -1 ] (max twenty pts)
1459         NHYDovf=["C1-OVF"]
1460 *%-----|-----|
1461 CONTINUOUS STANDHYD NHYD=["ST-5"], DT=[1] min, AREA=[0.45](ha), XI MP=[0.46],
TI MP=[0.57], DWF=[0](cms), LOSS=[1]:
1462 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1463 Previous areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1464 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%), LGI=[94.868](m),
MNI=[0.013], SCI=[0](min),
1465 Continuous simulation parameters:
1466 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
1467 *%-----|-----|
1468 ROUTE RESERVOIR NHYDout=["ST5STR"], NHYDin=["ST-5"], RDT=[1](min),
1469 TABLE of ( OUTFLOW STORAGE ) values
1470 (cms) - (ha-m)
1471 [ 0.000 , 0.0000 ]
1472 [ 0.040 , 0.0010 ]
1473 [ 0.041 , 0.0062 ]
1474 [ -1 , -1 ] (max twenty pts)
1475 NHYDovf=["ST5OVF"]
1476 *%-----|-----|
1477 ADD HYD NHYDs um=["ST5-E"], NHYDs t o
add=["DRAIN3"+"D3"+"C1-STR"+"C1-OVF"+"ST5STR"+"ST5OVF"]
1478 *%-----|-----|
1479 CONTINUOUS STANDHYD NHYD=["STRAND"], DT=[1](min), AREA=[7.59](ha),
1480 XI MP=[0.64], TI MP=[0.85], DWF=[0](cms), LOSS=[1]:
1481 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1482 Previous areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
1483 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%), LGI=[1230](m),
MNI=[0.013], SCI=[0](min),
1484 Continuous simulation parameters:
1485 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
1486 *%-----|-----|
1487 ROUTE RESERVOIR NHYDout=["S-POND"], NHYDin=["STRAND"], RDT=[1](min),
1488 TABLE of ( OUTFLOW STORAGE ) values
1489 (cms) - (ha-m)
1490 [ 0.000 , 0.000 ]
1491 [ 0.033 , 0.188 ]
1492 [ 0.057 , 0.253 ]
1493 [ 0.104 , 0.287 ]
1494 [ 0.160 , 0.336 ]
1495 [ 0.340 , 0.346 ]
1496 [ 0.471 , 0.360 ]
1497 [ 0.824 , 0.390 ]
1498 [ -1 , -1 ] (max twenty pts)
1499 NHYDovf=["S-OVF"]
1500 *%-----|-----|
1501 ADD HYD NHYDs um=["SSAOUT"], NHYDs t o add=["ST5-E"+"S-POND"+"S-OVF"]
1502 *%-----|-----|
1503 SAVE HYD NHYD=["SSAOUT"], # OF PCYCLES=[5], I CASEs h=[1]
1504 HYD_COMMENT=["SSAOUT"]
1505 *%-----|-----|
1506 CONTINUOUS STANDHYD NHYD=["Area-A"], DT=[1] min, AREA=[66.75](ha), XI MP=[0.64],
TI MP=[0.80], DWF=[0](cms), LOSS=[1]:

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1507      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1508      F=[ 0.00](mm),
1509      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1510      MNP=[ 0.250], SCP=[ 0](min),
1511      Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 0.5](%),
1512      LGI=[ 1155.422](m), MNI=[ 0.013], SCI=[ 0](min),
1513      Continuous simulation parameters:
1514      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1515      END=- 1
1516  *%-----|-----|
1517  SAVE HYD      NHYD=["Area-A"], # OF PCYCLES=[ 1], ICASEsh=[ 1]
1518  HYD_COMMENT=["SMMF-A Inflow"]
1519  *%-----|-----|
1520  ROUTE RESERVOIR  NHYDout=["SWMF-A"], NHYDin=["Area-A"], RDT=[ 1](min),
1521  TABLE of ( OUTFLOW STORAGE ) values
1522  (cms) - (ha-m)
1523  [ 0.000 , 0.000 ]
1524  [ 0.103 , 1.077 ]
1525  [ 0.128 , 1.749 ]
1526  [ 0.382 , 2.282 ]
1527  [ 0.703 , 2.582 ]
1528  [ 1.256 , 2.978 ]
1529  [ 1.567 , 3.202 ]
1530  [ 1.955 , 3.493 ]
1531  [ 2.100 , 3.600 ]
1532  [ -1 , -1 ] (max twenty pts)
1533  NHYDovf=["SWWAOV"]
1534  *%-----|-----|
1535  SAVE HYD      NHYD=["SWMF-A"], # OF PCYCLES=[ 1], ICASEsh=[ 1]
1536  HYD_COMMENT=["SMMF-A Outflow"]
1537  *%-----|-----|
1538  *ANALYSIS POINT 4 - TOTAL FLOW AT OUTLET OF STREET 5
1539  *%-----|-----|
1540  ADD HYD      NHYDsum=["PT4ST5"], NHYDsto add=["SSAOUT"+"SWMF-A"+"SWWAOV"]
1541  *%-----|-----|
1542  CONTINUOUS STANDHYD NHYD=["C6"], DT=[ 1]min, AREA=[ 1.87](ha), XI MP=[ 0.68], TI MP=[ 0.85],
1543  DWF=[ 0](cms), LOSS=[ 1]:
1544      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1545      F=[ 0.00](mm),
1546      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1547      MNP=[ 0.250], SCP=[ 0](min),
1548      Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 0.5](%),
1549      LGI=[ 193.391](m), MNI=[ 0.013], SCI=[ 0](min),
1550      Continuous simulation parameters:
1551      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1552      END=- 1
1553  *%-----|-----|
1554  ROUTE RESERVOIR  NHYDout=["C6-STR"], NHYDin=["C6"], RDT=[ 1](min),
1555  TABLE of ( OUTFLOW STORAGE ) values
1556  (cms) - (ha-m)
1557  [ 0.000 , 0.000 ]
1558  [ 0.026 , 0.029 ]
1559  [ 0.054 , 0.038 ]
1560  [ 0.075 , 0.044 ]
1561  [ 0.093 , 0.052 ]
1562  [ 0.120 , 0.056 ]
1563  [ 0.138 , 0.061 ]
1564  [ -1 , -1 ] (max twenty pts)
1565  NHYDovf=["C6-OVF"]
1566  *%-----|-----|
1567  CONTINUOUS STANDHYD NHYD=["C7"], DT=[ 1]min, AREA=[ 1.62](ha), XI MP=[ 0.68], TI MP=[ 0.85],
1568  DWF=[ 0](cms), LOSS=[ 1]:
1569      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1570      F=[ 0.00](mm),
1571      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1572      MNP=[ 0.250], SCP=[ 0](min),

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1561      Impervious areas: I Aimp=[ 1.57](mm), SLPI=[ 0.5](%),
1562      LGL=[ 180.000](m), MNI=[ 0.013], SCI=[ 0](min),
1563      Continuous simulation parameters:
1564      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1565      END=- 1
1566 *%-----|-----|
1567 ROUTE RESERVOIR NHYDout=["C7-STR"], NHYDin=["C7"], RDT=[ 1](min),
1568      TABLE of ( OUTFLOW STORAGE ) values
1569      (cms) - (ha-m)
1570      [ 0.000 , 0.000 ]
1571      [ 0.023 , 0.025 ]
1572      [ 0.047 , 0.033 ]
1573      [ 0.065 , 0.038 ]
1574      [ 0.081 , 0.045 ]
1575      [ 0.104 , 0.048 ]
1576      [ 0.120 , 0.053 ]
1577      [ -1 , -1 ] (max twenty pts)
1578      NHYDovf=["C7-OVF"]
1579 *%-----|-----|
1580 CONTINUOUS STANDHYD NHYD=["ST-6"], DT=[ 1]min, AREA=[ 0.41](ha), XI MP=[ 0.46], TI MP=[ 0.57],
1581      DWF=[ 0](cms), LOSS=[ 1]:
1582      Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
1583      F=[ 0.00](mm),
1584      Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1585      MNP=[ 0.250], SCP=[ 0](min),
1586      Impervious areas: I Aimp=[ 1.57](mm), SLPI=[ 0.5](%), LGL=[ 90.554](m),
1587      MNI=[ 0.013], SCI=[ 0](min),
1588      Continuous simulation parameters:
1589      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1590      END=- 1
1591 *%-----|-----|
1592 ROUTE RESERVOIR NHYDout=["ST6STR"], NHYDin=["ST-6"], RDT=[ 1](min),
1593      TABLE of ( OUTFLOW STORAGE ) values
1594      (cms) - (ha-m)
1595      [ 0.000 , 0.0000 ]
1596      [ 0.036 , 0.0010 ]
1597      [ 0.037 , 0.0058 ]
1598      [ -1 , -1 ] (max twenty pts)
1599      NHYDovf=["ST6OVF"]
1600 *%-----|-----|
1601 *ANALYSIS POINT 5 - TOTAL FLOW AT OUTLET OF STREET 6
1602 *%-----|-----|
1603 ADD HYD NHYDs um=["PT5ST6"], NHYDs to
1604      add=["PT4ST5"+"C6-STR"+"C6-OVF"+"C7-STR"+"C7-OVF"+"ST6STR"+"ST6OVF"]
1605 *%-----|-----|
1606 *ROUTE FLOW through O Keefe Drain 4
1607 ROUTE CHANNEL NHYDout=["DRAIN4"], NHYDin=["PT5ST6"], RDT=[ 1](min),
1608      CHLGTH=[ 324]{m}, CHSLOPE=[.10](%), FPSLOPE=[.10](%),
1609      SECNUM=[ 1], NSEG=[ 3]
1610      ( SEGROUGH, SEGDIST (m))=[ 0.07, 12.00 -0.043, 18.00 0.07, 30.00] NSEG
1611      times
1612      ( DISTANCE (m), ELEVATION (m))=[ 0.00, 2.00]
1613      ( 2.00, 1.20)
1614      ( 12.00, 1.00)
1615      ( 14.00, 0.00)
1616      ( 16.00, 0.00)
1617      ( 18.00, 1.00)
1618      ( 28.00, 1.20)
1619      ( 30.00, 2.00)
1620 *%-----|-----|
1621 CONTINUOUS NASHYD NHYD=["D4"], DT=[ 1]min, AREA=[ 1.73](ha), DWF=[ 0](cms), CNVC=[ 88],
1622      IA=[ 8.4](mm),
1623      N=[ 3], TP=[ 0.60]hrs,
1624      Continuous simulation parameters:
1625      IARECper=[ 4](hrs),
1626      SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),

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1617 InterEventTime=[ 12]( hrs)
1618 Baseflow simulation parameters:
1619 BaseFlowOption=[ 1] ,
1620 InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
1621 VHydCond=[ 0.055]( mm/ hr) , END=- 1
1622 *%-----|-----|
1623 CONTINUOUS STANDHYD NHYD=[ "Area- B" ] , DT=[ 1] mi n , AREA=[ 24.04]( ha) , XI MP=[ 0.62] ,
TI MP=[ 0.77] , DWF=[ 0]( cms) , LOSS=[ 1] :
1624 Horton: Fo=[ 76.20]( mm/ hr) , Fc=[ 13.20]( mm/ hr) , DCAY=[ 4.14]( / hr) ,
F=[ 0.00]( mm) ,
1625 Pervious areas: IAper=[ 4.67]( mm) , SLPP=[ 1.4]( %) , LGP=[ 50]( m) ,
MNP=[ 0.250] , SCP=[ 0]( mi n) ,
1626 Impervious areas: IAimp=[ 1.57]( mm) , SLPI=[ 1.4]( %) ,
LGI=[ 693.397]( m) , MNI=[ 0.013] , SCI=[ 0]( mi n) ,
1627 Continuous simulation parameters:
1628 IaRECper=[ 4]( hrs) , IaRECI mp=[ 4]( hrs) , InterEventTime=[ 12]( hrs) ,
END=- 1
1629 *%-----|-----|
1630 ROUTE RESERVOIR NHYDout=[ "SWMF- B" ] , NHYDin=[ "Area- B" ] , RDT=[ 1]( mi n) ,
1631 TABLE of ( OUTFLOW STORAGE ) values
1632 ( cms) - ( ha- m)
1633 [ 0.000 , 0.000 ]
1634 [ 0.025 , 0.090 ]
1635 [ 0.175 , 0.510 ]
1636 [ 0.350 , 0.710 ]
1637 [ 0.495 , 0.820 ]
1638 [ 0.648 , 0.980 ]
1639 [ 0.965 , 1.045 ]
1640 [ 1.072 , 1.140 ]
1641 [ -1 , -1 ] (max twenty pts)
1642 NHYDovf=[ "SWMBOVF" ]
1643 *%-----|-----|
1644 ADD HYD NHYDs um=[ "D4- EX" ] , NHYDs to add=[ "DRAIN4"+"D4"+"SWMF- B"+"SWMBOVF" ]
1645 *%-----|-----|
1646 *ROUTE FLOW THROUGH O'Keefe Drain 5
1647 * JFSA: Nov. 2020, added en points to close X-Section
1648 ROUTE CHANNEL NHYDout=[ "DRAIN5" ] , NHYDin=[ "D4- EX" ] , RDT=[ 1]( mi n) ,
1649 CHLGTH=[ 413.0]( m) , CHSLOPE=[ 0.16]( %) , FPSLOPE=[ 0.16]( %) ,
1650 SECNUM=[ 1] , NSEG=[ 3]
1651 ( SEGROUGH, SEGDIST ( m) )=[ 0.043, 12.29 -0.033, 17.97
1652 0.043, 32.84] NSEG times
1653 ( DISTANCE ( m) , ELEVATION ( m) )=(-0.01, 2.50)
1654 [ 0.00, 1.41]
1655 [ 6.13, 0.97]
1656 [ 12.29, 0.89]
1657 [ 15.71, 0.00]
1658 [ 17.97, 0.39]
1659 [ 23.04, 0.35]
1660 [ 32.83, 0.96]
1661 ( 32.84, 2.50)
1662 *%-----|-----|
1663 CONTINUOUS NASHYD NHYD=[ "D5" ] , DT=[ 1] mi n , AREA=[ 1.90]( ha) ,
1664 DWF=[ 0]( cms) , CNVC=[ 86] , IA=[ 8.7]( mm) , N=[ 3] , TP=[ 0.69] hrs ,
1665 Continuous simulation parameters:
1666 IaRECper=[ 4]( hrs) ,
1667 SMN=[ -1]( mm) , SMAX=[ -1]( mm) , SK=[ 0.010]/( mm) ,
1668 InterEventTime=[ 12]( hrs)
1669 Baseflow simulation parameters:
1670 BaseFlowOption=[ 1] ,
1671 InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
1672 VHydCond=[ 0.055]( mm/ hr) , END=- 1
1673 *%-----|-----|
1674 *EXTERNAL FLOWS SOUTHEAST OF THE SITE NORTH OF McKENNA CASEY DR.
1675 CONTINUOUS NASHYD NHYD=[ "O-13SDF" ] , DT=[ 1] mi n , AREA=[ 9.74]( ha) ,
1676 DWF=[ 0]( cms) , CNVC=[ 81] , IA=[ 4.0]( mm) , N=[ 3] , TP=[ .43] hrs ,
1677 Continuous simulation parameters:

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1678 IaRECper=[ 4] (hrs),
1679 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
1680 InterEventTime=[ 12] (hrs)
1681 Baseflow simulation parameters:
1682 BaseFlowOption=[ 1] ,
1683 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/day/mm)
1684 VHydCond=[ 0.055] (mm/hr), END=-1
1685 *%-----|-----
1686 *SNOW DISPOSAL FACILITY
1687 *PARAMETERS BASED ON ROBINSON 2006 MODEL
1688 ROUTE RESERVOIR NHYDout=["SDF"], NHYDin=["O-13SDF"], RDT=[ 1] (min),
1689 TABLE of ( OUTFLOW STORAGE ) values
1690 (cms) - (ha-m)
1691 [0.000, 0.000]
1692 [0.150, 0.600]
1693 (0.200, 1.500)
1694 [ -1 , -1 ] (max twenty pts)
1695 NHYDovf=["OVFSDF"]
1696 *%-----|-----
1697 *ANALYSIS POINT 6 - McKenna Casey Dr.
1698 *%-----|-----
1699 ADD HYD NHYDs um=["PT6MC"], NHYDs to add=["DRAIN5"+"D5"+"SDF"]
1700 *%-----|-----
1701 CONTINUOUS NASHYD NHYD=["O-15"], DT=[ 1] min, AREA=[ 10.67] (ha),
1702 DWF=[ 0] (cms), CNVC=[ 82], IA=[ 7.5] (mm), N=[ 3], TP=[ 0.30] hrs,
1703 Continuous simulation parameters:
1704 IaRECper=[ 4] (hrs),
1705 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
1706 InterEventTime=[ 12] (hrs)
1707 Baseflow simulation parameters:
1708 BaseFlowOption=[ 1] ,
1709 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/day/mm)
1710 VHydCond=[ 0.055] (mm/hr), END=-1
1711 *%-----|-----
1712 *TOTAL FLOW NORTH OF MCKENNA CASEY DR.
1713 ADD HYD NHYDs um=["MC"], NHYDs to add=["PT6MC"+"O-15"]
1714 *%-----|-----
1715 *ROUTE FLOW THROUGH AREA O-14
1716 * JFSA: Nov. 2020, added end points to close X-section
1717 ROUTE CHANNEL NHYDout=["O-14Ch"], NHYDin=["MC"], RDT=[ 1] (min),
1718 CHLGTH=[ 845.3] (m), CHSLOPE=[ 0.10] (%), FPSLOPE=[ 0.10] (%),
1719 SECNUM=[ 1], NSEG=[ 3]
1720 ( SEGROUGH, SEGDIST (m))=[ 0.06, 15.00 -0.033, 18.04 0.06, 31.85] NSEG
times
1721 ( DISTANCE (m), ELEVATION (m))=[ -0.01, 2.5
1722 (0.00, 1.53]
1723 (5.56, 1.47)
1724 (9.21, 1.45)
1725 (12.45, 1.53)
1726 (13.70, 1.50)
1727 (15.00, 0.69)
1728 (15.34, 0.00)
1729 (16.51, 0.05)
1730 (17.30, 0.17)
1731 (18.04, 0.74)
1732 (19.29, 1.32)
1733 (22.73, 1.47)
1734 (31.84, 1.41)
1735 (31.85, 2.50)
1736 *%-----|-----
1737 *% -Change O-14 from NASHYD to STANDHYD, name it "S-1-Okeefe" and add it to S-1
subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
1738 *% -JFSA 2021-02-16, add detailed subcatchment drainage area for each subcatchment
in Corrigan sub-catchment. After adding part of O-14 to S_1 sub-catchment so O-14
becomes 5 ha instead of 30.02 ha and TP becomes 0.133 (5*0.8/30.02) instead of 0.8
1739 CONTINUOUS NASHYD NHYD=["O-14"], DT=[ 1] min, AREA=[ 5] (ha),

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1740 DWF=[ 0](cms), CN C=[ 82], IA=[ 7.5](mm), N=[ 3], TP=[ 0.133]hrs,
1741 Continuous simulation parameters:
1742 IaRECper=[ 4](hrs),
1743 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1744 InterEventTime=[ 12](hrs)
1745 Baseflow simulation parameters:
1746 BaseFlowOption=[ 1],
1747 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
1748 VHydCond=[ 0.055](mm/hr), END=- 1
1749 *
1750 *%-----|-----|
1751 *ANALYSIS POINT 7 - JOCK RIVER
1752 * 2020-12-01 To Foster Drain
1753 * 2020-12-01 replace ("PT7JR") by ("OKEEFE")
1754 *%-----|-----|
1755 ADD HYD NHYDsum=[ "OKEEFE"], NHYDs to add=[ "O-14Ch"+"O-14"]
1756 *%-----|-----|
1757 *CONTINUOUS STANDHYD NHYD=[ "OKEEFE"], DT=[ 1](min), AREA=[ 448](ha),
1758 * XI MP=[ 0.65], TI MP=[ 0.65], DWF=[ 0](cms), LOSS=[ 2],
1759 * SCS curve number CN=[ 77],
1760 * Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 0.5](%),
1761 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
1762 * Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 0.5](%),
1763 * LGI=[ 1728](m), MNI=[ 0.013], SCI=[ 0](min),
1764 * Continuous simulation parameters:
1765 * IaRECper=[ 4](hrs), IaRECimp=[ 4](hrs),
1766 * SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1767 * InterEventTime=[ 18](hrs), END=- 1
1768 *#*****
1769 *# Okeefe Pond
1770 *# - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1771 *# and a ratio of the catchment area to the West Clarke pond rating curve
1772 *# from the MS for the next coordinates
1773 *#*****
1774 *ROUTE RESERVOIR NHYDout=[ "P_OKE"], NHYDin=[ "OKEEFE"],
1775 * RDT=[ 1](min),
1776 * TABLE of ( OUTFLOW STORAGE ) values
1777 * (cms) - (ha-m)
1778 * [ 0.0 , 0.0]
1779 * [ 14.13 , 13.0]
1780 * [ -1 , -1 ] (maximum one hundred pairs of points)
1781 * NHYDovf=[ "ok-OVF"],
1782 *%-----|-----|
1783 * -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
1784 * moved to drain before station 6215 on Jock River
1785 *CONTINUOUS STANDHYD NHYD=[ "S-1-D2"], DT=[ 1](min), AREA=[ 18.67](ha), XI MP=[ 0.65],
1786 * TI MP=[ 0.65], DWF=[ 0](cms),
1787 * LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
1788 * IAper=[ 4.67](mm), SLPP=[ 2.0](%),
1789 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min), Impervious surfaces:
1790 * IAimp=[ 1.57](mm), SLPI=[ 0.75](%),
1791 * LGI=[ 352.798](m), MNI=[ 0.013], SCI=[ 0](min),
1792 * IaRECper=[ 4](hrs), IaRECimp=[ 4](hrs),
1793 * SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1794 * InterEventTime=[ 12](hrs), END=- 1
1795 *%-----|-----|
1796 CONTINUOUS NASHYD NHYD=[ "S-1-D2"], DT=[ 1]min, AREA=[ 18.67](ha),
1797 DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
1798 N=[ 3], TP=[ 1.120]hrs,
1799 Continuous simulation parameters:
1800 IaRECper=[ 4](hrs),
SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
InterEventTime=[ 12](hrs)
Baseflow simulation parameters:

```

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1801 BaseFlowOpt ion=[ 1] ,
1802 Ini t GWRes Vol =[ 50] ( mm) , GWRes K=[ 0. 96] ( mm/ day/ mm)
1803 VHydCond=[ 0. 055] ( mm/ hr) , END=- 1
1804 *%-----|-----|
1805 *COMPUTE DUALHYD NHYDi n=[ " S- 1- D2" ] , CI NLET=[ 2. 062] ( cms) , NI NLET=[ 1] ,
1806 * Mj NHYD=[ " S- 1- D2J" ]
1807 * M nNHYD=[ " S- 1- D2N" ]
1808 * TM STO=[ 9999999] ( cu- m)
1809 *%-----|-----|
1810 *ADD HYD NHYDs um=[ " S- 1- D2S" ] , NHYDs t o add=[ " S- 1- D2J" +" S- 1- D2N" ]
1811 *%-----|-----|
1812 *ROUTE RESERVOIR NHYDout =[ " S- 1- D2R" ] , NHYDi n=[ " S- 1- D2S" ] ,
1813 * RDT=[ 1] ( mi n) ,
1814 * TABLE of ( OUTFLOW STORAGE ) values
1815 * ( cms) - ( ha- m)
1816 * [ 0. 0 , 0. 0 ]
1817 * [ 0. 2231, 0. 7445 ]
1818 * [ -1 , -1 ] (max twenty pts)
1819 * NHYDovf=[ " S- 1- D2Rovf" ]
1820 *%-----|-----|
1821 *CONTINUOUS STANDHYD NHYD=[ " S- 1- D3" ] , DT=[ 1] ( mi n) , AREA=[ 6. 79] ( ha) , XI MP=[ 0. 65] ,
1822 TI MP=[ 0. 65] , DWF=[ 0] ( cms) ,
1823 * LOSS=[ 2] , SCS curve number CN=[ 75] , Pervious surfaces:
1824 I Aper =[ 4. 67] ( mm) , SLPP=[ 2. 0] ( % ) ,
1825 * LGP=[ 40] ( m) , MNP=[ 0. 25] , SCP=[ 0] ( mi n) , Impervious surfaces:
1826 I Ai mp=[ 1. 57] ( mm) , SLPI =[ 0. 75] ( % ) ,
1827 * LGI =[ 212. 760] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1828 * Continuous simulation parameters:
1829 I aRECper =[ 4] ( hr s) , I aRECI mp=[ 4] ( hr s) ,
1830 * SM N=[ -1] ( mm) , SMAX=[ -1] ( mm) , SK=[ 0. 010] / ( mm) ,
1831 * InterEvent Ti me=[ 12] ( hr s) , END=- 1
1832 *%-----|-----|
1833 CONTINUOUS NASHYD NHYD=[ " S- 1- D3" ] , DT=[ 1] mi n , AREA=[ 6. 79] ( ha) ,
1834 DWF=[ 0] ( cms) , CN C=[ 77] , I A=[ 4. 67] ( mm) ,
1835 N=[ 3] , TP=[ 1. 281] hr s ,
1836 * Continuous simulation parameters:
1837 I aRECper =[ 4] ( hr s) ,
1838 * SM N=[ -1] ( mm) , SMAX=[ -1] ( mm) , SK=[ 0. 010] / ( mm) ,
1839 * InterEvent Ti me=[ 12] ( hr s)
1840 * Baseflow simulation parameters:
1841 BaseFlowOpt ion=[ 1] ,
1842 Ini t GWRes Vol =[ 50] ( mm) , GWRes K=[ 0. 96] ( mm/ day/ mm)
1843 VHydCond=[ 0. 055] ( mm/ hr) , END=- 1
1844 *%-----|-----|
1845 *COMPUTE DUALHYD NHYDi n=[ " S- 1- D3" ] , CI NLET=[ 0. 719] ( cms) , NI NLET=[ 1] ,
1846 * Mj NHYD=[ " S- 1- D3J" ]
1847 * M nNHYD=[ " S- 1- D3N" ]
1848 * TM STO=[ 9999999] ( cu- m)
1849 *%-----|-----|
1850 *ADD HYD NHYDs um=[ " S- 1- D3S" ] , NHYDs t o add=[ " S- 1- D3J" +" S- 1- D3N" ]
1851 *%-----|-----|
1852 *ROUTE RESERVOIR NHYDout =[ " S- 1- D3R" ] , NHYDi n=[ " S- 1- D3S" ] ,
1853 * RDT=[ 1] ( mi n) ,
1854 * TABLE of ( OUTFLOW STORAGE ) values
1855 * ( cms) - ( ha- m)
1856 * [ 0. 0 , 0. 0 ]
1857 * [ 0. 0811, 0. 2708 ]
1858 * [ -1 , -1 ] (max twenty pts)
1859 * NHYDovf=[ " S- 1- D3Rovf" ]
1860 *%-----|-----|
1861 ADD HYD NHYDs um=[ " SN_ OK" ] , NHYDs t o add=[ " N_ OK" +" OKEEFE" +" S- 1- D2" +" S- 1- D3" ]
1862 *%-----|-----|
1863 SAVE HYD NHYD=[ " SN_ OK" ] , # OF PCYCLES=[ -1] , I CASEs h=[ 1]
1864 *%-----|-----|
1865 *# HYD_ COMMENT=[ "Tot al Fl ows at Okeefe Dr ai n" ]

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1864  *# Hydrograph from Node Okeefe routed to Node at Foster Drain
1865  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6215
1866  *#
1867  ROUTE CHANNEL      NHYDout=[ "N_FO" ] , NHYDin=[ "SN_OK" ] ,
1868                    RDT=[ 1 ] ( min ),
1869                    CHLGTH=[ 1183 ] ( m ) ,    CHSLOPE=[ 0.0761 ] ( % ) ,
1870                                                    FPSLOPE=[ 0.0761 ] ( % ) ,
1871                    SECNUM=[ 1.0 ] ,          NSEG=[ 3 ]
1872                    ( SEGROUGH, SEGDIST ( m ) ) =
1873                    [ 0.050, -33.89
1874                    -0.035, 31.59
1875                    0.050, 34.41 ] NSEG times
1876                    ( DISTANCE ( m ) , ELEVATION ( m ) ) =
1877                    [ -794.18, 91.00 ]
1878                    [ -775.41, 91.50 ]
1879                    [ -702.63, 91.50 ]
1880                    [ -546.19, 91.50 ]
1881                    [ -529.54, 91.50 ]
1882                    [ -323.44, 91.00 ]
1883                    [ -320.71, 91.00 ]
1884                    [ -183.59, 91.00 ]
1885                    [ -182.54, 90.50 ]
1886                    [ -181.36, 90.00 ]
1887                    [ -177.37, 90.00 ]
1888                    [ -87.70, 90.00 ]
1889                    [ -33.89, 90.00 ]
1890                    [ -18.52, 86.88 ]
1891                    [ 0.00, 85.20 ]
1892                    [ 16.20, 86.83 ]
1893                    [ 31.59, 90.00 ]
1894                    [ 33.03, 90.50 ]
1895                    [ 34.41, 91.00 ]
1896  *%-----|-----|
1897  *#*****
1898  *# Catchment FOSTER
1899  *# - To Foster ditch (north of the Jock)
1900  *# - Partially developed (medium density); remaining agricultural
1901  *# - 2020-12-01 JFSA Foster area is 332 as per Foster SWF Environmental Study
1902  *# - 2020-12-01 decrease Foster drainage area from (373 HA) to (307.98 HA) after
1903  *# - 2021-02-12 update Foster area to 325.44 ha as measured from QGIS
1904  *#*****
1905  CONTINUOUS STANDHYD NHYD=[ "FOSTER" ] , DT=[ 1 ] min , AREA=[ 325.44 ] ( ha ) ,
1906                    XI MP=[ 0.55 ] , TI MP=[ 0.55 ] , DWF=[ 0 ] ( cms ) , LOSS=[ 2 ] ,
1907                    SCS curve number CN=[ 74 ] ,
1908                    Pervious surfaces: I A per=[ 4.67 ] ( mm ) , SLPP=[ 0.5 ] ( % ) ,
1909                    LGP=[ 40 ] ( m ) , MNP=[ 0.25 ] , SCP=[ 0 ] ( min ) ,
1910                    Impervious surfaces: I A i mp=[ 1.57 ] ( mm ) , SLPI=[ 0.5 ] ( % ) ,
1911                    LGI=[ 1472.956 ] ( m ) , MNI=[ 0.013 ] , SCI=[ 0 ] ( min ) ,
1912                    Continuous simulation parameters:
1913                    I a REC per=[ 4 ] ( hrs ) , I a REC i mp=[ 4 ] ( hrs ) ,
1914                    SM N=[ -1 ] ( mm ) , SMAX=[ -1 ] ( mm ) , SK=[ 0.010 ] / ( mm ) ,
1915                    Inter Event Time=[ 18 ] ( hrs ) , END=- 1
1916  *#*****
1917  *# Foster Pond
1918  *# - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1919  *# and a ratio of the catchment area to the West Clarke pond rating curve
1920  *# from the MSS for the next coordinates
1921  *#*****
1922  ROUTE RESERVOIR    NHYDout=[ "P_FOS" ] , NHYDin=[ "FOSTER" ] ,
1923                    RDT=[ 1 ] ( min ) ,
1924                    TABLE of ( OUTFLOW STORAGE ) values
1925                    ( cms ) - ( ha-m )
1926                    [ 0.0 , 0.0 ]

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1927          [ 10.34 , 10]
1928          [ -1 , -1 ] (max twenty pts)
1929          NHYDovf=[ "FO- OVF" ]
1930 *%-----|-----|
1931 ADD HYD          NHYDsum=[ "FOSTER- OUT" ], NHYDs to add=[ "P_FOS"+"FO- OVF" ]
1932 *%-----|-----|
1933 *#*****|
1934 * -Brazeau area from P 1800-19 =[71.751], change to 63.59 ha based on GIS measurements
1935 * -JFSA, 2021-01-19 update "W_CLAR_BRAZ" to 73.29 ha based on GIS measurements
1936 * -JFSA, 2021-01-22 Brazeau ("MS_P10"+"P10-OVF")brazeau pond discharges directly
to the jock river through a road side ditch on the west side of Borrisokane road
(station 6016)
1937 CONTINUOUS STANDHYD NHYD=[ "W_CLAR_BRAZ" ], DT=[ 1]mi n, AREA=[ 73.29](ha),
1938 XI MP=[ 0.6], TI MP=[ 0.65], DWF=[ 0](cms), LOSS=[ 2],
1939 SCS curve number CN=[ 77],
1940 Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
1941 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
1942 Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 0.5](%),
1943 LGI=[ 699.00](m), MNI=[ 0.013], SCI=[ 0](mi n),
1944 Continuous simulation parameters:
1945 I a REC per=[ 4](hr s), I a REC i mp=[ 4](hr s),
1946 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1947 I nter Event Ti me=[ 18](hr s), END=- 1
1948 *%-----|-----|
1949 * 2020-12-01 correct pond curve values
1950 ROUTE RESERVOIR NHYDout =["MS_P10"], NHYDi n=["W_CLAR_BRAZ"],
1951 RDT=[ 1](mi n),
1952 TABLE of ( OUTFLOW STORAGE ) values
1953 (cms) - (ha-m)
1954 [ 0.0 , 0.0 ]
1955 [ 0.068 , 0.001 ]
1956 [ 0.271 , 0.022 ]
1957 [ 0.379 , 0.051 ]
1958 [ 0.48 , 0.091 ]
1959 [ 0.853 , 0.341 ]
1960 [ 1.005 , 0.61 ]
1961 [ 1.128 , 1.231 ]
1962 [ 1.155 , 1.592 ]
1963 [ 1.194 , 1.876 ]
1964 [ 1.2 , 1.921 ]
1965 [ 1.259 , 2.369 ]
1966 [ 1.3 , 2.665 ]
1967 [ 1.349 , 2.813 ]
1968 [ -1 , -1 ] (max twenty pts)
1969 NHYDovf=[ "P10- OVF" ]
1970 *%-----|-----|
1971 * -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
1972 *CONTINUOUS STANDHYD NHYD=[ "S-1-FO-D2" ], DT=[ 1]mi n, AREA=[ 4.94](ha),
1973 * XI MP=[ 0.55], TI MP=[ 0.55], DWF=[ 0](cms), LOSS=[ 2],
1974 * SCS curve number CN=[ 74],
1975 * Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 0.5](%),
1976 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
1977 * Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 0.5](%),
1978 * LGI=[ 181.475](m), MNI=[ 0.013], SCI=[ 0](mi n),
1979 * Continuous simulation parameters:
1980 * I a REC per=[ 4](hr s), I a REC i mp=[ 4](hr s),
1981 * SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1982 * I nter Event Ti me=[ 18](hr s), END=- 1
1983 *%-----|-----|
1984 CONTINUOUS NASHYD NHYD=[ "S-1-FO-D2" ], DT=[ 1]mi n, AREA=[ 4.94](ha),
1985 DWF=[ 0](cms), CN C=[ 77], I A=[ 4.67](mm),
1986 N=[ 3], TP=[ 1.10]hr s,
1987 Continuous simulation parameters:
1988 I a REC per=[ 4](hr s),
1989 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),

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1990 InterEventTime=[ 12](hrs)
1991 Baseflow simulation parameters:
1992 BaseFlowOption=[ 1] ,
1993 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
1994 VHydCond=[ 0.055](mm/hr), END=- 1
1995 *%-----|-----|
1996 *COMPUTE DUALHYD NHYDin=["S-1-FO-D2"], CINLET=[ 0.508](cms), NINLET=[ 1],
1997 * MjNHYD=["S-1-FO-D2J"]
1998 * MnNHYD=["S-1-FO-D2N"]
1999 * TMSTO=[ 9999999](cu-m)
2000 *%-----|-----|
2001 *ADD HYD NHYDsum=["S-1-FO-D2S"], NHYDsto add=["S-1-FO-D2J"+"S-1-FO-D2N"]
2002 *%-----|-----|
2003 *ROUTE RESERVOIR NHYDout=["S-1-FO-D2R"], NHYDin=["S-1-FO-D2S"],
2004 * RDT=[ 1](min),
2005 * TABLE of ( OUTFLOW STORAGE ) values
2007 * [ 0.0 , 0.0 ]
2008 * [ 0.0590, 0.1970 ]
2009 * [ -1 , -1 ] (max twenty pts)
2010 * NHYDovf=["S-1FOD2ovf"]
2011 *%-----|-----|
2012 ADD HYD NHYDsum=["980"], NHYDsto add=["FOSTER-OUT"+"S-1-FO-D2"]
2013 *%-----|-----|
2014 SAVE HYD NHYD=["980"], # OF PCYCLES=[ -1], ICASEsh=[ 1]
2015 HYD_COMMENT=["Total Flows at Station 980 on Foster Drain"]
2016 *%-----|-----|
2017 *#
2018 *# Hydrograph from Node Foster SWM (Station 980) to Node at station 520
2019 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 980
2020 *#
2021 ROUTE CHANNEL NHYDout=["980-out"], NHYDin=["980"],
2022 RDT=[ 1](min),
2023 CHLGTH=[ 460](m), CHSLOPE=[ 0.04348](%),
2024 FPSLOPE=[ 0.04348](%),
2025 SECNUM=[ 1.0], NSEG=[ 3]
2026 ( SEGROUGH, SEGDI ST (m)) =
2027 [ 0.050, 45.90
2028 -0.035, 53.30
2029 0.050, 100] NSEG times
2030 ( DISTANCE (m), ELEVATION (m)) =
2031 [ 0, 91.75 ]
2032 [ 42.4, 92.18 ]
2033 [ 43.5, 92.16 ]
2034 [ 44.1, 92.1 ]
2035 [ 44.6, 92 ]
2036 [ 44.8, 91.86 ]
2037 [ 45.9, 91.04 ]
2038 [ 46.4, 90.65 ]
2039 [ 46.8, 90.36 ]
2040 [ 47.9, 90.32 ]
2041 [ 48.7, 90.35 ]
2042 [ 50.7, 90.33 ]
2043 [ 52.2, 90.38 ]
2044 [ 52.5, 90.59 ]
2045 [ 53.3, 91.28 ]
2046 [ 54, 91.83 ]
2047 [ 54.3, 92 ]
2048 [ 54.8, 92.08 ]
2049 [ 55.4, 92.12 ]
2050 [ 100, 91.84 ]
2051 *%-----|-----|
2052 * -JFSA 2021-02-26 "S-1-FO-D1" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2053 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D1"], DT=[ 1]min, AREA=[ 5.11](ha),
2054 * XI MP=[ 0.65], TI MP=[ 0.65], DWF=[ 0](cms), LOSS=[ 2],

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2055 *          SCS curve number CN=[ 74],
2056 *          Pervious surfaces: I Aper=[ 4.67] (mm), SLPP=[ 0.5] (%),
2057 *          LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (min),
2058 *          Impervious surfaces: I Aimp=[ 1.57] (mm), SLPI=[ 0.5] (%),
2059 *          LGI=[ 184.572] (m), MNI=[ 0.013], SCI=[ 0] (min),
2060 *          Continuous simulation parameters:
2061 *          I aRECper=[ 4] (hrs), I aRECimp=[ 4] (hrs),
2062 *          SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
2063 *          InterEventTime=[ 18] (hrs), END=- 1
2064 *%-----|-----|
2065 CONTINUOUS NASHYD  NHYD=[ "S-1-FO-DI" ], DT=[ 1] min, AREA=[ 5.11] (ha),
2066 DWF=[ 0] (cms), CNC=[ 77], I A=[ 4.67] (mm),
2067 N=[ 3], TP=[ 1.10] hrs,
2068 Continuous simulation parameters:
2069 I aRECper=[ 4] (hrs),
2070 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
2071 InterEventTime=[ 12] (hrs)
2072 Baseflow simulation parameters:
2073 BaseFlowOption=[ 1],
2074 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm3/day/mm)
2075 VHydCond=[ 0.055] (mm/hr), END=- 1
2076 *%-----|-----|
2077 *COMPUTE DUALHYD  NHYDin=[ "S-1-FO-DI" ], CINLET=[ 0.605] (cms), NINLET=[ 1],
2078 *          Maj NHYD=[ "S-1-FO-DIJ" ]
2079 *          Min NHYD=[ "S-1-FO-DIN" ]
2080 *          TMSTO=[ 9999999] (cu-m)
2081 *%-----|-----|
2082 *ADD HYD          NHYDsum=[ "S-1-FO-DIS" ], NHYDto add=[ "S-1-FO-DIN"+"S-1-FO-DIJ" ]
2083 *%-----|-----|
2084 *ROUTE RESERVOIR NHYDout=[ "S-1-FO-DIR" ], NHYDin=[ "S-1-FO-DIS" ],
2085 *          RDT=[ 1] (min),
2086 *          TABLE of ( OUTFLOW STORAGE ) values
2087 *          ( cms ) - ( ha-m)
2088 *          [ 0.0      , 0.0 ]
2089 *          [ 0.0611, 0.2038 ]
2090 *          [      -1 ,      -1 ] (max twenty pts)
2091 *          NHYDovf=[ "S-1FODlovf" ]
2092 *%-----|-----|
2093 ADD HYD          NHYDsum=[ "520" ], NHYDto add=[ "980-out"+"S-1-FO-DI" ]
2094 *%-----|-----|
2095 SAVE HYD        NHYD=[ "520" ], # OF PCYCLES=[ -1], I CASEsh=[ 1]
2096 HYD_COMMENT=[ "Total Flows at Sation 520 on Foster Drain" ]
2097 *%-----|-----|
2098 *# Hydrograph from Node at Station 520 (Foster Drain) to Node at station 6016 (Jock
River)
2099 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 520
2100 *#
2101 ROUTE CHANNEL   NHYDout=[ "520-out" ], NHYDin=[ "520" ],
2102 RDT=[ 1] (min),
2103 CHLGTH=[ 860] (m), CHSLOPE=[ 0.5872] (%),
2104 FPSLOPE=[ 0.5872] (%),
2105 SECNUM=[ 1.0], NSEG=[ 3]
2106 ( SEGROUGH, SEGDI ST (m))=
2107 [ 0.050, 45.90
2108 -0.035, 54.3
2109 0.050, 100.1097] NSEG times
2110 ( DI STANCE (m), ELEVATI ON (m))=
2111 [ 0, 91.26 ]
2112 [ 44.9, 91.46 ]
2113 [ 45.1, 91.37 ]
2114 [ 45.9, 90.84 ]
2115 [ 47, 90.32 ]
2116 [ 47.5, 90.22 ]
2117 [ 48, 90.17 ]
2118 [ 50.7, 90.19 ]
2119 [ 51.5, 90.17 ]

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2120 [ 52.2, 90.13 ]
2121 [ 52.7, 90.12 ]
2122 [ 53.3, 90.14 ]
2123 [ 53.5, 90.31 ]
2124 [ 53.9, 90.59 ]
2125 [ 54.3, 90.87 ]
2126 [ 54.7, 91.04 ]
2127 [ 55.3, 91.24 ]
2128 [ 55.5, 91.26 ]
2129 [ 63.7, 91.37 ]
2130 [ 100.1097, 91.43 ]
2131 *%-----|-----|
2132 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2133 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]mi n, AREA=[14.96](ha),
2134 * XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
2135 * SCS curve number CN=[74],
2136 * Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
2137 * LGP=[40](m), MNP=[0.25], SCP=[0](mi n),
2138 * Impervious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
2139 * LGI=[315.806](m), MNI=[0.013], SCI=[0](mi n),
2140 * Continuous simulation parameters:
2141 * I a REC per=[4](hrs), I a REC i mp=[4](hrs),
2142 * SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2143 * Inter Event Time=[18](hrs), END=-1
2144 *%-----|-----|
2145 CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]mi n, AREA=[14.96](ha),
2146 DWF=[0](cms), CN C=[77], I A=[4.67](mm),
2147 N=[3], TP=[1.007]hrs,
2148 Continuous simulation parameters:
2149 I a REC per=[4](hrs),
2150 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2151 Inter Event Time=[12](hrs)
2152 Baseflow simulation parameters:
2153 BaseFlowOption=[1],
2154 I n i t GWR es Vol=[50](mm), GWR es K=[0.96](mm/ day/ mm)
2155 VHydCond=[0.055](mm/ hr), END=-1
2156 *%-----|-----|
2157 *COMPUTE DUALHYD NHYD i n=["S-1-FO-F-D"], CI NLET=[1.749](cms), NI NLET=[1],
2158 * M i j NHYD=["S-1FO-F-DJ"]
2159 * M nNHYD=["S-1FO-F-DN"]
2160 * TM I STO=[9999999](cu-m)
2161 *%-----|-----|
2162 *ADD HYD NHYDs um=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2163 *%-----|-----|
2164 *ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"], NHYD i n=["S-1FO-F-DS"],
2165 * RDT=[1](mi n),
2166 * TABLE of ( OUTFLOW STORAGE ) values
2167 * ( cms ) - ( ha-m )
2168 * [ 0.0 , 0.0 ]
2169 * [ 0.1788, 0.5966 ]
2170 * [ -1 , -1 ] (max twenty pts)
2171 * NHYDovf=["S-1FoFDovf"]
2172 *%-----|-----|
2173 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2174 * -JFSA 2021-03-02 "S-1-D8" is Borriskane Rd. so it will remain STANDHYD in all
scenarios
2175 CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[1](mi n), AREA=[5.27](ha), XI MP=[0.65],
TI MP=[0.65], DWF=[0](cms),
2176 LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
I A per=[4.67](mm), SLPP=[2.0](%),
2177 LGP=[40](m), MNP=[0.25], SCP=[0](mi n), Impervious surfaces:
I A i mp=[1.57](mm), SLPI=[0.75](%),
2178 LGI=[187.439](m), MNI=[0.013], SCI=[0](mi n),
2179 Continuous simulation parameters:

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2180 IaRECper=[ 4](hr s), IaRECI mp=[ 4](hr s),
2181 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2182 InterEventTime=[ 12](hr s), END=- 1
2183 *%-----|-----
2184 * This is a road so it is always STANDHYD
2185 *CONTINUOUS NASHYD NHYD=[ "S-1-D8"], DT=[ 1]mi n, AREA=[ 5.27](ha),
2186 * DWF=[ 0](cms), CNV C=[ 77], IA=[ 4.67](mm),
2187 * N=[ 3], TP=[ 1.10]hr s,
2188 * Continuous simulation parameters:
2189 * IaRECper=[ 4](hr s),
2190 * SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2191 * InterEventTime=[ 12](hr s)
2192 * Baseflow simulation parameters:
2193 * BaseFlowOption=[ 1],
2194 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2195 * VHydCond=[ 0.055](mm/hr), END=- 1
2196 *%-----|-----
2197 *COMPUTE DUALHYD NHYDin=[ "S-1-D8"], CINLET=[ 2.279](cms), NI NLET=[ 1],
2198 * Mj NHYD=[ "S-1-D8J"]

2200 * TMJ STO=[ 9999999](cu-m)
2201 *%-----|-----
2202 *ADD HYD NHYDs um=[ "S-1-D8S"], NHYDs to add=[ "S-1-D8J"+"S-1-D8N"]
2203 *%-----|-----
2204 *ADD HYD NHYDs um=[ "S-1-D"], NHYDs to add=[ "S-1-Okeefe"+"S-1"+"S-1-Fost "]
2205 *%-----|-----
2206 *COMPUTE DUALHYD NHYDin=[ "S-1-D"], CINLET=[ 11.616](cms), NI NLET=[ 1],
2207 * Mj NHYD=[ "S-1-D-M"]
2208 * MnNHYD=[ "S-1-D-MN"]
2209 * TMJ STO=[ 5974](cu-m)
2210 *%-----|-----
2211 *ADD HYD NHYDs um=[ "S-1-DEV"], NHYDs to add=[ "S-1-D-M"+"S-1-D-MN"]
2212 *%-----|-----
2213 *ROUTE RESERVOIR NHYDout=[ "S-1-D8R"], NHYDin=[ "S-1-D8S"],
2214 * RDT=[ 1](mi n),
2215 * TABLE of ( OUTFLOW STORAGE ) values
2216 * (cms) - (ha-m)
2217 * [ 0.0 , 0.0 ]
2218 * [ 0.0630, 0.2102 ]
2219 * [ -1 , -1 ] (max twenty pts)
2220 * NHYDovf=[ "S-1-D8Rovf"]
2221 *%-----|-----
2222 * -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock Ri ver
2223 CONTINUOUS NASHYD NHYD=[ "S-1-A"], DT=[ 1]mi n, AREA=[ 75.88](ha),
2224 DWF=[ 0](cms), CNV C=[ 77], IA=[ 4.67](mm),
2225 N=[ 3], TP=[ 0.619]hr s,
2226 Continuous simulation parameters:
2227 IaRECper=[ 4](hr s),
2228 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2229 InterEventTime=[ 12](hr s)
2230 Baseflow simulation parameters:
2231 BaseFlowOption=[ 1],
2232 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2233 VHydCond=[ 0.055](mm/hr), END=- 1
2234 *%-----|-----
2235 * -JFSA 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2236 CONTINUOUS NASHYD NHYD=[ "W_CLAR_UNDE"], DT=[ 1]mi n, AREA=[ 35.65](ha),
2237 DWF=[ 0](cms), CNV C=[ 77], IA=[ 4.67](mm),
2238 N=[ 3], TP=[ 1.10]hr s,
2239 Continuous simulation parameters:
2240 IaRECper=[ 4](hr s),
2241 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2242 InterEventTime=[ 12](hr s)

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2243 Baseflow simulation parameters:
2244 BaseFlowOption=[ 1] ,
2245 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
2246 VHydCond=[ 0.055] (mm/ hr), END=- 1
2247 *%-----|-----
2248 ADD HYD NHYDsum=[ "SN_FO" ], NHYDs to
add=[ "N_FO"+"520-out"+"MS_P10"+"P10-OVF"+"W_CLAR_UNDE"+"S-1-FO-F-D"+"S-1-D8"+"S-1-A"]
2249 *%-----|-----
2250 SAVE HYD NHYD=[ "SN_FO" ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
2251 HYD_COMMENT=[ "Total Flows at Foster Drain"]
2252 *%-----|-----
2253 *# Hydrograph from Node Foster routed to Node at Cedarview Road
2254 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
2255 *#
2256 ROUTE CHANNEL NHYDout=[ "N_CE" ], NHYDin=[ "SN_FO" ],
2257 RDT=[ 1] (min),
2258 CHLGTH=[ 159] (m), CHSLOPE=[ 0.0818] ( %),
2259 FPSLOPE=[ 0.0818] ( %),
2260 SECNUM=[ 1.0], NSEG=[ 3]
2261 ( SEGROUGH, SEGDI ST (m))=
2262 [ 0.050, -15.46
2263 -0.035, 26.55
2264 0.050, 116.76] NSEG times
2265 ( DI STANCE (m), ELEVATI ON (m))=
2266 [- 645.23, 91.50]
2267 [- 391.20, 91.50]
2268 [- 91.00, 91.50]
2269 [- 85.52, 91.50]
2270 [- 15.46, 89.40]
2271 [- 9.79, 89.31]
2272 [- 3.22, 86.24]
2273 [ 3.22, 85.07]
2274 [ 10.96, 85.79]
2275 [ 16.44, 86.49]
2276 [ 26.55, 89.45]
2277 [ 29.03, 90.27]
2278 [ 35.76, 90.67]
2279 [ 36.67, 91.00]
2280 [ 108.08, 91.00]
2281 [ 109.82, 90.50]
2282 [ 112.04, 90.50]
2283 [ 114.62, 91.00]
2284 [ 116.76, 91.50]
2285 *%-----|-----
2286 *#*****
2287 *# Catchment S-1
2288 *# - To Jock River (north and south of Jock)
2289 *# - Primarily agricultural fields; portion of sand quarry
2290 *%-----|-----
2291 *% -2020-12-17 "S-1-Undev" and "S-1-Fost" was a part of Foster drain, they are below
the foster pond. Now they are added to S-1 subcatchment based on Project 1474-BCDC,
JFSA, Nov. 2020
2292 *% -2020-12-17 Change O-14 (it was part of Okeefe drain) to "S-1-Okeefe" and add it
to S-1 subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
2293 *% -2020-12-17 Add "S-1-BCDC" as NASHYD
2294 *% -2020-12-17 all other S-1 subcatchment as STANDHYD with DUALHYD and ROUTE RESERVOIR
2295 *%-----|-----
2296 *#*****
2297 * -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2298 *CONTINUOUS NASHYD NHYD=[ "S-1-A" ], DT=[ 1] min, AREA=[ 75.88] (ha),
2299 * DWF=[ 0] (cms), CNVC=[ 77], IA=[ 4.67] (mm),
2300 * N=[ 3], TP=[ 0.619] hrs,
2301 * Continuous simulation parameters:
2302 * IaRECper=[ 4] (hrs),
2303 * SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),

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2304 * InterEventTime=[ 12](hrs)
2305 * Baseflow simulation parameters:
2306 * BaseFlowOption=[ 1] ,
2307 * InitGWResVol=[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2308 * VHydCond=[ 0.055](mm/hr) , END=- 1
2309 *%-----|-----|
2310 CONTINUOUS NASHYD NHYD=["S-1-B"], DT=[ 1]min, AREA=[ 55.36](ha),
2311 DWF=[ 0](cms) , CNVC=[ 77] , IA=[ 4.67](mm) ,
2312 N=[ 3] , TP=[ 0.451]hrs ,
2313 Continuous simulation parameters:
2314 IaRECper=[ 4](hrs) ,
2315 SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2316 InterEventTime=[ 12](hrs)
2317 Baseflow simulation parameters:
2318 BaseFlowOption=[ 1] ,
2319 InitGWResVol=[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2320 VHydCond=[ 0.055](mm/hr) , END=- 1
2321 *%-----|-----|
2322 *# - JFSA 2021-02-24 change the name from S-1-BCDC to S-1-A and S-1-B. Change their
TP values based on the new areas compared to the old ones.
2323 *CONTINUOUS NASHYD NHYD=["S-1-BCDC"], DT=[ 1]min, AREA=[ 134.9](ha),
2324 * DWF=[ 0](cms) , CNVC=[ 77] , IA=[ 4.67](mm) ,
2325 * N=[ 3] , TP=[ 1.10]hrs ,
2326 * Continuous simulation parameters:
2327 * IaRECper=[ 4](hrs) ,
2328 * SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2329 * InterEventTime=[ 12](hrs)
2330 * Baseflow simulation parameters:
2331 * BaseFlowOption=[ 1] ,
2332 * InitGWResVol=[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2333 * VHydCond=[ 0.055](mm/hr) , END=- 1
2334 *%-----|-----|
2335 *# - JFSA 2021-02-24 "S-1-BCDC-1" and "S-1-BCDC-2" are not existing anymore.
"S-1-BCDC-1" is part of "S-1-FO-D2" and "S-1-BCDC-2" is part of "S-1-D2" and "S-1-D3"
2336 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-1"], DT=[ 1]min, AREA=[ 0.3](ha),
2337 * DWF=[ 0](cms) , CNVC=[ 77] , IA=[ 4.67](mm) ,
2338 * N=[ 3] , TP=[ 1.10]hrs ,
2339 * Continuous simulation parameters:
2340 * IaRECper=[ 4](hrs) ,
2341 * SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2342 * InterEventTime=[ 12](hrs)
2343 * Baseflow simulation parameters:
2344 * BaseFlowOption=[ 1] ,
2345 * InitGWResVol=[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2346 * VHydCond=[ 0.055](mm/hr) , END=- 1
2347 *%-----|-----|
2348 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-2"], DT=[ 1]min, AREA=[ 1.3](ha),
2349 * DWF=[ 0](cms) , CNVC=[ 77] , IA=[ 4.67](mm) ,
2350 * N=[ 3] , TP=[ 1.10]hrs ,
2351 * Continuous simulation parameters:
2352 * IaRECper=[ 4](hrs) ,
2353 * SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2354 * InterEventTime=[ 12](hrs)
2355 * Baseflow simulation parameters:
2356 * BaseFlowOption=[ 1] ,
2357 * InitGWResVol=[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2358 * VHydCond=[ 0.055](mm/hr) , END=- 1
2359 *%-----|-----|
2360 *# - JFSA 2021-01-19, after adding Greenbank pond, "S-1-BCDC-3" is not existing
anymore
2361 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-3"], DT=[ 1]min, AREA=[ 3.9](ha),
2362 * DWF=[ 0](cms) , CNVC=[ 77] , IA=[ 4.67](mm) ,
2363 * N=[ 3] , TP=[ 1.10]hrs ,
2364 * Continuous simulation parameters:
2365 * IaRECper=[ 4](hrs) ,
2366 * SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,

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2367 * InterEventTime=[ 12](hrs)
2368 * Baseflow simulation parameters:
2369 * BaseFlowOption=[ 1] ,
2370 * InitGWResVol=[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2371 * VHydCond=[ 0.055](mm/hr) , END=- 1
2372 *%-----|-----
2373 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
2374 *CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[ 1](min) , AREA=[ 44.93](ha) , XI MP=[ 0.65] ,
TI MP=[ 0.65] , DWF=[ 0](cms) ,
2375 * LOSS=[ 2] , SCS curve number CN=[ 75] , Pervious surfaces:
I A per=[ 4.67](mm) , SLPP=[ 2.0](%) ,
2376 * LGP=[ 40](m) , MNP=[ 0.25] , SCP=[ 0](min) , Impervious surfaces:
I A i mp=[ 1.57](mm) , SLPI=[ 0.75](%) ,
2377 * LGI=[ 547.296](m) , MNI=[ 0.013] , SCI=[ 0](min) ,
2378 * Continuous simulation parameters:
2379 * I a REC per=[ 4](hrs) , I a REC i mp=[ 4](hrs) ,
2380 * SM N=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2381 * InterEventTime=[ 12](hrs) , END=- 1
2382 *%-----|-----
2383 *COMPUTE DUALHYD NHYDin=["S-1-Okeefe"], CI NLET=[ 4.796](cms) , NI NLET=[ 1] ,
2384 * M a j NHYD=["S-1-OkM"]
2385 * M n NHYD=["S-1-OkMN"]
2386 * TM I STO=[ 9999999](cu-m)
2387 *%-----|-----
2388 *ADD HYD NHYDs um=["S-1-OkS"] , NHYDs t o add=["S-1-OkM"+"S-1-OkMN"]
2389 *%-----|-----
2390 *ROUTE RESERVOIR NHYDout=["S-1-OkSR"] , NHYDin=["S-1-OkS"] ,
2391 * RDT=[ 1](min) ,
2392 * TABLE of ( OUTFLOW STORAGE ) values
2393 * ( cms ) - ( ha-m)
2394 * [ 0.0 , 0.0 ]
2395 * [ 0.5370, 1.7917 ]
2396 * [ -1 , -1 ] (max twenty pts)
2397 * NHYDovf=["S-1-OkSovf"]
2398 *%-----|-----
2399 *CONTINUOUS NASHYD NHYD=["S-1-Okeefe"], DT=[ 1]mi n , AREA=[ 44.93](ha) ,
2400 * DWF=[ 0](cms) , CN C=[ 77] , I A=[ 4.67](mm) ,
2401 * N=[ 3] , TP=[ 1.049]hrs ,
2402 * Continuous simulation parameters:
2403 * I a REC per=[ 4](hrs) ,
2404 * SM N=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2405 * InterEventTime=[ 12](hrs)
2406 * Baseflow simulation parameters:
2407 * BaseFlowOption=[ 1] ,
2408 * InitGWResVol=[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2409 * VHydCond=[ 0.055](mm/hr) , END=- 1
2410 *%-----|-----
2411 * -JFSA 2021-02-26 "S-1-FO-DI" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2412 *CONTINUOUS STANDHYD NHYD=["S-1-FO-DI"] , DT=[ 1]mi n , AREA=[ 5.11](ha) ,
2413 * XI MP=[ 0.65] , TI MP=[ 0.65] , DWF=[ 0](cms) , LOSS=[ 2] ,
2414 * SCS curve number CN=[ 74] ,
2415 * Pervious surfaces: I A per=[ 4.67](mm) , SLPP=[ 0.5](%) ,
2416 * LGP=[ 40](m) , MNP=[ 0.25] , SCP=[ 0](min) ,
2417 * Impervious surfaces: I A i mp=[ 1.57](mm) , SLPI=[ 0.5](%) ,
2418 * LGI=[ 184.572](m) , MNI=[ 0.013] , SCI=[ 0](min) ,
2419 * Continuous simulation parameters:
2420 * I a REC per=[ 4](hrs) , I a REC i mp=[ 4](hrs) ,
2421 * SM N=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2422 * InterEventTime=[ 18](hrs) , END=- 1
2423 *%-----|-----
2424 *COMPUTE DUALHYD NHYDin=["S-1-FO-DI"] , CI NLET=[ 0.605](cms) , NI NLET=[ 1] ,
2425 * M a j NHYD=["S-1-FO-DIJ"]
2426 * M n NHYD=["S-1-FO-DIN"]
2427 * TM I STO=[ 9999999](cu-m)

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2428 *%-----|-----|
2429 *ADD HYD          NHYDs um=[ " S- 1- FO- DIS" ], NHYDs to add=[ " S- 1- FO- DIN" +" S- 1- FO- DIJ" ]
2430 *%-----|-----|
2431 *ROUTE RESERVOIR NHYDout=[ " S- 1- FO- DIR" ] , NHYDin=[ " S- 1- FO- DIS" ] ,
2432 *                RDT=[ 1 ] ( mi n),
2433 *                TABLE of ( OUTFLOW STORAGE ) values
2434 *                ( cms ) - ( ha- m)
2435 *                [ 0.0      , 0.0 ]
2436 *                [ 0.0611, 0.2038 ]
2437 *                [ -1      , -1      ] (max twenty pts)
2438 *                NHYDovf=[ " S- 1FOD1ovf" ]
2439 *%-----|-----|
2440 *CONTINUOUS NASHYD NHYD=[ " S- 1- FO- DI" ], DT=[ 1 ] mi n, AREA=[ 5.11 ] ( ha),
2441 *                DWF=[ 0 ] ( cms), CN C=[ 77], IA=[ 4.67 ] ( mm),
2442 *                N=[ 3], TP=[ 1.10 ] hr s,
2443 *                Continuous simulation parameters:
2444 *                IaRECper=[ 4 ] ( hr s),
2445 *                SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0.010 ] / ( mm),
2446 *                InterEventTime=[ 12 ] ( hr s)
2447 *                Baseflow simulation parameters:
2448 *                BaseFlowOption=[ 1 ] ,
2449 *                InitGWResVol=[ 50 ] ( mm), GWResK=[ 0.96 ] ( mm/ day/ mm)
2450 *                VHydCond=[ 0.055 ] ( mm/ hr), END=- 1
2451 *%-----|-----|
2452 * -JFSA 2021-02-26 " S- 1- FO- D2" is a part of S- 1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
2453 *CONTINUOUS STANDHYD NHYD=[ " S- 1- FO- D2" ], DT=[ 1 ] mi n, AREA=[ 4.94 ] ( ha),
2454 *                XI MP=[ 0.55], TI MP=[ 0.55], DWF=[ 0 ] ( cms), LOSS=[ 2],
2455 *                SCS curve number CN=[ 74],
2456 *                Pervious surfaces: IAper=[ 4.67 ] ( mm), SLPP=[ 0.5 ] ( %),
2457 *                LGP=[ 40 ] ( m), MNP=[ 0.25], SCP=[ 0 ] ( mi n),
2458 *                Impervious surfaces: IAimp=[ 1.57 ] ( mm), SLPI=[ 0.5 ] ( %),
2459 *                LGI=[ 181.475 ] ( m), MNI=[ 0.013], SCI=[ 0 ] ( mi n),
2460 *                Continuous simulation parameters:
2461 *                IaRECper=[ 4 ] ( hr s), IaRECImp=[ 4 ] ( hr s),
2462 *                SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0.010 ] / ( mm),
2463 *                InterEventTime=[ 18 ] ( hr s), END=- 1
2464 *%-----|-----|
2465 *CONTINUOUS NASHYD NHYD=[ " S- 1- FO- D2" ], DT=[ 1 ] mi n, AREA=[ 4.94 ] ( ha),
2466 *                DWF=[ 0 ] ( cms), CN C=[ 77], IA=[ 4.67 ] ( mm),
2467 *                N=[ 3], TP=[ 1.10 ] hr s,
2468 *                Continuous simulation parameters:
2469 *                IaRECper=[ 4 ] ( hr s),
2470 *                SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0.010 ] / ( mm),
2471 *                InterEventTime=[ 12 ] ( hr s)
2472 *                Baseflow simulation parameters:
2473 *                BaseFlowOption=[ 1 ] ,
2474 *                InitGWResVol=[ 50 ] ( mm), GWResK=[ 0.96 ] ( mm/ day/ mm)
2475 *                VHydCond=[ 0.055 ] ( mm/ hr), END=- 1
2476 *%-----|-----|
2477 *COMPUTE DUALHYD NHYDin=[ " S- 1- FO- D2" ], CINLET=[ 0.508 ] ( cms), NINLET=[ 1],
2478 *                Mj NHYD=[ " S- 1- FO- D2J" ]
2479 *                MnNHYD=[ " S- 1- FO- D2N" ]
2480 *                TMI STO=[ 9999999 ] ( cu- m)
2481 *%-----|-----|
2482 *ADD HYD          NHYDs um=[ " S- 1- FO- D2S" ], NHYDs to add=[ " S- 1- FO- D2J" +" S- 1- FO- D2N" ]
2483 *%-----|-----|
2484 *ROUTE RESERVOIR NHYDout=[ " S- 1- FO- D2R" ] , NHYDin=[ " S- 1- FO- D2S" ] ,
2485 *                RDT=[ 1 ] ( mi n),
2486 *                TABLE of ( OUTFLOW STORAGE ) values
2487 *                ( cms ) - ( ha- m)
2488 *                [ 0.0      , 0.0 ]
2489 *                [ 0.0590, 0.1970 ]
2490 *                [ -1      , -1      ] (max twenty pts)
2491 *                NHYDovf=[ " S- 1FOD2ovf" ]
2492 *%-----|-----|

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2493 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2494 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]mi n, AREA=[14.96](ha),
2495 * XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
2496 * SCS curve number CN=[74],
2497 * Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
2498 * LGP=[40](m), MNP=[0.25], SCP=[0](mi n),
2499 * Imper vious surfaces: I A i mp=[1.57](mm), SLPI =[0.5](%),
2500 * LGI =[315.806](m), MNI =[0.013], SCI =[0](mi n),
2501 * Continuous simulation parameters:
2502 * I a REC per =[4](hrs), I a REC i mp =[4](hrs),
2503 * SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2504 * Inter Event Ti me =[18](hrs), END=-1
2505 *%-----|-----|
2506 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[1]mi n, AREA=[14.96](ha),
2507 * DWF=[0](cms), CN C=[77], I A=[4.67](mm),
2508 * N=[3], TP=[1.007]hrs,
2509 * Continuous simulation parameters:
2510 * I a REC per =[4](hrs),
2511 * SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2512 * Inter Event Ti me =[12](hrs)
2513 * Baseflow simulation parameters:
2514 * BaseFl owOpt ion=[1],
2515 * Ini t GWRes Vol =[50](mm), GWRes K=[0.96](mm/ day/ mm)
2516 * VHydCond=[0.055](mm/ hr), END=-1
2517 *%-----|-----|
2518 *COMPUTE DUALHYD NHYD i n=["S-1-FO-F-D"], CI NLET=[1.749](cms), NI NLET=[1],
2519 * M a j NHYD=["S-1FO-F-DJ"]
2520 * M nNHYD=["S-1FO-F-DN"]
2521 * TM I STO=[9999999](cu-m)
2522 *%-----|-----|
2523 *ADD HYD NHYDs um=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2524 *%-----|-----|
2525 *ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"], NHYD i n=["S-1FO-F-DS"],
2526 * RDT=[1](mi n),
2527 * TABLE of ( OUTFLOW STORAGE ) values
2528 * ( cms ) - ( ha-m)
2529 * [ 0.0 , 0.0 ]
2530 * [ 0.1788, 0.5966 ]
2531 * [ -1 , -1 ] (max twenty pts)
2532 * NHYDovf=["S-1FoFDovf"]
2533 *%-----|-----|
2534 *CONTINUOUS STANDHYD NHYD=["S-1-DI"], DT=[1](mi n), AREA=[21.67](ha), XI MP=[0.65],
TI MP=[0.65], DWF=[0](cms),
2535 * LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
I A per=[4.67](mm), SLPP=[2.0](%),
2536 * LGP=[40](m), MNP=[0.25], SCP=[0](mi n), Imper vious surfaces:
I A i mp=[1.57](mm), SLPI =[0.75](%),
2537 * LGI =[380.088](m), MNI =[0.013], SCI =[0](mi n),
2538 * Continuous simulation parameters:
2539 * I a REC per =[4](hrs), I a REC i mp =[4](hrs),
2540 * SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2541 * Inter Event Ti me =[12](hrs), END=-1
2542 *%-----|-----|
2543 CONTINUOUS NASHYD NHYD=["S-1-DI"], DT=[1]mi n, AREA=[21.67](ha),
2544 DWF=[0](cms), CN C=[77], I A=[4.67](mm),
2545 N=[3], TP=[1.066]hrs,
2546 Continuous simulation parameters:
2547 I a REC per =[4](hrs),
2548 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2549 Inter Event Ti me =[12](hrs)
2550 Baseflow simulation parameters:
2551 BaseFl owOpt ion=[1],
2552 Ini t GWRes Vol =[50](mm), GWRes K=[0.96](mm/ day/ mm)
2553 VHydCond=[0.055](mm/ hr), END=-1
2554 *%-----|-----|

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2614 * IaRECper=[ 4](hr s), IaRECImp=[ 4](hr s),
2615 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2616 * InterEventTime=[ 12](hr s), END=- 1
2617 *%-----|-----
2618 *CONTINUOUS NASHYD NHYD=["S-1-D3"], DT=[ 1]mi n, AREA=[ 6.79](ha),
2619 * DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
2620 * N=[ 3], TP=[ 1.281]hr s,
2621 * Continuous simulation parameters:
2622 * IaRECper=[ 4](hr s),
2623 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2624 * InterEventTime=[ 12](hr s)
2625 * Baseflow simulation parameters:
2626 * BaseFlowOption=[ 1],
2627 * InItGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2628 * VHydCond=[ 0.055](mm/hr), END=- 1
2629 *%-----|-----
2630 *COMPUTE DUALHYD NHYDin=["S-1-D3"], CINLET=[ 0.719](cms), NINLET=[ 1],
2631 * MjNHYD=["S-1-D3J"]
2632 * MnNHYD=["S-1-D3N"]
2633 * TMI STO=[ 9999999](cu-m)
2634 *%-----|-----
2635 *ADD HYD NHYDs um=["S-1-D3S"], NHYDs to add=["S-1-D3J"+"S-1-D3N"]
2636 *%-----|-----
2637 *ROUTE RESERVOIR NHYDout=["S-1-D3R"], NHYDin=["S-1-D3S"],
2638 * RDT=[ 1](mi n),
2639 * TABLE of ( OUTFLOW STORAGE ) values
2640 * ( cms ) - ( ha-m)
2641 * [ 0.0 , 0.0 ]
2642 * [ 0.0811, 0.2708 ]
2643 * [ -1 , -1 ] (max twenty pts)
2644 * NHYDovf=["S-1-D3Rovf"]
2645 *%-----|-----
2646 *CONTINUOUS STANDHYD NHYD=["S-1-D4"], DT=[ 1](mi n), AREA=[ 3.28](ha), XI MP=[ 0.65],
TI MP=[ 0.65], DWF=[ 0](cms),
2647 * LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
I Aper=[ 4.67](mm), SLPP=[ 2.0](%),
2648 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n), Impervious surfaces:
I Ai mp=[ 1.57](mm), SLPI=[ 0.75](%),
2649 * LGI=[ 147.874](m), MNI=[ 0.013], SCI=[ 0](mi n),
2650 * Continuous simulation parameters:
2651 * IaRECper=[ 4](hr s), IaRECImp=[ 4](hr s),
2652 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2653 * InterEventTime=[ 12](hr s), END=- 1
2654 *%-----|-----
2655 CONTINUOUS NASHYD NHYD=["S-1-D4"], DT=[ 1]mi n, AREA=[ 3.28](ha),
2656 DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
2657 N=[ 3], TP=[ 1.10]hr s,
2658 Continuous simulation parameters:
2659 IaRECper=[ 4](hr s),
2660 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2661 InterEventTime=[ 12](hr s)
2662 Baseflow simulation parameters:
2663 BaseFlowOption=[ 1],
2664 InItGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2665 VHydCond=[ 0.055](mm/hr), END=- 1
2666 *%-----|-----
2667 *COMPUTE DUALHYD NHYDin=["S-1-D4"], CINLET=[ 0.373](cms), NINLET=[ 1],
2668 * MjNHYD=["S-1-D4J"]
2669 * MnNHYD=["S-1-D4N"]
2670 * TMI STO=[ 9999999](cu-m)
2671 *%-----|-----
2672 *ADD HYD NHYDs um=["S-1-D4S"], NHYDs to add=["S-1-D4J"+"S-1-D4N"]
2673 *%-----|-----
2674 *ROUTE RESERVOIR NHYDout=["S-1-D4R"], NHYDin=["S-1-D4S"],
2675 * RDT=[ 1](mi n),
2676 * TABLE of ( OUTFLOW STORAGE ) values

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```

2677 *                ( cms ) - ( ha - m )
2678 *                [ 0.0      , 0.0 ]
2679 *                [ 0.0392, 0.1308 ]
2680 *                [ -1      , -1      ] (max twenty pts)
2681 *                NHYDovf=[ "S-1-D4Rovf" ]
2682 *%-----|-----|
2683 *CONTINUOUS STANDHYD NHYD=[ "S-1-D5" ], DT=[ 1 ](mi n), AREA=[ 12.84 ](ha), XI MP=[ 0.65 ],
TI MP=[ 0.65 ], DWF=[ 0 ]( cms ),
2685 *                LGP=[ 40 ](m), MNP=[ 0.25 ], SCP=[ 0 ](mi n), Impervious surfaces:
I Ai mp=[ 1.57 ](mm), SLPI=[ 0.75 ]( % ),
2686 *                LGI=[ 292.57 ](m), MNI=[ 0.013 ], SCI=[ 0 ](mi n),
2687 *                Continuous simulation parameters:
2688 *                IaRECper=[ 4 ](hrs), IaRECI mp=[ 4 ](hrs),
2689 *                SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2690 *                InterEvent Time=[ 12 ](hrs), END=- 1
2691 *%-----|-----|
2692 CONTINUOUS NASHYD NHYD=[ "S-1-D5" ], DT=[ 1 ]mi n, AREA=[ 12.84 ](ha),
2693 DWF=[ 0 ]( cms ), CNV C=[ 77 ], IA=[ 4.67 ](mm),
2694 N=[ 3 ], TP=[ 1.10 ]hrs,
2695 Continuous simulation parameters:
2696 IaRECper=[ 4 ](hrs),
2697 SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2698 InterEvent Time=[ 12 ](hrs)
2699 Baseflow simulation parameters:
2700 BaseFlowOption=[ 1 ],
2701 Ini tGWRes Vol=[ 50 ](mm), GWRes K=[ 0.96 ](mm/ day/ mm)
2702 VHydCond=[ 0.055 ](mm/ hr), END=- 1
2703 *%-----|-----|
2704 *COMPUTE DUALHYD NHYDi n=[ "S-1-D5" ], CI NLET=[ 1.395 ]( cms ), NI NLET=[ 1 ],
2705 *                Mi j NHYD=[ "S-1-D5J" ]
2706 *                M nNHYD=[ "S-1-D5N" ]
2707 *                TM I STO=[ 9999999 ](cu - m)
2708 *%-----|-----|
2709 *ADD HYD NHYDs um=[ "S-1-D5S" ], NHYDs to add=[ "S-1-D5J"+"S-1-D5N" ]
2710 *%-----|-----|
2711 *ROUTE RESERVOIR NHYDout=[ "S-1-D5R" ], NHYDi n=[ "S-1-D5S" ],
2712 *                RDT=[ 1 ](mi n),
2713 *                TABLE of ( OUTFLOW STORAGE ) values
2714 *                ( cms ) - ( ha - m )
2715 *                [ 0.0      , 0.0 ]
2717 *                [ -1      , -1      ] (max twenty pts)
2718 *                NHYDovf=[ "S-1-D5Rovf" ]
2719 *%-----|-----|
2720 *CONTINUOUS STANDHYD NHYD=[ "S-1-D6" ], DT=[ 1 ](mi n), AREA=[ 1.75 ](ha), XI MP=[ 0.65 ],
TI MP=[ 0.65 ], DWF=[ 0 ]( cms ),
2721 *                LOSS=[ 2 ], SCS curve number CN=[ 75 ], Pervious surfaces:
I Aper=[ 4.67 ](mm), SLPP=[ 2.0 ]( % ),
2722 *                LGP=[ 40 ](m), MNP=[ 0.25 ], SCP=[ 0 ](mi n), Impervious surfaces:
I Ai mp=[ 1.57 ](mm), SLPI=[ 0.75 ]( % ),
2723 *                LGI=[ 108.01 ](m), MNI=[ 0.013 ], SCI=[ 0 ](mi n),
2724 *                Continuous simulation parameters:
2725 *                IaRECper=[ 4 ](hrs), IaRECI mp=[ 4 ](hrs),
2726 *                SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2727 *                InterEvent Time=[ 12 ](hrs), END=- 1
2728 *%-----|-----|
2729 CONTINUOUS NASHYD NHYD=[ "S-1-D6" ], DT=[ 1 ]mi n, AREA=[ 1.75 ](ha),
2730 DWF=[ 0 ]( cms ), CNV C=[ 77 ], IA=[ 4.67 ](mm),
2731 N=[ 3 ], TP=[ 1.10 ]hrs,
2732 Continuous simulation parameters:
2733 IaRECper=[ 4 ](hrs),
2734 SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2735 InterEvent Time=[ 12 ](hrs)
2736 Baseflow simulation parameters:

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2737 BaseFlowOpt ion=[ 1] ,
2738 Ini t GWRes Vol =[ 50] ( mm) , GWRes K=[ 0. 96] ( mm/ day/ mm)
2739 VHydCond=[ 0. 055] ( mm/ hr) , END=- 1
2740 *%-----|-----
2741 *COMPUTE DUALHYD NHYDi n=[ " S- 1- D6" ] , CI NLET=[ 0. 218] ( cms) , NI NLET=[ 1] ,
2742 * Mj NHYD=[ " S- 1- D6J" ]
2743 * M nNHYD=[ " S- 1- D6N" ]
2744 * TM STO=[ 9999999] ( cu- m)
2745 *%-----|-----
2746 *ADD HYD NHYDs um=[ " S- 1- D6S" ] , NHYDs to add=[ " S- 1- D6J" +" S- 1- D6N" ]
2747 *%-----|-----
2748 *ROUTE RESERVOIR NHYDout =[ " S- 1- D6R" ] , NHYDi n=[ " S- 1- D6S" ] ,
2749 * RDT=[ 1] ( mi n) ,
2750 * TABLE of ( OUTFLOW STORAGE ) values
2751 * ( cms) - ( ha- m)
2752 * [ 0. 0 , 0. 0 ]
2753 * [ 0. 0209, 0. 0698 ]
2754 * [ -1 , -1 ] ( max twenty pts)
2755 * NHYDovf=[ " S- 1- D6Rovf" ]
2756 *%-----|-----
2757 *CONTINUOUS STANDHYD NHYD=[ " S- 1- D7" ] , DT=[ 1] ( mi n) , AREA=[ 2. 03] ( ha) , XI MP=[ 0. 65] ,
TI MP=[ 0. 65] , DWF=[ 0] ( cms) ,
2758 * LOSS=[ 2] , SCS curve number CN=[ 75] , Pervious surfaces:
IAper=[ 4. 67] ( mm) , SLPP=[ 2. 0] ( % ) ,

2760 * LGI=[ 116. 33] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
2761 * Continuous simulation parameters:
2762 * IaRECper=[ 4] ( hrs) , IaRECI mp=[ 4] ( hrs) ,
2763 * SM N=[ -1] ( mm) , SMAX=[ -1] ( mm) , SK=[ 0. 010] / ( mm) ,
2764 * Inter Event Ti me=[ 12] ( hrs) , END=- 1
2765 *%-----|-----
2766 CONTINUOUS NASHYD NHYD=[ " S- 1- D7" ] , DT=[ 1] mi n , AREA=[ 2. 03] ( ha) ,
2767 DWF=[ 0] ( cms) , CN C=[ 77] , IA=[ 4. 67] ( mm) ,
2768 N=[ 3] , TP=[ 1. 10] hrs ,
2769 Continuous simulation parameters:
2770 IaRECper=[ 4] ( hrs) ,
2771 SM N=[ -1] ( mm) , SMAX=[ -1] ( mm) , SK=[ 0. 010] / ( mm) ,
2772 Inter Event Ti me=[ 12] ( hrs)
2773 Baseflow simulation parameters:
2774 BaseFlowOpt ion=[ 1] ,
2775 Ini t GWRes Vol =[ 50] ( mm) , GWRes K=[ 0. 96] ( mm/ day/ mm)
2776 VHydCond=[ 0. 055] ( mm/ hr) , END=- 1
2777 *%-----|-----
2778 *COMPUTE DUALHYD NHYDi n=[ " S- 1- D7" ] , CI NLET=[ 2. 279] ( cms) , NI NLET=[ 1] ,
2779 * Mj NHYD=[ " S- 1- D7J" ]
2780 * M nNHYD=[ " S- 1- D7N" ]
2781 * TM STO=[ 9999999] ( cu- m)
2782 *%-----|-----
2783 *ADD HYD NHYDs um=[ " S- 1- D7S" ] , NHYDs to add=[ " S- 1- D7J" +" S- 1- D7N" ]
2784 *%-----|-----
2785 *ROUTE RESERVOIR NHYDout =[ " S- 1- D7R" ] , NHYDi n=[ " S- 1- D7S" ] ,
2786 * RDT=[ 1] ( mi n) ,
2787 * TABLE of ( OUTFLOW STORAGE ) values
2788 * ( cms) - ( ha- m)
2789 * [ 0. 0 , 0. 0 ]
2790 * [ 0. 0243, 0. 0810 ]
2791 * [ -1 , -1 ] ( max twenty pts)
2792 * NHYDovf=[ " S- 1- D8Rovf" ]
2793 *%-----|-----
2794 * -JFSA 2021-02-26 " S- 1- D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2795 *CONTINUOUS STANDHYD NHYD=[ " S- 1- D8" ] , DT=[ 1] ( mi n) , AREA=[ 5. 27] ( ha) , XI MP=[ 0. 65] ,
TI MP=[ 0. 65] , DWF=[ 0] ( cms) ,
2796 * LOSS=[ 2] , SCS curve number CN=[ 75] , Pervious surfaces:
IAper=[ 4. 67] ( mm) , SLPP=[ 2. 0] ( % ) ,

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2797 *                LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min), Impervious surfaces:
I Ai mp=[ 1.57](mm), SLPI=[ 0.75](%),
2798 *                LGI=[ 187.439](m), MNI=[ 0.013], SCI=[ 0](min),
2799 *                Continuous simulation parameters:
2800 *                IaRECPper=[ 4](hrs), IaRECImp=[ 4](hrs),
2801 *                SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2802 *                InterEventTime=[ 12](hrs), END=- 1
2803 *%-----|-----
2804 *CONTINUOUS NASHYD NHYD=[ "S-1-D8"], DT=[ 1]min, AREA=[ 5.27](ha),
2805 *                DWF=[ 0](cms), CN/C=[ 77], IA=[ 4.67](mm),
2806 *                N=[ 3], TP=[ 1.10]hrs,
2807 *                Continuous simulation parameters:
2808 *                IaRECPper=[ 4](hrs),
2809 *                SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2810 *                InterEventTime=[ 12](hrs)
2811 *                Baseflow simulation parameters:
2812 *                BaseFlowOption=[ 1],
2813 *                InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2814 *                VHydCond=[ 0.055](mm/hr), END=- 1
2815 *%-----|-----
2816 *COMPUTE DUALHYD NHYDin=[ "S-1-D8"], CINLET=[ 2.279](cms), NI NLET=[ 1],
2817 *                Mij NHYD=[ "S-1-D8J"]
2818 *                MnNHYD=[ "S-1-D8N"]
2819 *                TMI STO=[ 9999999](cu-m)
2820 *%-----|-----
2821 *ADD HYD NHYDs um=[ "S-1-D8S"], NHYDs to add=[ "S-1-D8J"+"S-1-D8N"]
2822 *%-----|-----
2823 *ADD HYD NHYDs um=[ "S-1-D"], NHYDs to add=[ "S-1-Okeefe"+"S-1"+"S-1-Fost"]
2824 *%-----|-----
2825 *COMPUTE DUALHYD NHYDin=[ "S-1-D"], CINLET=[ 11.616](cms), NI NLET=[ 1],
2826 *                Mij NHYD=[ "S-1-D-M"]
2827 *                MnNHYD=[ "S-1-D-MN"]
2828 *                TMI STO=[ 5974](cu-m)
2829 *%-----|-----
2830 *ADD HYD NHYDs um=[ "S-1-DEV"], NHYDs to add=[ "S-1-D-M"+"S-1-D-MN"]
2831 *%-----|-----
2832 *ROUTE RESERVOIR NHYDout=[ "S-1-D8R"], NHYDin=[ "S-1-D8S"],
2833 *                RDT=[ 1](min),
2834 *                TABLE of ( OUTFLOW STORAGE ) values
2835 *                (cms) - (ha-m)
2836 *                [ 0.0, 0.0 ]
2837 *                [ 0.0630, 0.2102 ]
2838 *                [ -1, -1 ] (max twenty pts)
2839 *                NHYDovf=[ "S-1-D8Rovf"]
2840 *%-----|-----
2841 *%-----|-----
2842 *                -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
2843 *# Catchment W_CLAR
2844 *# - To West Clarke Drain (south of the Jock)
2845 *# - Subdivision with 43% imp. as per Barrhaven South MS
2846 *# - 2020-11-30 update CLARKE Tributary Drainage Area to = 121 ha based on
P598(04)-11
2847 *# - 2020-11-30 split CLARKE Drainage Area to MAJOR and ALL
2848 *#*****
2849 CONTINUOUS STANDHYD NHYD=[ "W_CLAR_M"], DT=[ 1]min, AREA=[ 1.772](ha),
2850 *                XI MP=[ 0.46], TI MP=[ 0.59], DWF=[ 0](cms), LOSS=[ 2],
2851 *                SCS curve number CN=[ 77],
2852 *                Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
2853 *                LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
2854 *                Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),
2855 *                LGI=[ 109](m), MNI=[ 0.013], SCI=[ 0](min),
2856 *                Continuous simulation parameters:
2857 *                IaRECPper=[ 4](hrs), IaRECImp=[ 4](hrs),
2858 *                SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2859 *                InterEventTime=[ 18](hrs), END=- 1

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2860 *%-----|-----|
2861 *COMPUTE DUALHYD      NHYDin=["W_CLAR_M"],  CILET=[ 0.213](cms),  NILET=[ 1],
2862 *                      MjNHYD=["W_CLAR_Mj"]
2863 *                      MnNHYD=["W_CLAR_Mn"]
2864 *                      TMSTO=[ 0.1](cu-m)
2865 *%-----|-----|
2866 *# 5-Year + 12% Capture
2867 ROUTE RESERVOIR      NHYDout=["W_CLAR_Mn"],  NHYDin=["W_CLAR_M"],
2868                      RDT=[ 1](min),
2869                      TABLE of ( OUTFLOW STORAGE ) values
2870                      (cms) - (ha-m)
2871                      [ 0.0      , 0.0 ]
2872                      [ 0.213  , 0.0001 ]
2873                      [      -1  , -1    ] (max twenty pts)
2874                      NHYDovf=["W_CLAR_Mj"],
2875 *%-----|-----|
2876 *      -Clarke_All area from P 598(04)-11 = 120.207 ha, change to 127.298 ha based on
GIS measurements,
2877 *      -JFSA, 2021-01-19 update W_CLAR_ALL to (121.17-1.772=119.398) ha based on GIS
measurements W_CLAR is 121.17 ha and W_CLAR_M is 1.772 ha
2878 CONTINUOUS STANDHYD NHYD=["W_CLAR_ALL"],  DT=[ 1]min,  AREA=[ 119.398](ha),
2879                      XI MP=[ 0.60],  TI MP=[ 0.65],  DWF=[ 0](cms),  LOSS=[ 2],
2880                      SCS curve number CN=[ 77],
2881                      Pervious surfaces: IAper=[ 4.67](mm),  SLPP=[ 1](%),
2882                      LGP=[ 40](m),  MNP=[ 0.25],  SCP=[ 0](min),
2883                      Impervious surfaces: IAimp=[ 1.57](mm),  SLPI=[ 1](%),
2884                      LGI=[ 892.18](m),  MNI=[ 0.013],  SCI=[ 0](min),
2885                      Continuous simulation parameters:
2886                      IaREcper=[ 4](hrs),  IaREcimp=[ 4](hrs),
2887                      SMN=[ -1](mm),  SMAX=[ -1](mm),  SK=[ 0.010]/(mm),
2888                      InterEventTime=[ 18](hrs),  END=-1
2889 *%-----|-----|
2890 ADD HYD              NHYDsum=["W_CLAR"],  NHYDs to add=["W_CLAR_ALL"+"W_CLAR_Mj"]
2891 *%-----|-----|
2892 SAVE HYD             NHYD=["W_CLAR"],  # OF PCYCLES=[ -1],  ICASEsh=[ 1]
2893                      HYD_COMMENT=["Total Flows to West Clarke"]
2894 *#*****
2895 *#      West Clarke Pond 2
2896 *#      - Rating curve obtained from Barrhaven South M5S modeling
2897 *#      - Tributary Drainage Area to M5S Pond 2 = 241 ha
2898 *#*****
2899 ROUTE RESERVOIR      NHYDout=["M5_P2"],  NHYDin=["W_CLAR"],
2900                      RDT=[ 1](min),
2901                      TABLE of ( OUTFLOW STORAGE ) values
2902                      (cms) - (ha-m)
2903                      [ 0.0      , 0.0 ]
2904                      [ 0.128  , 0.161 ]
2905                      [ 0.138  , 0.409 ]
2906                      [ 0.148  , 0.68 ]
2907                      [ 0.227  , 0.931 ]
2908                      [ 0.354  , 1.223 ]
2909                      [ 0.505  , 1.52 ]
2910                      [ 0.666  , 1.821 ]
2911                      [ 0.831  , 2.123 ]
2912                      [ 0.995  , 2.434 ]
2913                      [ 1.069  , 2.583 ]
2914                      [ 1.51   , 2.647 ]
2915                      [ 4.904  , 2.861 ]
2916                      [ 13.048 , 3.188 ]
2917                      [ 23.745 , 3.523 ]
2918                      [ 36.474 , 3.871 ]
2919                      [ 45.938 , 4.127 ]
2920                      [ 61.652 , 4.539 ]
2921                      [      -1  , -1    ] (max twenty pts)
2922                      NHYDovf=["P2-OVF"]
2923 *%-----|-----|

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2924 *#*****
2925 * -JFSA 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2926 *CONTINUOUS NASHYD NHYD=["W_CLAR_UNDE"], DT=[1]min, AREA=[35.65](ha),
2927 * DWF=[0](cms), CNVC=[77], IA=[4.67](mm),
2928 * N=[3], TP=[1.10]hrs,
2929 * Continuous simulation parameters:
2930 * IaRECper=[4](hrs),
2931 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2932 * InterEventTime=[12](hrs)
2933 * Baseflow simulation parameters:
2934 * BaseFlowOption=[1],
2935 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2936 * VHydCond=[0.055](mm/hr), END=-1
2937 *%-----|-----|
2938 ADD HYD NHYDsum=["SN_CE"], NHYDs to add=["N_CE"+
2939 +"S-1-D4"+"S-1-D5"+"MS_P2"+"P2-OVF"]
2940 *%-----|-----|
2941 SAVE HYD NHYD=["SN_CE"], # OF PCYCLES=[-1], ICASEsh=[1]
2942 HYD_COMMENT=["Total Flows before Station 5737 on Jock River"]
2943 *%-----|-----|
2944 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5737
2945 *# JFSA 2021-02-25 add station 5737 before station 5002. Station 5737 was extracted
from the HEC-RAS model
T:\PROJ\1474-16\Design\20201026-QuantityControlAnalysis\HEC-RAS\JockLidar2005
2946 *# JFSA 2021-03-02 change the slope to 0.1% instead of 0.0175 to stabilize the model
2947 ROUTE CHANNEL NHYDout=["5737"], NHYDin=["SN_CE"],
2948 RDT=[1](min),
2949 CHLGTH=[270](m), CHSLOPE=[0.0175](%),
2950 FPSLOPE=[0.0175](%),
2951 SECNUM=[1.0], NSEG=[3]
2952 (SEGROUGH, SEGDI ST (m))=
2953 [0.050, -24.04
2954 -0.035, 23.92
2955 0.050, 1130.8] NSEG times
2956 (DISTANCE (m), ELEVATION (m))=
2957 [-1060.52, 94]
2958 [-268.6, 91.5]
2959 [-259.43, 91.5]
2960 [-179.48, 91.5]
2961 [-67.9, 91.5]
2962 [-59.21, 91.5]
2963 [-33.19, 91]
2964 [-26.08, 90.5]
2965 [-24.04, 90]
2966 [-13.14, 86.77]
2967 [0, 85]
2968 [14.68, 86.74]
2969 [23.92, 90]
2970 [25.78, 90.5]
2971 [31.91, 91]
2972 [91.95, 91.5]
2973 [772.15, 92]
2974 [961.49, 92.5]
2975 [1044.69, 93]
2976 [1130.8, 95]
2977 *%-----|-----|
2978 ADD HYD NHYDsum=["5002"], NHYDs to add=["5737"+
2979 +"S-1-D1"+"S-1-D6"+"S-1-D7"]
2980 *%-----|-----|
2981 SAVE HYD NHYD=["5002"], # OF PCYCLES=[-1], ICASEsh=[1]
2982 HYD_COMMENT=["Total Flows before Station 5002 on Jock River"]
2983 *%-----|-----|
2984 *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
2985 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5002

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2986 *# JFSA 2021-02-19 Change the slope from 0.01 % (as per Stantec Report 2007) to 0.0255
 % so the model will be more stable and give reasonable results. It is justifiable as
 ROUTE CHANNELs aren't well suited to really flat slopes.

2987 *# JFSA 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one
 with 825 m length so the model will be more stable

2988 *# JFSA 2021-02-26 change the length of 5002 route channel from 825 m to 736 m That is
 because of adding station 5737 between station 6016 and station 5002. Then the length
 from station 5737 to station 5002 is 736 m Change the slope from 0.0255 % to 0.09511 %

2989 *
 2990 ROUTE CHANNEL NHYDout=["N_WCa"] , NHYDin=["5002"] ,
 2991 RDT=[1] (m i n) ,
 2992 CHLGTH=[245. 33333] (m) , CHSLOPE=[0. 09511] (%) ,
 2993 FPSLOPE=[0. 09511] (%) ,
 2994 SECNUM=[1. 0] , NSEG=[3]
 2995 (SEGROUGH, SEGDI ST (m)) =
 2996 [0. 050, - 37. 5
 2997 - 0. 035, 37. 50
 2998 0. 050, 157. 05] NSEG t i m e s
 2999 (DI STANCE (m) , ELEVATI ON (m)) =
 3000 [- 601. 81, 91. 5]
 3001 [- 37. 50, 90. 00]
 3002 [- 19. 61, 87. 04]
 3003 [0. 00, 85. 70]
 3004 [14. 87, 86. 93]
 3005 [37. 50, 90. 00]
 3006 [38. 54, 90. 50]
 3007 [42. 23, 91]
 3008 [157. 05, 91. 50]
 3009 * [161. 44, 91. 50]
 3010 * [236. 48, 93. 00]
 3011 * [385. 47, 92. 50]
 3012 * [390. 78, 92. 50]

3013 *%-----|
 3014 ROUTE CHANNEL NHYDout=["N_WCb"] , NHYDin=["N_WCa"] ,
 3015 RDT=[1] (m i n) ,
 3016 CHLGTH=[245. 33333] (m) , CHSLOPE=[0. 09511] (%) ,
 3017 FPSLOPE=[0. 09511] (%) ,
 3018 SECNUM=[1. 0] , NSEG=[3]
 3019 (SEGROUGH, SEGDI ST (m)) =
 3020 [0. 050, - 37. 5
 3021 - 0. 035, 37. 50
 3022 0. 050, 157. 05] NSEG t i m e s
 3023 (DI STANCE (m) , ELEVATI ON (m)) =
 3024 [- 601. 81, 91. 5]
 3025 [- 37. 50, 90. 00]
 3026 [- 19. 61, 87. 04]
 3027 [0. 00, 85. 70]
 3028 [14. 87, 86. 93]
 3029 [37. 50, 90. 00]
 3030 [38. 54, 90. 50]
 3031 [42. 23, 91]
 3032 [157. 05, 91. 50]

3033 *%-----|
 3034 ROUTE CHANNEL NHYDout=["N_WC"] , NHYDin=["N_WCb"] ,
 3035 RDT=[1] (m i n) ,
 3036 CHLGTH=[245. 33333] (m) , CHSLOPE=[0. 09511] (%) ,
 3037 FPSLOPE=[0. 09511] (%) ,
 3038 SECNUM=[1. 0] , NSEG=[3]
 3039 (SEGROUGH, SEGDI ST (m)) =
 3040 [0. 050, - 37. 5
 3041 - 0. 035, 37. 50
 3042 0. 050, 157. 05] NSEG t i m e s
 3043 (DI STANCE (m) , ELEVATI ON (m)) =
 3044 [- 601. 81, 91. 5]
 3045 [- 37. 50, 90. 00]
 3046 [- 19. 61, 87. 04]

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3047 [ 0.00, 85.70]
3048 [ 14.87, 86.93]
3049 [ 37.50, 90.00]
3050 [ 38.54, 90.50]
3051 [ 42.23, 91]
3052 [ 157.05, 91.50]
3053 *#*****
3054 * -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
3055 *ADD HYD NHYDs um=["SN_WC"], NHYDs to
add=["MS_P2"+"P2-OVF"+"N_WC"+"W_CLAR_UNDE"]
3056 *%-----|-----|
3057 *SAVE HYD NHYD=["SN_WC"], # OF PCYCLES=[-1], ICASEsh=[1]
3058 * HYD_COMMENT=["Total Flows at West Clarke Pond Outlet"]
3059 *%-----|-----|
3060 *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
3061 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
3062 *#
3063 ROUTE CHANNEL NHYDout=["N_KB"], NHYDin=["N_WC"],
3064 RDT=[1](min),
3065 CHLGTH=[1020](m), CHSLOPE=[0.0498](%),
3066 FPSLOPE=[0.0498](%),
3067 SECNUM=[1.0], NSEG=[3]
3068 ( SEGROUGH, SEGDI ST (m))=
3069 [0.050, -23.63
3070 -0.035, 23.63
3071 0.050, 728.3] NSEG times
3072 ( DISTANCE (m), ELEVATION (m))=
3073 [-1082.01, 94]
3074 [-1028.17, 92.5]
3075 [-992.3, 93.5]
3076 [-279.34, 90]
3077 [-23.63, 90]
3078 [-13.45, 87.13]
3079 [-0.07, 86.24]
3080 [10.54, 87.15]
3081 [23.63, 90]
3082 [24.86, 90.5]
3083 [26.72, 91]
3084 [45.07, 91.5]
3085 [128.17, 91.5]
3086 [270.7, 92.5]
3087 [728.3, 95]
3088 *%-----|-----|
3089 *#*****
3090 *# Catchment KEN_BU
3091 *# - To Kennedy-Burnett SWM Facility
3092 *# - Outlets to Fraser-Clarke drain (north of the Jock)
3093 *# - Medium density residential subdivision
3094 * - Add Kennedy Burnett model (Convert PCSWMM from NOVATECH June, 2020 to SWWHYMD)
3095 *#*****
3096 *CONTINUOUS STANDHYD NHYD=["KEN_BU"], DT=[1]min, AREA=[281](ha),
3097 * XI MP=[0.55], TI MP=[0.55], DWF=[0](cms), LOSS=[2],
3098 * SCS curve number CN=[71],
3099 * Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
3100 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3101 * Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
3102 * LGI=[1369](m), MNI=[0.013], SCI=[0](min),
3103 * Continuous simulation parameters:
3104 * I a REC per=[4](hrs), I a REC i mp=[4](hrs),
3105 * SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3106 * Inter Event Time=[18](hrs), END=-1
3107 *%-----|-----|
3108 *#*****
3109 *# Existing Kennedy-Burnett SWM Facility
3110 *# - Rating curve obtained from URTKBP

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3111 *# - Tributary Drainage Area to Pond = 160 ha
3112 *#*****
3113 *ROUTE RESERVOIR NHYDout=[ "KEN_P"], NHYDin=[ "KEN_BU"],
3114 * RDT=[ 1](mi n),
3115 * TABLE of ( OUTFLOW STORAGE ) values
3116 * (cms) - (ha-m)
3117 * [ 0.0 , 0.0 ]
3118 * [ 0.13 , 0.26]
3119 * [ 0.43 , 0.56]
3120 * [ 0.67 , 0.90]
3121 * [ 0.86 , 1.32]
3122 * [ 1.01 , 1.79]
3123 * [ 1.15 , 2.33]
3124 * [ -1 , -1 ] (max twenty pts)
3125 * NHYDovf=[ "KEN-OV" ]
3126 *%-----|-----
3127 * -JFSA, 2021-01-19 update all KEN_BU areas based on GIS measurements
3128 CONTINUOUS STANDHYD NHYD=[ "KB-01A"], DT=[ 1]mi n, AREA=[ 40.82](ha), XI MP=[ 0.097],
3129 T I M P=[ 0.4], DWF=[ 0](cms), LOSS=[ 1]:
3129 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3130 F=[ 0.00](mm),
3130 Pervious areas: I A p e r=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 40](m),
3131 MNP=[ 0.250], SCP=[ 0](mi n),
3131 Impervious areas: I A i m p=[ 0.785](mm), SLPI=[ 0.5](%),
3132 L G I=[ 521.664](m), M N I=[ 0.013], S C I=[ 0](mi n),
3132 Continuous simulation parameters:
3133 I a R E C p e r=[ 4](hrs), I a R E C i m p=[ 4](hrs), I n t e r E v e n t T i m e=[ 12](hrs),
3133 E N D=- 1
3134 *%-----|-----
3135 COMPUTE DUALHYD NHYDin=[ "KB-01A"], CI NLET=[ 3.6](cms), NI NLET=[ 1],
3136 M a j N H Y D=[ "KB-01A-M"]
3137 M n N H Y D=[ "KB-01A-MN"]
3138 T M S T O=[ 4995](cu-m)
3139 *%-----|-----
3140 ADD HYD NHYDs um=[ "KB-01A-S"], NHYDs t o a d d=[ "KB-01A-M"+"KB-01A-MN"]
3141 *%-----|-----
3142 CONTINUOUS STANDHYD NHYD=[ "KB-01B"], DT=[ 1]mi n, AREA=[ 31.1](ha), XI MP=[ 0.1875],
3143 T I M P=[ 0.375], DWF=[ 0](cms), LOSS=[ 1]:
3143 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3144 F=[ 0.00](mm),
3144 Pervious areas: I A p e r=[ 4.67](mm), SLPP=[ 0.42](%), LGP=[ 40](m),
3145 MNP=[ 0.250], SCP=[ 0](mi n),
3145 Impervious areas: I A i m p=[ 0.785](mm), SLPI=[ 0.42](%),
3146 L G I=[ 455.339](m), M N I=[ 0.013], S C I=[ 0](mi n),
3146 Continuous simulation parameters:
3147 I a R E C p e r=[ 4](hrs), I a R E C i m p=[ 4](hrs), I n t e r E v e n t T i m e=[ 12](hrs),
3147 E N D=- 1
3148 *%-----|-----
3149 COMPUTE DUALHYD NHYDin=[ "KB-01B"], CI NLET=[ 1.585](cms), NI NLET=[ 1],
3150 M a j N H Y D=[ "KB-01B-M"]
3151 M n N H Y D=[ "KB-01B-MN"]
3152 T M S T O=[ 6075](cu-m)
3153 *%-----|-----
3154 ADD HYD NHYDs um=[ "KB-01B-S"], NHYDs t o a d d=[ "KB-01B-M"+"KB-01B-MN"]
3155 *%-----|-----
3156 CONTINUOUS STANDHYD NHYD=[ "KB-01C"], DT=[ 1]mi n, AREA=[ 13.78](ha), XI MP=[ 0.2045],
3157 T I M P=[ 0.409], DWF=[ 0](cms), LOSS=[ 1]:
3157 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3158 F=[ 0.00](mm),
3158 Pervious areas: I A p e r=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3159 MNP=[ 0.250], SCP=[ 0](mi n),
3159 Impervious areas: I A i m p=[ 0.785](mm), SLPI=[ 0.5](%),
3160 L G I=[ 303.095](m), M N I=[ 0.013], S C I=[ 0](mi n),
3160 Continuous simulation parameters:
3161 I a R E C p e r=[ 4](hrs), I a R E C i m p=[ 4](hrs), I n t e r E v e n t T i m e=[ 12](hrs),
3161 E N D=- 1

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3162 *%-----|-----|
3163 COMPUTE DUALHYD NHYDin=["KB-01C"], CILET=[1.35](cms), NILET=[1],
3164 Maj NHYD=["KB-01C-M"]
3165 MnNHYD=["KB-01C-MN"]
3166 TMSTO=[1880](cu-m)
3167 *%-----|-----|
3168 ADD HYD NHYDsum=["KB-01C-S"], NHYDsto add=["KB-01C-M"+"KB-01C-MN"]
3169 *%-----|-----|
3170 CONTINUOUS STANDHYD NHYD=["KB-03"], DT=[1]min, AREA=[84.78](ha), XI MP=[0.197],
TI MP=[0.394], DWF=[0](cms), LOSS=[1]:
3171 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3172 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3173 Impervious areas: I A imp=[0.785](mm), SLPI=[0.63](%),
LGI=[751.798](m), MNI=[0.013], SCI=[0](min),
3174 Continuous simulation parameters:
3175 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3176 *%-----|-----|
3177 COMPUTE DUALHYD NHYDin=["KB-03"], CILET=[5.27](cms), NILET=[1],
3178 Maj NHYD=["KB-03-M"]
3179 MnNHYD=["KB-03-MN"]
3180 TMSTO=[15500](cu-m)
3181 *%-----|-----|
3182 ADD HYD NHYDsum=["KB-03-S"], NHYDsto add=["KB-03-M"+"KB-03-MN"]
3183 *%-----|-----|
3184 CONTINUOUS STANDHYD NHYD=["KB-04"], DT=[1]min, AREA=[6.95](ha), XI MP=[0.85],
TI MP=[0.85], DWF=[0](cms), LOSS=[1]:
3185 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3186 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3187 Impervious areas: I A imp=[0.942](mm), SLPI=[0.5](%),
LGI=[215.252](m), MNI=[0.013], SCI=[0](min),
3188 Continuous simulation parameters:
3189 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3190 *%-----|-----|
3191 COMPUTE DUALHYD NHYDin=["KB-04"], CILET=[0.503](cms), NILET=[1],
3192 Maj NHYD=["KB-04-M"]
3193 MnNHYD=["KB-04-MN"]
3194 TMSTO=[1972](cu-m)
3195 *%-----|-----|
3196 ADD HYD NHYDsum=["KB-04-S"], NHYDsto add=["KB-04-M"+"KB-04-MN"]
3197 *%-----|-----|
3198 CONTINUOUS STANDHYD NHYD=["KB-05"], DT=[1]min, AREA=[5.19](ha), XI MP=[0.93],
TI MP=[0.93], DWF=[0](cms), LOSS=[1]:
3199 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3200 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3201 Impervious areas: I A imp=[1.57](mm), SLPI=[0.5](%),
LGI=[186.011](m), MNI=[0.013], SCI=[0](min),
3202 Continuous simulation parameters:
3203 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3204 *%-----|-----|
3205 *%-----|-----|
3206 CONTINUOUS STANDHYD NHYD=["KB-06"], DT=[1]min, AREA=[12.93](ha), XI MP=[0.873],
TI MP=[0.873], DWF=[0](cms), LOSS=[1]:
3207 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3208 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3209 Impervious areas: I A imp=[0.942](mm), SLPI=[4.75](%),

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3210          LGI=[ 293.598](m), MNI=[ 0.013], SCI=[ 0](min),
3211          Continuous simulation parameters:
          I aRECper=[ 4](hrs), I aRECImp=[ 4](hrs), I nterEventTime=[ 12](hrs),
          END=- 1
3212  *%-----|-----|
3213  COMPUTE DUALHYD  NHYDin=["KB-06"], CINLET=[ 2.262](cms), NINLET=[ 1],
3214                   MajNHYD=["KB-06-M"]
3215                   MnNHYD=["KB-06-MN"]
3216                   TMSTO=[ 1950](cu-m)
3217  *%-----|-----|
3218  ADD HYD          NHYDsum=["KB-06-S"], NHYDsto add=["KB-06-M"+"KB-06-MN"]
3219  *%-----|-----|
3220  CONTINUOUS STANDHYD  NHYD=["KB-11"], DT=[ 1]min, AREA=[ 4.03](ha), XI MP=[ 0.675],
          TIMP=[ 0.675], DWF=[ 0](cms), LOSS=[ 1]:
3221          Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
          F=[ 0.00](mm),
3222          Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
          MNP=[ 0.250], SCP=[ 0](min),
3223          Impervious areas: I Ai mp=[ 0.785](mm), SLPI=[ 2.0](%),
          LGI=[ 163.911](m), MNI=[ 0.013], SCI=[ 0](min),
3224          Continuous simulation parameters:
3225          I aRECper=[ 4](hrs), I aRECImp=[ 4](hrs), I nterEventTime=[ 12](hrs),
          END=- 1
3226  *%-----|-----|
3227  COMPUTE DUALHYD  NHYDin=["KB-11"], CINLET=[ 0.5773](cms), NINLET=[ 1],
3228                   MajNHYD=["KB-11-M"]
3229                   MnNHYD=["KB-11-MN"]
3230                   TMSTO=[ 597](cu-m)
3231  *%-----|-----|
3232  ADD HYD          NHYDsum=["KB-11-S"], NHYDsto add=["KB-11-M"+"KB-11-MN"]
3233  *%-----|-----|
3234  CONTINUOUS STANDHYD  NHYD=["S1"], DT=[ 1]min, AREA=[ 4.99](ha), XI MP=[ 0.93], TIMP=[ 0.93],
          DWF=[ 0](cms), LOSS=[ 1]:
3235          Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
          F=[ 0.00](mm),
3236          Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
          MNP=[ 0.250], SCP=[ 0](min),
3237          Impervious areas: I Ai mp=[ 1.57](mm), SLPI=[ 2.0](%),
          LGI=[ 182.392](m), MNI=[ 0.013], SCI=[ 0](min),
3238          Continuous simulation parameters:
3239          I aRECper=[ 4](hrs), I aRECImp=[ 4](hrs), I nterEventTime=[ 12](hrs),
          END=- 1
3240  *%-----|-----|
3241  CONTINUOUS STANDHYD  NHYD=["KB-15"], DT=[ 1]min, AREA=[ 2.15](ha), XI MP=[ 0.79],
          TIMP=[ 0.79], DWF=[ 0](cms), LOSS=[ 1]:
3242          Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
          F=[ 0.00](mm),
3243          Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
          MNP=[ 0.250], SCP=[ 0](min),
3244          Impervious areas: I Ai mp=[ 0.157](mm), SLPI=[ 0.3](%),
          LGI=[ 119.722](m), MNI=[ 0.013], SCI=[ 0](min),
3245          Continuous simulation parameters:
3246          I aRECper=[ 4](hrs), I aRECImp=[ 4](hrs), I nterEventTime=[ 12](hrs),
          END=- 1
3247  *%-----|-----|
3248  *%-----|-----|
3249  ADD HYD          NHYDsum=["KB-P1"], NHYDsto
          add=["KB-01A-S"+"KB-01B-S"+"KB-01C-S"+"KB-03-S"+"KB-04-S"+"KB-05"+"KB-06-S"+"KB-11-S"+"KB
          -15"+"S1"]
3250  *%-----|-----|
3251  ROUTE RESERVOIR  NHYDout=["KB-P1R"], NHYDin=["KB-P1"],
3252                   RDT=[ 1](min),
3253                   TABLE of ( OUTFLOW STORAGE ) values
3254                   ( cms ) - ( ha-m)
3255                   [ 0.0 , 0.0 ]
3256                   [ 0.076, 0.003 ]

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```

3257 [ 0.088, 0.006]
3258 [ 0.136, 0.011]
3259 [ 0.301, 0.017]
3260 [ 0.454, 0.027]
3261 [ 0.631, 0.041]
3262 [ 1.173, 0.068]
3263 [ 1.91, 0.111]
3264 [ 4.847, 0.231]
3265 [ 9.813, 0.436]
3266 [ 12.134, 0.617]
3267 [ 12.438, 0.732]
3268 [ 12.424, 0.811]
3269 [ 12.425, 0.894]
3270 [ -1 , -1 ] (max twenty pts)
3271 NHYDovf=[ "KB- P1ovf" ]
3272 *%-----|-----|
3273 ADD HYD NHYDsum=[ "KB- Pond1" ], NHYDs to add=[ "KB- P1R"+"KB- P1ovf" ]
3274 *%-----|-----|
3275 SAVE HYD NHYD=[ "KB- Pond1" ], # OF PCYCLES=[ -1 ], ICASEsh=[ 1 ]
3276 HYD_COMMENT=[ "Total Flows at KB first pond" ]
3277 *%-----|-----|
3278 CONTINUOUS STANDHYD NHYD=[ "KB- 07" ], DT=[ 1 ] min, AREA=[ 10.86 ] (ha), XI MP=[ 0.86 ],
TI MP=[ 0.86 ], DWF=[ 0 ] (cms), LOSS=[ 1 ]:
3279 Horton: Fo=[ 76.20 ] (mm/ hr), Fc=[ 13.20 ] (mm/ hr), DCAY=[ 4.14 ] (/ hr),
F=[ 0.00 ] (mm),
3280 Pervious areas: I A per=[ 4.67 ] (mm), SLPP=[ 2.0 ] (%), LGP=[ 40 ] (m),
MNP=[ 0.250 ], SCP=[ 0 ] (min),
3281 Impervious areas: I A i mp=[ 0.785 ] (mm), SLPI=[ 2.0 ] (%),
LGI=[ 269.072 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (min),
3282 Continuous simulation parameters:
3283 I a REC per=[ 4 ] (hrs), I a REC i mp=[ 4 ] (hrs), I nter Event Ti me=[ 12 ] (hrs),
END=- 1
3284 *%-----|-----|
3285 COMPUTE DUALHYD NHYD i n=[ "KB- 07" ], CI NLET=[ 2.094 ] (cms), NI NLET=[ 1 ],
3286 M a j NHYD=[ "KB- 07- M" ]
3287 M n NHYD=[ "KB- 07- MN" ]
3288 T M S TO=[ 1378 ] (cu- m)
3289 *%-----|-----|
3290 ADD HYD NHYDsum=[ "KB- 07- S" ], NHYDs to add=[ "KB- 07- M"+"KB- 07- MN" ]
3291 *%-----|-----|
3292 CONTINUOUS STANDHYD NHYD=[ "KB- 08" ], DT=[ 1 ] min, AREA=[ 6.61 ] (ha), XI MP=[ 0.64 ],
TI MP=[ 0.64 ], DWF=[ 0 ] (cms), LOSS=[ 1 ]:
3293 Horton: Fo=[ 76.20 ] (mm/ hr), Fc=[ 13.20 ] (mm/ hr), DCAY=[ 4.14 ] (/ hr),
F=[ 0.00 ] (mm),
3294 Pervious areas: I A per=[ 4.67 ] (mm), SLPP=[ 2.0 ] (%), LGP=[ 40 ] (m),
MNP=[ 0.250 ], SCP=[ 0 ] (min),
3295 Impervious areas: I A i mp=[ 0.785 ] (mm), SLPI=[ 2.0 ] (%),
LGI=[ 209.921 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (min),
3296 Continuous simulation parameters:
3297 I a REC per=[ 4 ] (hrs), I a REC i mp=[ 4 ] (hrs), I nter Event Ti me=[ 12 ] (hrs),
END=- 1
3298 *%-----|-----|
3299 COMPUTE DUALHYD NHYD i n=[ "KB- 08" ], CI NLET=[ 1.058 ] (cms), NI NLET=[ 1 ],
3300 M a j NHYD=[ "KB- 08- M" ]
3301 M n NHYD=[ "KB- 08- MN" ]
3302 T M S TO=[ 787 ] (cu- m)
3303 *%-----|-----|
3304 ADD HYD NHYDsum=[ "KB- 08- S" ], NHYDs to add=[ "KB- 08- M"+"KB- 08- MN" ]
3305 *%-----|-----|
3306 CONTINUOUS STANDHYD NHYD=[ "KB- 09" ], DT=[ 1 ] min, AREA=[ 2.6 ] (ha), XI MP=[ 0.86 ],
TI MP=[ 0.86 ], DWF=[ 0 ] (cms), LOSS=[ 1 ]:
3307 Horton: Fo=[ 76.20 ] (mm/ hr), Fc=[ 13.20 ] (mm/ hr), DCAY=[ 4.14 ] (/ hr),
F=[ 0.00 ] (mm),
3308 Pervious areas: I A per=[ 4.67 ] (mm), SLPP=[ 2.0 ] (%), LGP=[ 40 ] (m),
MNP=[ 0.250 ], SCP=[ 0 ] (min),
3309 Impervious areas: I A i mp=[ 1.57 ] (mm), SLPI=[ 2.0 ] (%),

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3310          LGI=[ 131.656](m), MNI=[ 0.013], SCI=[ 0](min),
3311          Continuous simulation parameters:
3312          IaRECPer=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3313          END=- 1
3314          *%-----|-----|
3315          CONTINUOUS STANDHYD NHYD=["KB- 10_1"], DT=[ 1]min, AREA=[ 2.37](ha), XI MP=[ 0.86],
3316          T I MP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3317          Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3318          F=[ 0.00](mm),
3319          Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3320          MNP=[ 0.250], SCP=[ 0](min),
3321          Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 2.0](%),
3322          LGI=[ 125.698](m), MNI=[ 0.013], SCI=[ 0](min),
3323          Continuous simulation parameters:
3324          IaRECPer=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3325          END=- 1
3326          *%-----|-----|
3327          CONTINUOUS STANDHYD NHYD=["KB- 10_2"], DT=[ 1]min, AREA=[ 1.14](ha), XI MP=[ 0.86],
3328          T I MP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3329          Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3330          F=[ 0.00](mm),
3331          Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3332          MNP=[ 0.250], SCP=[ 0](min),
3333          Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 2.0](%), LGI=[ 87.178](m),
3334          MNI=[ 0.013], SCI=[ 0](min),
3335          Continuous simulation parameters:
3336          IaRECPer=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3337          END=- 1
3338          *%-----|-----|
3339          CONTINUOUS STANDHYD NHYD=["KB- 12"], DT=[ 1]min, AREA=[ 4.86](ha), XI MP=[ 0.79],
3340          T I MP=[ 0.79], DWF=[ 0](cms), LOSS=[ 1]:
3341          Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3342          F=[ 0.00](mm),
3343          Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3344          MNP=[ 0.250], SCP=[ 0](min),
3345          Impervious areas: IAimp=[ 1.099](mm), SLPI=[ 2.0](%),
3346          LGI=[ 180.000](m), MNI=[ 0.013], SCI=[ 0](min),
3347          Continuous simulation parameters:
3348          IaRECPer=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3349          END=- 1
3350          *%-----|-----|
3351          COMPUTE DUALHYD NHYDin=["KB- 12"], C I NLET=[ 0.8665](cms), N I NLET=[ 1],
3352          M a j NHYD=["KB- 12- M "]
3353          M n NHYD=["KB- 12- MN "]
3354          T M S T O=[ 632](cu-m)
3355          *%-----|-----|
3356          ADD HYD NHYDs um=["KB- 12- S"], NHYDs t o add=["KB- 12- M "+"KB- 12- MN "]
3357          *%-----|-----|
3358          CONTINUOUS STANDHYD NHYD=["KB- 13"], DT=[ 1]min, AREA=[ 10.19](ha), XI MP=[ 0.64],
3359          T I MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3360          Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3361          F=[ 0.00](mm),
3362          Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3363          MNP=[ 0.250], SCP=[ 0](min),
3364          Impervious areas: IAimp=[ 0.785](mm), SLPI=[ 2.0](%),
3365          LGI=[ 260.640](m), MNI=[ 0.013], SCI=[ 0](min),
3366          Continuous simulation parameters:
3367          IaRECPer=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3368          END=- 1
3369          *%-----|-----|
3370          COMPUTE DUALHYD NHYDin=["KB- 13"], C I NLET=[ 1.722](cms), N I NLET=[ 1],
3371          M a j NHYD=["KB- 13- M "]
3372          M n NHYD=["KB- 13- MN "]
3373          T M S T O=[ 1077](cu-m)

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3354 *%-----|-----|
3355 ADD HYD          NHYDs um=[ " KB- 13- S" ], NHYDs to add=[ " KB- 13- M" +" KB- 13- MN" ]
3356 *%-----|-----|
3357 CONTINUOUS STANDHYD NHYD=[ " KB- 14" ], DT=[ 1] mi n, AREA=[ 5. 47] ( ha ), XI MP=[ 0. 64 ],
TI MP=[ 0. 64 ], DWF=[ 0] ( cms ), LOSS=[ 1 ] :
3358 Horton: Fo=[ 76. 20] ( mm/ hr ), Fc=[ 13. 20] ( mm/ hr ), DCAY=[ 4. 14] ( / hr ),
F=[ 0. 00] ( mm ),
3359 Pervious areas: IAper=[ 4. 67] ( mm ), SLPP=[ 2. 0] ( % ), LGP=[ 40] ( m ),
MNP=[ 0. 250 ], SCP=[ 0] ( mi n ),
3360 Impervious areas: IAi mp=[ 0. 785] ( mm ), SLPI =[ 2. 0] ( % ),
LGI =[ 190. 962] ( m ), MNI =[ 0. 013 ], SCI =[ 0] ( mi n ),
3361 Continuous simulation parameters:
3362 IaREcper=[ 4] ( hr s ), IaRECi mp=[ 4] ( hr s ), Int er Event Ti me=[ 12] ( hr s ),
END=- 1
3363 *%-----|-----|
3364 COMPUTE DUALHYD   NHYDi n=[ " KB- 14" ], CI NLET=[ 0. 8734] ( cms ), NI NLET=[ 1 ],
3365 M aj NHYD=[ " KB- 14- M" ]
3366 M nNHYD=[ " KB- 14- MN" ]
3367 TM STO=[ 631] ( cu- m )
3368 *%-----|-----|
3369 ADD HYD          NHYDs um=[ " KB- 14- S" ], NHYDs to add=[ " KB- 14- M" +" KB- 14- MN" ]
3370 *%-----|-----|
3371 *%-----|-----|
3372 CONTINUOUS STANDHYD NHYD=[ " KB- 16_2" ], DT=[ 1] mi n, AREA=[ 3. 42] ( ha ), XI MP=[ 0. 71 ],
TI MP=[ 0. 71 ], DWF=[ 0] ( cms ), LOSS=[ 1 ] :
3373 Horton: Fo=[ 76. 20] ( mm/ hr ), Fc=[ 13. 20] ( mm/ hr ), DCAY=[ 4. 14] ( / hr ),
F=[ 0. 00] ( mm ),
3374 Pervious areas: IAper=[ 4. 67] ( mm ), SLPP=[ 2. 0] ( % ), LGP=[ 40] ( m ),
MNP=[ 0. 250 ], SCP=[ 0] ( mi n ),
3375 Impervious areas: IAi mp=[ 0. 157] ( mm ), SLPI =[ 0. 3] ( % ),
LGI =[ 150. 997] ( m ), MNI =[ 0. 013 ], SCI =[ 0] ( mi n ),
3376 Continuous simulation parameters:
3377 IaREcper=[ 4] ( hr s ), IaRECi mp=[ 4] ( hr s ), Int er Event Ti me=[ 12] ( hr s ),
END=- 1
3378 *%-----|-----|
3379 ADD HYD          NHYDs um=[ " KB- P2" ], NHYDs to
add=[ " KB- Pond1" +" KB- 07- S" +" KB- 08- S" +" KB- 09" +" KB- 10_1" +" KB- 10_2" +" KB- 12- S" +" KB- 13- S" +" KB- 1
4- S" +" KB- 16_2" ]
3380 *%-----|-----|
3381 ROUTE RESERVOIR  NHYDout =[ " KB- P2R" ], NHYDi n=[ " KB- P2" ],
3382 RDT=[ 1] ( mi n ),
3383 TABLE of ( OUTFLOW STORAGE ) values
3384 ( cms ) - ( ha- m )
3385 [ 0. 0 , 0. 0 ]
3386 [ 0. 053, 0. 005 ]
3387 [ 0. 132, 0. 009 ]
3388 [ 0. 269, 0. 014 ]
3389 [ 0. 455, 0. 023 ]
3390 [ 0. 699, 0. 037 ]
3391 [ 0. 947, 0. 056 ]
3392 [ 1. 853, 0. 09 ]
3393 [ 2. 712, 0. 146 ]
3394 [ 6. 626, 0. 287 ]
3395 [ 11. 228, 0. 515 ]
3396 [ 14. 885, 0. 738 ]
3397 [ 16. 473, 0. 893 ]
3398 [ 17. 311, 0. 998 ]
3399 [ 17. 633, 1. 063 ]
3400 [ 17. 634, 1. 112 ]
3401 [ - 1 , - 1 ] ( max twenty pts )
3402 NHYDovf=[ " KB- P2ovf" ]
3403 *%-----|-----|
3404 ADD HYD          NHYDs um=[ " KB- Pond2" ], NHYDs to add=[ " KB- P2R" +" KB- P2ovf" ]
3405 *%-----|-----|
3406 SAVE HYD        NHYD=[ " KB- Pond2" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
3407 HYD_COMMENT=[ " Total Flows at KB second pond" ]

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3408 *%-----|-----|
3409 CONTINUOUS STANDHYD NHYD=["KB-16_1"], DT=[1]min, AREA=[2.8](ha), XI MP=[0.75],
TI MP=[0.75], DWF=[0](cms), LOSS=[1]:
3410 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3411 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3412 Impervious areas: I A i mp=[0.157](mm), SLPI=[0.3](%),
LGI=[136.626](m), MNI=[0.013], SCI=[0](min),
3413 Continuous simulation parameters:
3414 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3415 *%-----|-----|
3416 ADD HYD NHYDsum=["KB-P3"], NHYDs to add=["KB-Pond2"+"KB-16_1"]
3417 *%-----|-----|
3418 *%-----|-----|
3419 * One inflow node from pond 3 is added to the model (ROUTE RESERVOIR)
3420 * Another inflow node from right side of pond 3 is not added to the model
3421 ROUTE RESERVOIR NHYDout=["KB-P3R"], NHYDin=["KB-P3"],
3422 RDT=[1](min),
3423 TABLE of ( OUTFLOW STORAGE ) values
3424 (cms) - (ha-m)
3425 [ 0.0 , 0.0 ]
3426 [0.051, 0.002]
3427 [0.048, 0.003]
3428 [0.057, 0.029]
3429 [0.089, 0.045]
3430 [0.133, 0.069]
3431 [0.199, 0.106]
3432 [0.321, 0.172]
3433 [1.029, 0.306]
3434 [4.036, 0.527]
3435 [8.332, 0.761]
3436 [11.727, 0.941]
3437 [14.125, 1.067]
3438 [15.675, 1.149]
3439 [16.555, 1.196]
3440 [16.911, 1.214]
3441 [ -1 , -1 ] (max twenty pts)
3442 NHYDovf=["KB-P3ovf"]
3443 *%-----|-----|
3444 ADD HYD NHYDsum=["KB-Pond3"], NHYDs to add=["KB-P3R"+"KB-P3ovf"]
3445 *%-----|-----|
3446 SAVE HYD NHYD=["KB-Pond3"], # OF PCYCLES=[-1], I CASEs h=[1]
3447 HYD_COMMENT=["Total Flows at KB third pond"]
3448 *%-----|-----|
3449 *#*****
3450 *# EXISTING / PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM
Modeling Approach, NOVATECH Report June, 2020)
3451 *# - TO FRASER-CLARKE DRAIN
3452 *#*****
3453 CONTINUOUS STANDHYD NHYD=["FC-01"], DT=[1]min, AREA=[8.03](ha), XI MP=[0.47],
TI MP=[0.47], DWF=[0](cms), LOSS=[1]:
3454 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3455 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3456 Impervious areas: I A i mp=[1.57](mm), SLPI=[1.0](%),
LGI=[231.373](m), MNI=[0.013], SCI=[0](min),
3457 Continuous simulation parameters:
3458 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3459 *%-----|-----|
3460 COMPUTE DUALHYD NHYDin=["FC-01"], C I NLET=[0.756](cms), N I NLET=[1],
3461 M i j NHYD=["FC-01-M"]
3462 M n NHYD=["FC-01-MN"]

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3463          TMI STO=[ 714] ( cu- m)
3464 *%-----|-----|
3465 ADD HYD      NHYDs um=[ "FC- 01- S" ], NHYDs  to  add=[ "FC- 01- M" +"FC- 01- MN" ]
3466 *%-----|-----|
3467 CONTI NUOUS STANDHYD NHYD=[ "FC- 02" ], DT=[ 1] mi n, AREA=[ 16. 05] ( ha), XI MP=[ 0. 93],
TI MP=[ 0. 93], DWF=[ 0] ( cms), LOSS=[ 1]:
3468          Hort on: Fo=[ 76. 20] ( mm/ hr), Fc=[ 13. 20] ( mm/ hr), DCAY=[ 4. 14] (/ hr),
F=[ 0. 00] ( mm),
3469          Perv ious  areas: IAper=[ 4. 67] ( mm), SLPP=[ 2. 0] ( %), LGP=[ 40] ( m),
MNP=[ 0. 250], SCP=[ 0] ( mi n),
3470          Imperv ious  areas: IAi mp=[ 1. 57] ( mm), SLPI=[ 1. 0] ( %),
LGI=[ 327. 109] ( m), MNI=[ 0. 013], SCI=[ 0] ( mi n),
3471          Continuous simulation parameters:
3472          IaRECper=[ 4] ( hr s), IaRECI mp=[ 4] ( hr s), Int er Event Ti me=[ 12] ( hr s),
END=- 1
3473 *%-----|-----|
3474 COMPUTE DUALHYD NHYDi n=[ "FC- 02" ], CI NLET=[ 1. 159] ( cms), NI NLET=[ 1],
3475          Mj NHYD=[ "FC- 02- M" ]
3476          MnNHYD=[ "FC- 02- MN" ]
3477          TMI STO=[ 2385] ( cu- m)
3478 *%-----|-----|
3479 ADD HYD      NHYDs um=[ "FC- 02- S" ], NHYDs  to  add=[ "FC- 02- M" +"FC- 02- MN" ]
3480 *%-----|-----|
3481 CONTI NUOUS STANDHYD NHYD=[ "FC- 03" ], DT=[ 1] mi n, AREA=[ 7. 37] ( ha), XI MP=[ 0. 64],
TI MP=[ 0. 64], DWF=[ 0] ( cms), LOSS=[ 1]:
3482          Hort on: Fo=[ 76. 20] ( mm/ hr), Fc=[ 13. 20] ( mm/ hr), DCAY=[ 4. 14] (/ hr),
F=[ 0. 00] ( mm),
3483          Perv ious  areas: IAper=[ 4. 67] ( mm), SLPP=[ 2. 0] ( %), LGP=[ 40] ( m),
MNP=[ 0. 250], SCP=[ 0] ( mi n),
3484          Imperv ious  areas: IAi mp=[ 1. 57] ( mm), SLPI=[ 1. 0] ( %),
LGI=[ 221. 660] ( m), MNI=[ 0. 013], SCI=[ 0] ( mi n),
3485          Continuous simulation parameters:
3486          IaRECper=[ 4] ( hr s), IaRECI mp=[ 4] ( hr s), Int er Event Ti me=[ 12] ( hr s),
END=- 1
3487 *%-----|-----|
3488 COMPUTE DUALHYD NHYDi n=[ "FC- 03" ], CI NLET=[ 0. 358] ( cms), NI NLET=[ 1],
3489          Mj NHYD=[ "FC- 03- M" ]
3490          MnNHYD=[ "FC- 03- MN" ]
3491          TMI STO=[ 1131] ( cu- m)
3492 *%-----|-----|
3493 ADD HYD      NHYDs um=[ "FC- 03- S" ], NHYDs  to  add=[ "FC- 03- M" +"FC- 03- MN" ]
3494 *%-----|-----|
3495 CONTI NUOUS STANDHYD NHYD=[ "FC- 04" ], DT=[ 1] mi n, AREA=[ 12. 87] ( ha), XI MP=[ 0. 64],
TI MP=[ 0. 64], DWF=[ 0] ( cms), LOSS=[ 1]:
3496          Hort on: Fo=[ 76. 20] ( mm/ hr), Fc=[ 13. 20] ( mm/ hr), DCAY=[ 4. 14] (/ hr),
F=[ 0. 00] ( mm),
3497          Perv ious  areas: IAper=[ 4. 67] ( mm), SLPP=[ 2. 0] ( %), LGP=[ 40] ( m),
MNP=[ 0. 250], SCP=[ 0] ( mi n),
3498          Imperv ious  areas: IAi mp=[ 1. 57] ( mm), SLPI=[ 1. 0] ( %),
LGI=[ 292. 916] ( m), MNI=[ 0. 013], SCI=[ 0] ( mi n),
3499          Continuous simulation parameters:
3500          IaRECper=[ 4] ( hr s), IaRECI mp=[ 4] ( hr s), Int er Event Ti me=[ 12] ( hr s),
END=- 1
3501 *%-----|-----|
3502 COMPUTE DUALHYD NHYDi n=[ "FC- 04" ], CI NLET=[ 0. 741] ( cms), NI NLET=[ 1],
3503          Mj NHYD=[ "FC- 04- M" ]
3504          MnNHYD=[ "FC- 04- MN" ]
3505          TMI STO=[ 1794] ( cu- m)
3506 *%-----|-----|
3507 ADD HYD      NHYDs um=[ "FC- 04- S" ], NHYDs  to  add=[ "FC- 04- M" +"FC- 04- MN" ]
3508 *%-----|-----|
3509 *#*****
3510 *#      PROPOSED Subcatchments ( Kennedy- Burnett SWM Facility ( 118080), SWM Mdeling
Approach, NOVATECH Report June, 2020)
3511 *#      - TO JOCK RI VER
3512 *#*****

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3513 CONTINUOUS STANDHYD NHYD=["JR-01"], DT=[1] min, AREA=[8.24](ha), XI MP=[0.64],
TI MP=[0.64], DWF=[0](cms), LOSS=[1]:
3514 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3515 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3516 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[234.379](m), MNI=[0.013], SCI=[0](min),
3517 Continuous simulation parameters:
3518 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3519 *%-----|
3520 COMPUTE DUALHYD NHYDin=["JR-01"], CINLET=[0.563](cms), NINLET=[1],
3521 MajNHYD=["JR-01-M"]
3522 MinNHYD=["JR-01-MN"]
3523 TMSSTO=[1040](cu-m)
3524 *%-----|
3525 ADD HYD NHYDsum=["JR-01-S"], NHYDsto add=["JR-01-M"+"JR-01-MN"]
3526 *%-----|
3527 CONTINUOUS STANDHYD NHYD=["JR-02"], DT=[1] min, AREA=[1.59](ha), XI MP=[0.64],
TI MP=[0.64], DWF=[0](cms), LOSS=[1]:
3528 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3529 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3530 Impervious areas: IAimp=[1.57](mm), SLPI=[1.0](%),
LGI=[102.956](m), MNI=[0.013], SCI=[0](min),
3531 Continuous simulation parameters:
3532 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
END=-1
3533 *%-----|
3534 COMPUTE DUALHYD NHYDin=["JR-02"], CINLET=[0.153](cms), NINLET=[1],
3535 MajNHYD=["JR-02-M"]
3536 MinNHYD=["JR-02-MN"]
3537 TMSSTO=[153](cu-m)
3538 *%-----|
3539 ADD HYD NHYDsum=["JR-02-S"], NHYDsto add=["JR-02-M"+"JR-02-MN"]
3540 *%-----|
3541 *#*****
3542 *# Catchment FRASER
3543 *# - To Fraser-Clarke drain (north of the Jock)
3544 *# - Developed land with assumed 43% imp.
3545 *# - 2020-12-17 Change Fraser area to be 35.1 as measured from QGIS
3546 *# - 2020-12-17 All Fraser is undeveloped (Nashyd)
3547 *#*****
3548 CONTINUOUS NASHYD NHYD=["FRASER-DRN"], DT=[1] min, AREA=[13.65](ha),
3549 DWF=[0](cms), CNVC=[77], IA=[4.67](mm),
3550 N=[3], TP=[0.4258]hrs,
3551 Continuous simulation parameters:
3552 IaRECper=[4](hrs),
3553 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3554 InterEventTime=[12](hrs)
3555 Baseflow simulation parameters:
3556 BaseFlowOption=[1],
3557 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3558 VHydCond=[0.055](mm/hr), END=-1
3559 *%-----|
3560 *CONTINUOUS STANDHYD NHYD=["FRASER-D"], DT=[1] min, AREA=[21.61](ha),
3561 * XI MP=[0.585], TI MP=[0.585], DWF=[0](cms), LOSS=[2],
3562 * SCS curve number CN=[80],
3563 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3564 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3565 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3566 * LGI=[379.561](m), MNI=[0.013], SCI=[0](min),
3567 * Continuous simulation parameters:
3568 * IaRECper=[4](hrs), IaRECImp=[4](hrs),

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3569 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3570 * InterEventTime=[18](hrs), END=-1
3571 */-----|-----|
3572 CONTINUOUS NASHYD NHYD=["FRASER-D"], DT=[1]min, AREA=[21.61](ha),
3573 DWF=[0](cms), CNVC=[77], IA=[4.67](mm),
3574 N=[3], TP=[0.674]hrs,
3575 Continuous simulation parameters:
3576 IaRECper=[4](hrs),
3577 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3578 InterEventTime=[12](hrs)
3579 Baseflow simulation parameters:
3580 BaseFlowOption=[1],
3581 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
3582 VHydCond=[0.055](mm/hr), END=-1
3583 */-----|-----|
3584 * COMPUTE DUALHYD NHYDin=["FRASER-D"], CINLET=[3.545](cms), NINLET=[1],
3585 * MjNHYD=["FRASER-J"]
3586 * MnNHYD=["FRASER-N"]
3587 * TMSTO=[9999999](cu-m)
3588 */-----|-----|
3589 * ADD HYD NHYDs um=["FRASER-S"], NHYDs to add=["FRASER-J"+"FRASER-N"]
3590 */-----|-----|
3591 * ROUTE RESERVOIR NHYDout=["MS_P20"], NHYDin=["FRASER"],
3592 * RDT=[1](min),
3593 * TABLE of ( OUTFLOW STORAGE ) values
3594 * ( cms ) - ( ha-m )
3595 * [ 0.0 , 0.0 ]
3596 * [ 0.04 , 0.36 ]
3597 * [ -1 , -1 ] (max twenty pts)
3598 * NHYDovf=["P20-OVF"]
3599 */-----|-----|
3600 * ADD HYD NHYDs um=["4241"], NHYDs to
add=["KB-Pond3"+"S-1-B"+"FRASER-DRN"+"FRASER-D"+"N_KB"+"FC-01-S"+"FC-02-S"+"FC-03-S"]
3601 */-----|-----|
3602 * SAVE HYD NHYD=["4241"], # OF PCYCLES=[-1], ICASEsh=[1]
3603 HYD_COMMENT=["Total Flows at Ken-Burnett Outlet"]
3604 */-----|-----|
3605 *# Hydrograph from Node Ken-Burnett to station 3633
3606 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4241
3607 *#
3608 * ROUTE CHANNEL NHYDout=["4241-out"], NHYDin=["4241"], RDT=[1](min),
3609 CHLGTH=[294](m), CHSLOPE=[0.1088](%), FPSLOPE=[0.1088](%),
3610 SECNUM=[1.0], NSEG=[3]
3611 ( SEGROUGH, SEGDIST (m) )=[0.05, -20.12
3612 -0.035, 45.26
3613 0.05, 403.84] NSEG times
3614 ( DISTANCE (m), ELEVATION (m) )=[
3615 [-909.72, 95 ]
3616 [-907.09, 94.5 ]
3617 [-904.65, 94 ]
3618 [-902.26, 93.5 ]
3619 [-44.51, 91.5 ]
3620 [-25.1, 91.5 ]
3621 [-20.98, 91 ]
3622 [-20.61, 90.5 ]
3623 [-20.12, 90 ]
3624 [-6.13, 87.26 ]
3625 [17.51, 86.56 ]
3626 [31.37, 87.2 ]
3627 [45.26, 90 ]
3628 [50.41, 90.5 ]
3629 [63.06, 91 ]
3630 [134.5, 91.5 ]
3631 [190.63, 92 ]
3632 [251.98, 92.5 ]
3633 [321.32, 93.5 ]

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3634 [ 403.84, 95 ]
3635 *%-----|-----|
3636 ADD HYD NHYDsum=["SN_KB"], NHYDs to
add=["4241-out"+"FC-04-S"+"JR-01-S"+"JR-02-S"]
3637 *%-----|-----|
3638 SAVE HYD NHYD=["SN_KB"], # OF PCYCLES=[-1], ICASEsh=[1]
3639 HYD_COMMENT=["Total Flows before Station 3633"]
3640 *%-----|-----|
3641 *# Hydrograph from Station 3633 to Node Todd
3642 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
3643 *# JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and
change the slope from 0.0498% to 0.24671% That is because of adding station 4241
between station 4534 and station 3633
3644 *#
3645 ROUTE CHANNEL NHYDout=["N_TO"], NHYDin=["SN_KB"], RDT=[1](min),
3646 CHLGTH=[608](m), CHSLOPE=[0.24671](%), FPSLOPE=[0.24671](%),
3647 SECNUM=[1.0], NSEG=[3]
3648 (SEGROUGH, SEGDI ST (m))=[0.05, -23.74
3649 -0.035, 23.74
3650 0.05, 26.50] NSEG times
3651 (DISTANCE (m), ELEVATION (m))=[
3652 -29.24, 91.0
3653 -27.41, 90.5
3654 -25.64, 90
3655 -23.74, 89.5
3656 -22, 89.26
3657 -20, 88.51
3658 -19, 88.32
3659 -15, 88.1
3660 -10, 88.11
3661 -5, 88.17
3662 0, 88.27
3663 5, 88.19
3664 10, 88.06
3665 15, 88.48
3666 16, 88.7
3667 23.74, 89.5
3668 24.68, 90
3669 25.57, 90.5
3670 26.50, 91.0
3671 * [-29.24, 91]
3672 * [-27.41, 90.5]
3673 * [-25.64, 90]
3674 * [-23.74, 89.5]
3675 * [-22, 89.26]
3676 * [-20, 88.51]
3677 * [-19, 88.32]
3678 * [-15, 88.1]
3679 * [-10, 88.11]
3680 * [-5, 88.17]
3681 * [0, 88.27]
3682 * [5, 88.19]
3683 * [10, 88.06]
3684 * [15, 88.48]
3685 * [16, 88.7]
3686 * [23.74, 89.5]
3687 * [24.68, 90]
3688 * [25.57, 90.5]
3689 *%-----|-----|
3690 *#*****
3691 *# Catchment Greenbank
3692 *# - To Greenbank Drain (south of the Jock)
3693 *# - JFSA 2021-01-18 add Greenbank pond as per JFSA, P598(06)-15, June 2016
3694 *# - JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
3695 *#*****
3696 CONTINUOUS STANDHYD NHYD=["Greenbank"], DT=[1]min, AREA=[36.6](ha),

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3697      XI MP=[ 0. 639], TI MP=[ 0. 682], DWF=[ 0](cms), LOSS=[ 2],
3698      SCS curve number CN=[ 77],
3699      Pervious surfaces: I A per=[ 4. 67](mm), SLPP=[ 1](%),
3700      LGP=[ 40](m), MNP=[ 0. 25], SCP=[ 0](mi n),
3701      Impervious surfaces: I A i mp=[ 1. 57](mm), SLPI=[ 1](%),
3702      LGI=[ 493. 96](m), MNI=[ 0. 013], SCI=[ 0](mi n),
3703      Continuous simulation parameters:
3704      I a REC per=[ 4](hrs), I a REC i mp=[ 4](hrs),
3705      SM N=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0. 010]/(mm),
3706      Inter Event Ti me=[ 18](hrs), END=- 1
3707  *%-----|-----|
3708  ROUTE RESERVOIR  NHYDout=["GreenB_MN"], NHYDin=["Greenbank"],
3709      RDT=[ 1](mi n),
3710      TABLE of ( OUTFLOW STORAGE ) values
3711      (cms) - (ha-m)
3712      [ 0. 0 , 0. 0 ]
3713      [ 0. 033 , 0. 084 ]
3714      [ 0. 039 , 0. 201 ]
3715      [ 0. 113 , 0. 292 ]
3716      [ 0. 237 , 0. 386 ]
3717      [ 0. 382 , 0. 484 ]
3718      [ 0. 539 , 0. 585 ]
3719      [ 0. 7 , 0. 692 ]
3720      [ 0. 86 , 0. 804 ]
3721      [ 4. 684 , 0. 922 ]
3722      [ 11. 539 , 1. 052 ]
3723      [ 20. 867 , 1. 168 ]
3724      [ 103. 616 , 1. 974 ]
3725      [ - 1 , - 1 ] (max twenty pts)
3726      NHYDovf=["GreenB_M"],
3727  *%-----|-----|
3728  *%-----|-----|
3729  ADD HYD          NHYDs um=["GreenB"], NHYDs to add=["N_TO"+"GreenB_M"+"GreenB_MN"]
3730  *%-----|-----|
3731  SAVE HYD        NHYD=["GreenB"], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
3732      HYD_COMMENT=["Total Flows at Greenbank Drain"]
3733  *%-----|-----|
3734  *#*****|*****|
3735  *# Catchment TODD
3736  *# - To Todd Drain (south of the Jock)
3737  *# - Subdivision with 43% imp. as per Barrhaven South MSS
3738  *# - 2020-11-30 increase imp. based on P598(04)-11
3739  *# - 2020-11-30 update TODD Tributary Drainage Area to = 146.015 ha based on
3740  P598(04)-11
3741  *# - 2020-11-30 split TODD Drainage Area to MAJOR, MINOR, POND and ALL
3742  *#*****|*****|
3743  *# - JFSA 2021-01-19 add "TODD_MN1" as part of Clarke("W_CLAR_M") and remove it
3744  from Todd
3745  *CONTINUOUS STANDHYD NHYD=["TODD_MN1"], DT=[ 1]mi n, AREA=[ 1. 772](ha),
3746  * XI MP=[ 0. 53], TI MP=[ 0. 57], DWF=[ 0](cms), LOSS=[ 2],
3747  * SCS curve number CN=[ 77],
3748  * Pervious surfaces: I A per=[ 4. 67](mm), SLPP=[ 1](%),
3749  * LGP=[ 40](m), MNP=[ 0. 25], SCP=[ 0](mi n),
3750  * Impervious surfaces: I A i mp=[ 1. 57](mm), SLPI=[ 1](%),
3751  * LGI=[ 108. 689](m), MNI=[ 0. 013], SCI=[ 0](mi n),
3752  * Continuous simulation parameters:
3753  * I a REC per=[ 4](hrs), I a REC i mp=[ 4](hrs),
3754  * SM N=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0. 010]/(mm),
3755  * Inter Event Ti me=[ 18](hrs), END=- 1
3756  *%-----|-----|
3757  CONTINUOUS STANDHYD NHYD=["TODD_MN2"], DT=[ 1]mi n, AREA=[ 2. 1](ha),
3758  XI MP=[ 0. 53], TI MP=[ 0. 57], DWF=[ 0](cms), LOSS=[ 2],
3759  SCS curve number CN=[ 77],
3760  Pervious surfaces: I A per=[ 4. 67](mm), SLPP=[ 1](%),
3761  LGP=[ 40](m), MNP=[ 0. 25], SCP=[ 0](mi n),
3762  Impervious surfaces: I A i mp=[ 1. 57](mm), SLPI=[ 1](%),

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3761             LGI=[ 118.322](m), MNI=[ 0.013], SCI=[ 0](min),
3762 Continuous simulation parameters:
3763 IaRECPER=[ 4](hrs), IaRECI MP=[ 4](hrs),
3764 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3765 InterEventTime=[ 18](hrs), END=- 1
3766 *%-----|-----|
3767 CONTINUOUS STANDHYD NHYD=["TODD_MN3"], DT=[ 1]min, AREA=[ 0.117](ha),
3768 XI MP=[ 0.53], TI MP=[ 0.57], DWF=[ 0](cms), LOSS=[ 2],
3769 SCS curve number CN=[ 77],
3770 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
3771             LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
3772 Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),
3773             LGI=[ 27.928](m), MNI=[ 0.013], SCI=[ 0](min),
3774 Continuous simulation parameters:
3775 IaRECPER=[ 4](hrs), IaRECI MP=[ 4](hrs),
3776 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3777 InterEventTime=[ 18](hrs), END=- 1
3778 *%-----|-----|
3779 CONTINUOUS STANDHYD NHYD=["TODD_M"], DT=[ 1]min, AREA=[ 30.230](ha),
3780 XI MP=[ 0.52], TI MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 2],
3781 SCS curve number CN=[ 77],
3782 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
3783             LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
3784 Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),
3785             LGI=[ 448.925](m), MNI=[ 0.013], SCI=[ 0](min),
3786 Continuous simulation parameters:
3787 IaRECPER=[ 4](hrs), IaRECI MP=[ 4](hrs),
3788 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3789 InterEventTime=[ 18](hrs), END=- 1
3790 *%-----|-----|
3791 * -JFSA, 2021-01-19 update "TODD_ALL" area from 108.741 ha to 112.908 ha based on
GIS measurements (148.41-30.23-0.117-2.1-3.055=112.908 ha)
3792 CONTINUOUS STANDHYD NHYD=["TODD_ALL"], DT=[ 1]min, AREA=[ 112.908](ha),
3793 XI MP=[ 0.52], TI MP=[ 0.57], DWF=[ 0](cms), LOSS=[ 2],
3794 SCS curve number CN=[ 77],
3795 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
3796             LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
3797 Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),
3798             LGI=[ 867.594](m), MNI=[ 0.013], SCI=[ 0](min),
3799 Continuous simulation parameters:
3800 IaRECPER=[ 4](hrs), IaRECI MP=[ 4](hrs),
3801 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3802 InterEventTime=[ 18](hrs), END=- 1
3803 *%-----|-----|
3804 CONTINUOUS STANDHYD NHYD=["TODD_P"], DT=[ 1]min, AREA=[ 3.055](ha),
3805 XI MP=[ 0.63], TI MP=[ 0.63], DWF=[ 0](cms), LOSS=[ 2],
3806 SCS curve number CN=[ 77],
3807 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
3808             LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
3809 Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),
3810             LGI=[ 142.712](m), MNI=[ 0.013], SCI=[ 0](min),
3811 Continuous simulation parameters:
3812 IaRECPER=[ 4](hrs), IaRECI MP=[ 4](hrs),
3813 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3814 InterEventTime=[ 18](hrs), END=- 1
3815 *%-----|-----|
3816 *%-----|-----|
3817 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3818 *CONTINUOUS STANDHYD NHYD=["TODD_DEVL"], DT=[ 1]min, AREA=[ 15.87](ha),
3819 * XI MP=[ 0.63], TI MP=[ 0.63], DWF=[ 0](cms), LOSS=[ 2],
3820 * SCS curve number CN=[ 77],
3821 * Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
3822 *             LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
3823 * Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),

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3824 *                LGI=[ 325.27](m), MNI=[ 0.013], SCI=[ 0](min),
3825 *                Continuous simulation parameters:
3826 *                IaRECper=[ 4](hrs), IaRECimp=[ 4](hrs),
3827 *                SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3828 *                InterEventTime=[ 18](hrs), END=- 1
3829 *%-----|-----
3830 *                -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
3831 *                is called "corr2" and its parameters remain the same.
3832 *CONTINUOUS NASHYD NHYD=[ "TODD_UnD" ], DT=[ 1]min, AREA=[ 12.47](ha),
3833 *                DWF=[ 0](cms), CNVC=[ 77], IA=[ 4.67](mm),
3834 *                N=[ 3], TP=[ 1.10]hrs,
3835 *                Continuous simulation parameters:
3836 *                IaRECper=[ 4](hrs),
3837 *                SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3838 *                InterEventTime=[ 12](hrs)
3839 *                Baseflow simulation parameters:
3840 *                BaseFlowOption=[ 1],
3841 *                InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
3842 *                VHydCond=[ 0.055](mm/hr), END=- 1
3843 *%-----|-----
3844 *# 5-Year + 12% Capture
3845 *COMPUTE DUALHYD NHYDin=[ "TODD_Mj" ], CINLET=[ 3.314](cms), NINLET=[ 1],
3846 *                MjNHYD=[ "TODD_Mj" ]
3847 *                MnNHYD=[ "TODD_Mjn" ]
3848 *                TMSTO=[ 0.1](cu-m)
3849 ROUTE RESERVOIR NHYDout=[ "TODD_Mjn" ], NHYDin=[ "TODD_Mj" ],
3850 RDT=[ 1](min),
3851 TABLE of ( OUTFLOW STORAGE ) values
3852 ( cms ) - ( ha-m )
3853 [ 0.0 , 0.0 ]
3854 [ 3.314 , 0.0001 ]
3855 [ -1 , -1 ] (max twenty pts)
3856 NHYDovf=[ "TODD_Mj" ],
3857 *%-----|-----
3858 *# 5-Year + 12% Capture
3859 *COMPUTE DUALHYD NHYDin=[ "TODD_MN1" ], CINLET=[ 0.227](cms), NINLET=[ 1],
3860 *                MjNHYD=[ "TODD_MN1j" ]
3861 *                MnNHYD=[ "TODD_MN1n" ]
3862 *                TMSTO=[ 0.1](cu-m)
3863 ROUTE RESERVOIR NHYDout=[ "TODD_MN1n" ], NHYDin=[ "TODD_MN1" ],
3864 RDT=[ 1](min),
3865 TABLE of ( OUTFLOW STORAGE ) values
3866 ( cms ) - ( ha-m )
3867 [ 0.0 , 0.0 ]
3868 [ 0.227 , 0.0001 ]
3869 [ -1 , -1 ] (max twenty pts)
3870 NHYDovf=[ "TODD_MN1j" ],
3871 *%-----|-----
3872 *COMPUTE DUALHYD NHYDin=[ "TODD_MN2" ], CINLET=[ 0.268](cms), NINLET=[ 1],
3873 *                MjNHYD=[ "TODD_MN2j" ]
3874 *                MnNHYD=[ "TODD_MN2n" ]
3875 *                TMSTO=[ 0.1](cu-m)
3876 ROUTE RESERVOIR NHYDout=[ "TODD_MN2n" ], NHYDin=[ "TODD_MN2" ],
3877 RDT=[ 1](min),
3878 TABLE of ( OUTFLOW STORAGE ) values
3879 ( cms ) - ( ha-m )
3880 [ 0.0 , 0.0 ]
3881 [ 0.268 , 0.0001 ]
3882 [ -1 , -1 ] (max twenty pts)
3883 NHYDovf=[ "TODD_MN2j" ],
3884 *%-----|-----
3885 *COMPUTE DUALHYD NHYDin=[ "TODD_MN3" ], CINLET=[ 0.016](cms), NINLET=[ 1],
3886 *                MjNHYD=[ "TODD_MN3j" ]
3887 *                MnNHYD=[ "TODD_MN3n" ]
3888 *                TMSTO=[ 0.1](cu-m)
3889 ROUTE RESERVOIR NHYDout=[ "TODD_MN3n" ], NHYDin=[ "TODD_MN3" ],

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3889          RDT=[ 1 ] ( m i n ) ,
3890          TABLE of ( OUTFLOW STORAGE ) values
3891              ( c m s ) - ( h a - m )
3892              [ 0.0 , 0.0 ]
3893              [ 0.016 , 0.0001 ]
3894              [ -1 , -1 ] ( m a x t w e n t y p t s )
3895          NHYDovf=[ " TODD_MN3j " ] ,
3896 *%-----|-----|
3897 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
3898 CONTINUOUS STANDHYD NHYD=[ " A2 " ] , DT=[ 1 ] m i n , AREA=[ 25.5 ] ( h a ) ,
3899 XI MP=[ 0.42 ] , TI MP=[ 0.52 ] , DWF=[ 0 ] ( c m s ) , LOSS=[ 2 ] ,
3900 SCS curve number CN=[ 75 ] ,
3901 Pervious surfaces: I A p e r=[ 4.67 ] ( m m ) , SLPP=[ 1 ] ( % ) ,
3902 L G P=[ 40 ] ( m ) , M N P=[ 0.25 ] , S C P=[ 0 ] ( m i n ) ,
3903 I m p e r v i o u s s u r f a c e s : I A i m p=[ 1.57 ] ( m m ) , S L P I=[ 1 ] ( % ) ,
3904 L G I=[ 566 ] ( m ) , M N I=[ 0.013 ] , S C I=[ 0 ] ( m i n ) ,
3905 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
3906 I a R E C p e r=[ 4 ] ( h r s ) , I a R E C i m p=[ 4 ] ( h r s ) ,
3907 S M N=[ -1 ] ( m m ) , S M A X=[ -1 ] ( m m ) , S K=[ 0.010 ] / ( m m ) ,
3908 I n t e r E v e n t T i m e=[ 18 ] ( h r s ) , E N D=-1
3909 *%-----|-----|
3910 COMPUTE DUALHYD NHYD i n=[ " A2 " ] , C I N L E T=[ 1.818 ] ( c m s ) , N I N L E T=[ 1 ] ,
3911 M a j N H Y D=[ " A2 - M " ]
3912 M i n N H Y D=[ " A2 - M N " ]
3913 T M S T O=[ 924 ] ( c u - m )
3914 *%-----|-----|
3915 ADD HYD NHYD s u m=[ " TODD " ] , NHYD s t o
add=[ " TODD_MN2n " + " TODD_MN3n " + " TODD_Mj " + " TODD_P " + " TODD_ALL " + " W_CLAR_Mn " ]
3916 *%-----|-----|
3917 SAVE HYD NHYD=[ " TODD " ] , # O F P C Y C L E S=[ -1 ] , I C A S E s h=[ 1 ]
3918 HYD_COMMENT=[ " T o t a l F l o w s a t T o d d D r a i n " ]
3919 *%-----|-----|
3920 *#*****|*****|
3921 *# Todd Pond 3
3922 *# - Rating curve obtained from Barrhaven South M&S modeling
3923 *# - stantec 2007, Tributary Drainage Area to M&S Pond 3 = 193 ha
3924 *#*****|*****|
3925 ROUTE RESERVOIR NHYDout=[ " M&S_P3 " ] , NHYD i n=[ " TODD " ] ,
3926 RDT=[ 1 ] ( m i n ) ,
3927          TABLE of ( OUTFLOW STORAGE ) values
3928              ( c m s ) - ( h a - m )
3929              [ 0.0 , 0.0 ]
3930              [ 0.014 , 0.155 ]
3931              [ 0.048 , 0.394 ]
3932              [ 0.061 , 0.56 ]
3933              [ 0.08 , 0.909 ]
3934              [ 0.088 , 1.089 ]
3935              [ 0.109 , 1.652 ]
3936              [ 0.118 , 1.952 ]
3937              [ 0.122 , 2.099 ]
3938              [ 1.972 , 2.269 ]
3939              [ 9.135 , 2.598 ]
3940              [ 15.608 , 2.826 ]
3941              [ 19.256 , 2.942 ]
3942              [ 27.282 , 3.181 ]
3943              [ 40.957 , 3.55 ]
3944              [ 56.372 , 3.929 ]
3945              [ 73.349 , 4.317 ]
3946              [ 85.469 , 4.579 ]
3947              [ 104.771 , 4.977 ]
3948              [ -1 , -1 ] ( m a x t w e n t y p t s )
3949          NHYDovf=[ " P3 - O V F " ]
3950 *%-----|-----|
3951 ADD HYD NHYD s u m=[ " SN_TO " ] , NHYD s t o
add=[ " GreenB " + " M&S_P3 " + " P3 - O V F " + " TODD_MN2j " + " A2 - M " ]

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3952 *%-----|-----|
3953 SAVE HYD      NHYD=["SN_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3954              HYD_COMMENT=["Total Flows at Todd Drain"]
3955 *%-----|-----|
3956 *#
3957 *# Hydrograph from Todd Drain routed to Corrigan Drain
3958 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
3959 *# 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so
the model will be more stable and give reasonable results. It is justifiable as ROUTE
CHANNELS aren't well suited to really flat slopes.
*
3960
3961 ROUTE CHANNEL  NHYDout=["N_TO"], NHYDin=["SN_TO"],
3962              RDT=[1](min),
3963              CHLGTH=[280](m), CHSLOPE=[0.05](%),
3964              FPSLOPE=[0.05](%),
3965              SECNUM=[1.0], NSEG=[3]
3966              ( SEGROUGH, SEGDIST (m) )=
3967              [ 0.075, -17.72
3968              -0.045, 17.72
3969              0.075, 80.62] NSEG times
3970              ( DISTANCE (m), ELEVATION (m) )=
3971              [-83.32, 90.00]
3972              [-81.36, 89.50]
3973              [-79.12, 89.00]
3974              [-76.13, 88.50]
3975              [-20.46, 88.00]
3976              [-19.36, 87.50]
3977              [-18.51, 87.00]
3978              [-17.72, 86.50]
3979              [-11.95, 85.24]
3980              [-0.11, 85.12]
3981              [11.49, 85.20]
3982              [17.72, 86.50]
3983              [19.74, 87.00]
3984              [21.22, 87.50]
3985              [22.68, 88.00]
3986              [24.28, 88.50]
3987              [26.79, 89.00]
3988              [71.98, 90.00]
3989              [80.62, 90.50]
3990 *%-----|-----|
3991 SAVE HYD      NHYD=["N_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3992              HYD_COMMENT=["Total inflows at Station 2462"]
3993 *%-----|-----|
3994 *#*****
3995 *# Catchment CORRIG
3996 *# - To Corrigan Drain (south of the Jock)
3997 *# - Primarily Developed (medium density)
3998 *# - JFSA JAN 2021, add Corrigan subcatchments as per IBI, July 2008
3999 *#*****
4000 *ROUTE RESERVOIR NHYDout=["MS_P1"], NHYDin=["CORRIG"],
4001 *              RDT=[1](min),
4002 *              TABLE of ( OUTFLOW STORAGE ) values
4003 *              (cms) - (ha-m)
4004 *              [ 0.0 , 0.0 ]
4005 *              [ 0.06 , 0.58]
4006 *              [ -1 , -1 ] (max twenty pts)
4007 *              NHYDovf=["P1-OVF"]
4008 *%-----|-----|
4009 *ADD HYD      NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
4010 *%-----|-----|
4011 *SAVE HYD     NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
4012 *              HYD_COMMENT=["Total Flows at Corrigan Drain"]
4013 *%-----|-----|
4014 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"

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and its parameters remain the same.
4015 CONTINUOUS STANDHYD NHYD=["corr1"], DT=[1]min, AREA=[15.87](ha),
4016 XI MP=[0.63], TI MP=[0.63], DWF=[0](cms), LOSS=[2],
4017 SCS curve number CN=[77],
4018 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4019 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4020 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4021 LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
4022 Continuous simulation parameters:
4023 IaRECper=[4](hrs), IaRECImp=[4](hrs),
4024 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4025 InterEventTime=[18](hrs), END=-1
4026 *%-----|-----|
4027 * -JFSA 2021-02-23 add DUALHYD for "corr1". "corr1" DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to "corr1".
4028 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
4029 COMPUTE DUALHYD NHYDin=["corr1"], CINLET=[1.818](cms), NINLET=[1],
4030 MjNHYD=["corr1-M"]
4031 MnNHYD=["corr1-MN"]
4032 TMSTO=[924](cu-m)
4033 *%-----|-----|
4034 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
4035 CONTINUOUS NASHYD NHYD=["corr2"], DT=[1]min, AREA=[12.47](ha),
4036 DWF=[0](cms), CN C=[77], IA=[4.67](mm),
4037 N=[3], TP=[1.10]hrs,
4038 Continuous simulation parameters:
4039 IaRECper=[4](hrs),
4040 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4041 InterEventTime=[12](hrs)
4042 Baseflow simulation parameters:
4043 BaseFlowOption=[1],
4044 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4045 VHydCond=[0.055](mm/hr), END=-1
4046 *%-----|-----|
4047 * -JFSA 2021-01-19 change Al-Corrig to be developed as per geottawa website and
apply the parameters of A2, the nearest sub-catchment to Al-Corrig, LGI is calculated
based on Al-Corrig area
4048 * -JFSA 2021-01-19 update all Corrigan areas based on GIS measurements, and keep
LGI as it is from Corrigan Report, IBI Group, 2008 because LGI calculated is less than
LGI from the Corrigan Report
4049 CONTINUOUS STANDHYD NHYD=["Al-Corrig"], DT=[1]min, AREA=[15.75](ha),
4050 XI MP=[0.42], TI MP=[0.52], DWF=[0](cms), LOSS=[2],
4051 SCS curve number CN=[75],
4052 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4053 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4054 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4055 LGI=[324.037](m), MNI=[0.013], SCI=[0](min),
4056 Continuous simulation parameters:
4057 IaRECper=[4](hrs), IaRECImp=[4](hrs),
4058 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4059 InterEventTime=[18](hrs), END=-1
4060 *
4061 * -JFSA 2021-01-25 add DUALHYD for Al-Corrig. Al-Corrig DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to Al-Corrig.
4062 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
4063 COMPUTE DUALHYD NHYDin=["Al-Corrig"], CINLET=[1.818](cms), NINLET=[1],
4064 MjNHYD=["Al-M"]
4065 MnNHYD=["Al-MN"]
4066 TMSTO=[924](cu-m)
4067 *%-----|-----|
4068 *CONTINUOUS NASHYD NHYD=["Al-Corrig"], DT=[1]min, AREA=[15.75](ha),
4069 * DWF=[0](cms), CN C=[66], IA=[2.5](mm),
4070 * N=[3.0], TP=[0.36]hrs,

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4071 *          Continuous simulation parameters:
4072 *          IaRECper=[ 4]( hrs),
4073 *          SMN=[ -1]( mm),   SMAX=[ -1]( mm),   SK=[ 0.010]/( mm),
4074 *          InterEventTime=[ 12]( hrs)
4075 *          Baseflow simulation parameters:
4076 *          BaseFlowOption=[ 1] ,
4077 *          InitGWResVol =[ 50]( mm),   GWResK=[ 0.96]( mm/ day/ mm)
4078 *          VHydCond=[ 0.055]( mm/ hr),   END=- 1
4079 *%-----|-----|
4080 CONTINUOUS NASHYD  NHYD=[ "B1"], DT=[ 1] mi n, AREA=[ 2.77]( ha),
4081 DWF=[ 0]( cms),   CN C=[ 56], IA=[ 2.5]( mm),
4082 N=[ 3.0], TP=[ 0.23] hr s,
4083 Continuous simulation parameters:
4084 IaRECper=[ 4]( hrs),
4085 SMN=[ -1]( mm),   SMAX=[ -1]( mm),   SK=[ 0.010]/( mm),
4086 InterEventTime=[ 12]( hrs)
4087 Baseflow simulation parameters:
4088 BaseFlowOption=[ 1] ,
4089 InitGWResVol =[ 50]( mm),   GWResK=[ 0.96]( mm/ day/ mm)
4090 VHydCond=[ 0.055]( mm/ hr),   END=- 1
4091 *%-----|-----|
4092 CONTINUOUS STANDHYD  NHYD=[ "A4"], DT=[ 1] mi n, AREA=[ 1.27]( ha),
4093 XI MP=[ 0.65], TI MP=[ 0.65], DWF=[ 0]( cms), LOSS=[ 2],
4094 SCS curve number CN=[ 75],
4095 Pervious surfaces: IAper=[ 4.67]( mm), SLPP=[ 1]( %),
4096                    LGP=[ 40]( m), MNP=[ 0.25], SCP=[ 0]( mi n),
4097 Impervious surfaces: IAi mp=[ 1.57]( mm), SLPI=[ 1]( %),
4098                    LGI=[ 253]( m), MNI =[ 0.013], SCI=[ 0]( mi n),
4099 Continuous simulation parameters:
4100 IaRECper=[ 4]( hrs),   IaRECI mp=[ 4]( hr s),
4101 SMN=[ -1]( mm),   SMAX=[ -1]( mm), SK=[ 0.010]/( mm),
4102 InterEventTime=[ 18]( hr s),   END=- 1
4103 *%-----|-----|
4104 COMPUTE DUALHYD  NHYDin=[ "A4"], CINLET=[ 0.405]( cms), NI NLET=[ 1],
4105 Maj NHYD=[ "A4- M"]
4106 MinNHYD=[ "A4- MN"]
4107 TMI STO=[ 68]( cu- m)
4108 *%-----|-----|
4109 ADD HYD  NHYDsum=[ "MH101"], NHYDs to
add=[ "A1- M" +"A1- MN" +"corr1- M" +"corr1- MN" +"corr2" +"B1" +"A4- MN"]
4110 *%-----|-----|
4111 SAVE HYD  NHYD=[ "MH101"], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4112 HYD_COMMENT=[ "Total Flows at MH101"]
4113 *%-----|-----|
4114 ROUTE PIPE  PTYPE=[ 1] circ, NHYDout=[ "101- 102"], RNUMBER=[ 1.0], PDI AM=[ 1050]( mm),
4115 PLNGTH=[ 368]( m), PROUGH=[ 0.013], PSLOPE=[ 0.0054]( m/ m),
NHYDin=[ "MH101"], RDT=[ 1]
4116 *%-----|-----|
4117 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
4118 *CONTINUOUS STANDHYD  NHYD=[ "A2"], DT=[ 1] mi n, AREA=[ 25.5]( ha),
4119 * XI MP=[ 0.42], TI MP=[ 0.52], DWF=[ 0]( cms), LOSS=[ 2],
4120 * SCS curve number CN=[ 75],
4121 * Pervious surfaces: IAper=[ 4.67]( mm), SLPP=[ 1]( %),
4122 *                    LGP=[ 40]( m), MNP=[ 0.25], SCP=[ 0]( mi n),
4123 * Impervious surfaces: IAi mp=[ 1.57]( mm), SLPI=[ 1]( %),
4124 *                    LGI=[ 566]( m), MNI =[ 0.013], SCI=[ 0]( mi n),
4125 * Continuous simulation parameters:
4126 * IaRECper=[ 4]( hrs),   IaRECI mp=[ 4]( hr s),
4127 * SMN=[ -1]( mm),   SMAX=[ -1]( mm), SK=[ 0.010]/( mm),
4128 * InterEventTime=[ 18]( hr s),   END=- 1
4129 *%-----|-----|
4130 *COMPUTE DUALHYD  NHYDin=[ "A2"], CINLET=[ 1.818]( cms), NI NLET=[ 1],
4131 * Maj NHYD=[ "A2- M"]
4132 * MinNHYD=[ "A2- MN"]
4133 * TMI STO=[ 924]( cu- m)

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4134 *%-----|-----|
4135 ADD HYD          NHYDs um=[ "MH102" ], NHYDs to add=[ "A2-MN"+"101-102" ]
4136 *%-----|-----|
4137 SAVE HYD         NHYD=[ "MH102" ], # OF PCYCLES=[ -1 ], I CASEs h=[ 1 ]
4138 HYD_COMMENT=[ "Total Flows at MH102" ]
4139 *%-----|-----|
4140 CONTINUOUS STANDHYD NHYD=[ "A5" ], DT=[ 1 ] mi n, AREA=[ 1.6 ] (ha),
4141 XI MP=[ 0.71 ], TI MP=[ 0.71 ], DWF=[ 0 ] (cms), LOSS=[ 2 ],
4142 SCS curve number CN=[ 75 ],
4143 Pervious surfaces: I Aper=[ 4.67 ] (mm), SLPP=[ 1 ] ( % ),
4144                    LGP=[ 40 ] (m), MNP=[ 0.25 ], SCP=[ 0 ] (mi n),
4145 ImperVIOUS surfaces: I Ai mp=[ 1.57 ] (mm), SLPI=[ 1 ] ( % ),
4146                    LGI=[ 300 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),
4147 Continuous simulation parameters:
4148 I a REc per=[ 4 ] (hrs), I a REc i mp=[ 4 ] (hrs),
4149 SM N=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
4150 InterEvent Ti me=[ 18 ] (hrs), END=- 1
4151 *%-----|-----|
4152 ADD HYD          NHYDs um=[ "A5T" ], NHYDs to add=[ "A4-M"+"A5" ]
4153 *%-----|-----|
4154 COMPUTE DUALHYD  NHYDi n=[ "A5T" ], CI NLET=[ 0.357 ] (cms), NI NLET=[ 1 ],
4155 M aj NHYD=[ "A5-M" ]
4156 M nNHYD=[ "A5-MN" ]
4157 TM STO=[ 60 ] (cu-m)
4158 *%-----|-----|
4159 * -JFSA Jan. 2021, A3 is a part of Todd so it is removed
4160 * -JFSA Jan. 2021, "A2-M" added to "Todd"
4161 *CONTINUOUS STANDHYD NHYD=[ "A3" ], DT=[ 1 ] mi n, AREA=[ 18.4 ] (ha),
4162 * XI MP=[ 0.58 ], TI MP=[ 0.65 ], DWF=[ 0 ] (cms), LOSS=[ 2 ],
4163 * SCS curve number CN=[ 75 ],
4164 * Pervious surfaces: I Aper=[ 4.67 ] (mm), SLPP=[ 1 ] ( % ),
4165 *                    LGP=[ 40 ] (m), MNP=[ 0.25 ], SCP=[ 0 ] (mi n),
4166 * ImperVIOUS surfaces: I Ai mp=[ 1.57 ] (mm), SLPI=[ 1 ] ( % ),
4167 *                    LGI=[ 450 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),
4168 * Continuous simulation parameters:
4169 * I a REc per=[ 4 ] (hrs), I a REc i mp=[ 4 ] (hrs),
4170 * SM N=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
4171 * InterEvent Ti me=[ 18 ] (hrs), END=- 1
4172 *%-----|-----|
4173 *ADD HYD          NHYDs um=[ "A3-A2M" ], NHYDs to add=[ "A2-M"+"A3" ]
4174 *%-----|-----|
4175 *COMPUTE DUALHYD  NHYDi n=[ "A3-A2M" ], CI NLET=[ 2.208 ] (cms), NI NLET=[ 1 ],
4176 * M aj NHYD=[ "A3R-M" ]
4177 * M nNHYD=[ "A3R-MN" ]
4178 * TM STO=[ 908 ] (cu-m)
4179 *%-----|-----|
4180 ROUTE PIPE       PTYPE=[ 1 ] circ, NHYDout=[ "102-103" ], RNUMBER=[ 1.0 ], PDI AM=[ 1500 ] (mm),
4181 PLNGTH=[ 504 ] (m), PROUGH=[ 0.013 ], PSLOPE=[ 0.0028 ] (m/m),
NHYDi n=[ "MH102" ], RDT=[ 1 ]
4182 *%-----|-----|
4183 ADD HYD          NHYDs um=[ "MH103" ], NHYDs to add=[ "102-103"+"A5-MN" ]
4184 *%-----|-----|
4185 SAVE HYD         NHYD=[ "MH103" ], # OF PCYCLES=[ -1 ], I CASEs h=[ 1 ]
4186 HYD_COMMENT=[ "Total Flows at MH103" ]
4187 *%-----|-----|
4188 ROUTE PIPE       PTYPE=[ 1 ] circ, NHYDout=[ "103-104" ], RNUMBER=[ 1.0 ], PDI AM=[ 1650 ] (mm),
4189 PLNGTH=[ 438 ] (m), PROUGH=[ 0.013 ], PSLOPE=[ 0.0046 ] (m/m),
NHYDi n=[ "MH103" ], RDT=[ 1 ]
4190 *%-----|-----|
4191 CONTINUOUS STANDHYD NHYD=[ "A6" ], DT=[ 1 ] mi n, AREA=[ 1.56 ] (ha),
4192 XI MP=[ 0.71 ], TI MP=[ 0.71 ], DWF=[ 0 ] (cms), LOSS=[ 2 ],
4193 SCS curve number CN=[ 75 ],
4194 Pervious surfaces: I Aper=[ 4.67 ] (mm), SLPP=[ 1 ] ( % ),
4195                    LGP=[ 40 ] (m), MNP=[ 0.25 ], SCP=[ 0 ] (mi n),
4196 ImperVIOUS surfaces: I Ai mp=[ 1.57 ] (mm), SLPI=[ 1 ] ( % ),
4197                    LGI=[ 280 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),

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4198 Continuous simulation parameters:
4199 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4200 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4201 InterEventTime=[ 18](hrs), END=- 1
4202 *%-----|
4203 ADD HYD NHYDsum=["A6T"], NHYDs to add=["A5-M"+"A6"]
4204 *%-----|
4205 COMPUTE DUALHYD NHYDin=["A6T"], CINLET=[ 0.357](cms), NINLET=[ 1],
4206 MjNHYD=["A6-M"]
4207 MnNHYD=["A6-MN"]
4208 TMSSTO=[ 60](cu-m)
4209 *%-----|
4210 * -JFSA Jan. 2021, A7-corrig is a part of Todd so it is removed
4211 *CONTINUOUS STANDHYD NHYD=["A7-corrig"], DT=[ 1]min, AREA=[ 11.8](ha),
4212 * XIMP=[ 0.41], TIMP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4213 * SCS curve number CN=[ 75],
4214 * Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4215 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4216 * Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4217 * LGI=[ 438](m), MNI=[ 0.013], SCI=[ 0](min),
4218 * Continuous simulation parameters:
4219 * IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4220 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4221 * InterEventTime=[ 18](hrs), END=- 1
4222 *%-----|
4223 *ADD HYD NHYDsum=["A7-A3RM"], NHYDs to add=["A3R-M"+"A7-corrig"]
4224 *%-----|
4225 *COMPUTE DUALHYD NHYDin=["A7-A3RM"], CINLET=[ 1.003](cms), NINLET=[ 1],
4226 * MjNHYD=["A7R-M"]
4227 * MnNHYD=["A7R-MN"]
4228 * TMSSTO=[ 496](cu-m)
4229 *%-----|
4230 ADD HYD NHYDsum=["MH104"], NHYDs to add=["A6-MN"+"103-104"+"TODD_Min"]
4231 *%-----|
4232 SAVE HYD NHYD=["MH104"], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4233 HYD_COMMENT=["Total Flows at MH104"]
4234 *%-----|
4235 CONTINUOUS STANDHYD NHYD=["B2"], DT=[ 1]min, AREA=[ 12.31](ha),
4236 XIMP=[ 0.41], TIMP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4237 SCS curve number CN=[ 75],
4238 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4239 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4240 Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4241 LGI=[ 417](m), MNI=[ 0.013], SCI=[ 0](min),
4242 Continuous simulation parameters:
4243 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4244 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4245 InterEventTime=[ 18](hrs), END=- 1
4246 *%-----|
4247 COMPUTE DUALHYD NHYDin=["B2"], CINLET=[ 1.029](cms), NINLET=[ 1],
4248 MjNHYD=["B2-M"]
4249 MnNHYD=["B2-MN"]
4250 TMSSTO=[ 508](cu-m)
4251 *%-----|
4252 ROUTE PIPE PTYPE=[ 1]circ, NHYDout=["315-333"], RNUMBER=[ 1.0], PDIAM=[ 1200](mm),
4253 PLNGTH=[ 254](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
NHYDin=["B2-MN"], RDT=[ 1]
4254 *%-----|
4255 CONTINUOUS STANDHYD NHYD=["B3"], DT=[ 1]min, AREA=[ 5.59](ha),
4256 XIMP=[ 0.41], TIMP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4257 SCS curve number CN=[ 75],
4258 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4259 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4260 Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4261 LGI=[ 345](m), MNI=[ 0.013], SCI=[ 0](min),
4262 Continuous simulation parameters:

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4263 IaRECper=[ 4](hr s), IaRECImp=[ 4](hr s),
4264 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4265 InterEventTime=[ 18](hr s), END=- 1
4266 *%-----|-----|
4267 COMPUTE DUALHYD NHYDin=[" B3"], CI NLET=[ 0.459](cms), NI NLET=[ 1],
4268 Mj NHYD=[" B3- M"]
4269 MnNHYD=[" B3- MN"]
4270 TM STO=[ 227](cu- m)
4271 *%-----|-----|
4272 ADD HYD NHYDs um=[" MH333"], NHYDs to add=[" B3- MN"+" 315- 333"]
4273 *%-----|-----|
4274 SAVE HYD NHYD=[" MH333"], # OF PCYCLES=[ -1], I CASEs h=[ 1]
4275 HYD_COMMENT=[" Total Fl ows at MH333"]
4276 *%-----|-----|
4277 ROUTE PI PE PTYPE=[ 1]circ, NHYDout=[" 333- 335"], RNUMBER=[ 1.0], PDI AM=[ 1200](mm),
4278 PLNGTH=[ 251](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
NHYDin=[" MH333"], RDT=[ 1]
4279 *%-----|-----|
4280 ROUTE PI PE PTYPE=[ 1]circ, NHYDout=[" 335- 338"], RNUMBER=[ 1.0], PDI AM=[ 1200](mm),
4281 PLNGTH=[ 185](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
NHYDin=[" 333- 335"], RDT=[ 1]
4282 *%-----|-----|
4283 ROUTE PI PE PTYPE=[ 1]circ, NHYDout=[" 338- 340"], RNUMBER=[ 1.0], PDI AM=[ 1350](mm),
4284 PLNGTH=[ 233](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
NHYDin=[" 335- 338"], RDT=[ 1]
4285 *%-----|-----|
4286 CONTINUOUS STANDHYD NHYD=[" B4"], DT=[ 1]min, AREA=[ 7.6](ha),
4287 XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4288 SCS curve number CN=[ 75],
4289 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4290 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4291 ImperVIOUS surfaces: IAi mp=[ 1.57](mm), SLPI =[ 1](%),
4292 LGI =[ 388](m), MNI =[ 0.013], SCI =[ 0](min),
4293 Continuous simulation parameters:
4294 IaRECper=[ 4](hr s), IaRECImp=[ 4](hr s),
4295 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4296 InterEventTime=[ 18](hr s), END=- 1
4297 *%-----|-----|
4298 COMPUTE DUALHYD NHYDin=[" B4"], CI NLET=[ 0.655](cms), NI NLET=[ 1],
4299 Mj NHYD=[" B4- M"]
4300 MnNHYD=[" B4- MN"]
4301 TM STO=[ 323](cu- m)
4302 *%-----|-----|
4303 ADD HYD NHYDs um=[" MH340"], NHYDs to add=[" 338- 340"+" B4- MN"]
4304 *%-----|-----|
4305 SAVE HYD NHYD=[" MH340"], # OF PCYCLES=[ -1], I CASEs h=[ 1]
4306 HYD_COMMENT=[" Total Fl ows at MH340"]
4307 *%-----|-----|
4308 ROUTE PI PE PTYPE=[ 1]circ, NHYDout=[" 340- 104"], RNUMBER=[ 1.0], PDI AM=[ 1650](mm),
4309 PLNGTH=[ 240](m), PROUGH=[ 0.013], PSLOPE=[ 0.0015](m/m),
NHYDin=[" MH340"], RDT=[ 1]
4310 *%-----|-----|
4311 ADD HYD NHYDs um=[" MH104T"], NHYDs to add=[" 340- 104"+" MH104"]
4312 *%-----|-----|
4313 ROUTE PI PE PTYPE=[ 2]rect, NHYDout=[" 104- 105"], RNUMBER=[ 1.0],
4314 PW DTH=[ 2400](mm) by PHEI GHT=[ 2100](mm),
PLNGTH=[ 380](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
NHYDin=[" MH104T"], RDT=[ 1]
4315 *%-----|-----|
4316 CONTINUOUS STANDHYD NHYD=[" B5"], DT=[ 1]min, AREA=[ 2.2](ha),
4317 XI MP=[ 0.57], TI MP=[ 0.57], DWF=[ 0](cms), LOSS=[ 2],
4318 SCS curve number CN=[ 75],
4319 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4320 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4321 ImperVIOUS surfaces: IAi mp=[ 1.57](mm), SLPI =[ 1](%),

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4322             LGI=[ 187](m), MNI=[ 0.013], SCI=[ 0](min),
4323 Continuous simulation parameters:
4324 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4325 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4326 InterEventTime=[ 18](hrs), END=- 1
4327 *%-----|
4328 COMPUTE DUALHYD NHYDin=[ "B5" ], CINLET=[ 0.260](cms), NINLET=[ 1],
4329 MijNHYD=[ "B5-M" ]
4330 MnNHYD=[ "B5-MN" ]
4331 TMSSTO=[ 250](cu-m)
4332 *%-----|
4333 CONTINUOUS STANDHYD NHYD=[ "A8" ], DT=[ 1]min, AREA=[ 0.96](ha),
4334 XI MP=[ 0.71], TIMP=[ 0.71], DWF=[ 0](cms), LOSS=[ 2],
4335 SCS curve number CN=[ 75],
4336 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4337 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4338 Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4339 LGI=[ 186](m), MNI=[ 0.013], SCI=[ 0](min),
4340 Continuous simulation parameters:
4341 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4342 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4343 InterEventTime=[ 18](hrs), END=- 1
4344 *%-----|
4345 ADD HYD NHYDsum=[ "A8T" ], NHYDs to add=[ "A6-M" + "A8" ]
4346 *%-----|
4347 COMPUTE DUALHYD NHYDin=[ "A8T" ], CINLET=[ 0.238](cms), NINLET=[ 1],
4348 MijNHYD=[ "A8-M" ]
4349 MnNHYD=[ "A8-MN" ]
4350 TMSSTO=[ 40](cu-m)
4351 *%-----|
4352 ADD HYD NHYDsum=[ "MH105" ], NHYDs to
4353 add=[ "104-105" + "B5-MN" + "A8-MN" + "TODD_MN3j" ]
4354 *%-----|
4354 SAVE HYD NHYD=[ "MH105" ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4355 HYD_COMMENT=[ "Total Flows at MH105" ]
4356 *%-----|
4357 DI VERT HYD NHYDin=[ "A8-M" ] NIDout=[ 2]max five,
4358 outflow hydrographs (NHYDs)=[ "A8-M-JR" "A8-M-B6" ]
4359 flow distribution table: (modify as necessary)
4360 Note: all flows are in (cms)
4361 QIDi + QIDi = QTOTAL
4362 [ 0 + 0 = 0 ]
4363 [ 50 + 50 = 100 ] end
4364 *%-----|
4365 DI VERT HYD NHYDin=[ "MH105" ] NIDout=[ 2]max five,
4366 outflow hydrographs (NHYDs)=[ "MH105-JR" "MH105-B6" ]
4367 flow distribution table: (modify as necessary)
4368 Note: all flows are in (cms)
4369 QIDi + QIDi = QTOTAL
4370 [ 0 + 0 = 0 ]
4371 [ 0 + 3.0 = 3.0 ]
4372 [ 96.9+ 3.1 = 100 ] end
4373 *%-----|
4374 CONTINUOUS STANDHYD NHYD=[ "B7" ], DT=[ 1]min, AREA=[ 7.19](ha),
4375 XI MP=[ 0.41], TIMP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4376 SCS curve number CN=[ 75],
4377 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4378 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4379 Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4380 LGI=[ 211](m), MNI=[ 0.013], SCI=[ 0](min),
4381 Continuous simulation parameters:
4382 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4383 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4384 InterEventTime=[ 18](hrs), END=- 1

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4385 *%-----|-----|
4386 ADD HYD NHYDs um=[ " B7- B4M " ], NHYDs to add=[ " B4- M " + " B7 " ]
4387 *%-----|-----|
4388 COMPUTE DUALHYD NHYD i n=[ " B7- B4M " ], CI NLET=[ 0. 629 ] ( c m s ), NI NLET=[ 1 ],
4389 M a j NHYD=[ " B7R- M " ]
4390 M n NHYD=[ " B7R- MN " ]
4391 T M S T O=[ 311 ] ( c u- m )
4392 *%-----|-----|
4393 ROUTE PI PE PTYPE=[ 1 ] c i r c, NHYD out=[ " 360- 106A " ], RNUMBER=[ 1. 0 ], PDI AM=[ 1050 ] ( m m ),
4394 PLNGTH=[ 167 ] ( m ), PROUGH=[ 0. 013 ], PSLOPE=[ 0. 001 ] ( m / m ),
NHYD i n=[ " B7R- MN " ], RDT=[ 1 ]
4395 *%-----|-----|
4396 * -JFSA 2021-01-19 change B6 to be developed as per geottawa website and apply the
parameters of A7, the nearest sub-catchment to B6, LGI is calculated based on B6 area
CONTINUOUS STANDHYD NHYD=[ " B6 " ], DT=[ 1 ] m i n, AREA=[ 3. 29 ] ( h a ),
4397 XI MP=[ 0. 41 ], TI MP=[ 0. 54 ], DWF=[ 0 ] ( c m s ), LOSS=[ 2 ],
4398 SCS curve number CN=[ 75 ],
4400 P e r v i o u s s u r f a c e s : I A p e r=[ 4. 67 ] ( m m ), S L P P=[ 1 ] ( % ),
4401 L G P=[ 40 ] ( m ), M N P=[ 0. 25 ], S C P=[ 0 ] ( m i n ),
4402 I m p e r v i o u s s u r f a c e s : I A i m p=[ 1. 57 ] ( m m ), S L P I=[ 1 ] ( % ),
4403 L G I=[ 148. 099 ] ( m ), M N I=[ 0. 013 ], S C I=[ 0 ] ( m i n ),
4404 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
4405 I a R E C p e r=[ 4 ] ( h r s ), I a R E C i m p=[ 4 ] ( h r s ),
4406 S M N=[ - 1 ] ( m m ), S M A X=[ - 1 ] ( m m ), S K=[ 0. 010 ] / ( m m ),
4407 I n t e r E v e n t T i m e=[ 18 ] ( h r s ), E N D=- 1
4408 *%-----|-----|
4409 * -JFSA 2021-01-25 add B1 DUALHYD as per Corrigan Report, IBI Group, 2008
COMPUTE DUALHYD NHYD i n=[ " B6 " ], CI NLET=[ 0. 064 ] ( c m s ), NI NLET=[ 1 ],
4410 M a j NHYD=[ " B6- M " ]
4411 M n NHYD=[ " B6- MN " ]
4412 T M S T O=[ 5484 ] ( c u- m )
4413 *%-----|-----|
4414 *CONTINUOUS NASHYD NHYD=[ " B6 " ], DT=[ 1 ] m i n, AREA=[ 3. 29 ] ( h a ),
4415 DWF=[ 0 ] ( c m s ), C N C=[ 75 ], I A=[ 2. 5 ] ( m m ),
4416 N=[ 3. 0 ], T P=[ 0. 36 ] h r s,
4417 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
4418 I a R E C p e r=[ 4 ] ( h r s ),
4419 S M N=[ - 1 ] ( m m ), S M A X=[ - 1 ] ( m m ), S K=[ 0. 010 ] / ( m m ),
4420 I n t e r E v e n t T i m e=[ 12 ] ( h r s )
4421 B a s e f l o w s i m u l a t i o n p a r a m e t e r s :
4422 B a s e F l o w O p t i o n=[ 1 ],
4423 I n i t G W R e s V o l=[ 50 ] ( m m ), G W R e s K=[ 0. 96 ] ( m m / d a y / m m )
4424 V H y d C o n d=[ 0. 055 ] ( m m / h r ), E N D=- 1
4425 *%-----|-----|
4426 *% -EX-LAND is external land. It is a part of JOCKVA sub-catchment as per Corrigan
Report, IBI Group, 2008
CONTINUOUS STANDHYD NHYD=[ " EX- LAND " ], DT=[ 1 ] m i n, AREA=[ 32. 5 ] ( h a ),
4427 XI MP=[ 0. 50 ], TI MP=[ 0. 50 ], DWF=[ 0 ] ( c m s ), LOSS=[ 2 ],
4428 SCS curve number CN=[ 74 ],
4429 P e r v i o u s s u r f a c e s : I A p e r=[ 4. 67 ] ( m m ), S L P P=[ 1 ] ( % ),
4430 L G P=[ 40 ] ( m ), M N P=[ 0. 25 ], S C P=[ 0 ] ( m i n ),
4431 I m p e r v i o u s s u r f a c e s : I A i m p=[ 1. 57 ] ( m m ), S L P I=[ 1 ] ( % ),
4432 L G I=[ 465. 475 ] ( m ), M N I=[ 0. 013 ], S C I=[ 0 ] ( m i n ),
4433 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
4434 I a R E C p e r=[ 4 ] ( h r s ), I a R E C i m p=[ 4 ] ( h r s ),
4435 S M N=[ - 1 ] ( m m ), S M A X=[ - 1 ] ( m m ), S K=[ 0. 010 ] / ( m m ),
4436 I n t e r E v e n t T i m e=[ 18 ] ( h r s ), E N D=- 1
4437 *%-----|-----|
4438 *CONTINUOUS NASHYD NHYD=[ " EX- LAND " ], DT=[ 1 ] m i n, AREA=[ 32. 5 ] ( h a ),
4439 DWF=[ 0 ] ( c m s ), C N C=[ 75 ], I A=[ 2. 5 ] ( m m ),
4440 N=[ 3. 0 ], T P=[ 0. 36 ] h r s,
4441 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
4442 I a R E C p e r=[ 4 ] ( h r s ),
4443 S M N=[ - 1 ] ( m m ), S M A X=[ - 1 ] ( m m ), S K=[ 0. 010 ] / ( m m ),
4444 I n t e r E v e n t T i m e=[ 12 ] ( h r s )
4445 B a s e f l o w s i m u l a t i o n p a r a m e t e r s :
4446 B a s e F l o w O p t i o n=[ 1 ],
4447 I n i t G W R e s V o l=[ 50 ] ( m m ), G W R e s K=[ 0. 96 ] ( m m / d a y / m m )
4448 V H y d C o n d=[ 0. 055 ] ( m m / h r ), E N D=- 1
4449 *%-----|-----|
4450 *% -EX-LAND is external land. It is a part of JOCKVA sub-catchment as per Corrigan
Report, IBI Group, 2008
COMPUTE DUALHYD NHYD i n=[ " EX- LAND " ], CI NLET=[ 2. 275 ] ( c m s ), NI NLET=[ 1 ],
4451 M a j NHYD=[ " EX- LAND- M " ]
4452 M n NHYD=[ " EX- LAND- MN " ]
4453 T M S T O=[ 1365 ] ( c u- m )
4454 *%-----|-----|
4455 ADD HYD NHYDs um=[ " B6- B7ExM " ], NHYDs to
add=[ " B7R- M " + " EX- LAND- M " + " B5- M " + " B6- M " + " B6- MN " + " A8- M - B6 " ]
4456 *%-----|-----|

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4447 COMPUTE DUALHYD      NHYDIn=[ " B6- B7ExM " ], CINLET=[ 0.064 ] ( cms ), NINLET=[ 1 ],
4448                        MajNHYD=[ " B6R- M " ]
4449                        MnNHYD=[ " B6R- MN " ]
4450                        TMSTO=[ 5484 ] ( cu- m )
4451 *%-----|-----|
4452 ROUTE PIPE            PTYPE=[ 1 ] circ, NHYDout=[ " 105- 106A " ], RNUMBER=[ 1.0 ], PDIAM=[ 1800 ] ( mm ),
4453                        PLNGTH=[ 208 ] ( m ), PROUGH=[ 0.013 ], PSLOPE=[ 0.001 ] ( m/ m ),
                        NHYDIn=[ " MHI05- B6 " ], RDT=[ 1 ]
4454 *%-----|-----|
4455 ADD HYD              NHYDsum=[ " MHI06A " ], NHYDs to
add=[ " 360- 106A " + " 105- 106A " + " B6R- MN " + " B6R- M " ]
4456 *%-----|-----|
4457 SAVE HYD            NHYD=[ " MHI06A " ], # OF PCYCLES=[ - 1 ], ICASEsh=[ 1 ]
4458                        HYD_COMMENT=[ " Total Flows at MHI06A " ]
4459 *%-----|-----|
4460 *%      -JFSA 2021-01-12 THE MANHOLE MHI06 is called MHI17/106 in Corrigan Report, IBI
Group, July 2008
4461 *%
4462 ROUTE PIPE            PTYPE=[ 1 ] circ, NHYDout=[ " 106A- 106 " ], RNUMBER=[ 1.0 ], PDIAM=[ 1800 ] ( mm ),
4463                        PLNGTH=[ 190 ] ( m ), PROUGH=[ 0.013 ], PSLOPE=[ 0.001 ] ( m/ m ),
                        NHYDIn=[ " MHI06A " ], RDT=[ 1 ]
4464 *%-----|-----|
4465 CONTINUOUS STANDHYD NHYD=[ " A9 " ], DT=[ 1 ] min, AREA=[ 2.44 ] ( ha ),
4466                        XI MP=[ 0.71 ], TI MP=[ 0.71 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
4467                        SCS curve number CN=[ 75 ],
4468                        Pervious surfaces: I A per=[ 4.67 ] ( mm ), SLPP=[ 1 ] ( % ),
4469                        LGP=[ 40 ] ( m ), MNP=[ 0.25 ], SCP=[ 0 ] ( min ),
4470                        Impervious surfaces: I A i mp=[ 1.57 ] ( mm ), SLPI=[ 1 ] ( % ),
4471                        LGI=[ 262 ] ( m ), MNI=[ 0.013 ], SCI=[ 0 ] ( min ),
4472                        Continuous simulation parameters:
4473                        I a REC per=[ 4 ] ( hrs ), I a REC i mp=[ 4 ] ( hrs ),
4474                        SMN=[ - 1 ] ( mm ), SMAX=[ - 1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
4475                        InterEvent Time=[ 18 ] ( hrs ), END=- 1
4476 *%-----|-----|
4477 COMPUTE DUALHYD      NHYDIn=[ " A9 " ], CINLET=[ 0.547 ] ( cms ), NINLET=[ 1 ],
4478                        MajNHYD=[ " A9- M " ]
4479                        MnNHYD=[ " A9- MN " ]
4480                        TMSTO=[ 0 ] ( cu- m )
4481 *%-----|-----|
4482 ADD HYD              NHYDsum=[ " MHI06 " ], NHYDs to add=[ " 106A- 106 " + " A9- MN " ]
4483 *%-----|-----|
4484 SAVE HYD            NHYD=[ " MHI06 " ], # OF PCYCLES=[ - 1 ], ICASEsh=[ 1 ]
4485                        HYD_COMMENT=[ " Total Flows at MHI06 " ]
4486 *%-----|-----|
4487 *%      -JFSA 2021-01-12 THE MANHOLE MHI07 is called MHI18/107 in Corrigan Report, IBI
Group, July 2008
4488 *%
4489 ROUTE PIPE            PTYPE=[ 1 ] circ, NHYDout=[ " 106- 107 " ], RNUMBER=[ 1.0 ], PDIAM=[ 1800 ] ( mm ),
4490                        PLNGTH=[ 122.5 ] ( m ), PROUGH=[ 0.013 ], PSLOPE=[ 0.001 ] ( m/ m ),
                        NHYDIn=[ " MHI06 " ], RDT=[ 1 ]
4491 *%-----|-----|
4492 CONTINUOUS STANDHYD NHYD=[ " A10 " ], DT=[ 1 ] min, AREA=[ 4.14 ] ( ha ),
4493                        XI MP=[ 0.35 ], TI MP=[ 0.47 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
4494                        SCS curve number CN=[ 75 ],
4495                        Pervious surfaces: I A per=[ 4.67 ] ( mm ), SLPP=[ 1 ] ( % ),
4496                        LGP=[ 40 ] ( m ), MNP=[ 0.25 ], SCP=[ 0 ] ( min ),
4497                        Impervious surfaces: I A i mp=[ 1.57 ] ( mm ), SLPI=[ 1 ] ( % ),
4498                        LGI=[ 183 ] ( m ), MNI=[ 0.013 ], SCI=[ 0 ] ( min ),
4499                        Continuous simulation parameters:
4500                        I a REC per=[ 4 ] ( hrs ), I a REC i mp=[ 4 ] ( hrs ),
4501                        SMN=[ - 1 ] ( mm ), SMAX=[ - 1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
4502                        InterEvent Time=[ 18 ] ( hrs ), END=- 1
4503 *%-----|-----|
4504 COMPUTE DUALHYD      NHYDIn=[ " A10 " ], CINLET=[ 0.310 ] ( cms ), NINLET=[ 1 ],
4505                        MajNHYD=[ " A10- M " ]
4506                        MnNHYD=[ " A10- MN " ]

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4507          TMI STO=[ 228] (cu-m)
4508 *%-----|-----|
4509 CONTINUOUS STANDHYD NHYD=[ "A11"], DT=[ 1] min, AREA=[ 10.61] (ha),
4510          XI MP=[ 0.53], TI MP=[ 0.62], DWF=[ 0] (cms), LOSS=[ 2],
4511          SCS curve number CN=[ 75],
4512          Pervious surfaces: IAper=[ 4.67] (mm), SLPP=[ 1] (%),
4513          LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (min),
4514          Impervious surfaces: IAimp=[ 1.57] (mm), SLPI=[ 1] (%),
4515          LGI=[ 379] (m), MNI=[ 0.013], SCI=[ 0] (min),
4516          Continuous simulation parameters:
4517          IARECper=[ 4] (hrs), IARECimp=[ 4] (hrs),
4518          SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
4519          InterEventTime=[ 18] (hrs), END=- 1
4520 *%-----|-----|
4521 COMPUTE DUALHYD      NHYDin=[ "A11"], CILET=[ 0.993] (cms), NILET=[ 1],
4522          MajNHYD=[ "A11-M"]
4523          MnNHYD=[ "A11-MN"]
4524          TMI STO=[ 556] (cu-m)
4525 *%-----|-----|
4526 ADD HYD              NHYDsum=[ "MH107"], NHYDs to add=[ "106-107"+"A10-MN"+"A11-MN"]
4527 *%-----|-----|
4528 SAVE HYD            NHYD=[ "MH107"], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4529          HYD_COMMENT=[ "Total Flows at MH107"]
4530 *%-----|-----|
4531 ROUTE PIPE          PTYPE=[ 1] circ, NHYDout=[ "107-119"], RNUMBER=[ 1.0], PDIAM=[ 1800] (mm),
4532          PLNGTH=[ 114] (m), PROUGH=[ 0.013], PSLOPE=[ 0.0012] (m/m),
          NHYDin=[ "MH107"], RDT=[ 1]
4533 *%-----|-----|
4534 *% -JFSA 2021-01-12 THE MANHOLE MH108 is called MH20/108 in Corrigan Report, IBI
          Group, July 2008
4535 *%
4536 ROUTE PIPE          PTYPE=[ 1] circ, NHYDout=[ "119-108"], RNUMBER=[ 1.0], PDIAM=[ 1800] (mm),
4537          PLNGTH=[ 65.8] (m), PROUGH=[ 0.013], PSLOPE=[ 0.0012] (m/m),
          NHYDin=[ "107-119"], RDT=[ 1]
4538 *%-----|-----|
4539 CONTINUOUS STANDHYD NHYD=[ "A12"], DT=[ 1] min, AREA=[ 12.29] (ha),
4540          XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0] (cms), LOSS=[ 2],
4541          SCS curve number CN=[ 75],
4542          Pervious surfaces: IAper=[ 4.67] (mm), SLPP=[ 1] (%),
4543          LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (min),
4544          Impervious surfaces: IAimp=[ 1.57] (mm), SLPI=[ 1] (%),
4545          LGI=[ 183] (m), MNI=[ 0.013], SCI=[ 0] (min),
4546          Continuous simulation parameters:
4547          IARECper=[ 4] (hrs), IARECimp=[ 4] (hrs),
4548          SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
4549          InterEventTime=[ 18] (hrs), END=- 1
4550 *%-----|-----|
4551 COMPUTE DUALHYD      NHYDin=[ "A12"], CILET=[ 1.029] (cms), NILET=[ 1],
4552          MajNHYD=[ "A12-M"]
4553          MnNHYD=[ "A12-MN"]
4554          TMI STO=[ 672] (cu-m)
4555 *%-----|-----|
4556 CONTINUOUS STANDHYD NHYD=[ "A13"], DT=[ 1] min, AREA=[ 2.59] (ha),
4557          XI MP=[ 0.71], TI MP=[ 0.71], DWF=[ 0] (cms), LOSS=[ 2],
4558          SCS curve number CN=[ 75],
4559          Pervious surfaces: IAper=[ 4.67] (mm), SLPP=[ 1] (%),
4560          LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (min),
4561          Impervious surfaces: IAimp=[ 1.57] (mm), SLPI=[ 1] (%),
4562          LGI=[ 379] (m), MNI=[ 0.013], SCI=[ 0] (min),
4563          Continuous simulation parameters:
4564          IARECper=[ 4] (hrs), IARECimp=[ 4] (hrs),
4565          SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
4566          InterEventTime=[ 18] (hrs), END=- 1
4567 *%-----|-----|
4568 COMPUTE DUALHYD      NHYDin=[ "A13"], CILET=[ 0.571] (cms), NILET=[ 1],
4569          MajNHYD=[ "A13-M"]

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4570 M nNHYD=[ " A13- MN" ]
4571 TMJ STO=[ 0] ( cu- m)
4572 *%-----|-----|
4573 * -JFSA 2021-01-22 add the Corrigan pond area ("Pond-Block")
4574 CONTINUOUS STANDHYD NHYD=[ " Pond- Block" ], DT=[ 1] mi n, AREA=[ 2.94] ( ha),
4575 XI MP=[ 0.415], TI MP=[ 0.415], DWF=[ 0] ( cms), LOSS=[ 2],
4576 SCS curve number CN=[ 75],
4577 Pervious surfaces: I Aper=[ 4.67] ( mm), SLPP=[ 1] ( %),
4578 LGP=[ 40] ( m), MNP=[ 0.25], SCP=[ 0] ( mi n),
4579 Impervious surfaces: I Ai mp=[ 1.57] ( mm), SLPI = [ 1] ( %),
4580 LGI = [ 183] ( m), MNI = [ 0.013], SCI = [ 0] ( mi n),
4581 Continuous simulation parameters:
4582 IaRECper=[ 4] ( hrs), IaRECI mp=[ 4] ( hrs),
4583 SM N=[ -1] ( mm), SMAX=[ -1] ( mm), SK=[ 0.010] / ( mm),
4584 InterEvent Time=[ 18] ( hrs), END=- 1
4585 *%-----|-----|
4586 ADD HYD NHYDsum=[ " MHI08" ], NHYDs to add=[ " 119- 108" + " A13- MN" + " A12- MN" ]
4587 *%-----|-----|
4588 SAVE HYD NHYD=[ " MHI08" ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4589 HYD_COMMENT=[ " Total Flows at MHI08" ]
4590 *%-----|-----|
4591 ROUTE PIPE PTYPE=[ 1] circ, NHYDout=[ " 108- 116" ], RNUMBER=[ 1.0], PDI AM=[ 1800] ( mm),
4592 PLNGTH=[ 76.6] ( m), PROUGH=[ 0.013], PSLOPE=[ 0.0013] ( m/ m),
4593 NHYDin=[ " MHI08" ], RDT=[ 1]
4594 *%-----|-----|
4595 ROUTE PIPE PTYPE=[ 1] circ, NHYDout=[ " 116- corrig" ], RNUMBER=[ 1.0],
4596 PDI AM=[ 1800] ( mm),
4597 PLNGTH=[ 79.5] ( m), PROUGH=[ 0.013], PSLOPE=[ 0.0013] ( m/ m),
4598 NHYDin=[ " 108- 116" ], RDT=[ 1]
4599 *%-----|-----|
4600 ADD HYD NHYDsum=[ " Corrigan" ], NHYDs to add=[ " 116- corrig" + " Pond- Block" ]
4601 *%-----|-----|
4602 SAVE HYD NHYD=[ " Corrigan" ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4603 HYD_COMMENT=[ " Total Flows at Corrigan Pond" ]
4604 *%-----|-----|
4605 ROUTE RESERVOIR NHYDout = [ " Co- P" ], NHYDin=[ " Corrigan" ],
4606 RDT=[ 1] ( mi n),
4607 TABLE of ( OUTFLOW STORAGE ) values
4608 ( cms ) - ( ha- m)
4609 [ 0.0 , 0.0 ]
4610 [ 0.015 , 0.04118]
4611 [ 0.030 , 0.08297]
4612 [ 0.045 , 0.12537]
4613 [ 0.060 , 0.16837]
4614 [ 0.075 , 0.21199]
4615 [ 0.090 , 0.27545]
4616 [ 0.105 , 0.34650]
4617 [ 0.120 , 0.42049]
4618 [ 0.135 , 0.50188]
4619 [ 0.186 , 0.60307]
4620 [ 2.110 , 0.79083]
4621 [ 5.874 , 1.00271]
4622 [ 11.395 , 1.29643]
4623 [ 18.770 , 1.62054]
4624 [ 28.143 , 1.97516]
4625 [ -1 , -1 ] (max twenty pts)
4626 NHYDovf=[ " Co- P- OVF" ]
4627 *%-----|-----|
4628 ADD HYD NHYDsum=[ " corrig" ], NHYDs to
4629 add=[ " Co- P- OVF" + " Co- P" + " N_ TO" + " MHI05- JR" + " A8- M" + " JR" + " A9- M" + " A10- M" + " A11- M" + " A12- M" + " A
4630 13- M" ]
4631 *%-----|-----|
4632 SAVE HYD NHYD=[ " corrig" ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4633 HYD_COMMENT=[ " Total Flows at Corrigan Pond" ]
4634 *%-----|-----|
4635 *#*****

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4631  *#      Corrigan Pond 1
4632  *#      - Rating curve obtained from Barrhaven South M&S modeling
4633  *#      - Tributary Drainage Area to M&S Pond 1 = 145 ha
4634  *#*****
4635  *ROUTE RESERVOIR      NHYDout=[ "MS_P1" ],  NHYDin=[ "CORRIG" ],
4636  *                      RDT=[ 1 ] ( mi n ),
4637  *                      TABLE of ( OUTFLOW STORAGE ) values
4638  *                      ( cms ) - ( ha- m )
4639  *                      [  0.0 ,  0.0 ]
4640  *                      [  0.06 , 0.58 ]
4641  *                      [  -1 ,  -1 ] (max twenty pts)
4642  *                      NHYDovf=[ "P1-OVF" ]
4643  *%-----|-----
4644  *ADD HYD              NHYDsum=[ "SN_CO" ], NHYDs to add=[ "N_TO"+"P1-OVF"+"MS_P1" ]
4645  *%-----|-----
4646  *SAVE HYD            NHYD=[ "SN_CO" ], # OF PCYCLES=[ -1 ], ICASEsh=[ 1 ]
4647  *                      HYD_COMMENT=[ "Total Flows at Corrigan Drain" ]
4648  *%-----|-----
4649  *#
4650  *# Hydrograph from Corrigan Drain routed to Jockvale Road
4651  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
4652  *#
4653  ROUTE CHANNEL      NHYDout=[ "N_M" ] , NHYDin=[ "corrig" ] ,
4654  RDT=[ 1 ] ( mi n ),
4655  CHLGTH=[ 580 ] ( m ),  CHSLOPE=[ 0.4448 ] ( % ),
4656  FPSLOPE=[ 0.4448 ] ( % ),
4657  SECNUM=[ 1.0 ],      NSEG=[ 3 ]
4658  ( SEGROUGH, SEGDI ST ( m ) ) =
4659  [ 0.075, -17.72
4660  -0.045, 17.72
4661  0.075, 80.62 ] NSEG times
4662  ( DI STANCE ( m ), ELEVATI ON ( m ) ) =
4663  [ -83.32, 90.00 ]
4664  [ -81.36, 89.50 ]
4665  [ -79.12, 89.00 ]
4666  [ -76.13, 88.50 ]
4667  [ -20.46, 88.00 ]
4668  [ -19.36, 87.50 ]
4669  [ -18.51, 87.00 ]
4670  [ -17.72, 86.50 ]
4671  [ -11.95, 85.24 ]
4672  [ -0.11, 85.12 ]
4673  [ 11.49, 85.20 ]
4674  [ 17.72, 86.50 ]
4675  [ 19.74, 87.00 ]
4676  [ 21.22, 87.50 ]
4677  [ 22.68, 88.00 ]
4678  [ 24.28, 88.50 ]
4679  [ 26.79, 89.00 ]
4680  [ 71.98, 90.00 ]
4681  [ 80.62, 90.50 ]
4682  *%-----|-----
4683  *#*****
4684  *#      Catchment MLLS
4685  *#      - To SWM Facility north of the Jock
4686  *#      - Primarily residential development
4687  *#*****
4688  CONTINUOUS STANDHYD NHYD=[ "MLLS" ], DT=[ 1 ] mi n, AREA=[ 175.99 ] ( ha ),
4689  XI MP=[ 0.38 ], TI MP=[ 0.38 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
4690  SCS curve number CN=[ 74 ],
4691  Pervious surfaces: I A per=[ 4.67 ] ( mm ), SLPP=[ 1 ] ( % ),
4692  LGP=[ 40 ] ( m ), MNP=[ 0.25 ], SCP=[ 0 ] ( mi n ),
4693  Impervious surfaces: I A i mp=[ 1.57 ] ( mm ), SLPI=[ 1 ] ( % ),
4694  LGI=[ 1118.123 ] ( m ), MNI=[ 0.013 ], SCI=[ 0 ] ( mi n ),
4695  Continuous simulation parameters:
4696  I a REC per=[ 4 ] ( hr s ), I a REC i mp=[ 4 ] ( hr s ),

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4697 SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
4698 Inter Event Time=[ 18 ] ( hr s ) , END=- 1
4699 *%-----|-----|
4700 *#*****|
4701 *# Chapman Mills SWM Pond
4702 *# - Rating curve obtained from CCL hydraulic modeling
4703 *#*****|
4704 ROUTE RESERVOIR NHYDout =[ " M LL_P " ] , NHYDin =[ " M LLS " ] ,
4705 RDT=[ 1 ] ( mi n ) ,
4706 TABLE of ( OUTFLOW STORAGE ) values
4707 ( cms ) - ( ha- m )
4708 [ 0. 0 , 0. 0 ]
4709 [ 0. 01 , 0. 01 ]
4710 [ 0. 05 , 0. 06 ]
4711 [ 0. 09 , 0. 11 ]
4712 [ 0. 13 , 0. 15 ]
4713 [ 0. 18 , 0. 19 ]
4714 [ 0. 28 , 0. 28 ]
4715 [ 0. 37 , 0. 34 ]
4716 [ 0. 45 , 0. 40 ]
4717 [ 0. 51 , 0. 44 ]
4718 [ 0. 56 , 0. 47 ]
4719 [ 0. 64 , 0. 52 ]
4720 [ 0. 76 , 0. 59 ]
4721 [ 0. 86 , 0. 65 ]
4722 [ 1. 09 , 0. 78 ]
4723 [ 1. 44 , 0. 96 ]
4724 [ 3. 18 , 1. 84 ]
4725 [ 4. 05 , 2. 31 ]
4726 [ - 1 , - 1 ] ( max t went y pts )
4727 NHYDovf =[ " M L- OV " ]
4728 *%-----|-----|
4729 ADD HYD NHYDs um=[ " SN_M " ] , NHYDs to add=[ " N_M " + " M L- OV " + " M LL_P " ]
4730 *%-----|-----|
4731 SAVE HYD NHYD=[ " SN_M " ] , # OF PCYCLES=[ - 1 ] , I CASEs h=[ 1 ]
4732 HYD_COMMENT=[ " Tot al Fl ows at Jockval e Road " ]
4733 *%-----|-----|
4734 *#
4735 *# Hydrograph from Jockvale Road routed to Heart's Desire
4736 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689
4737 *#
4738 ROUTE CHANNEL NHYDout =[ " N_DE " ] , NHYDin =[ " SN_M " ] ,
4739 RDT=[ 1 ] ( mi n ) ,
4740 CHLGTH=[ 1962 ] ( m ) , CHSLOPE=[ 0. 2227 ] ( % ) ,
4741 FPSLOPE=[ 0. 2227 ] ( % ) ,
4742 SECNUM=[ 1. 0 ] , NSEG=[ 3 ]
4743 ( SEGROUGH, SEGDI ST ( m ) ) =
4744 [ 0. 075 , - 17. 56
4745 - 0. 045 , 18. 27
4746 0. 075 , 32. 51 ] NSEG t i mes
4747 ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
4748 [ - 54. 07 , 85. 00 ]
4749 [ - 39. 43 , 84. 50 ]
4750 [ - 28. 30 , 84. 00 ]
4751 [ - 24. 12 , 83. 50 ]
4752 [ - 22. 30 , 83. 00 ]
4753 [ - 20. 55 , 82. 50 ]
4754 [ - 17. 56 , 82. 00 ]
4755 [ - 12. 63 , 81. 22 ]
4756 [ - 0. 11 , 80. 75 ]
4757 [ 11. 55 , 81. 22 ]
4758 [ 18. 27 , 82. 00 ]
4759 [ 19. 82 , 82. 50 ]
4760 [ 22. 48 , 83. 00 ]
4761 [ 27. 90 , 83. 50 ]
4762 [ 29. 31 , 84. 00 ]

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4763 [ 30.81, 84.50]
4764 [ 32.51, 85.00]
4765 *%-----|-----|
4766 *#*****|
4767 *# Catchment DESIRE
4768 *# - To Jock River (north of the Jock)
4769 *# - Rural-estate subdivision (Heart's Desire Community)
4770 *#*****|
4771 CONTINUOUS STANDHYD NHYD=["DESIRE"], DT=[1] min, AREA=[23.78](ha),
4772 XI MP=[0.25], TI MP=[0.25], DWF=[0](cms), LOSS=[2],
4773 SCS curve number CN=[77],
4774 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4775 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4776 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4777 LGI=[400](m), MNI=[0.013], SCI=[0](min),
4778 Continuous simulation parameters:
4779 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
4780 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4781 Inter Event Time=[18](hrs), END=-1
4782 *%-----|-----|
4783 *#*****|
4784 *# Catchment JOCKVA
4785 *# - To Jockvale SWM Facility
4786 *# - Residential development & golf course
4787 *# - JFSA 2021-01-11 update JOCKVA after updating CORRIG as per IBI GROUP, July 2008.
4788 *# JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two
areas JOCKVA and EX-LAND 32.5 ha as per IBI GROUP, July 2008.
4789 *#*****|
4790 CONTINUOUS STANDHYD NHYD=["JOCKVA"], DT=[1] min, AREA=[225.13](ha),
4791 XI MP=[0.50], TI MP=[0.50], DWF=[0](cms), LOSS=[2],
4792 SCS curve number CN=[74],
4793 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4794 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4795 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4796 LGI=[1310.55](m), MNI=[0.013], SCI=[0](min),
4797 Continuous simulation parameters:
4798 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
4799 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4800 Inter Event Time=[18](hrs), END=-1
4801 *%-----|-----|
4802 ADD HYD NHYDsum=["JOCKVA-TO"], NHYDsto
add=["EX-LAND-MN"+"JOCKVA"+"B2-M"+"B3-M"]
4803 *%-----|-----|
4804 SAVE HYD NHYD=["JOCKVA-TO"], # OF PCYCLES=[-1], ICASEsh=[1]
4805 HYD_COMMENT=["Total Flows at KB first pond"]
4806 *%-----|-----|
4807 *#*****|
4808 *# Jockvale SWM Facility
4809 *# - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
4810 *#*****|
4811 ROUTE RESERVOIR NHYDout=["JOCK_P"], NHYDin=["JOCKVA-TO"],
4812 RDT=[1](min),
4813 TABLE of ( OUTFLOW STORAGE ) values
4814 (cms) - (ha-m)
4815 [ 0.0 , 0.0 ]
4816 [ 0.27 , 0.03]
4817 [ 0.28 , 0.55]
4818 [ 0.29 , 1.14]
4819 [ 0.30 , 1.80]
4820 [ 0.31 , 2.32]
4821 [ 1.12 , 2.87]
4822 [ 2.92 , 3.45]
4823 [ 4.64 , 4.07]
4824 [ 6.69 , 4.72]
4825 [ 9.02 , 5.39]
4826 [ 11.62 , 6.10]

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4827 [ 14.42 , 6.85]
4828 [ 17.45 , 7.62]
4829 [ 20.69 , 8.44]
4830 [ 24.08 , 9.28]
4831 [ 27.68 , 10.17]
4832 [ -1 , -1 ] (max twenty pts)
4833 NHYDovf=["JO-OVF"]
4834 *%-----|-----|
4835 ADD HYD NHYDs um=["SN_DE"], NHYDs to add=["N_DE"+"DESIRE"+"JO-OVF"+"JOCK_P"]
4836 *%-----|-----|
4837 SAVE HYD NHYD=["SN_DE"], # OF PCYCLES=[-1], ICASEsh=[1]
4838 HYD_COMMENT=["Total Flows at Heart's Desire"]
4839 *%-----|-----|
4840 *#
4841 *# Hydrograph from Heart's Desire routed to Rideau River
4842 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 0
4843 *#
4844 ROUTE CHANNEL NHYDout=["NI"], NHYDin=["SN_DE"],
4845 RDT=[1](min),
4846 CHLGTH=[563](m), CHSLOPE=[0.9668](%),
4847 FPSLOPE=[0.9668](%),
4848 SECNUM=[1.0], NSEG=[3]
4849 ( SEGROUGH, SEGDIST (m))=
4850 [0.075, -30.20
4851 -0.045, 30.20
4852 0.075, 48.48] NSEG times
4853 ( DISTANCE (m), ELEVATION (m))=
4854 [-98.46, 81.50]
4855 [-92.24, 81.00]
4856 [-86.88, 80.50]
4857 [-81.54, 80.00]
4858 [-74.36, 79.50]
4859 [-63.54, 79.00]
4860 [-39.23, 78.50]
4861 [-34.51, 78.00]
4862 [-33.01, 77.50]
4863 [-30.20, 77.00]
4864 [-13.42, 76.18]
4865 [-1.14, 76.09]
4866 [17.06, 76.18]
4867 [30.20, 77.00]
4868 [32.95, 77.50]
4869 [34.06, 78.00]
4870 [35.11, 78.50]
4871 [36.32, 79.00]
4872 [37.74, 79.50]
4873 [48.48, 81.50]
4874 *%-----|-----|
4875 *#*****
4876 *# Catchment S-2
4877 *# - To Jock River (north and south)
4878 *# - Undeveloped floodplain and river
4879 *#*****
4880 CONTINUOUS NASHYD NHYD=["S-2"], DT=[1]min, AREA=[102.94](ha),
4881 DWF=[0](cms), CN/C=[72], IA=[4.67](mm),
4882 N=[3], TP=[0.40]hrs,
4883 Continuous simulation parameters:
4884 IareCper=[4](hrs),
4885 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4886 InterEventTime=[12](hrs)
4887 Baseflow simulation parameters:
4888 BaseFlowOption=[1],
4889 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4890 VHydCond=[0.055](mm/hr), END=-1
4891 *%-----|-----|
4892 ADD HYD NHYDs um=["SN_NI"], NHYDs to add=["NI"+"S-2"]

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4893 *%-----|-----|
4894 SAVE HYD          NHYD=["SN_NI"], # OF PCYCLES=[-1], ICASEs h=[1]
4895                 HYD_COMMENT=["Total Flows at Rideau River"]
4896 *%-----|-----|
4897 *#####
4898 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4899 START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[5]
4900 *%              ["C24SC005.stm"] <--storm filename, one per line for NSTORMtime
4901 *%-----|-----|
4902 *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4903 START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[10]
4904 *%              ["C24SC010.stm"] <--storm filename, one per line for NSTORMtime
4905 *%-----|-----|
4906 *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4907 START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[25]
4908 *%              ["C24SC025.stm"] <--storm filename, one per line for NSTORMtime
4909 *%-----|-----|
4910 *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4911 START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[50]
4912 *%              ["C24SC050.stm"] <--storm filename, one per line for NSTORMtime
4913 *%-----|-----|
4914 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4915 *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4916 *%              ["100YC3H.STM"] <--storm filename, one per line for NSTORMtime
4917 *%-----|-----|
4918 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4919 START            TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4920 *%              ["C24SC100.stm"] <--storm filename, one per line for NSTORMtime
4921 *%-----|-----|
4922 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4923 *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4924 *%              ["C24SC100.stm"] <--storm filename, one per line for NSTORMtime
4925 *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[101]
4926 *%              ["A24SC100.stm"] <--storm filename, one per line for NSTORMtime
4927 *START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
4928 *%              ["A24SC100_60.stm"] <--storm filename, one per line for NSTORMtime
4929 FINISH
4930

```


Table with multiple columns including codes (e.g., 014977, 014978), parameters, values, and descriptions. It contains a dense list of technical data points and notes, such as 'CONTINUES STANDBY', 'MJOR SYSTEM', and 'COMPUTE DUAL/D'. Includes a 'PROPOSED Subcatchment' section for the Kennedy-Burnett SWM Facility.

Table with multiple columns containing alphanumeric codes (e.g., 022445, 022446), descriptions (e.g., ROUTE PIPE, COMPUTE DUALJND), and numerical data (e.g., 1.0, 0.01, 146.76, 4.493, 28.07, 26.52, n/a, 0.00).

```

026198 overflow cul 1 0 03:JO:DFP 00 000 No.date 0:00 0:00 n/a 0:00
026208 (M&Stoked, 2334ft) Total Vol. 0.0000400 cu_N(C) r (0_TotD) r (0_hrs)
026211 R0002: CO0396.....DfIn-n-1D NND.....AREA-QPEAKm-TpPeakDte-hh-mm--RvMm-R-C--DFWm
026223 ADD HYD + 1 0 02:DESIRE 55476.26 49.617 No.date 38:54 13.23 n/a 0:00
026223 23.78 9316 No.date 28:03 19.26 n/a 0:00
026248 + 1 0 02:JO:DFP 00 000 No.date 0:00 0:00 n/a 0:00
026258 + 1 0 02:LOCK.P 257.63 2.500 No.date 29:05 26.83 n/a 0:00
026268 SUM 1 0 01:SN,DE 55476.26 49.619 No.date 38:49 13.23 n/a 0:00
026273 R0002: CO0397.....DfIn-n-1D NND.....AREA-QPEAKm-TpPeakDte-hh-mm--RvMm-R-C--DFWm
026288 SAVE HYD 1 0 01:SN,DE 55476.26 49.619 No.date 38:49 13.23 n/a 0:00
026298 *****
026303 *****
026313 *****
026328 # Hydrograph from Heart's Desire routed to Rideau River
026333 # Channel X-Section obtained from RCHA Hydraulic Model - Station 0
026348 *****
026358 R0002: CO0398.....DfIn-n-1D NND.....AREA-QPEAKm-TpPeakDte-hh-mm--RvMm-R-C--DFWm
026368 ROUTE CHANNEL 1 0 01:SN,DE 55476.26 49.619 No.date 38:49 13.23 n/a 0:00
026373 (RDf=1.0) out.c. 1 0 01:SN 55476.26 49.617 No.date 38:54 13.23 n/a 0:00
026388 (L/S= 563 / 967/045)
026398 *****
026403 *****
026428 # To Lock River (north and south)
026433 # Undeveloped floodplain and river
026448 *****
026458 R0002: CO0399.....DfIn-n-1D NND.....AREA-QPEAKm-TpPeakDte-hh-mm--RvMm-R-C--DFWm
026468 CONTINUES NASHID 1 0 01:SW 102.94 1.373 No.date 28:20 13.01 286 0:00
026473 (Cm= 72.0; No. 3.00; Tpe= 4.0)
026488 (InterEventTime= 12.00)
026498 *****
026508 R0002: CO0400.....DfIn-n-1D NND.....AREA-QPEAKm-TpPeakDte-hh-mm--RvMm-R-C--DFWm
026518 ADD HYD + 1 0 02:DESIRE 55476.26 49.617 No.date 38:54 13.23 n/a 0:00
026528 + 1 0 02: S.2 102.94 1.373 No.date 28:20 13.01 n/a 0:00
026538 SUM 1 0 01:LOCK.P 55779.20 49.715 No.date 38:54 13.23 n/a 0:00
026548 R0002: CO0401.....DfIn-n-1D NND.....AREA-QPEAKm-TpPeakDte-hh-mm--RvMm-R-C--DFWm
026558 SAVE HYD 1 0 01:SN,NO 55779.20 49.715 No.date 38:54 13.23 n/a 0:00
026568 *****
026573 *****
026588 *****
026598 *****
026603 *****
026613 *****
026628 *****
026638 *****
026648 *****
026658 *****
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029928 *****

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Table with multiple columns containing alphanumeric codes (e.g., 033677, R0005, C00098), technical specifications, and numerical data. The table is organized into vertical columns and contains a dense array of alphanumeric characters and numbers.

Table with columns for ID, description, and numerical data. Includes entries for COMPUTE DUAL/D, Mjr System, Msr System, and various R005 codes. Includes a 'PROPOSED Subcatchments' section for Kennedy-Burnett SWM Facility.

Table with columns for ID, description, and numerical values. The table lists various system components and their associated data points across multiple rows.

Table with columns for ID, description, and numerical data. Rows include hydrograph data for various nodes (e.g., Node 7, Node 5, Node 4) and detailed model parameters for numerous subwatersheds (e.g., R0101, R0102, R0103) and channels. The table lists parameters like flow rates, dates, and model identifiers.

Table with columns for station ID, name, location, and various technical parameters. Includes entries for ROUTE 101, ROUTE 102, and ROUTE 103.

Table with columns for station ID, name, location, and various technical parameters. Includes entries for ROUTE 104, ROUTE 105, and ROUTE 106.


```

078555 # License # 2549257
078556 *****
07857 # CALIBRATION OF SUMMER FLOW PARAMETERS
07858 # USING CONTINUOUS SIMULATIONS
07859 # Rainfall data from JFSa rain gauge installed at site + other gauges by the City
07860 # Use data collected from May 1st to July 14, 2001
07861 # 2020-12-01 change TMSSTO to COMPUTE TROUDED (TMSSTO = 1, instead of 0.0001)
07862 # 2020-12-01 correct pond curve values
07863 # 2020-12-01 change WCLAB ROAD 2.0 MP to 0.55 SLP(=0.51)(% (imperial slope), and LGI up to 700m
07864 # 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NDRout="N_TOI", NDRin="SN_TOI") from 0.033 % (as per S
07865 # 2021-02-19 Change the slope for ROUTE CHANNEL Station 5022 (NDRout="N_NC", NDRin="SN_NC") from 0.019 % (as per S
07866 #
07867 # R025: C0002
07868 #
07869 # READ STORM
07870 # Filename = storm001
07871 # Comments = Pluic PCS 24 hrs 1.25 ans pour Ctawm CDA
07872 # [SID=10.00;SIDR= 24.00;PFR= 74.39]
07873 # R025: C0003
07874 # KEFV STORM
07875 # [RFact= 1.00;TSH=PT= 96.00 min]
07876 # [SID=10.00;SIDR= 24.00;PFR= 74.39]
07877 # R025: C0004
07878 #
07879 # REFLECT VALUES
07880 #
07881 # Filename = T:\PROJ\1474-16\Design\2020\1026-QuantityControlAnalysis\SWMM6a-SMR-Model\updated\cd3\CI_G1_eaf_DEF
07882 # [CSave= 1 (read and print data)]
07883 # File title: File comment: Based on various calibration exercises in Ont
07884 # THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDARD COM
07885 #
07886 # Avton's infiltration equation parameters:
07887 # [Infil= 70.20 m/hr] [K=0.1320 m/hr] [ICAV= 4.14 hr] [P= .00 mm]
07888 # Parameters for PERFOR surfaces in STANDARD
07889 # [Infil= 4.67 mm] [Lq=50.00 mm] [DWP= .250]
07890 # Parameters for IMPERVIOUS surfaces in STANDARD
07891 # [Infil= 1.57 mm] [Lq= 1.50] [DWP= .013]
07892 # Parameters used in NASIDD
07893 # [Infil= 0.00] [DWP= .01]
07894 # Average monthly Pan Evaporation data in mm
07895 #
07896 #
07897 #
07898 #
07899 #
07900 #
07901 #
07902 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07903 # R025: C0006
07904 # CONTINUES NASIDD 1.0 0.01;SCL 3074.00 15.500 No.date 36.57 25.80 34.7 000
07905 # [Cm= 6.0; N= 3.00; Tp= 1.31]
07906 # [IAREC= 4.00; SM= 55.05; SMM=330.32; SK= .010]
07907 # [InterEventTime= 12.00]
07908 #
07909 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07910 # of 1.32
07911 # R025: C0007
07912 # CONTINUES NASIDD 1.0 0.01;SCL 971.00 5.778 No.date 32.34 24.02 32.3 000
07913 # [Cm= 6.0; N= 3.00; Tp= 1.32]
07914 # [IAREC= 4.00; SM= 64.50; SMM=430.01; SK= .010]
07915 # [InterEventTime= 12.00]
07916 #
07917 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07918 # of 1.30
07919 # R025: C0008
07920 # CONTINUES NASIDD 1.0 0.01;SCL 3074.00 15.500 No.date 39.59 20.65 27.8 000
07921 # [Cm= 5.5; N= 3.00; Tp= 1.31]
07922 # [IAREC= 4.00; SM= 83.24; SMM=554.96; SK= .010]
07923 # [InterEventTime= 12.00]
07924 # R025: C0009
07925 # CONTINUES NASIDD 1.0 0.01;SCL 1781.00 14.166 No.date 32.40 31.50 42.3 000
07926 # [Cm= 7.0; N= 3.00; Tp= 1.31]
07927 # [IAREC= 4.00; SM= 75.32; SMM=264.99; SK= .010]
07928 # [InterEventTime= 12.00]
07929 # R025: C0010
07930 # CONTINUES NASIDD 1.0 0.01;SCL 3074.00 15.500 No.date 29.22 27.01 36.3 000
07931 # [Cm= 6.0; N= 3.00; Tp= 1.24]
07932 # [IAREC= 4.00; SM= 52.62; SMM=350.79; SK= .010]
07933 # [InterEventTime= 12.00]
07934 #
07935 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07936 # of 1.80
07937 # R025: C0011
07938 # CONTINUES NASIDD 1.0 0.01;SCL 1917.00 10.351 No.date 34.27 27.01 36.3 000
07939 # [Cm= 6.0; N= 3.00; Tp= 1.80]
07940 # [IAREC= 4.00; SM= 52.62; SMM=350.79; SK= .010]
07941 # [InterEventTime= 12.00]
07942 #
07943 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07944 # of 1.52
07945 # R025: C0012
07946 # CONTINUES NASIDD 1.0 0.01;SCL 5666.00 27.457 No.date 37.54 31.50 42.3 000
07947 # [Cm= 7.0; N= 3.00; Tp= 1.32]
07948 # [IAREC= 4.00; SM= 75.32; SMM=264.99; SK= .010]
07949 # [InterEventTime= 12.00]
07950 #
07951 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07952 # of 1.75
07953 # R025: C0013
07954 # CONTINUES NASIDD 1.0 0.01;SCL 3074.00 15.500 No.date 39.59 27.01 36.3 000
07955 # [Cm= 6.0; N= 3.00; Tp= 1.11]
07956 # [IAREC= 4.00; SM= 52.62; SMM=350.79; SK= .010]
07957 # [InterEventTime= 12.00]
07958 #
07959 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07960 # of 1.68
07961 # R025: C0014
07962 # CONTINUES NASIDD 1.0 0.01;SCL 1132.00 11.752 No.date 30.54 30.18 40.6 000
07963 # [Cm= 7.0; N= 3.00; Tp= 1.68]
07964 # [IAREC= 4.00; SM= 43.07; SMM=287.10; SK= .010]
07965 # [InterEventTime= 12.00]
07966 #
07967 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07968 # of 1.82
07969 # R025: C0015
07970 # CONTINUES NASIDD 1.0 0.01;SCL 4064.00 13.075 No.date 39.59 24.61 33.1 000
07971 # [Cm= 6.0; N= 3.00; Tp= 1.32]
07972 # [IAREC= 4.00; SM= 61.90; SMM=412.66; SK= .010]
07973 # [InterEventTime= 12.00]
07974 #
07975 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07976 # of 1.80
07977 # R025: C0016
07978 # CONTINUES NASIDD 1.0 0.01;SCL 131.00 2.266 No.date 28.57 25.20 33.9 000
07979 # [Cm= 6.0; N= 3.00; Tp= .901]
07980 # [IAREC= 4.00; SM= 59.42; SMM=396.11; SK= .010]
07981 # [InterEventTime= 12.00]
07982 #
07983 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07984 # of 1.65
07985 # R025: C0017
07986 # CONTINUES NASIDD 1.0 0.01;SCL 3854.00 15.333 No.date 38.34 27.01 36.3 000
07987 # [Cm= 6.0; N= 3.00; Tp= 1.65]
07988 # [IAREC= 4.00; SM= 52.62; SMM=350.79; SK= .010]
07989 # [InterEventTime= 12.00]
07990 #
07991 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
07992 # of 1.82
07993 # R025: C0018
07994 # CONTINUES NASIDD 1.0 0.01;SCL 11663.00 36.24 21.75 29.2 000
07995 # [Cm= 7.0; N= 3.00; Tp= 1.82]
07996 # [IAREC= 4.00; SM= 75.32; SMM=264.99; SK= .010]
07997 # [InterEventTime= 12.00]
07998 #
07999 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08000 # of 1.75
08001 # R025: C0019
08002 # CONTINUES NASIDD 1.0 0.01;SCL 165.00 1.076 No.date 33.03 27.63 37.1 000
08003 # [IAREC= 4.00; SM= 50.55; SMM=336.97; SK= .010]
08004 # [InterEventTime= 12.00]
08005 #
08006 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08007 # of 1.63
08008 # R025: C0020
08009 # CONTINUES NASIDD 1.0 0.01;SCL 1332.00 7.882 No.date 35.14 31.50 42.3 000
08010 # [Cm= 7.0; N= 3.00; Tp= 1.63]
08011 # [IAREC= 4.00; SM= 39.75; SMM=244.99; SK= .010]
08012 # [InterEventTime= 12.00]
08013 #
08014 # R025: C0021
08015 # CONTINUES NASIDD 1.0 0.01;SCL 224.00 6.882 No.date 28.45 35.66 47.9 000
08016 # [Cm= 7.0; N= 3.00; Tp= .75]
08017 # [IAREC= 4.00; SM= 31.15; SMM=207.66; SK= .010]
08018 # [InterEventTime= 12.00]
08019 #
08020 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08021 # of 1.20
08022 # R025: C0022
08023 # CONTINUES NASIDD 1.0 0.01;SCL 945.00 17.664 No.date 33.18 32.85 44.2 000
08024 # [Cm= 7.4; N= 3.00; Tp= 1.20]
08025 # [IAREC= 4.00; SM= 36.30; SMM=244.99; SK= .010]
08026 # [InterEventTime= 12.00]
08027 # R025: C0023
08028 # CONTINUES NASIDD 1.0 0.01;SCL 20.00 7.98 No.date 28.35 39.36 52.9 000
08029 # [Cm= 8.1; N= 3.00; Tp= .621]
08030 # [IAREC= 4.00; SM= 25.21; SMM=168.09; SK= .010]
08031 # [InterEventTime= 12.00]
08032 #
08033 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08034 # of 1.61
08035 # R025: C0024
08036 # CONTINUES NASIDD 1.0 0.01;SCL 1412.00 7.480 No.date 37.50 34.24 46.0 000
08037 # [Cm= 7.0; N= 3.00; Tp= 1.61]
08038 # [IAREC= 4.00; SM= 33.81; SMM=225.43; SK= .010]
08039 # [InterEventTime= 12.00]
08040 # R025: C0025
08041 # CONTINUES NASIDD 1.0 0.01;SCL 585.00 10.942 No.date 29.56 39.36 52.9 000

```

Table with multiple columns containing alphanumeric codes (e.g., 082729, R0025, C00051), descriptions of hydrographs and flow types, and numerical data points (e.g., 38743.00, 44.08, 26.55, n/a, 0.00). The table lists various hydrograph events and flow types across different nodes and sections.

13091 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13092 # of 1.80
13093 ROUTE: C00008..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13094 CONTINUES NASHDD 1.0 01:SW11 3074.00 10.428 No.date 39:59 28.29 n/a 000
13095 [Cm 55.0 No 3.00: Tp=1.33]
13096 [Iarec 4.00: SMN= 83.24: SMW=554.96: SKE= 010]
13097 [InterEventTime= 12.00]
13098 ROUTE: C00009..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13099 CONTINUES NASHDD 1.0 01:SW11 1781.00 19.695 No.date 32:38 42.49 n/a 000
13100 [Cm 72.0 No 3.00: Tp= 3.91]
13101 [Iarec 4.00: SMN= 264.99: SKE= 010]
13102 [InterEventTime= 12.00]
13103 ROUTE: C00010..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13104 CONTINUES NASHDD 1.0 01:SW11 500.00 10.735 No.date 29:21 36.76 n/a 000
13105 [Cm 66.0 No 3.00: Tp= 1.24]
13106 [Iarec 4.00: SMN= 52.62: SMW=350.79: SKE= 010]
13107 [InterEventTime= 12.00]
13108 #
13109 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13110 # of 1.80
13111 ROUTE: C00011..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13112 CONTINUES NASHDD 1.0 01:SW11 1917.00 14.496 No.date 34:24 36.76 n/a 000
13113 [Cm 66.0 No 3.00: Tp= 2.99]
13114 [Iarec 4.00: SMN= 52.62: SMW=350.79: SKE= 010]
13115 [InterEventTime= 12.00]
13116 #
13117 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13118 # of 1.52
13119 ROUTE: C00012..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13120 CONTINUES NASHDD 1.0 01:SW10 5666.00 37.663 No.date 37:48 42.49 n/a 000
13121 [Cm 72.0 No 3.00: Tp= 8.00]
13122 [Iarec 4.00: SMN= 39.75: SMW=264.99: SKE= 010]
13123 [InterEventTime= 12.00]
13124 #
13125 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13126 # of 1.75
13127 ROUTE: C00013..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13128 CONTINUES NASHDD 1.0 01:SW11 876.00 36.118 No.date 39:59 36.76 n/a 000
13129 [Cm 66.0 No 3.00: Tp=1.66]
13130 [Iarec 4.00: SMN= 52.62: SMW=350.79: SKE= 010]
13131 [InterEventTime= 12.00]
13132 #
13133 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13134 # of 1.68
13135 ROUTE: C00014..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13136 CONTINUES NASHDD 1.0 01:SW9 1132.00 16.501 No.date 30:52 40.82 n/a 000
13137 [Cm 72.0 No 3.00: Tp= 8.00]
13138 [Iarec 4.00: SMN= 43.07: SMW=287.10: SKE= 010]
13139 [InterEventTime= 12.00]
13140 #
13141 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13142 # of 1.82
13143 ROUTE: C00015..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13144 CONTINUES NASHDD 1.0 01:SW10 4484.00 18.060 No.date 39:59 33.61 n/a 000
13145 [Cm 62.0 No 3.00: Tp=1.33]
13146 [Iarec 4.00: SMN= 61.90: SMW=412.66: SKE= 010]
13147 [InterEventTime= 12.00]
13148 #
13149 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13150 # of 1.80
13151 ROUTE: C00016..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13152 CONTINUES NASHDD 1.0 01:SW8 131.00 3.259 No.date 28:57 34.39 n/a 000
13153 [Cm 63.0 No 3.00: Tp= 9.01]
13154 [Iarec 4.00: SMN= 52.62: SMW=396.11: SKE= 010]
13155 [InterEventTime= 12.00]
13156 #
13157 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13158 # of 1.65
13159 ROUTE: C00017..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13160 CONTINUES NASHDD 1.0 01:HB1RR 3854.00 21.218 No.date 38:28 36.76 n/a 000
13161 [Cm 66.0 No 3.00: Tp= 1.66]
13162 [Iarec 4.00: SMN= 52.62: SMW=350.79: SKE= 010]
13163 [InterEventTime= 12.00]
13164 #
13165 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13166 # of 1.82
13167 ROUTE: C00018..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13168 CONTINUES NASHDD 1.0 01:SW7 3197.00 16.421 No.date 36:21 29.79 n/a 000
13169 [Cm 77.0 No 3.00: Tp= 6.51]
13170 [Iarec 4.00: SMN= 508.81: SKE= 010]
13171 [InterEventTime= 12.00]
13172 #
13173 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13174 # of 1.75
13175 ROUTE: C00019..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13176 CONTINUES NASHDD 1.0 01:SW6 165.00 1.511 No.date 33:01 37.57 n/a 000
13177 [Cm 67.0 No 3.00: Tp= 4.17]
13178 [Iarec 4.00: SMN= 35.92: SMW=336.97: SKE= 010]
13179 [InterEventTime= 12.00]
13180 #
13181 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13182 # of 1.63
13183 ROUTE: C00020..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13184 CONTINUES NASHDD 1.0 01:VGR 1332.00 10.882 No.date 35:10 42.49 n/a 000
13185 [Cm 72.0 No 3.00: Tp= 3.91]
13186 [Iarec 4.00: SMN= 39.75: SMW=264.99: SKE= 010]
13187 [InterEventTime= 12.00]
13188 ROUTE: C00021..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13189 CONTINUES NASHDD 1.0 01:SW5 224.00 9.576 No.date 28:44 47.62 n/a 000
13190 [Cm 72.0 No 3.00: Tp= 7.52]
13191 [Iarec 4.00: SMN= 31.15: SMW=207.66: SKE= 010]
13192 [InterEventTime= 12.00]
13193 #
13194 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13195 # of 1.20
13196 ROUTE: C00022..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13197 CONTINUES NASHDD 1.0 01:FLC 4945.00 52.056 No.date 33:16 44.17 n/a 000
13198 [Cm 74.0 No 3.00: Tp= 4.45]
13199 [Iarec 4.00: SMN= 244.49: SKE= 010]
13200 [InterEventTime= 12.00]
13201 ROUTE: C00023..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13202 CONTINUES NASHDD 1.0 01:SW5A2 20.00 1.097 No.date 28:35 52.06 n/a 000
13203 [Cm 81.0 No 3.00: Tp= 6.21]
13204 [Iarec 4.00: SMN= 25.32: SMW=168.09: SKE= 010]
13205 [InterEventTime= 12.00]
13206 #
13207 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
13208 # of 1.61
13209 ROUTE: C00024..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13210 CONTINUES NASHDD 1.0 01:SW5A1 1412.00 10.184 No.date 37:44 45.88 n/a 000
13211 [Cm 72.0 No 3.00: Tp= 3.91]
13212 [Iarec 4.00: SMN= 35.81: SMW=225.43: SKE= 010]
13213 [InterEventTime= 12.00]
13214 ROUTE: C00025..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13215 CONTINUES NASHDD 1.0 01:SW4 585.00 14.953 No.date 29:55 52.06 n/a 000
13216 [Cm 81.0 No 3.00: Tp= 7.52]
13217 [Iarec 4.00: SMN= 25.21: SMW=168.09: SKE= 010]
13218 [InterEventTime= 12.00]
13219 ROUTE: C00026..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13220 CONTINUES NASHDD 1.0 01:SM CK 1021.00 19.782 No.date 30:45 51.16 n/a 000
13221 [Cm 80.0 No 3.00: Tp= 2.46]
13222 [Iarec 4.00: SMN= 26.32: SMW=175.50: SKE= 010]
13223 [InterEventTime= 12.00]
13224 ROUTE: C00027..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13225 CONTINUES NASHDD 1.0 01:SW2 177.00 7.567 No.date 28:44 47.62 n/a 000
13226 [Cm 77.0 No 3.00: Tp= 1.51]
13227 [Iarec 4.00: SMN= 31.15: SMW=207.66: SKE= 010]
13228 [InterEventTime= 12.00]
13229 ROUTE: C00028..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13230 CONTINUES NASHDD 1.0 01:SM CR 1122.00 52.056 No.date 31:42 52.06 n/a 000
13231 [Cm 81.0 No 3.00: Tp= 3.25]
13232 [Iarec 4.00: SMN= 25.21: SMW=168.09: SKE= 010]
13233 [InterEventTime= 12.00]
13234 ROUTE: C00029..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13235 CONTINUES NASHDD 1.0 01:SW3 2977.00 40.730 No.date 31:28 46.75 n/a 000
13236 [Cm 76.0 No 3.00: Tp= 3.01]
13237 [Iarec 4.00: SMN= 32.46: SMW=216.39: SKE= 010]
13238 [InterEventTime= 12.00]
13239 #
13240 # Routing hydrographs
13241 #
13242 # Starting with the addition of Jock River Headwater and Subwatershed 13
13243 #
13244 ROUTE: C00030..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13245 ADD HYD + 1.0 02:JR_HW 3680.00 21.616 No.date 36:52 35.18 n/a 000
13246 [Iarec 4.00: SMN= 13.71 971.00 8.203 No.date 32:53 32.84 n/a 000
13247 [Iarec 4.00: SMN= 13.71 4651.00 27.600 No.date 35:21 34.69 n/a 000
13248 #
13249 # Sum of hydrographs from Node 13 routed to Node 13A
13250 # (Approximated cross-section - see cross-section 258)
13251 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
13252 #
13253 ROUTE: C00031..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13254 ROUTE CHANNEL > 1.0 02:NS3 4651.00 22.598 No.date 38:56 34.69 n/a 000
13255 [RFE 1.00] out c. 1.0 01:NS3A 4651.00 22.598 No.date 38:56 34.69 n/a 000
13256 [L/S n= 5926. / 0.76 / 0.40]
13257 [Vms = 598. Dmax = 4.178]
13258 #
13259 # Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
13260 #
13261 ROUTE: C00032..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13262 ADD HYD + 1.0 02:NS3 4651.00 22.598 No.date 38:56 34.69 n/a 000
13263 [Iarec 4.00: SMN= 32.46: SMW=216.39: SKE= 010]
13264 SIM + 1.0 01:NS3A 7725.00 32.845 No.date 39:44 32.14 n/a 000
13265 #
13266 # Insertion of a reservoir to simulate the effects of the Goodwood Marsh
13267 #
13268 ROUTE: C00033..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13269 ROUTE RESERVOIR > 1.0 02:NS3A 7725.00 32.845 No.date 39:44 32.14 n/a 000
13270 [Iarec 4.00: SMN= 32.46: SMW=216.39: SKE= 010]
13271 [MSt oleda: 1.796E+03 m]
13272 #
13273 ROUTE: C00034..... Dfma 1-D NND..... AREHA-QPEAKm-TpeakDate hh:mm - Rvmm R.C. - Dfwm
13274 SAVE HYD 1.0 01:RES_GM 7725.00 3.950 No.date 62:26 32.14 n/a 000
13275 [Iarec 4.00: SMN= 32.46: SMW=216.39: SKE= 010]
13276 remark: Outflow from Res GM
13277 # Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12

Table with 4 columns: ID, Description, Date, Value. Rows include various engineering notes, station data, and subcatchment information.


```
157098 *** WARNING: New pipe size used for routing.
157108 R0100: C00370 ROUTE PIPE ->
157118 *** WARNING: New pipe size used for routing.
157128 R0050: C00378 ROUTE PIPE ->
157138 *** WARNING: New pipe size used for routing.
157148 R0050: C00379 ROUTE PIPE ->
157158 *** WARNING: New pipe size used for routing.
157168 R0100: C00303 ROUTE PIPE ->
157178 *** WARNING: New pipe size used for routing.
157188 R0100: C00309 ROUTE PIPE ->
157198 *** WARNING: New pipe size used for routing.
157208 R0100: C00325 ROUTE PIPE ->
157218 *** WARNING: New pipe size used for routing.
157228 R0100: C00326 ROUTE PIPE ->
157238 *** WARNING: New pipe size used for routing.
157248 R0100: C00334 ROUTE PIPE ->
157258 *** WARNING: New pipe size used for routing.
157268 R0100: C00342 IR VERT HYD ->
157278 *** NOTE: Inflow hyd. is dry and cannot be diverted.
157288 R0100: C00357 ROUTE PIPE ->
157298 *** WARNING: New pipe size used for routing.
157308 R0100: C00362 ROUTE PIPE ->
157318 *** WARNING: New pipe size used for routing.
157328 R0100: C00369 ROUTE PIPE ->
157338 *** WARNING: New pipe size used for routing.
157348 R0100: C00370 ROUTE PIPE ->
157358 *** WARNING: New pipe size used for routing.
157368 R0100: C00378 ROUTE PIPE ->
157378 *** WARNING: New pipe size used for routing.
157388 R0100: C00379 ROUTE PIPE ->
157398 *** WARNING: New pipe size used for routing.
157408 Simulation ended on 2021-03-04 at 11:53:36
157418 =====
157428
157438
```

Attachment D

Model 4A – Jock River Reach One Future Conditions – Without SWM controls

JFSA, 2021

SWMHYMO Input & Summary files

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1  20      Metric units / ID numbers OFF
2  *****
3  *# SWHYMO Ver: 5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *****
5  *# Project Name: [Jock River]      Project Number: [1474-16]
6  *# Date       : 04-03-2021
7  *# Modeller   : [MM]
8  *# Company    : JFSA Inc.
9  *# License #  : 2549237
10 *****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *# 2020-11-30 change TMSSTO in COMPUTE DUALHYD (TMSSTO = 0.1 instead of 0.0001)
16 *# 2020-12-01 correct pond curve values
17 *# 2020-12-01 change WCLAR_BRAZ_XIMP to 0.55, SLPI=[0.5](%) (impervious slope), and
    LGI up to 700m
18 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NHYDout=["N_TO"]
    ,NHYDin=["SN_TO"]) from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model
    will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELS
    aren't well suited to really flat slopes.
19 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 5002 (NHYDout=["N_WC"]
    ,NHYDin=["SN_CE"]) from 0.01 % (as per Stantec Report 2007) to 0.0255 % so the model
    will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELS
    aren't well suited to really flat slopes.
20 *
21 * Calibrated parameters for Summer 2003 data: APII=50, APIK=0.85, CN=varies,
22 *                                             SK=0.01, InterEventTime=12,
23 *                                             GWResk=0.96, VHydCond=0.055
24 *
25 *# -----
26 *
27 *START          TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
28 *              ["XAVG0315.STM"] average storm data a 15 minute time step
29 *              The above rainf file is an average of the JFSA gauge data
30 *              with the City of Ottawa rainfall data collected during
31 *              the same period.
32 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
33 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
34              ["C24SC002.stm"] <-- storm filename, one per line for NSTORM time
35 *%-----|-----|
36 *%-----|-----|
37 READ STORM     STORM_FILENAME=["storm 001"]
38 *%-----|-----|
39 MODIFY STORM   ICASEms=[1], NSHIFT=[96],
40              RedFACT=[1],
41 *%-----|-----|
42 DEFAULT VALUES ICASEdef=[1], read and print values
43              DEFVAL_FILENAME=["CitiGate.DEF"]
44 *%-----|-----|
45 COMPUTE API    APII=[50], APIK=[.85]/day
46 *%-----|-----|
47 *%-----|-----|
48 *#
49 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
50 *# of 1.32
51 *%-----|-----|
52 CONTINUOUS NASHYD NHYD=["JR_HW"], DT=[1]min, AREA=[3680](ha),
53              DWF=[0](cms), CNVC=[64], IA=[2.5](mm),
54              N=[3.0], TP=[7.13]hrs,
55              Continuous simulation parameters:
56              IARECper=[4](hrs),
57              SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
58              InterEventTime=[12](hrs)
59              Baseflow simulation parameters:

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60 BaseFlowOption=[ 1] ,
61 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
62 VHydCond=[ 0.055](mm/hr), END=- 1
63 *%-----|-----
64 *#
65 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
66 *# of 1.32
67 *%-----|-----
68 CONTINUOUS NASHYD NHYD=["SW_13"], DT=[ 1]min, AREA=[ 971](ha),
69 DWF=[ 0](cms), CNVC=[ 61], IA=[ 2.5](mm),
70 N=[ 3.0], TP=[ 3.76]hrs,
71 Continuous simulation parameters:
72 IaRECper=[ 4](hrs),
73 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
74 InterEventTime=[ 12](hrs)
75 Baseflow simulation parameters:
76 BaseFlowOption=[ 1] ,
77 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
78 VHydCond=[ 0.055](mm/hr), END=- 1
79 *%-----|-----
80 *#
81 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
82 *# of 1.80
83 *%-----|-----
84 CONTINUOUS NASHYD NHYD=["JR_GWM"], DT=[ 1]min, AREA=[ 3074](ha),
85 DWF=[ 0](cms), CNVC=[ 55], IA=[ 2.5](mm),
86 N=[ 3], TP=[ 11.33]hrs,
87 Continuous simulation parameters:
88 IaRECper=[ 4](hrs),
89 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
90 InterEventTime=[ 12](hrs)
91 Baseflow simulation parameters:
92 BaseFlowOption=[ 1] ,
93 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
94 VHydCond=[ 0.055](mm/hr), END=- 1
95 *%-----|-----
96 CONTINUOUS NASHYD NHYD=["JR_ASH"], DT=[ 1]min, AREA=[ 1781](ha),
97 DWF=[ 0](cms), CNVC=[ 72], IA=[ 2.5](mm),
98 N=[ 3.0], TP=[ 3.91]hrs,
99 Continuous simulation parameters:
100 IaRECper=[ 4](hrs),
101 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
102 InterEventTime=[ 12](hrs)
103 Baseflow simulation parameters:
104 BaseFlowOption=[ 1] ,
105 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
106 VHydCond=[ 0.055](mm/hr), END=- 1
107 *%-----|-----
108 CONTINUOUS NASHYD NHYD=["SW_11"], DT=[ 1]min, AREA=[ 500](ha),
109 DWF=[ 0](cms), CNVC=[ 66], IA=[ 2.5](mm),
110 N=[ 3.0], TP=[ 1.24]hrs,
111 Continuous simulation parameters:
112 IaRECper=[ 4](hrs),
113 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
114 InterEventTime=[ 12](hrs)
115 Baseflow simulation parameters:
116 BaseFlowOption=[ 1] ,
117 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
118 VHydCond=[ 0.055](mm/hr), END=- 1
119 *%-----|-----
120 *#
121 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
122 *# of 1.80
123 *%-----|-----
124 CONTINUOUS NASHYD NHYD=["NN_CK"], DT=[ 1]min, AREA=[ 1917](ha),
125 DWF=[ 0](cms), CNVC=[ 66], IA=[ 2.5](mm),

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126 N=[ 3. 0], TP=[ 5. 29] hrs,
127 Continuous simulation parameters:
128 IaRECPper=[ 4] (hrs),
129 SMN=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010]/ (mm),
130 InterEventTime=[ 12] (hrs)
131 Baseflow simulation parameters:
132 BaseFlowOption=[ 1] ,
133 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
134 VHydCond=[ 0. 055] (mm/ hr), END=- 1
135 *%-----|-----
136 *#
137 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
138 *# of 1.52
139 *%-----|-----
140 CONTINUOUS NASHYD NHYD=[ "SW_10"], DT=[ 1] min, AREA=[ 5666] (ha),
141 DWF=[ 0] (cms), CNVC=[ 72], IA=[ 2. 5] (mm),
142 N=[ 3. 0], TP=[ 8. 00] hrs,
143 Continuous simulation parameters:
144 IaRECPper=[ 4] (hrs),
145 SMN=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010]/ (mm),
146 InterEventTime=[ 12] (hrs)
147 Baseflow simulation parameters:
148 BaseFlowOption=[ 1] ,
149 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
150 VHydCond=[ 0. 055] (mm/ hr), END=- 1
151 *%-----|-----
152 *#
153 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
154 *# of 1.75
155 *%-----|-----
156 CONTINUOUS NASHYD NHYD=[ "KG_CK"], DT=[ 1] min, AREA=[ 8376] (ha),
157 DWF=[ 0] (cms), CNVC=[ 66], IA=[ 2. 5] (mm),
158 N=[ 3. 0], TP=[ 11. 66] hrs,
159 Continuous simulation parameters:
160 IaRECPper=[ 4] (hrs),
161 SMN=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010]/ (mm),
162 InterEventTime=[ 12] (hrs)
163 Baseflow simulation parameters:
164 BaseFlowOption=[ 1] ,
165 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
166 VHydCond=[ 0. 055] (mm/ hr), END=- 1
167 *%-----|-----
168 *#
169 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
170 *# of 1.68
171 *%-----|-----
172 CONTINUOUS NASHYD NHYD=[ "SW_9"], DT=[ 1] min, AREA=[ 1132] (ha),
173 DWF=[ 0] (cms), CNVC=[ 70], IA=[ 2. 5] (mm),
174 N=[ 3. 0], TP=[ 2. 51] hrs,
175 Continuous simulation parameters:
176 IaRECPper=[ 4] (hrs),
177 SMN=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010]/ (mm),
178 InterEventTime=[ 12] (hrs)
179 Baseflow simulation parameters:
180 BaseFlowOption=[ 1] ,
181 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
182 VHydCond=[ 0. 055] (mm/ hr), END=- 1
183 *%-----|-----
184 *#
185 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
186 *# of 1.82
187 *%-----|-----
188 CONTINUOUS NASHYD NHYD=[ "NC_CK"], DT=[ 1] min, AREA=[ 4464] (ha),
189 DWF=[ 0] (cms), CNVC=[ 62], IA=[ 2. 5] (mm),
190 N=[ 3. 0], TP=[ 11. 32] hrs,
191 Continuous simulation parameters:

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192 IaREcper=[ 4] (hr s),
193 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
194 InterEventTime=[ 12] (hr s)
195 Baseflow simulation parameters:
196 BaseFlowOption=[ 1] ,
197 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
198 VHydCond=[ 0. 055] (mm/ hr), END=- 1
199 *%-----|-----
200 *#
201 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
202 *# of 1.80
203 *%-----|-----
204 CONTINUOUS NASHYD NHYD=[ "SW_8" ], DT=[ 1] mi n, AREA=[ 131] (ha),
205 DWF=[ 0] (cms), CN C=[ 63], IA=[ 2. 5] (mm),
206 N=[ 3. 0], TP=[ 0. 90] hr s,
207 Continuous simulation parameters:
208 IaREcper=[ 4] (hr s),
209 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
210 InterEventTime=[ 12] (hr s)
211 Baseflow simulation parameters:
212 BaseFlowOption=[ 1] ,
213 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
214 VHydCond=[ 0. 055] (mm/ hr), END=- 1
215 *%-----|-----
216 *#
217 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
218 *# of 1.65
219 *%-----|-----
220 CONTINUOUS NASHYD NHYD=[ "HB_DR" ], DT=[ 1] mi n, AREA=[ 3854] (ha),
221 DWF=[ 0] (cms), CN C=[ 66], IA=[ 2. 5] (mm),
222 N=[ 3. 0], TP=[ 8. 42] hr s,
223 Continuous simulation parameters:
224 IaREcper=[ 4] (hr s),
225 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
226 InterEventTime=[ 12] (hr s)
227 Baseflow simulation parameters:
228 BaseFlowOption=[ 1] ,
229 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
230 VHydCond=[ 0. 055] (mm/ hr), END=- 1
231 *%-----|-----
232 *#
233 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
234 *# of 1.82
235 *%-----|-----
236 CONTINUOUS NASHYD NHYD=[ "SW_7" ], DT=[ 1] mi n, AREA=[ 3197] (ha),
237 DWF=[ 0] (cms), CN C=[ 57], IA=[ 2. 5] (mm),
238 N=[ 3. 0], TP=[ 6. 65] hr s,
239 Continuous simulation parameters:
240 IaREcper=[ 4] (hr s),
241 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
242 InterEventTime=[ 12] (hr s)
243 Baseflow simulation parameters:
244 BaseFlowOption=[ 1] ,
245 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
246 VHydCond=[ 0. 055] (mm/ hr), END=- 1
247 *%-----|-----
248 *#
249 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
250 *# of 1.75
251 *%-----|-----
252 CONTINUOUS NASHYD NHYD=[ "SW_6" ], DT=[ 1] mi n, AREA=[ 165] (ha),
253 DWF=[ 0] (cms), CN C=[ 67], IA=[ 2. 5] (mm),
254 N=[ 3. 0], TP=[ 4. 18] hr s,
255 Continuous simulation parameters:
256 IaREcper=[ 4] (hr s),
257 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),

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258 InterEventTime=[ 12] (hrs)
259 Baseflow simulation parameters:
260 BaseFlowOption=[ 1] ,
261 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
262 VHydCond=[ 0.055] (mm/ hr), END=- 1
263 *%-----|-----
264 *#
265 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
266 *# of 1.67
267 *%-----|-----
268 CONTINUOUS NASHYD NHYD=[ "VG_DR" ], DT=[ 1] min, AREA=[ 1332] (ha),
269 DWF=[ 0] (cms), CNVC=[ 72], IA=[ 2.5] (mm),
270 N=[ 3.0], TP=[ 5.95] hrs,
271 Continuous simulation parameters:
272 IaRECper=[ 4] (hrs),
273 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
274 InterEventTime=[ 12] (hrs)
275 Baseflow simulation parameters:
276 BaseFlowOption=[ 1] ,
277 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
278 VHydCond=[ 0.055] (mm/ hr), END=- 1
279 *%-----|-----
280 CONTINUOUS NASHYD NHYD=[ "SW_5" ], DT=[ 1] min, AREA=[ 224] (ha),
281 DWF=[ 0] (cms), CNVC=[ 77], IA=[ 2.5] (mm),
282 N=[ 3.0], TP=[ 0.75] hrs,
283 Continuous simulation parameters:
284 IaRECper=[ 4] (hrs),
285 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
286 InterEventTime=[ 12] (hrs)
287 Baseflow simulation parameters:
288 BaseFlowOption=[ 1] ,
289 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
290 VHydCond=[ 0.055] (mm/ hr), END=- 1
291 *%-----|-----
292 *#
293 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
294 *# of 1.20
295 *%-----|-----
296 CONTINUOUS NASHYD NHYD=[ "FL_CK" ], DT=[ 1] min, AREA=[ 4945] (ha),
297 DWF=[ 0] (cms), CNVC=[ 74], IA=[ 2.5] (mm),
298 N=[ 3.0], TP=[ 4.45] hrs,
299 Continuous simulation parameters:
300 IaRECper=[ 4] (hrs),
301 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
302 InterEventTime=[ 12] (hrs)
303 Baseflow simulation parameters:
304 BaseFlowOption=[ 1] ,
305 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
306 VHydCond=[ 0.055] (mm/ hr), END=- 1
307 *%-----|-----
308 CONTINUOUS NASHYD NHYD=[ "SW_5A2" ], DT=[ 1] min, AREA=[ 20] (ha),
309 DWF=[ 0] (cms), CNVC=[ 81], IA=[ 2.5] (mm),
310 N=[ 3.0], TP=[ 0.62] hrs,
311 Continuous simulation parameters:
312 IaRECper=[ 4] (hrs),
313 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
314 InterEventTime=[ 12] (hrs)
315 Baseflow simulation parameters:
316 BaseFlowOption=[ 1] ,
317 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
318 VHydCond=[ 0.055] (mm/ hr), END=- 1
319 *%-----|-----
320 *#
321 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
322 *# of 1.61
323 *%-----|-----

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324 CONTI NUOUS NASHYD NYHD=["SW_5A1"], DT=[1] mi n, AREA=[1412] (ha),
325 DWF=[0] (cms), CNV C=[75], IA=[2. 5] (mm),
326 N=[3. 0], TP=[8. 00] hr s,
327 Continuous simulation parameters:
328 IaRECper=[4] (hr s),
329 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
330 InterEventTime=[12] (hr s)
331 Baseflow simulation parameters:
332 BaseFlowOpti on=[1] ,
333 In it GWR es Vol = [50] (mm), GWR es K=[0. 96] (mm / day / mm)
334 VHydCond=[0. 055] (mm / hr), END=- 1

*%-----|

336 CONTI NUOUS NASHYD NYHD=["SW_4"], DT=[1] mi n, AREA=[585] (ha),
337 DWF=[0] (cms), CNV C=[81], IA=[2. 5] (mm),
338 N=[3. 0], TP=[1. 75] hr s,
339 Continuous simulation parameters:
340 IaRECper=[4] (hr s),
341 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
342 InterEventTime=[12] (hr s)
343 Baseflow simulation parameters:
344 BaseFlowOpti on=[1] ,
345 In it GWR es Vol = [50] (mm), GWR es K=[0. 96] (mm / day / mm)
346 VHydCond=[0. 055] (mm / hr), END=- 1

*%-----|

348 CONTI NUOUS NASHYD NYHD=["LM_CK"], DT=[1] mi n, AREA=[1021] (ha),
349 DWF=[0] (cms), CNV C=[80], IA=[2. 5] (mm),
350 N=[3. 0], TP=[2. 46] hr s,
351 Continuous simulation parameters:
352 IaRECper=[4] (hr s),
353 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
354 InterEventTime=[12] (hr s)
355 Baseflow simulation parameters:
356 BaseFlowOpti on=[1] ,
357 In it GWR es Vol = [50] (mm), GWR es K=[0. 96] (mm / day / mm)
358 VHydCond=[0. 055] (mm / hr), END=- 1

*%-----|

360 CONTI NUOUS NASHYD NYHD=["SW_2"], DT=[1] mi n, AREA=[177] (ha),
361 DWF=[0] (cms), CNV C=[77], IA=[2. 5] (mm),
362 N=[3. 0], TP=[0. 75] hr s,
363 Continuous simulation parameters:
364 IaRECper=[4] (hr s),
365 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
366 InterEventTime=[12] (hr s)
367 Baseflow simulation parameters:
368 BaseFlowOpti on=[1] ,
369 In it GWR es Vol = [50] (mm), GWR es K=[0. 96] (mm / day / mm)
370 VHydCond=[0. 055] (mm / hr), END=- 1

*%-----|

372 CONTI NUOUS NASHYD NYHD=["SM_DR"], DT=[1] mi n, AREA=[1122] (ha),
373 DWF=[0] (cms), CNV C=[81], IA=[2. 5] (mm),
374 N=[3. 0], TP=[3. 25] hr s,
375 Continuous simulation parameters:
376 IaRECper=[4] (hr s),
377 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
378 InterEventTime=[12] (hr s)
379 Baseflow simulation parameters:
380 BaseFlowOpti on=[1] ,
381 In it GWR es Vol = [50] (mm), GWR es K=[0. 96] (mm / day / mm)
382 VHydCond=[0. 055] (mm / hr), END=- 1

*%-----|

384 CONTI NUOUS NASHYD NYHD=["MO_DR"], DT=[1] mi n, AREA=[2737] (ha),
385 DWF=[0] (cms), CNV C=[76], IA=[2. 5] (mm),
386 N=[3. 0], TP=[3. 03] hr s,
387 Continuous simulation parameters:
388 IaRECper=[4] (hr s),
389 SMN=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),

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390 InterEventTime=[ 12]( hrs)
391 Baseflow simulation parameters:
392 BaseFlowOption=[ 1] ,
393 InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
394 VHydCond=[ 0.055]( mm/ hr) , END=- 1
395 *%-----|-----|
396 * -JFSA 2020 replaced SW_1 with a detailed model from Stantec Report 2007
397 *CONTINUOUS NASHYD NHYD=["SW_1"], DT=[ 1]mi n, AREA=[ 3176]( ha),
398 * DWF=[ 0]( cms) , CN/C=[ 78] , IA=[ 2.5]( mm),
399 * N=[ 3.0] , TP=[ 3.56]hrs,
400 * Continuous simulation parameters:
401 * IaRECper=[ 4]( hrs),
402 * SMN=[ -1]( mm) , SMAX=[ -1]( mm) , SK=[ 0.010]/( mm),
403 * InterEventTime=[ 12]( hrs)
404 * Baseflow simulation parameters:
405 * BaseFlowOption=[ 1] ,
406 * InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
407 * VHydCond=[ 0.055]( mm/ hr) , END=- 1
408 *%-----|-----|
409 *#
410 *# Routing hydrographs
411 *#
412 *# Starting with the addition of Jock River Headwater and Subwatershed 13
413 *#
414 ADD HYD NHYDsum=["S_N13"], NHYDs to add=["JR_HW"+"SW_13"]
415 *%-----|-----|
416 *#
417 *# Sum of hydrographs from Node 13 routed to Node 13A
418 *# (Approximated cross-section - see cross-section 258)
419 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
420 *#
421 ROUTE CHANNEL NHYDout=["N13A"] , NHYDin=["S_N13"],
422 RDT=[ 1]( mi n),
423 CHLGTH=[ 9074]( m) , CHSLOPE=[ 0.0220]( %),
424 FPSLOPE=[ 0.0220]( %),
425 SECNUM=[ 1.0] , NSEG=[ 1]
426 ( SEGROUGH, SEGDI ST ( m))=[ 0.04, 15.5] NSEG times
427 ( DI STANCE ( m) , ELEVATI ON ( m))=
428 [- 40, 132.5]
429 [- 30, 132]
430 [- 25, 131.5]
431 [- 13, 130]
432 [- 8, 127.00]
433 [- 7, 126.50]
434 [- 6, 126]
435 [- 5.5, 125.50]
436 [0, 123.75]
437 [4.5, 125.50]
438 [6, 126]
439 [7.5, 126.5]
440 [9, 127]
441 [10, 127.5]
442 [11.5, 128.0]
443 [15.5, 129.5]
444 *%-----|-----|
445 *#
446 *# Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
447 *#
448 ADD HYD NHYDsum=["SN13A"], NHYDs to add=["N13A"+"JR_GWM"]
449 *%-----|-----|
450 *#
451 *# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
452 *#
453 ROUTE RESERVOIR NHYDout=["RES_GM"] , NHYDin=["SN13A"],
454 RDT=[ 1]( mi n),
455 TABLE of ( OUTFLOW STORAGE ) values

```

```

456 (cms) - (ha-m)
457 [ 0.0 , 0.0 ]
458 [ 1.991, 2.144 ]
459 [ 2.693, 39.826 ]
460 [ 3.509, 81.697 ]
461 [ 4.578, 318.774 ]
462 [ 5.647, 594.947 ]
463 [ 7.109, 910.219 ]
464 [ 8.616, 1264.589 ]
465 [ 10.371, 1658.057 ]
466 [ 12.402, 2090.622 ]
467 [ 22.056, 3462.487 ]
468 [ -1 , -1 ] (max twenty pts)
469
470 NHYDovf=[" " ] ,
471 *%-----|-----|
472 *#
473 SAVE HYD NHYD=["RES_GM"], # OF PCYCLES=[-1], ICASEsh=[-1]
474 HYD_FILENAMES=["H_RESGM"]
475 HYD_COMMENT=["Out flow from Res GM"]
476 *%-----|-----|
477 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
478 *# (Approximated cross-section - see cross-section 258)
479 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
480 ROUTE CHANNEL NHYDout=["N12"], NHYDin=["RES_GM"],
481 RDT=[1] (min),
482 CHLGTH=[5926] (m), CHSLOPE=[0.0759] (%),
483 FPSLOPE=[0.0759] (%),
484 SECNUM=[1.0], NSEG=[1]
485 (SEGROUGH, SEGDIST (m))=[0.04, 15.5] NSEG times
486 (DISTANCE (m), ELEVATION (m))=
487 [-40, 132.5]
488 [-30, 132]
489 [-25, 131.5]
490 [-13, 130]
491 [-8, 127.00]
492 [-7, 126.50]
493 [-6, 126]
494 [-5.5, 125.50]
495 [0, 123.75]
496 [4.5, 125.50]
497 [6, 126]
498 [7.5, 126.5]
499 [9, 127]
500 [10, 127.5]
501 [11.5, 128.00]
502 [15.5, 129.5]
503 *%-----|-----|
504 *#
505 *# Addition of Subwatershed Jock River at Ashton to Node 12
506 *#
507 ADD HYD NHYDsum=["S_N12"], NHYDsto add=["N12"+"JR_ASH"]
508 SAVE HYD NHYD=["S_N12"], # OF PCYCLES=[-1], ICASEsh=[-1]
509 HYD_FILENAMES=["H_SN12"]
510 HYD_COMMENT=["flow at S_N12 near Ashton"]
511 *%-----|-----|
512 *#
513 *# Sum of hydrographs from Node 12 routed to Node 11
514 *# (Approximated cross-section - see cross-section 258)
515 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
516 *ROUTE CHANNEL NHYDout=["N11"], NHYDin=["S_N12"],
517 * RDT=[1] (min),
518 * CHLGTH=[972] (m), CHSLOPE=[0.0514] (%),
519 * FPSLOPE=[0.0514] (%),
520 * SECNUM=[1.0], NSEG=[1]
521 * (SEGROUGH, SEGDIST (m))=[0.04, 15.5] NSEG times
522 * (DISTANCE (m), ELEVATION (m))=

```

```

522 * [- 40, 132.5]
523 * [- 30, 132]
524 * [- 25, 131.5]
525 * [- 13, 130]
526 * [- 8, 127.00]
527 * [- 7, 126.50]
528 * [- 6, 126]
529 * [- 5.5, 125.50]
530 * [0, 123.75]
531 * [4.5, 125.50]
532 * [6, 126]
533 * [7.5, 126.5]
534 * [9, 127]
535 * [10, 127.5]
536 * [11.5, 128.00]
537 * [15.5, 129.5]
538 *%-----|-----
539 *#
540 *# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
541 *#
542 ROUTE CHANNEL NHYDout=["Duml1"], NHYDin=["S_N12"],
543 RDT=[1](min),
544 CHLGTH=[972](m), CHSLOPE=[0.054](%),
545 FPSLOPE=[0.054](%),
546 SECNUM=[1.0], NSEG=[1]
547 ( SEGROUGH, SEGDIST (m))=[0.04, 15.5] NSEG times
548 ( DISTANCE (m), ELEVATION (m))=
549 [- 40, 132.5]
550 [- 30, 132]
551 [- 25, 131.5]
552 [- 13, 130]
553 [- 8, 127.00]
554 [- 7, 126.50]
555 [- 6, 126]
556 [- 5.5, 125.50]
557 [0, 123.75]
558 [4.5, 125.50]
559 [6, 126]
560 [7.5, 126.5]
561 [9, 127]
562 [10, 127.5]
563 [11.5, 128.00]
564 [15.5, 129.5]
565 *%-----|-----
566 *#
567 *# Addition of Subwatershed 11 and No Name Creek to Node 11
568 *#
569 ADD HYD NHYDsum=["S_N11"], NHYDstoadd=["Duml1"+"SW_11"+"NN_CK"]
570 *%-----|-----
571 *#
572 *# Sum of hydrographs from Node 11 routed to Node 10
573 *# Section 1
574 *#
575 ROUTE CHANNEL NHYDout=["N10"], NHYDin=["S_N11"],
576 RDT=[1](min),
577 CHLGTH=[14028](m), CHSLOPE=[0.1568](%),
578 FPSLOPE=[0.1568](%),
579 SECNUM=[1.0], NSEG=[5]
580 ( SEGROUGH, SEGDIST (m))=
581 [0.04, -52.82
582 0.1, -6.47
583 -0.05, 6.47
584 0.1, 45.36
585 0.04, 423.88] NSEG times
586 ( DISTANCE (m), ELEVATION (m))=
587 [- 226.24 , 112.50]

```



```

588             [- 167.50 , 111.50]
589             [- 106.81 , 111.00]
590             [- 92.37 , 110.00]
591             [- 52.82 , 109.00]
592             [- 24.90, 109.00]
593             [- 17.02, 108.50]
594             [- 6.47, 108.00]
595             [ 6.47, 108.00]
596             [ 15.67, 108.50]
597             [ 18.95, 109.00]
598             [ 45.36, 109.50]
599             [ 120.79, 110.00]
600             [ 145.72, 111.00]
601             [ 181.56, 111.50]
602             [ 423.88, 112.50]
603 *%-----|-----
604 *#
605 *# Addition of Subwatershed 10 to Node 10
606 *#
607 ADD HYD          NHYDs um=["S_N10"], NHYDs to add=["N10"+"SW_10"]
608 *%-----|-----
609 SAVE HYD        NHYD=["S_N10"], # OF PCYCLES=[- 1], I CASEs h=[- 1]
610                 HYD_FILE NAME=["H_SN10"]
611                 HYD_COMMENT=["flow at S_N10: N10 + SW_10"]
612 *%-----|-----
613 *# Addition of Kings Creek to S_N10
614 *#
615 ADD HYD          NHYDs um=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
616 *%-----|-----
617 *#
618 *# Sum of hydrographs from Node 10 routed to Node 9
619 *# Section 2
620 *#
621 ROUTE CHANNEL    NHYDout=["N9"] , NHYDin=["S_N10A"] ,
622                 RDT=[ 1] ( mi n) ,
623                 CHLGTH=[ 3982] ( m) ,   CHSLOPE=[ 0.0753] ( % ) ,
624                                     FPSLOPE=[ 0.0753] ( % ) ,
625                 SECNUM=[ 1.0] ,       NSEG=[ 4]
626                 ( SEGROUGH, SEGDI ST ( m) ) =
627                 [ 0.04, - 30.27
628                 0.05, - 18.42
629                 - 0.05, 18.42
630                 0.04, 131.58] NSEG times
631                 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
632                 [- 446.74, 106.00]
633                 [- 415.68, 105.50]
634                 [- 285.40, 105.00]
635                 [- 173.77, 104.50]
636                 [- 144.95, 104.00]
637                 [- 111.18, 103.50]
638                 [- 94.06, 103.00]
639                 [- 71.02, 102.50]
640                 [- 30.27, 102.00]
641                 [- 19.33, 100.00]
642                 [- 18.42, 99.50]
643                 [ 18.42, 99.50]
644                 [ 20.77, 100.00]
645                 [ 27.93, 101.00]
646                 [ 52.29, 101.00]
647                 [ 68.80, 101.50]
648                 [ 79.66, 103.00]
649                 [ 91.50, 103.50]
650                 [ 131.58, 104.00]
651 *%-----|-----
652 *#
653 *# Addition of Subwatershed 9 and Nichols Creek to Node 9

```

```

654  *#
655  ADD HYD          NHYDs um=[ "S_N9" ], NHYDs  to  add=[ "N9"+"SW_9"+"NC_CK" ]
656  *%-----|-----|
657  *#
658  *# Sum of hydrographs from Node 9 routed to Node 8
659  *# Section 3
660  *#
661  ROUTE CHANNEL    NHYDout =[ "N8" ] , NHYDin =[ "S_N9" ] ,
662                  RDT=[ 1 ] ( mi n ) ,
663                  CHLGTH=[ 2269 ] ( m ) ,   CHSLOPE=[ 0.0882 ] ( % ) ,
664                                                    FPSLOPE=[ 0.0882 ] ( % ) ,
665                  SECNUM=[ 1.0 ] ,          NSEG=[ 3 ]
666                  ( SEGROUGH, SEGDI ST ( m ) ) =
667                    [ 0.1, -17.99
668                      -0.045, 17.31
669                      0.1, 456.58 ] NSEG t i m e s
670                  ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
671                    [ -201.19, 100.50 ]
672                    [ -135.21, 100.00 ]
673                    [ -94.83, 99.50 ]
674                    [ -67.05, 99.00 ]
675                    [ -17.99, 98.50 ]
676                    [ -16.02, 98.00 ]
677                    [ -13.95, 97.50 ]
678                    [ 13.95, 97.50 ]
679                    [ 15.64, 98.00 ]
680                    [ 17.31, 98.50 ]
681                    [ 162.02, 98.50 ]
682                    [ 172.89 , 99.00 ]
683                    [ 314.38, 99.00 ]
684                    [ 343.78, 99.50 ]
685                    [ 365.67, 100.00 ]
686                    [ 376.68, 100.00 ]
687                    [ 393.11, 99.50 ]
688                    [ 404.97, 99.50 ]
689                    [ 431.70, 100.00 ]
690                    [ 456.58, 100.50 ]
691  *%-----|-----|
692  *#
693  *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
694  *#
695  ADD HYD          NHYDs um=[ "S_N8" ], NHYDs  to  add=[ "N8"+"SW_8"+"HB_DR" ]
696  *%-----|-----|
697  *#
698  *# Sum of hydrographs from Node 8 routed to Node 7
699  *# Section 4
700  *#
701  ROUTE CHANNEL    NHYDout =[ "N7" ] , NHYDin =[ "S_N8" ] ,
702                  RDT=[ 1 ] ( mi n ) ,
703                  CHLGTH=[ 3750 ] ( m ) ,   CHSLOPE=[ 0.0533 ] ( % ) ,
704                                                    FPSLOPE=[ 0.0533 ] ( % ) ,
705                  SECNUM=[ 1.0 ] ,          NSEG=[ 3 ]
706                  ( SEGROUGH, SEGDI ST ( m ) ) =
707                    [ 0.12, -18.11
708                      -0.07, 17.22
709                      0.12, 590.05 ] NSEG t i m e s
710                  ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
711                    [ -433.21, 102.00 ]
712                    [ -425.34, 101.50 ]
713                    [ -377.56, 101.50 ]
714                    [ -366.23, 101.00 ]
715                    [ -202.60, 100.50 ]
716                    [ -96.25, 99.50 ]
717                    [ -68.36 99.00 ]
718                    [ -18.11, 98.50 ]
719                    [ -13.81, 97.50 ]

```

```

720 [ 13. 81, 97. 50]
721 [ 17. 22, 98. 50]
722 [ 161. 95, 98. 50]
723 [ 173. 11, 99. 00]
724 [ 314. 05, 99. 00]
725 [ 365. 52, 100. 00]
726 [ 404. 70, 99. 50]
727 [ 476. 74, 100. 50]
728 [ 502. 31, 101. 00]
729 [ 584. 69, 101. 00]
730 [ 585. 79, 101. 00]
731 [ 590. 05, 102. 00]
732 *%-----|-----
733 *#
734 *# Addition of Subwatershed 7 to Node 7
735 *#
736 ADD HYD          NHYDs um=[ "S_N7" ], NHYDs to add=[ "N7"+"SW_7" ]
737 *%-----|-----
738 SAVE HYD         NHYD=[ "S_N7" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ - 1 ]
739                 HYD_FI LENAME=[ "H_SN7" ]
740                 HYD_COMMENT=[ "flow at S_N7: N7 + SW_7" ]
741 *%-----|-----
742 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
743 *# Storage area and volumes were estimated from available topo maps.
744 *# Release rate from fen was assumed to be controlled by the downstream
745 *# river cross-section for summer conditions. It is was assumed that for up to
746 *# 0.75 m of water, the main channel of the river provided the storage. Above
747 *# this depth, the wetland starts to signigicantly store water.
748 *#
749 ROUTE RESERVOIR NHYDout =[ "RES_RF" ] , NHYDi n=[ "S_N7" ] ,
750                 RDT=[ 1 ] ( mi n ) ,
751                 TABLE of ( OUTFLOW STORAGE ) values
752                         ( cms ) - ( ha- m )
753                 TABLE of ( OUTFLOW STORAGE ) values
754                         ( cms ) - ( ha- m )
755                         [ 0. 0 , 0. 0 ]
756                         [ 0. 9051, 2. 40]
757                         [ 2. 907, 4. 13]
758                         [ 9. 744, 9. 18]
759                         [ 20. 304, 14. 96]
760                         [ 34. 167, 310. 21]
761                         [ 74. 993, 605. 46]
762                         [ 104. 876, 900. 71]
763                         [ 140. 56, 2892. 00]
764                         [ 225. 00, 3615. 63]
765                         [ - 1 , - 1 ] (max t wenty pts)
766                 NHYDovf=[ " " ] ,
767 *%-----|-----
768 SAVE HYD         NHYD=[ "RES_RF" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ - 1 ]
769                 HYD_FI LENAME=[ "H_Res RF" ]
770                 HYD_COMMENT=[ "out flow of Ri chmond Fen" ]
771 *%-----|-----
772 *#
773 *# Sum of hydrographs from Node 7 routed to Node 6
774 *# Section 5
775 *#
776 ROUTE CHANNEL   NHYDout =[ "N6" ] , NHYDi n=[ "RES_RF" ] ,
777                 RDT=[ 1 ] ( mi n ) ,
778                 CHLGTH=[ 3056 ] ( m ) , CHSLOPE=[ 0. 0818 ] ( % ) ,
779                 FPSLOPE=[ 0. 0818 ] ( % ) ,
780                 SECNUM=[ 1. 0 ] , NSEG=[ 5 ]
781                 ( SEGROUGH, SEGDI ST ( m ) ) =
782                 [ 0. 025, - 70. 8
783                 0. 1, - 23. 9
784                 - 0. 05, 23. 9
785                 0. 06, 39. 8

```

```

786           0. 05, 96. 3] NSEG times
787           ( DI STANCE ( m), ELEVATI ON ( m))=
788             [- 100. 8, 97. 00]
789             [- 70. 8, 96. 50]
790             [- 52. 0, 96. 00]
791             [- 35. 1, 95. 50]
792             [- 30. 6, 95. 00]
793             [- 23. 9, 94. 54]
794             [ 23. 9, 94. 54]
795             [ 39. 8, 95. 00]
796             [ 50. 4, 95. 50]
797             [ 93. 5, 96. 00]
798             [ 94. 9, 96. 50]
799             [ 96. 3, 97. 00]
800 *%-----|-----
801 *#
802 *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
803 *#
804 ADD HYD           NHYDs um=[ "S_N6" ], NHYDs to add=[ "N6"+"SW_6"+"VG_DR" ]
805 *%-----|-----
806 *#
807 *# Sum of hydrographs from Node 6 routed to Node 5
808 *# Section 6
809 *#
810 ROUTE CHANNEL     NHYDout =[ "N5" ] , NHYDin =[ "S_N6" ] ,
811                   RDT=[ 1] ( mi n),
812                   CHLGTH=[ 1852] ( m),   CHSLOPE=[ 0. 0540] ( %),
813                                           FPSLOPE=[ 0. 0540] ( %),
814                   SECNUM=[ 1. 0],       NSEG=[ 3]
815                   ( SEGROUGH, SEGDI ST ( m))=
816                     [ 0. 035, - 131. 59
817                     - 0. 045, 48. 96
818                     0. 1, 239. 04] NSEG times
819                   ( DI STANCE ( m), ELEVATI ON ( m))=
820                     [- 686. 30, 94. 50]
821                     [- 675. 70, 94. 00]
822                     [- 492. 52, 93. 00]
823                     [- 467. 28, 94. 00]
824                     [- 131. 59, 94. 00]
825                     [- 92. 79, 92. 50]
826                     [- 18. 06, 91. 00]
827                     [ 18. 06, 91. 00]
828                     [ 43. 47, 92. 50]
829                     [ 48. 96, 94. 00]
830                     [ 177. 43, 94. 00]
831                     [ 239. 04, 94. 50]
832 *%-----|-----
833 *#
834 *# Addition of Subwatershed 5 and Flowing Creek to Node 5
835 *#
836 ADD HYD           NHYDs um=[ "S_N5" ], NHYDs to add=[ "N5"+"SW_5"+"FL_CK" ]
837 *%-----|-----
838 *#
839 *# Sum of hydrographs from Node 5 routed to Node 5A
840 *# Section 7
841 *#
842 ROUTE CHANNEL     NHYDout =[ "N5A" ] , NHYDin =[ "S_N5" ] ,
843                   RDT=[ 1] ( mi n),
844                   CHLGTH=[ 556] ( m),   CHSLOPE=[ 0. 0900] ( %),
845                                           FPSLOPE=[ 0. 0900] ( %),
846                   SECNUM=[ 1. 0],       NSEG=[ 4]
847                   ( SEGROUGH, SEGDI ST ( m))=
848                     [ 0. 04, - 41. 5
849                     0. 1, - 14. 0
850                     - 0. 045, 14. 0
851                     0. 1, 41. 1] NSEG times

```

```

852 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
853 [- 275. 8, 93. 00]
854 [- 248. 6, 92. 50]
855 [- 237. 0, 92. 00]
856 [- 219. 3, 91. 50]
857 [- 202. 1, 91. 50]
858 [- 186. 0, 92. 00]
859 [- 129. 2, 92. 00]
860 [- 117. 6, 91. 50]
861 [- 100. 6, 91. 00]
862 [- 41. 5, 91. 00]
863 [- 20. 0, 91. 00]
864 [- 14. 0, 90. 54]
865 [ 14. 0, 90. 54]
866 [ 15. 3, 91. 00]
867 [ 17. 3, 91. 50]
868 [ 38. 4, 92. 00]
869 [ 39. 8, 92. 50]
870 [ 41. 1, 93. 00]
871 *%-----|-----|
872 *#
873 *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
874 *#
875 ADD HYD NHYDs um=[ "S_N5A" ] , NHYDs t o add=[ "N5A"+"SW_5A2"+"SW_5A1" ]
876 *%-----|-----|
877 *#
878 *# Sum of hydrographs from Node 5A routed to Node 4
879 *# Section 8
880 *#
881 ROUTE CHANNEL NHYDout =[ "N4" ] , NHYDi n=[ "S_N5A" ] ,
882 RDT=[ 1 ] ( mi n) ,
883 CHLGTH=[ 4630 ] ( m) , CHSLOPE=[ 0. 0432 ] ( % ) ,
884 FPSLOPE=[ 0. 0432 ] ( % ) ,
885 SECNUM=[ 1. 0 ] , NSEG=[ 3 ]
886 ( SEGROUGH, SEGDI ST ( m) ) =
887 [ 0. 05, -28. 2
888 -0. 035, 28. 2
889 0. 05, 173. 1 ] NSEG t i mes
890 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
891 [- 38. 9, 92. 00]
892 [- 35. 8, 91. 50]
893 [- 33. 3, 91. 00]
894 [- 28. 2, 90. 50]
895 [- 15. 0, 87. 48]
896 [- 5. 0, 88. 34]
897 [ 5. 0, 86. 20]
898 [ 15. 0, 88. 55]
899 [ 28. 2, 90. 50]
900 [ 29. 7, 91. 00]
901 [ 46. 5, 91. 00]
902 [ 127. 8, 91. 00]
903 [ 148. 7, 91. 50]
904 [ 173. 1, 92. 00]
905 *%-----|-----|
906 *#
907 *# Addition of Subwatershed 4 and Leamy Creek to Node 4
908 *#
909 ADD HYD NHYDs um=[ "S_N4" ] , NHYDs t o add=[ "N4"+"SW_4"+"LM_CK" ]
910 SAVE HYD NHYD=[ "S_N4" ] , # OF PCYCLES=[ - 1 ] , I CASEs h=[ 1 ]
911 HYD_COMMENT=[ "f l ow at S_N4" ]
912 *%-----|-----|
913 *#
914 *# Sum of hydrographs from Node 4 routed to Node 2
915 *# Section 9
916 *#
917 ROUTE CHANNEL NHYDout =[ "N2" ] , NHYDi n=[ "S_N4" ] ,

```

```

918 RDT=[ 1] ( mi n),
919 CHLGTH=[ 1667] ( m), CHSLOPE=[ 0. 0600] ( %),
920 FPSLOPE=[ 0. 0600] ( %),
921 SECNUM=[ 1. 0], NSEG=[ 4]
922 ( SEGROUGH, SEGDI ST ( m))=
923 [ 0. 1, - 28. 0
924 - 0. 04, 28. 4
925 0. 06, 31. 7
926 0. 04, 80. 2] NSEG times
927 ( DI STANCE ( m), ELEVATI ON ( m))=
928 [- 36. 3, 92. 00]
929 [- 32. 6, 91. 50]
930 [- 30. 2, 91. 00]
931 [- 28. 0, 90. 45]
932 [- 15. 0, 87. 48]
933 [- 5. 0, 88. 34]
934 [ 5. 0, 86. 20]
935 [ 15. 0, 88. 55]
936 [ 28. 0, 90. 45]
937 [ 28. 4, 90. 50]
938 [ 30. 4, 91. 00]
939 [ 31. 7, 91. 50]
940 [ 80. 2, 92. 00]
941 *%-----|-----|
942 *#
943 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
944 *#
945 ADD HYD NHYDs um=[ "S_N2" ], NHYDs to add=[ "N2"+"SW_2"+"SM_DR"+"MO_DR" ]
946 *%-----|-----|
947 SAVE HYD NHYD=[ "S_N2" ], # OF PCYCLES=[ - 1], ICASEs h=[ - 1]
948 HYD_FI LENAME=[ "H_SN2" ]
949 HYD_COMMENT=[ "flow at S_N2 Jock River Gauge at Modie Dr." ]
950 *%-----|-----|
951 *#
952 *# Sum of hydrographs from Node 2 routed to Node 1
953 *# Section 10
954 *#
955 *#*****
956 *%READ HYD NHYD=[ "S_N2" ],
957 *% HYD_FI LENAME=[ "H_S_N2" ]
958 *%-----|-----|
959 *#
960 *# Hydrograph from Node 2 routed to Node 416
961 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
962 *#
963 ROUTE CHANNEL NHYDout =["N_416" ] , NHYDin=["S_N2" ] ,
964 RDT=[ 1] ( mi n),
965 CHLGTH=[ 2327] ( m), CHSLOPE=[ 0. 0498] ( %),
966 FPSLOPE=[ 0. 0498] ( %),
967 SECNUM=[ 1. 0], NSEG=[ 3]
968 ( SEGROUGH, SEGDI ST ( m))=
969 [ 0. 075, - 23. 96
970 - 0. 055, 23. 96
971 0. 075, 157. 38] NSEG times
972 ( DI STANCE ( m), ELEVATI ON ( m))=
973 [- 336. 97, 93. 5]
974 [- 318. 85, 93]
975 [- 259, 92. 5]
976 [- 133. 18, 92]
977 [- 33. 17, 92]
978 [- 27. 21, 92]
979 [- 26. 14, 91. 5]
980 [- 24. 99, 91]
981 [- 23. 96, 90. 5]
982 [- 14. 33, 88. 26]
983 [- 0. 68, 88. 12]

```

```

984 [ 14. 33, 88. 26]
985 [ 23. 96, 90. 5]
986 [ 32. 12, 91]
987 [ 43. 74, 91. 5]
988 [ 57. 09, 92]
989 [ 73. 53, 92. 5]
990 [ 108. 27, 93]
991 [ 125. 88, 93. 5]
992 [ 144. 81, 94]
993 [ 157. 38, 94. 5]
994 *%-----|-----|
995 *#*****|*****|
996 *# Catchment SW1a
997 *# - Portion of RVCA catchment SW1 outside of Reach 1 subwatershed
998 *# - Undeveloped agricultural land
999 *#*****|*****|
1000 CONTINUOUS NASHYD NHYD=[ "SW_1a" ], DT=[ 1] (mi n), AREA=[ 536. 42] ( ha),
1001 DWF=[ 0] ( cms), CN C=[ 72], I A=[ 4. 67] ( mm),
1002 N=[ 3], TP=[ 2. 79] hr s,
1003 Continuous simulation parameters:
1004 I aRE Cper=[ 4] ( hr s),
1005 SM N=[ - 1] ( mm), SMAX=[ - 1] ( mm), SK=[ 0. 010] / ( mm),
1006 I nter Event Ti me=[ 12] ( hr s)
1007 Baseflow simulation parameters:
1008 BaseFl owOpt ion=[ 1] ,
1009 I n i t GWRes Vol =[ 50] ( mm), GWRes K=[ 0. 96] ( mm/ day/ mm)
1010 VHydCond=[ 0. 055] ( mm/ hr), END=- 1
1011 *%-----|-----|
1012 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
1013 CONTINUOUS STANDHYD NHYD=[ "S-1-Okeefe" ], DT=[ 1] ( mi n), AREA=[ 44. 93] ( ha), XI MP=[ 0. 65],
TI MP=[ 0. 65], DWF=[ 0] ( cms),
1014 LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
I Aper=[ 4. 67] ( mm), SLPP=[ 2. 0] ( %),
1015 LGP=[ 40] ( m), MNP=[ 0. 25], SCP=[ 0] ( mi n), Impervious surfaces:
I Ai mp=[ 1. 57] ( mm), SLPI=[ 0. 75] ( %),
1016 LGI=[ 547. 296] ( m), MNI=[ 0. 013], SCI=[ 0] ( mi n),
1017 Continuous simulation parameters:
1018 I aRE Cper=[ 4] ( hr s), I aRE Ci mp=[ 4] ( hr s),
1019 SM N=[ - 1] ( mm), SMAX=[ - 1] ( mm), SK=[ 0. 010] / ( mm),
1020 I nter Event Ti me=[ 12] ( hr s), END=- 1
1021 *%-----|-----|
1022 *COMPUTE DUALHYD NHYDi n=[ "S-1-Okeefe" ], CI NLET=[ 4. 796] ( cms), NI NLET=[ 1],
1023 * M i j NHYD=[ "S-1-OkM" ]
1024 * M nNHYD=[ "S-1-OkMN" ]
1025 * TM I STO=[ 9999999] ( cu- m)
1026 *%-----|-----|
1027 *ADD HYD NHYDs um=[ "S-1-OkS" ], NHYDs t o add=[ "S-1-OkM" +"S-1-OkMN" ]
1028 *%-----|-----|
1029 *ROUTE RESERVOIR NHYDout=[ "S-1-OkSR" ], NHYDi n=[ "S-1-OkS" ],
1030 * RDT=[ 1] ( mi n),
1031 * TABLE of ( OUTFLOW STORAGE ) values
1032 * ( cms) - ( ha- m)
1033 * [ 0. 0 , 0. 0 ]
1034 * [ 0. 5370, 1. 7917 ]
1035 * [ - 1 , - 1 ] ( max twenty pts)
1036 * NHYDovf=[ "S-1-OkSovf" ]
1037 *%-----|-----|
1038 ADD HYD NHYDs um=[ "SN_416" ], NHYDs t o add=[ "N_416"+"SW_1a"+"S-1-Okeefe" ]
1039 *%-----|-----|
1040 SAVE HYD NHYD=[ "SN_416" ], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
1041 HYD_COMMENT=[ "Tot al Fl ows at Hi ghway 416" ]
1042 *%-----|-----|
1043 *#
1044 *# Hydrograph from Node 416 routed to Node at Okeefe drain
1045 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245

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1046 *#
1047 ROUTE CHANNEL NHYDout =["N_OK" ] , NHYDin=["SN_416" ] ,
1048 RDT=[ 1 ] ( mi n),
1049 CHLGTH=[ 497 ] ( m), CHSLOPE=[ 0. 3018 ] ( %),
1050 FPSLOPE=[ 0. 3018 ] ( %),
1051 SECNUM=[ 1. 0 ], NSEG=[ 3]
1052 ( SEGROUGH, SEGDI ST ( m) )=
1053 [ 0. 075, - 19. 40
1054 - 0. 055, 19. 40
1055 0. 075, 377. 02] NSEG t i m e s
1056 ( DI STANCE ( m), ELEVATI ON ( m) )=
1057 [- 1061. 41, 92. 50]
1058 [- 945. 91, 92. 00]
1059 [- 783. 64, 91. 50]
1060 [- 136. 74, 91. 00]
1061 [- 86. 04, 91. 00]
1062 [- 20. 86, 91. 00]
1063 [- 20. 18, 90. 50]
1064 [- 19. 40, 90. 00]
1065 [- 11. 68, 86. 89]
1066 [ 0. 00, 86. 10]
1067 [ 12. 09, 86. 81]
1068 [ 19. 40, 90. 00]
1069 [ 34. 68, 90. 50]
1070 [ 60. 56, 91. 00]
1071 [ 170. 14, 91. 00]
1072 [ 175. 05, 90. 50]
1073 [ 180. 29, 90. 00]
1074 [ 193. 41, 90. 00]
1075 [ 195. 98, 90. 50]
1076 [ 377. 02, 92. 50]
1077 *%-----|-----|
1078 *#*****|*****|
1079 *# Catchment OKEEFE
1080 *# - To O'Keefe drain (north of the Jock)
1081 *# - Developed with assumed 43% imp.
1082 *# - 2020-12-01 add Okeefe model (Area 513.02 HA) instead of current Okeefe (Area
1083 513.02 HA)
1084 *# - 2020-11-20 Okeefe detailed model was added as per the NOVATECH SWWHYMD model
1085 (Citi-Gate 2014).
1086 *%-----|-----|
1087 *#*****|*****|
1088 CONTINUOUS NASHYD NHYD=["O-1R"], DT=[ 1 ] mi n, AREA=[ 63. 72 ] ( ha),
1089 DWF=[ 0 ] ( cms), CNVC=[ 61 ], IA=[ 6. 2 ] ( mm), N=[ 3 ], TP=[ . 9 ] hr s,
1090 Continuous simulation parameters:
1091 IaRECper=[ 4 ] ( hr s),
1092 SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0. 010 ] / ( mm),
1093 InterEventTime=[ 12 ] ( hr s)
1094 Baseflow simulation parameters:
1095 BaseFlowOption=[ 1 ],
1096 InitGWResVol=[ 50 ] ( mm), GWResK=[ 0. 96 ] ( mm/ day/ mm)
1097 VHydCond=[ 0. 055 ] ( mm/ hr), END=- 1
1098 *%-----|-----|
1099 *ROUTE FLOW THROUGH AREA 0-2
1100 ROUTE CHANNEL NHYDout =["O-1R"], NHYDin=["O-1"], RDT=[ 1 ] ( mi n),
1101 CHLGTH=[ 960 ] ( m), CHSLOPE=[ 0. 63 ] ( %), FPSLOPE=[ 0. 63 ] ( %),
1102 SECNUM=[ 1 ], NSEG=[ 3]
1103 ( SEGROUGH, SEGDI ST ( m) )=[ 0. 06, 4 -. 043, 6 0. 06, 10] NSEG t i m e s
1104 ( DI STANCE ( m), ELEVATI ON ( m) )=[ 0. 00, 2. 0]
1105 [ 0. 0, 2. 0]
1106 [ 4. 0, 0. 0]
1107 [ 6. 0, 0. 0]
1108 [ 10. 0, 2. 0]

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1109 *%-----|-----|
1110 CONTINUOUS NASHYD NHYD=["O-2"], DT=[1] min, AREA=[28.61] (ha),
1111 DWF=[0] (cms), CN C=[57], IA=[5.2] (mm), N=[3], TP=[1.1] hrs,
1112 Continuous simulation parameters:
1113 IaRECper=[4] (hrs),
1114 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1115 InterEventTime=[12] (hrs)
1116 Baseflow simulation parameters:
1117 BaseFlowOption=[1],
1118 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
1119 VHydCond=[0.055] (mm/hr), END=-1
1120 *%-----|-----|
1121 CONTINUOUS NASHYD NHYD=["O-4"], DT=[1] min, AREA=[46.94] (ha),
1122 DWF=[0] (cms), CN C=[49], IA=[9.2] (mm), N=[3], TP=[0.9] hrs,
1123 Continuous simulation parameters:
1124 IaRECper=[4] (hrs),
1125 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1126 InterEventTime=[12] (hrs)
1127 Baseflow simulation parameters:
1128 BaseFlowOption=[1],
1129 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
1130 VHydCond=[0.055] (mm/hr), END=-1
1131 *%-----|-----|
1132 *TOTAL EXTERNAL FLOW NORTH OF O'KEEFE CT. CROSSING
1133 ADD HYD NHYDsum=["OKF-N"], NHYDstoadd=["O-1R"+"O-2"+"O-4"]
1134 *%-----|-----|
1135 *ROUTE FLOW THROUGH AREA O-6
1136 ROUTE CHANNEL ROUTE CHANNEL NHYDout=["OKF-NR"], NHYDin=["OKF-N"], RDT=[1] (min),
1137 CHLGTH=[210] (m), CHSLOPE=[.81] (%), FPSLOPE=[.81] (%),
1138 SECNUM=[1], NSEG=[3]
1139 (SEGROUGH, SEGDIST (m))=[0.043, 22.43 - 0.043, 25.07
1140 0.043, 45.54] NSEG times
1141 (DISTANCE (m), ELEVATION (m))=[0.00, 3.73]
1142 (14.62, 1.56)
1143 (18.41, 1.44)
1144 (22.43, 0.00)
1145 (25.07, 0.70)
1146 (29.10, 1.79)
1147 (33.73, 2.71)
1148 (45.54, 3.58)
1149 *%-----|-----|
1150 CONTINUOUS NASHYD NHYD=["O-6"], DT=[1] min, AREA=[16.46] (ha),
1151 DWF=[0] (cms), CN C=[43], IA=[9.2] (mm), N=[3], TP=[0.7] hrs,
1152 Continuous simulation parameters:
1153 IaRECper=[4] (hrs),
1154 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1155 InterEventTime=[12] (hrs)
1156 Baseflow simulation parameters:
1157 BaseFlowOption=[1],
1158 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
1159 VHydCond=[0.055] (mm/hr), END=-1
1160 *%-----|-----|
1161 CONTINUOUS STANDHYD NHYD=["O-3"], DT=[1] (min), AREA=[39.67] (ha), XI MP=[0.15],
1162 T I MP=[0.30], DWF=[0] (cms),
1163 LOSS=[2], SCS curve number CN=[50], Pervious surfaces:
1164 IAper=[4.67] (mm), SLPP=[0.32] (%),
1165 LGP=[440] (m), MNP=[0.035], SCP=[0] (min), Impervious surfaces:
1166 IAimp=[1.57] (mm), SLPI=[0.32] (%),
1167 LGI=[1880] (m), MNI=[0.013], SCI=[0] (min),
1168 Continuous simulation parameters:
1169 IaRECper=[4] (hrs), IaRECimp=[4] (hrs),
1170 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1171 InterEventTime=[12] (hrs), END=-1
1172 *%-----|-----|
1173 CONTINUOUS STANDHYD NHYD=["O-5"], DT=[1] (min), AREA=[60.63] (ha), XI MP=[0.13],
1174 T I MP=[0.26], DWF=[0] (cms),

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1171      LOSS=[ 2], SCS curve number CN=[ 61],
1172      Pervious surfaces: I Aper=[ 4.67] (mm), SLPP=[ 1.38] (%),
1173      LGP=[ 550] (m), MNP=[ 0.035], SCP=[ 0] (min), Impervious surfaces:
      I Aimp=[ 1.57] (mm), SLPI=[ 1.38] (%),
1174      LGI=[ 1450] (m), MNI=[ 0.013], SCI=[ 0] (min),
1175      Continuous simulation parameters:
1176      I aRECper=[ 4] (hrs), I aRECimp=[ 4] (hrs),
1177      SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1178      Inter Event Time=[ 12] (hrs), END=- 1
1179      *%-----|-----|
1180      *TOTAL EXTERNAL FLOWS WEST OF THE SITE AND NORTH OF O'KEEFE CRT
1181      *%-----|-----|
1182      ADD HYD      NHYDsum=[ "PT1"], NHYDs to add=[ "OKF-NR"+"O-3"+"O-5"+"O-6"]
1183      *%-----|-----|
1184      CONTINUOUS NASHYD      NHYD=[ "O-7"], DT=[ 1] min, AREA=[ 5.28] (ha),
1185      DWF=[ 0] (cms), CNC=[ 54], IA=[ 7.5] (mm), N=[ 3], TP=[ 0.6] hrs,
1186      Continuous simulation parameters:
1187      I aRECper=[ 4] (hrs),
1188      SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1189      Inter Event Time=[ 12] (hrs)
1190      Baseflow simulation parameters:
1191      BaseFlowOption=[ 1] ,
1192      Init GWRes Vol=[ 50] (mm), GWRes K=[ 0.96] (mm3/day/mm)
1193      VHydCond=[ 0.055] (mm/hr), END=- 1
1194      *%-----|-----|
1195      *ANALYSIS POINT 1 - TOTAL FLOW NORTH OF FALLOWFIELD DR. AND O'KEEFE CRT.
1196      ADD HYD      NHYDsum=[ "FF"], NHYDs to add=[ "PT1"+"O-7"]
1197      *%-----|-----|
1198      *ROUTE FLOW through O'Keefe Drain 1
1199      ROUTE CHANNEL      NHYDout=[ "DRAIN1"], NHYDin=[ "FF"], RDT=[ 1] (min),
1200      CHLGTH=[ 302] (m), CHSLOPE=[ 1.00] (%), FPSLOPE=[ 1.00] (%),
1201      SECNUM=[ 1], NSEG=[ 3]
1202      ( SEGROUGH, SEGDIST (m))=[ 0.07, 13.45 -0.043, 16.55 0.07, 30.00] NSEG
      times
1203      ( DISTANCE (m), ELEVATION (m))=[ 0.00, 1.70]
1204      ( 3.45, 0.60)
1205      ( 13.45, 0.50)
1206      ( 14.45, 0.00)
1207      ( 15.55, 0.00)
1208      ( 16.55, 0.50)
1209      ( 26.55, 0.60)
1210      ( 30.00, 1.70)
1211      *%-----|-----|
1212      CONTINUOUS NASHYD      NHYD=[ "DI"], DT=[ 1] min, AREA=[ 1.17] (ha),
1213      DWF=[ 0] (cms), CNC=[ 84], IA=[ 9.0] (mm), N=[ 3], TP=[ 0.28] hrs,
1214      Continuous simulation parameters:
1215      I aRECper=[ 4] (hrs),
1216      SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1217      Inter Event Time=[ 12] (hrs)
1218      Baseflow simulation parameters:
1219      BaseFlowOption=[ 1] ,
1220      Init GWRes Vol=[ 50] (mm), GWRes K=[ 0.96] (mm3/day/mm)
1221      VHydCond=[ 0.055] (mm/hr), END=- 1
1222      *%-----|-----|
1223      CONTINUOUS STANDHYD      NHYD=[ "AI"], DT=[ 1] min, AREA=[ 2.50] (ha), XI MP=[ 0.68], TI MP=[ 0.85],
      DWF=[ 0] (cms), LOSS=[ 1]:
1224      Horton: Fo=[ 76.20] (mm/hr), Fc=[ 13.20] (mm/hr), DCAY=[ 4.14] (/hr),
      F=[ 0.00] (mm),
1225      Pervious areas: I Aper=[ 4.67] (mm), SLPP=[ 0.5] (%), LGP=[ 50] (m),
      MNP=[ 0.250], SCP=[ 0] (min),
1226      Impervious areas: I Aimp=[ 1.57] (mm), SLPI=[ 0.5] (%),
      LGI=[ 223.607] (m), MNI=[ 0.013], SCI=[ 0] (min),
1227      Continuous simulation parameters:
1228      I aRECper=[ 4] (hrs), I aRECimp=[ 4] (hrs), Inter Event Time=[ 12] (hrs),
      END=- 1
1229      *%-----|-----|

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1230 ROUTE RESERVOIR NHYDout=["A1-STR"], NHYDin=["A1"], RDT=[1](min),
1231 TABLE of (OUTFLOW STORAGE) values
1232 (cms) - (ha-m)
1233 [0.000 , 0.000]
1234 [0.035 , 0.038]
1235 [0.072 , 0.051]
1236 [0.100 , 0.059]
1237 [0.125 , 0.070]
1238 [0.160 , 0.074]
1239 [0.185 , 0.081]
1240 [-1 , -1] (max twenty pts)
1241 NHYDovf=["A1-OVF"]
1242 *%-----|-----|
1243 CONTINUOUS STANDHYD NHYD=["ST-2"], DT=[1]min, AREA=[0.59](ha), XI MP=[0.46],
1244 TIMP=[0.57], DWF=[0](cms), LOSS=[1]:
1245 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1246 F=[0.00](mm),
1247 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1248 MNP=[0.250], SCP=[0](min),
1249 ImperVIOUS areas: IAmp=[1.57](mm), SLPI=[0.5](%),
1250 LGI=[108.628](m), MNI=[0.013], SCI=[0](min),
1251 Continuous simulation parameters:
1252 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
1253 END=-1
1254 *%-----|-----|
1255 ROUTE RESERVOIR NHYDout=["ST2STR"], NHYDin=["ST-2"], RDT=[1](min),
1256 TABLE of (OUTFLOW STORAGE) values
1257 (cms) - (ha-m)
1258 [0.000 , 0.0000]
1259 [0.052 , 0.0010]
1260 [0.053 , 0.0080]
1261 [-1 , -1] (max twenty pts)
1262 NHYDovf=["ST2OVF"]
1263 *%-----|-----|
1264 *%-----|-----|
1265 *TOTAL FLOW NORTH OF STRANDHERD DR. (EAST BRANCH) CROSSING
1266 *%-----|-----|
1267 CONTINUOUS NASHYD NHYD=["O-8"], DT=[1]min, AREA=[60.55](ha),
1268 DWF=[0](cms), CNVC=[69], IA=[4.0](mm), N=[3], TP=[1.0]hrs,
1269 Continuous simulation parameters:
1270 IaRECper=[4](hrs),
1271 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1272 InterEventTime=[12](hrs)
1273 Baseflow simulation parameters:
1274 BaseFlowOption=[1],
1275 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1276 VHydCond=[0.055](mm/hr), END=-1
1277 *%-----|-----|
1278 ROUTE PIPE PTYPE=[2]rect, NHYDout=["O8PIPE"], RNUMBER=[1], PWDTH=[1800](mm),
1279 PHEIGHT=[1200](mm), PLNGTH=[335.1](m),
1280 PROUGH=[0.013], PSLOPE=[0.001](m/m), NHYDin=["O-8"], RDT=[1](min)
1281 *%-----|-----|
1282 *%-----|-----|
1283 ADD HYD NHYDsum=["ST2-IN"], NHYDsto
1284 add=["DRAINI"+"DI"+"A1-STR"+"A1-OVF"+"ST2STR"+"ST2OVF"+"O8PIPE"]
1285 *%-----|-----|
1286 CONTINUOUS STANDHYD NHYD=["A7"], DT=[1]min, AREA=[3.51](ha), XI MP=[0.68], TIMP=[0.85],
1287 DWF=[0](cms), LOSS=[1]:
1288 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
1289 F=[0.00](mm),
1290 Pervious areas: IAper=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
1291 MNP=[0.250], SCP=[0](min),
1292 ImperVIOUS areas: IAmp=[1.57](mm), SLPI=[0.5](%),
1293 LGI=[264.953](m), MNI=[0.013], SCI=[0](min),
1294 Continuous simulation parameters:
1295 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),

END=- 1

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1285 *%-----|-----|
1286 ROUTE RESERVOIR NHYDout=["A7-STR"], NHYDin=["A7"], RDT=[1](min),
1287 TABLE of ( OUTFLOW STORAGE ) values
1288 (cms) - (ha-m)
1289 [ 0.000 , 0.000 ]
1290 [ 0.049 , 0.054 ]
1291 [ 0.102 , 0.072 ]
1292 [ 0.140 , 0.082 ]
1293 [ 0.175 , 0.099 ]
1294 [ 0.225 , 0.105 ]
1295 [ 0.260 , 0.114 ]
1296 [ -1 , -1 ] (max twenty pts)
1297 NHYDovf=["A7-OVF"]
1298 *%-----|-----|
1299 CONTINUOUS STANDHYD NHYD=["ST-3"], DT=[1]min, AREA=[0.71](ha), XI MP=[0.46],
TI MP=[0.57], DWF=[0](cms), LOSS=[1]:
1300 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1301 Pervious areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1302 Impervious areas: I A imp=[1.57](mm), SLPI=[0.5](%),
LGI=[119.164](m), MNI=[0.013], SCI=[0](min),
1303 Continuous simulation parameters:
1304 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=- 1
1305 *%-----|-----|
1306 ROUTE RESERVOIR NHYDout=["ST3STR"], NHYDin=["ST-3"], RDT=[1](min),
1307 TABLE of ( OUTFLOW STORAGE ) values
1308 (cms) - (ha-m)
1309 [ 0.000 , 0.0000 ]
1310 [ 0.063 , 0.0010 ]
1311 [ 0.064 , 0.0094 ]
1312 [ -1 , -1 ] (max twenty pts)
1313 NHYDovf=["ST3OVF"]
1314 *%-----|-----|
1315 *ANALYSIS POINT 2 - TOTAL FLOW AT OUTLET OF STREET 2/3 INTERSECTION
1316 *%-----|-----|
1317 ADD HYD NHYDs um=["PT2ST3"], NHYDs t o
add=["ST2-IN"+"A7-STR"+"A7-OVF"+"ST3STR"+"ST3OVF"]
1318 *%-----|-----|
1319 *ROUTE FLOW through O Keefe Drain 2
1320 ROUTE CHANNEL NHYDout=["DRAIN2"], NHYDin=["PT2ST3"], RDT=[1](min),
1321 CHLGTH=[592](m), CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1322 SECNUM=[1], NSEG=[3]
1323 ( SEGROUGH, SEGDI ST (m))=[0.07, 12.60 -0.043, 17.40 0.07, 30.00] NSEG
t i m e s
1324 ( DI STANCE (m), ELEVATI ON (m))=[0.00, 1.70]
1325 (2.60, 0.95)
1326 (12.60, 0.75)
1327 (14.10, 0.00)
1328 (15.90, 0.00)
1329 (17.40, 0.75)
1330 (27.40, 0.95)
1331 (30.00, 1.70)
1332 *%-----|-----|
1333 CONTINUOUS NASHYD NHYD=["D2"], DT=[1]min, AREA=[2.28](ha), DWF=[0](cms), CNV C=[84],
I A=[9.0](mm),
1334 N=[3], TP=[0.99]hrs,
1335 Continuous simulation parameters:
1336 I a REC per=[4](hrs),
1337 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1338 I n t e r E v e n t T i m e=[12](hrs)
1339 Baseflow simulation parameters:
1340 BaseFl owOpt i on=[1],
1341 I n i t G W R e s V o l =[50](mm), G W R e s K=[0.96](mm/day/mm)
```

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1342          VHydCond=[ 0. 055] ( mm/ hr ) ,      END=- 1
1343 *%-----|-----|
1344 CONTINUOUS STANDHYD NHYD=[ " A17" ] , DT=[ 1] mi n , AREA=[ 12. 04] ( ha) , XI MP=[ 0. 68] ,
TI MP=[ 0. 85] , DWF=[ 0] ( cms) , LOSS=[ 1] :
1345 Horton: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] ( / hr) ,
F=[ 0. 00] ( mm) ,
1346 Pervious areas: I A per =[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1347 ImperVIOUS areas: I A i mp=[ 1. 57] ( mm) , SLPI =[ 0. 5] ( % ) ,
LGI =[ 490. 714] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1348 Continuous simulation parameters:
1349 I a REC per =[ 4] ( hr s) , I a REC i mp=[ 4] ( hr s) , I n t e r E v e n t T i m e =[ 12] ( hr s) ,
END=- 1
1350 *%-----|-----|
1351 ROUTE RESERVOIR NHYDout =[ " A17STR" ] , NHYD i n=[ " A17" ] , RDT=[ 1] ( mi n) ,
1352 TABLE of ( OUTFLOW STORAGE ) values
1353 ( cms) - ( ha- m)
1354 [ 0. 000 , 0. 000 ]
1355 [ 0. 169 , 0. 185 ]
1356 [ 0. 349 , 0. 248 ]
1357 [ 0. 482 , 0. 283 ]
1358 [ 0. 602 , 0. 338 ]
1359 [ 0. 771 , 0. 359 ]
1360 [ 0. 891 , 0. 391 ]
1361 [ - 1 , - 1 ] ( m a x t w e n t y p t s)
1362 NHYDovf =[ " A17OVF" ]
1363 *%-----|-----|
1364 CONTINUOUS STANDHYD NHYD=[ " ST- 4" ] , DT=[ 1] mi n , AREA=[ 0. 35] ( ha) , XI MP=[ 0. 46] ,
TI MP=[ 0. 57] , DWF=[ 0] ( cms) , LOSS=[ 1] :
1365 Horton: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] ( / hr) ,
F=[ 0. 00] ( mm) ,
1366 Pervious areas: I A per =[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1367 ImperVIOUS areas: I A i mp=[ 1. 57] ( mm) , SLPI =[ 0. 5] ( % ) , LGI =[ 83. 666] ( m) ,
MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1368 Continuous simulation parameters:
1369 I a REC per =[ 4] ( hr s) , I a REC i mp=[ 4] ( hr s) , I n t e r E v e n t T i m e =[ 12] ( hr s) ,
END=- 1
1370 *%-----|-----|
1371 ROUTE RESERVOIR NHYDout =[ " ST4STR" ] , NHYD i n=[ " ST- 4" ] , RDT=[ 1] ( mi n) ,
1372 TABLE of ( OUTFLOW STORAGE ) values
1373 ( cms) - ( ha- m)
1374 [ 0. 000 , 0. 0000 ]
1375 [ 0. 031 , 0. 0010 ]
1376 [ 0. 032 , 0. 0050 ]
1377 [ - 1 , - 1 ] ( m a x t w e n t y p t s)
1378 NHYDovf =[ " ST4OVF" ]
1379 *%-----|-----|
1380 CONTINUOUS STANDHYD NHYD=[ " A18" ] , DT=[ 1] mi n , AREA=[ 5. 30] ( ha) , XI MP=[ 0. 68] , TI MP=[ 0. 85] ,
DWF=[ 0] ( cms) , LOSS=[ 1] :
1381 Horton: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] ( / hr) ,
F=[ 0. 00] ( mm) ,
1382 Pervious areas: I A per =[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1383 ImperVIOUS areas: I A i mp=[ 1. 57] ( mm) , SLPI =[ 0. 5] ( % ) ,
LGI =[ 325. 576] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1384 Continuous simulation parameters:
1385 I a REC per =[ 4] ( hr s) , I a REC i mp=[ 4] ( hr s) , I n t e r E v e n t T i m e =[ 12] ( hr s) ,
END=- 1
1386 *%-----|-----|
1387 ROUTE RESERVOIR NHYDout =[ " A18STR" ] , NHYD i n=[ " A18" ] , RDT=[ 1] ( mi n) ,
1388 TABLE of ( OUTFLOW STORAGE ) values
1389 ( cms) - ( ha- m)
1390 [ 0. 000 , 0. 000 ]
1391 [ 0. 074 , 0. 082 ]
1392 [ 0. 154 , 0. 109 ]

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1393         [ 0.212 , 0.125 ]
1394         [ 0.265 , 0.149 ]
1395         [ 0.339 , 0.158 ]
1396         [ 0.392 , 0.172 ]
1397         [ -1 , -1 ] (max twenty pts)
1398         NHYDovf=[ "A18OVF" ]
1399 *%-----|-----|
1400 *ANALYSIS POINT 3 - TOTAL FLOW AT OUTLET OF STREET 4
1401 *%-----|-----|
1402 ADD HYD          NHYDsum=[ "PT3ST4" ], NHYDsto
add=[ "DRAIN2"+"D2"+"A17STR"+"A17OVF"+"ST4STR"+"ST4OVF"+"A18STR"+"A18OVF" ]
1403 *%-----|-----|
1404 *ROUTE FLOW through O'Keefe Drain 3
1405 ROUTE CHANNEL   NHYDout=[ "DRAIN3" ], NHYDin=[ "PT3ST4" ], RDT=[ 1 ] (min),
1406                CHLGTH=[ 525 ] (m), CHSLOPE=[ .23 ] (%), FPSLOPE=[ .23 ] (%),
1407                SECNUM=[ 1 ], NSEG=[ 3 ]
1408                ( SEGROUGH, SEGDIST (m) )=[ 0.07, 12.50 -0.043, 17.50 0.07, 30.00 ] NSEG
times
1409                ( DISTANCE (m), ELEVATION (m) )=[ 0.00, 1.70 ]
1410                ( 2.50, 1.00 )
1411                ( 12.50, 0.80 )
1412                ( 14.10, 0.00 )
1413                ( 15.90, 0.00 )
1414                ( 17.50, 0.80 )
1415                ( 27.50, 1.00 )
1416                ( 30.00, 1.70 )
1417 *%-----|-----|
1418 CONTINUOUS NASHYD NHYD=[ "D3" ], DT=[ 1 ] min, AREA=[ 2.51 ] (ha),
1419                DWF=[ 0 ] (cms), CNVC=[ 86 ], IAC=[ 8.7 ] (mm), N=[ 3 ], TP=[ 0.73 ] hrs,
1420                Continuous simulation parameters:
1421                IARECper=[ 4 ] (hrs),
1422                SMIN=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
1423                InterEventTime=[ 12 ] (hrs)
1424                Baseflow simulation parameters:
1425                BaseFlowOption=[ 1 ],
1426                InitalGWResVol=[ 50 ] (mm), GWResK=[ 0.96 ] (mm/day/mm)
1427                VHydCond=[ 0.055 ] (mm/hr), END=-1
1428 *%-----|-----|
1429 CONTINUOUS STANDHYD NHYD=[ "C1" ], DT=[ 1 ] min, AREA=[ 3.41 ] (ha), XI MP=[ 0.68 ], TI MP=[ 0.85 ],
DWF=[ 0 ] (cms), LOSS=[ 1 ]:
1430                Horton: Fo=[ 76.20 ] (mm/hr), Fc=[ 13.20 ] (mm/hr), DCAY=[ 4.14 ] (/hr),
1431                F=[ 0.00 ] (mm),
1432                Pervious areas: IAPER=[ 4.67 ] (mm), SLPP=[ 0.5 ] (%), LGP=[ 50 ] (m),
1433                MNP=[ 0.250 ], SCP=[ 0 ] (min),
1434                Impervious areas: IAIMP=[ 1.57 ] (mm), SLPI=[ 0.5 ] (%),
1435                LGL=[ 261.151 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (min),
1436                Continuous simulation parameters:
1437                IARECper=[ 4 ] (hrs), IARECimp=[ 4 ] (hrs), InterEventTime=[ 12 ] (hrs),
1438                END=-1
1439 *%-----|-----|
1440 ROUTE RESERVOIR NHYDout=[ "C1-STR" ], NHYDin=[ "C1" ], RDT=[ 1 ] (min),
1441                TABLE of ( OUTFLOW STORAGE ) values
1442                ( cms ) - ( ha-m )
1443                [ 0.000 , 0.000 ]
1444                [ 0.048 , 0.052 ]
1445                [ 0.099 , 0.070 ]
1446                [ 0.136 , 0.080 ]
1447                [ 0.170 , 0.096 ]
1448                [ 0.218 , 0.102 ]
1449                [ 0.252 , 0.111 ]
1450                [ -1 , -1 ] (max twenty pts)
1451                NHYDovf=[ "C1-OVF" ]
1452 *%-----|-----|
1453 CONTINUOUS STANDHYD NHYD=[ "ST-5" ], DT=[ 1 ] min, AREA=[ 0.45 ] (ha), XI MP=[ 0.46 ],
1454                TI MP=[ 0.57 ], DWF=[ 0 ] (cms), LOSS=[ 1 ]:
1455                Horton: Fo=[ 76.20 ] (mm/hr), Fc=[ 13.20 ] (mm/hr), DCAY=[ 4.14 ] (/hr),

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1451 F=[ 0. 00] ( mm) ,
Pervious areas: I A per=[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1452 Impervious areas: I A i mp=[ 1. 57] ( mm) , SLPI=[ 0. 5] ( % ) , LGI =[ 94. 868] ( m) ,
MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1453 Continuous simulation parameters:
1454 I a REC per=[ 4] ( hrs) , I a REC i mp=[ 4] ( hrs) , I nter Event Ti me=[ 12] ( hrs) ,
END=- 1
1455 *%-----|
1456 ROUTE RESERVOIR NHYDout =[ " ST5STR" ] , NHYDi n=[ " ST- 5" ] , RDT=[ 1] ( mi n) ,
1457 TABLE of ( OUTFLOW STORAGE ) values
1458 ( cms) - ( ha- m)
1459 [ 0. 000 , 0. 0000 ]
1460 [ 0. 040 , 0. 0010 ]
1461 [ 0. 041 , 0. 0062 ]
1462 [ - 1 , - 1 ] ( max t went y pt s)
1463 NHYDovf=[ " ST5OVF" ]
1464 *%-----|
1465 ADD HYD NHYDs um=[ " ST5- E" ] , NHYDs t o
add=[ " DRAI N3" +" D3" +" C1- STR" +" C1- OVF" +" ST5STR" +" ST5OVF" ]
1466 *%-----|
1467 CONTINUOUS STANDHYD NHYD=[ " STRAND" ] , DT=[ 1] ( mi n) , AREA=[ 7. 59] ( ha) ,
1468 XI MP=[ 0. 64] , TI MP=[ 0. 85] , DWF=[ 0] ( cms) , LOSS=[ 1] :
1469 Horton: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] ( / hr) ,
F=[ 0. 00] ( mm) ,
1470 Pervious areas: I A per=[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 40] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1471 Impervious areas: I A i mp=[ 1. 57] ( mm) , SLPI=[ 0. 5] ( % ) , LGI =[ 1230] ( m) ,
MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1472 Continuous simulation parameters:
1473 I a REC per=[ 4] ( hrs) , I a REC i mp=[ 4] ( hrs) , I nter Event Ti me=[ 12] ( hrs) ,
END=- 1
1474 *%-----|
1475 ROUTE RESERVOIR NHYDout =[ " S- POND" ] , NHYDi n=[ " STRAND" ] , RDT=[ 1] ( mi n) ,
1476 TABLE of ( OUTFLOW STORAGE ) values
1477 ( cms) - ( ha- m)
1478 [ 0. 000 , 0. 000 ]
1479 [ 0. 033 , 0. 188 ]
1480 [ 0. 057 , 0. 253 ]
1481 [ 0. 104 , 0. 287 ]
1482 [ 0. 160 , 0. 336 ]
1483 [ 0. 340 , 0. 346 ]
1484 [ 0. 471 , 0. 360 ]
1485 [ 0. 824 , 0. 390 ]
1486 [ - 1 , - 1 ] ( max t went y pt s)
1487 NHYDovf=[ " S- OVF" ]
1488 *%-----|
1489 ADD HYD NHYDs um=[ " SSAOUT" ] , NHYDs t o add=[ " ST5- E" +" S- POND" +" S- OVF" ]
1490 *%-----|
1491 SAVE HYD NHYD=[ " SSAOUT" ] , # OF PCYCLES=[ 5] , I CASEs h=[ 1]
1492 HYD_ COMMENT=[ " SSAOUT" ]
1493 *%-----|
1494 CONTINUOUS STANDHYD NHYD=[ " Area- A" ] , DT=[ 1] mi n , AREA=[ 66. 75] ( ha) , XI MP=[ 0. 64] ,
1495 TI MP=[ 0. 80] , DWF=[ 0] ( cms) , LOSS=[ 1] :
1496 Horton: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] ( / hr) ,
F=[ 0. 00] ( mm) ,
1497 Pervious areas: I A per=[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1497 Impervious areas: I A i mp=[ 1. 57] ( mm) , SLPI=[ 0. 5] ( % ) ,
LGI =[ 1155. 422] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1498 Continuous simulation parameters:
1499 I a REC per=[ 4] ( hrs) , I a REC i mp=[ 4] ( hrs) , I nter Event Ti me=[ 12] ( hrs) ,
END=- 1
1500 *%-----|
1501 SAVE HYD NHYD=[ " Area- A" ] , # OF PCYCLES=[ 1] , I CASEs h=[ 1]
1502 HYD_ COMMENT=[ " SMMF- A I nflow" ]

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1503 *%-----|-----|
1504 ROUTE RESERVOIR NHYDout=["SWMF-A"], NHYDin=["Area-A"], RDT=[1](min),
1505 TABLE of ( OUTFLOW STORAGE ) values
1506 (cms) - (ha-m)
1507 [ 0.000 , 0.000 ]
1508 [ 0.103 , 1.077 ]
1509 [ 0.128 , 1.749 ]
1510 [ 0.382 , 2.282 ]
1511 [ 0.703 , 2.582 ]
1512 [ 1.256 , 2.978 ]
1513 [ 1.567 , 3.202 ]
1514 [ 1.955 , 3.493 ]
1515 [ 2.100 , 3.600 ]
1516 [ -1 , -1 ] (max twenty pts)
1517 NHYDovf=["SWVAOV"]
1518 *%-----|-----|
1519 SAVE HYD NHYD=["SWMF-A"], # OF PCYCLES=[1], ICASEsh=[1]
1520 HYD_COMMENT=["SMMF-A Outflow"]
1521 *%-----|-----|
1522 *ANALYSIS POINT 4 - TOTAL FLOW AT OUTLET OF STREET 5
1523 *%-----|-----|
1524 ADD HYD NHYDs um=["PT4ST5"], NHYDs to add=["SSAOUT"+"SWMF-A"+"SWVAOV"]
1525 *%-----|-----|
1526 CONTINUOUS STANDHYD NHYD=["C6"], DT=[1]min, AREA=[1.87](ha), XI MP=[0.68], TI MP=[0.85],
DWF=[0](cms), LOSS=[1]:
1527 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1528 Pervious areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1529 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%),
LGI=[193.391](m), MNI=[0.013], SCI=[0](min),
1530 Continuous simulation parameters:
1531 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
1532 *%-----|-----|
1533 ROUTE RESERVOIR NHYDout=["C6-STR"], NHYDin=["C6"], RDT=[1](min),
1534 TABLE of ( OUTFLOW STORAGE ) values
1535 (cms) - (ha-m)
1536 [ 0.000 , 0.000 ]
1537 [ 0.026 , 0.029 ]
1538 [ 0.054 , 0.038 ]
1539 [ 0.075 , 0.044 ]
1540 [ 0.093 , 0.052 ]
1541 [ 0.120 , 0.056 ]
1542 [ 0.138 , 0.061 ]
1543 [ -1 , -1 ] (max twenty pts)
1544 NHYDovf=["C6-OVF"]
1545 *%-----|-----|
1546 CONTINUOUS STANDHYD NHYD=["C7"], DT=[1]min, AREA=[1.62](ha), XI MP=[0.68], TI MP=[0.85],
DWF=[0](cms), LOSS=[1]:
1547 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1548 Pervious areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1549 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%),
LGI=[180.000](m), MNI=[0.013], SCI=[0](min),
1550 Continuous simulation parameters:
1551 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
1552 *%-----|-----|
1553 ROUTE RESERVOIR NHYDout=["C7-STR"], NHYDin=["C7"], RDT=[1](min),
1554 TABLE of ( OUTFLOW STORAGE ) values
1555 (cms) - (ha-m)
1556 [ 0.000 , 0.000 ]
1557 [ 0.023 , 0.025 ]
1558 [ 0.047 , 0.033 ]

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1559          [ 0.065 , 0.038 ]
1560          [ 0.081 , 0.045 ]
1561          [ 0.104 , 0.048 ]
1562          [ 0.120 , 0.053 ]
1563          [ -1 , -1 ] (max twenty pts)
1564          NHYDovf=["C7-OVF"]
1565 *%-----|-----|
1566 CONTINUOUS STANDHYD NHYD=["ST-6"], DT=[1] min, AREA=[0.41](ha), XI MP=[0.46], TI MP=[0.57],
DWF=[0](cms), LOSS=[1]:
1567 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1568 Pervious areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1569 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%), LGI=[90.554](m),
MNI=[0.013], SCI=[0](min),
1570 Continuous simulation parameters:
1571 I a RE C per=[4](hrs), I a RE C i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
1572 *%-----|-----|
1573 ROUTE RESERVOIR NHYDout=["ST6STR"], NHYDin=["ST-6"], RDT=[1](min),
1574 TABLE of ( OUTFLOW STORAGE ) values
1575 (cms) - (ha-m)
1576 [ 0.000 , 0.0000 ]
1577 [ 0.036 , 0.0010 ]
1578 [ 0.037 , 0.0058 ]
1579 [ -1 , -1 ] (max twenty pts)
1580 NHYDovf=["ST6OVF"]
1581 *%-----|-----|
1582 *ANALYSIS POINT 5 - TOTAL FLOW AT OUTLET OF STREET 6
1583 *%-----|-----|
1584 ADD HYD NHYDsum=["PT5ST6"], NHYDsto
add=["PT4ST5"+"C6-STR"+"C6-OVF"+"C7-STR"+"C7-OVF"+"ST6STR"+"ST6OVF"]
1585 *%-----|-----|
1586 ROUTE FLOW through O Keefe Drain 4
1587 ROUTE CHANNEL NHYDout=["DRAIN4"], NHYDin=["PT5ST6"], RDT=[1](min),
1588 CHLGTH=[324](m), CHSLOPE=[.10](%), FPSLOPE=[.10](%),
1589 SECNUM=[1], NSEG=[3]
1590 ( SEGROUGH, SEGDI ST (m))=[0.07, 12.00 -0.043, 18.00 0.07, 30.00] NSEG
t i m e s
1591 ( DI STANCE (m), ELEVATION (m))=[0.00, 2.00]
1592 (2.00, 1.20)
1593 (12.00, 1.00)
1594 (14.00, 0.00)
1595 (16.00, 0.00)
1596 (18.00, 1.00)
1597 (28.00, 1.20)
1598 (30.00, 2.00)
1599 *%-----|-----|
1600 CONTINUOUS NASHYD NHYD=["D4"], DT=[1] min, AREA=[1.73](ha), DWF=[0](cms), CNV C=[88],
I A=[8.4](mm),
1601 N=[3], TP=[0.60] hrs,
1602 Continuous simulation parameters:
1603 I a RE C per=[4](hrs),
1604 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1605 I n t e r E v e n t T i m e=[12](hrs)
1606 Baseflow simulation parameters:
1607 BaseFlowOption=[1],
1608 I n i t G W R e s V o l=[50](mm), G W R e s K=[0.96](mm/day/mm)
1609 VHydCond=[0.055](mm/hr), END=-1
1610 *%-----|-----|
1611 CONTINUOUS STANDHYD NHYD=["Area-B"], DT=[1] min, AREA=[24.04](ha), XI MP=[0.62],
TI MP=[0.77], DWF=[0](cms), LOSS=[1]:
1612 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1613 Pervious areas: I A per=[4.67](mm), SLPP=[1.4](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),

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1614      Impervious areas: IAImp=[ 1.57](mm), SLPI=[ 1.4](%),
1615      LGI=[ 693.397](m), MNI=[ 0.013], SCI=[ 0](min),
1616      Continuous simulation parameters:
1617      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1618      END=- 1
1619
1620 *%-----|-----|
1621 ROUTE RESERVOIR      NHYDout=["SWMF- B"], NHYDin=["Area- B"], RDT=[ 1](min),
1622      TABLE of ( OUTFLOW STORAGE ) values
1623      (cms) - (ha-m)
1624      [ 0.000 , 0.000 ]
1625      [ 0.025 , 0.090 ]
1626      [ 0.175 , 0.510 ]
1627      [ 0.350 , 0.710 ]
1628      [ 0.495 , 0.820 ]
1629      [ 0.648 , 0.980 ]
1630      [ 0.965 , 1.045 ]
1631      [ 1.072 , 1.140 ]
1632      [ -1 , -1 ] (max twenty pts)
1633      NHYDovf=["SWMBOVF"]
1634
1635 *%-----|-----|
1636 ADD HYD              NHYDs um=["D4- EX"], NHYDs to add=["DRAIN4"+"D4"+"SWMF- B"+"SWMBOVF"]
1637
1638 *%-----|-----|
1639 *ROUTE FLOW THROUGH O'Keefe Drain 5
1640 * JFSA: Nov. 2020, added endpoints to close X-Section
1641 ROUTE CHANNEL      NHYDout=["DRAIN5"], NHYDin=["D4- EX"], RDT=[ 1](min),
1642      CHLGTH=[ 413.0](m), CHSLOPE=[ 0.16](%), FPSLOPE=[ 0.16](%),
1643      SECNUM=[ 1], NSEG=[ 3]
1644      ( SEGROUGH, SEGDIST (m))=[ 0.043, 12.29 -0.033, 17.97
1645      0.043, 32.84] NSEG times
1646      ( DISTANCE (m), ELEVATION (m))=(-0.01, 2.50)
1647      [ 0.00, 1.41]
1648      [ 6.13, 0.97]
1649      [ 12.29, 0.89]
1650      [ 15.71, 0.00]
1651      [ 17.97, 0.39]
1652      [ 23.04, 0.35]
1653      [ 32.83, 0.96]
1654      (32.84, 2.50)
1655
1656 *%-----|-----|
1657 CONTINUOUS NASHYD   NHYD=["D5"], DT=[ 1]min, AREA=[ 1.90](ha),
1658      DWF=[ 0](cms), CNVC=[ 86], IA=[ 8.7](mm), N=[ 3], TP=[ 0.69]hrs,
1659      Continuous simulation parameters:
1660      IARECper=[ 4](hrs),
1661      SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1662      InterEventTime=[ 12](hrs)
1663      Baseflow simulation parameters:
1664      BaseFlowOption=[ 1],
1665      InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
1666      VHydCond=[ 0.055](mm/hr), END=- 1
1667
1668 *%-----|-----|
1669 *EXTERNAL FLOWS SOUTHEAST OF THE SITE NORTH OF McKENNA CASEY DR.
1670 CONTINUOUS NASHYD   NHYD=["O-13SDF"], DT=[ 1]min, AREA=[ 9.74](ha),
1671      DWF=[ 0](cms), CNVC=[ 81], IA=[ 4.0](mm), N=[ 3], TP=[ .43]hrs,
1672      Continuous simulation parameters:
1673      IARECper=[ 4](hrs),
1674      SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1675      InterEventTime=[ 12](hrs)
1676      Baseflow simulation parameters:
1677      BaseFlowOption=[ 1],
1678      InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
1679      VHydCond=[ 0.055](mm/hr), END=- 1
1680
1681 *%-----|-----|
1682 *SNOW DISPOSAL FACILITY
1683 *PARAMETERS BASED ON ROBINSON 2006 MODEL
1684 ROUTE RESERVOIR      NHYDout=["SDF"], NHYDin=["O-13SDF"], RDT=[ 1](min),
1685      TABLE of ( OUTFLOW STORAGE ) values

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1678             ( cms ) - ( ha - m )
1679             [ 0. 000, 0. 000 ]
1680             [ 0. 150, 0. 600 ]
1681             ( 0. 200, 1. 500 )
1682             [ - 1 , - 1 ] ( max t w e n t y p t s )
1683             NHYDovf=[ " OVFSDF " ]
1684 *%-----|-----|
1685 *ANALYSIS POINT 6 - McKenna Casey Dr.
1686 *%-----|-----|
1687 ADD HYD             NHYDsum=[ " PT6MC " ], NHYDs to add=[ " DRAIN5 "+" D5 "+" SDF " ]
1688 *%-----|-----|
1689 CONTINUOUS NASHYD  NHYD=[ " O- 15 " ], DT=[ 1 ] mi n, AREA=[ 10. 67 ] ( ha ),
1690                   DWF=[ 0 ] ( cms ), CNVC=[ 82 ], IA=[ 7. 5 ] ( mm ), N=[ 3 ], TP=[ 0. 30 ] hr s,
1691                   Continuous simulation parameters:
1692                   IaRECper=[ 4 ] ( hr s ),
1693                   SMN=[ - 1 ] ( mm ),   SMAX=[ - 1 ] ( mm ), SK=[ 0. 010 ] / ( mm ),
1694                   InterEventTime=[ 12 ] ( hr s )
1695                   Baseflow simulation parameters:
1696                   BaseFlowOption=[ 1 ] ,
1697                   InitGWResVol=[ 50 ] ( mm ),   GWResK=[ 0. 96 ] ( mm / day / mm )
1698                   VHydCond=[ 0. 055 ] ( mm / hr ),   END=- 1
1699 *%-----|-----|
1700 *TOTAL FLOW NORTH OF MCKENNA CASEY DR.
1701 ADD HYD             NHYDsum=[ " M C " ], NHYDs to add=[ " PT6MC "+" O- 15 " ]
1702 *%-----|-----|
1703 *ROUTE FLOW THROUGH AREA O- 14
1704 * JFSA: Nov. 2020, added end points to close X-section
1705 ROUTE CHANNEL      NHYDout=[ " O- 14Ch " ], NHYDin=[ " M C " ], RDT=[ 1 ] ( mi n ),
1706                   CHLGT=[ 845. 3 ] ( m ),   CHSLOPE=[ 0. 10 ] ( % ), FPSLOPE=[ 0. 10 ] ( % ),
1707                   SECNUM=[ 1 ], NSEG=[ 3 ]
1708                   ( SEGROUGH, SEGDIST ( m ) )=[ 0. 06, 15. 00 - 0. 033, 18. 04 0. 06, 31. 85 ] NSEG
1709                   t i m e s
1710                   ( DI STANCE ( m ), ELEVATI ON ( m ) )=[ - 0. 01, 2. 5
1711                   ( 0. 00, 1. 53 ]
1712                   ( 5. 56, 1. 47 )
1713                   ( 9. 21, 1. 45 )
1714                   ( 12. 45, 1. 53 )
1715                   ( 13. 70, 1. 50 )
1716                   ( 15. 00, 0. 69 )
1717                   ( 15. 34, 0. 00 )
1718                   ( 16. 51, 0. 05 )
1719                   ( 17. 30, 0. 17 )
1720                   ( 18. 04, 0. 74 )
1721                   ( 19. 29, 1. 32 )
1722                   ( 22. 73, 1. 47 )
1723                   ( 31. 84, 1. 41 )
1724                   ( 31. 85, 2. 50 )
1725 *%-----|-----|
1726 *% -Change O- 14 from NASHYD to STANDHYD, name it " S- 1- Okeefe " and add it to S- 1
1727 subcatchment based on Project 1474- BCDC, JFSA, Nov. 2020
1728 *% -JFSA 2021- 02- 16, add detailed subcatchment drainage area for each subcatchment
1729 in Corrigan sub-catchment. After adding part of O- 14 to S_ 1 sub-catchment so O- 14
1730 becomes 5 ha instead of 30. 02 ha and TP becomes 0. 133 ( 5 * 0. 8 / 30. 02 ) instead of 0. 8
1731 CONTINUOUS NASHYD  NHYD=[ " O- 14 " ], DT=[ 1 ] mi n, AREA=[ 5 ] ( ha ),
1732                   DWF=[ 0 ] ( cms ), CNVC=[ 82 ], IA=[ 7. 5 ] ( mm ), N=[ 3 ], TP=[ 0. 133 ] hr s,
1733                   Continuous simulation parameters:
1734                   IaRECper=[ 4 ] ( hr s ),
1735                   SMN=[ - 1 ] ( mm ),   SMAX=[ - 1 ] ( mm ), SK=[ 0. 010 ] / ( mm ),
1736                   InterEventTime=[ 12 ] ( hr s )
1737                   Baseflow simulation parameters:
1738                   BaseFlowOption=[ 1 ] ,
1739                   InitGWResVol=[ 50 ] ( mm ),   GWResK=[ 0. 96 ] ( mm / day / mm )
1740                   VHydCond=[ 0. 055 ] ( mm / hr ),   END=- 1
1741 *
1742 *%-----|-----|
1743 *ANALYSIS POINT 7 - JOCK RIVER

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1740 * 2020-12-01 To Foster Drain
1741 * 2020-12-01 replace ("PT7JR") by ("OKEEFE")
1742 *%-----|-----
1743 ADD HYD          NHYDsum=["OKEEFE"], NHYDs to add=["O-14Ch"+"O-14"]
1744 *%-----|-----
1745 *CONTINUOUS STANDHYD NHYD=["OKEEFE"], DT=[1](min), AREA=[448](ha),
1746 *          XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
1747 *          SCS curve number CN=[77],
1748 *          Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
1749 *          LGP=[40](m), MNP=[0.25], SCP=[0](min),
1750 *          Impervious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
1751 *          LGI=[1728](m), MNI=[0.013], SCI=[0](min),
1752 *          Continuous simulation parameters:
1753 *          I a REC per=[4](hrs), I a REC i mp=[4](hrs),
1754 *          SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1755 *          Inter Event Ti me=[18](hrs), END=-1
1756 *#*****
1757 *#      Okeefe Pond
1758 *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1759 *#      and a ratio of the catchment area to the West Clarke pond rating curve
1760 *#      from the M55 for the next coordinates
1761 *#*****
1762 *ROUTE RESERVOIR  NHYDout=["P_OKE"], NHYDin=["OKEEFE"],
1763 *          RDT=[1](min),
1764 *          TABLE of ( OUTFLOW STORAGE ) values
1765 *          ( cms ) - ( ha-m )
1766 *          [ 0.0 , 0.0 ]
1767 *          [ 14.13 , 13.0 ]
1768 *          [ -1 , -1 ] (maximum one hundred pairs of points)
1769 *          NHYDovf=["ok-OVF"],
1770 *%-----|-----
1771 * -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
1772 * moved to drain before station 6215 on Jock River
1773 *CONTINUOUS STANDHYD NHYD=["S-1-D2"], DT=[1](min), AREA=[18.67](ha), XI MP=[0.65],
1774 *          TI MP=[0.65], DWF=[0](cms),
1775 *          LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
1776 *          I A per=[4.67](mm), SLPP=[2.0](%),
1777 *          LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
1778 *          I A i mp=[1.57](mm), SLPI=[0.75](%),
1779 *          LGI=[352.798](m), MNI=[0.013], SCI=[0](min),
1780 *          Continuous simulation parameters:
1781 *          I a REC per=[4](hrs), I a REC i mp=[4](hrs),
1782 *          SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1783 *          Inter Event Ti me=[12](hrs), END=-1
1784 *%-----|-----
1785 *CONTINUOUS NASHYD  NHYD=["S-1-D2"], DT=[1]min, AREA=[18.67](ha),
1786 *          DWF=[0](cms), CNVC=[77], I A=[4.67](mm),
1787 *          N=[3], TP=[1.120]hrs,
1788 *          Continuous simulation parameters:
1789 *          I a REC per=[4](hrs),
1790 *          SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1791 *          Inter Event Ti me=[12](hrs)
1792 *          Baseflow simulation parameters:
1793 *          BaseFlowOption=[1],
1794 *          I ni t GWRes Vol =[50](mm), GWRes K=[0.96](mm/day/mm)
1795 *          VHydCond=[0.055](mm/hr), END=-1
1796 *%-----|-----
1797 *COMPUTE DUALHYD  NHYDin=["S-1-D2"], CINLET=[2.062](cms), NINLET=[1],
1798 *          M a j NHYD=["S-1-D2J"]
1799 *          M n NHYD=["S-1-D2N"]
1800 *          TMI STO=[9999999](cu-m)
1801 *%-----|-----
1802 *ADD HYD          NHYDsum=["S-1-D2S"], NHYDs to add=["S-1-D2J"+"S-1-D2N"]
1803 *%-----|-----
1804 *ROUTE RESERVOIR  NHYDout=["S-1-D2R"], NHYDin=["S-1-D2S"],

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```

1801 *          RDT=[ 1](mi n),
1802 *          TABLE of ( OUTFLOW STORAGE ) values
1803 *                ( cms ) - ( ha- m)
1804 *                [ 0.0      , 0.0 ]
1805 *                [ 0.2231, 0.7445 ]
1806 *                [   -1   ,  -1   ] (max twenty pts)
1807 *          NHYDovf=[ "S- 1- D2Rovf" ]
1808 *%-----|-----
1809 CONTINUOUS STANDHYD NHYD=[ "S- 1- D3" ], DT=[ 1](mi n), AREA=[ 6.79](ha), XI MP=[ 0.65],
TI MP=[ 0.65], DWF=[ 0](cms),
1810 LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
1811 IAper=[ 4.67](mm), SLPP=[ 2.0](%),
LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n), Impervious surfaces:
1812 IAi mp=[ 1.57](mm), SLPI=[ 0.75](%),
1813 LGI=[ 212.760](m), MNI=[ 0.013], SCI=[ 0](mi n),
1814 Continuous simulation parameters:
1815 IaRECPper=[ 4](hrs), IaRECI mp=[ 4](hrs),
1816 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1817 InterEvent Ti me=[ 12](hrs), END=- 1
1818 *%-----|-----
1819 *CONTINUOUS NASHYD NHYD=[ "S- 1- D3" ], DT=[ 1]mi n, AREA=[ 6.79](ha),
1820 * DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
1821 * N=[ 3], TP=[ 1.281]hrs,
1822 * Continuous simulation parameters:
1823 * IaRECPper=[ 4](hrs),
1824 * SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1825 * InterEvent Ti me=[ 12](hrs)
1826 * Baseflow simulation parameters:
1827 * BaseFl owOpt ion=[ 1],
1828 * Ini tGWRes Vol =[ 50](mm), GWRes K=[ 0.96](mm/ day/ mm)
1829 * VHydCond=[ 0.055](mm/ hr), END=- 1
1830 *%-----|-----
1831 *COMPUTE DUALHYD NHYDi n=[ "S- 1- D3" ], CI NLET=[ 0.719](cms), NI NLET=[ 1],
1832 * Mj NHYD=[ "S- 1- D3J" ]
1833 * M nNHYD=[ "S- 1- D3N" ]
1834 * TM STO=[ 9999999](cu- m)
1835 *%-----|-----
1836 *ADD HYD NHYDs um=[ "S- 1- D3S" ], NHYDs to add=[ "S- 1- D3J"+"S- 1- D3N" ]
1837 *%-----|-----
1838 *ROUTE RESERVOIR NHYDout=[ "S- 1- D3R" ], NHYDi n=[ "S- 1- D3S" ],
1839 * RDT=[ 1](mi n),
1840 *          TABLE of ( OUTFLOW STORAGE ) values
1841 *                ( cms ) - ( ha- m)
1842 *                [ 0.0      , 0.0 ]
1843 *                [ 0.0811, 0.2708 ]
1844 *                [   -1   ,  -1   ] (max twenty pts)
1845 *          NHYDovf=[ "S- 1- D3Rovf" ]
1846 *%-----|-----
1847 *ADD HYD NHYDs um=[ "SN_OK" ], NHYDs to add=[ "N_OK"+"OKEEFE"+"S- 1- D2"+"S- 1- D3" ]
1848 *%-----|-----
1849 *SAVE HYD NHYD=[ "SN_OK" ], # OF PCYCLES=[ -1], I CASEs h=[ 1]
HYD_COMMENT=[ "Total Fl ows at Okeefe Drain" ]
1850 *%-----|-----
1851 *#
1852 *# Hydrograph from Node Okeefe routed to Node at Foster Drain
1853 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6215
1854 *#
1855 *ROUTE CHANNEL NHYDout=[ "N_FO" ], NHYDi n=[ "SN_OK" ],
1856 * RDT=[ 1](mi n),
1857 * CHLGTH=[ 1183](m), CHSLOPE=[ 0.0761](%),
1858 * FPSLOPE=[ 0.0761](%),
1859 * SECNUM=[ 1.0], NSEG=[ 3]
1860 * ( SEGROUGH, SEGDI ST (m))=
1861 * [ 0.050, -33.89
1862 * -0.035, 31.59
1863 * 0.050, 34.41] NSEG ti mes

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1864      ( DI STANCE ( m), ELEVATI ON ( m) ) =
1865      [- 794. 18, 91. 00]
1866      [- 775. 41, 91. 50]
1867      [- 702. 63, 91. 50]
1868      [- 546. 19, 91. 50]
1869      [- 529. 54, 91. 50]
1870      [- 323. 44, 91. 00]
1871      [- 320. 71, 91. 00]
1872      [- 183. 59, 91. 00]
1873      [- 182. 54, 90. 50]
1874      [- 181. 36, 90. 00]
1875      [- 177. 37, 90. 00]
1876      [- 87. 70, 90. 00]
1877      [- 33. 89, 90. 00]
1878      [- 18. 52, 86. 88]
1879      [ 0. 00, 85. 20]
1880      [ 16. 20, 86. 83]
1881      [ 31. 59, 90. 00]
1882      [ 33. 03, 90. 50]
1883      [ 34. 41, 91. 00]
1884      *%-----|-----|
1885      *#*****|
1886      *#      Catchment FOSTER
1887      *#      - To Foster ditch (north of the Jock)
1888      *#      - Partially developed (medium density); remaining agricultural
1889      *#      - 2020-12-01 JFSA Foster area is 332 as per Foster SWF Environmental Study
1890      *#      - 2020-12-01 decrease Foster drainage area from (373 HA) to (307.98 HA) after
1891      *#      - 2021-02-12 update Foster area to 325.44 ha as measured from QGIS
1892      *#*****|
1893      CONTINUOUS STANDHYD NHYD=[ "FOSTER" ], DT=[ 1] mi n, AREA=[ 325. 44] ( ha),
1894      XI MP=[ 0. 55], TI MP=[ 0. 55], DWF=[ 0] ( cms), LOSS=[ 2],
1895      SCS curve number CN=[ 74],
1896      Pervious surfaces: I A per=[ 4. 67] ( mm), SLPP=[ 0. 5] ( %),
1897      LGP=[ 40] ( m), MNP=[ 0. 25], SCP=[ 0] ( mi n),
1898      Impervious surfaces: I A i mp=[ 1. 57] ( mm), SLPI=[ 0. 5] ( %),
1899      LGI=[ 1472. 956] ( m), MNI=[ 0. 013], SCI=[ 0] ( mi n),
1900      Continuous simulation parameters:
1901      I a REC per=[ 4] ( hr s), I a REC i mp=[ 4] ( hr s),
1902      SM N=[ - 1] ( mm), SMAX=[ - 1] ( mm), SK=[ 0. 010] / ( mm),
1903      Inter Event Ti me=[ 18] ( hr s), END=- 1
1904      *#*****|
1905      *#      Foster Pond
1906      *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1907      *#      and a ratio of the catchment area to the West Clarke pond rating curve
1908      *#      from the MS for the next coordinates
1909      *#*****|
1910      ROUTE RESERVOIR      NHYDout =[ "P_FOS" ], NHYDin=[ "FOSTER" ],
1911      RDT=[ 1] ( mi n),
1912      TABLE of ( OUTFLOW STORAGE ) values
1913      ( cms ) - ( ha- m)
1914      [ 0. 0 , 0. 0 ]
1915      [ 10. 34 , 10]
1916      [ - 1 , - 1 ] ( max twenty pts)
1917      NHYDovf=[ "FO- OVF" ]
1918      *%-----|-----|
1919      ADD HYD      NHYDsum=[ "FOSTER- OUT" ], NHYDs to add=[ "P_FOS"+"FO- OVF" ]
1920      *%-----|-----|
1921      *#*****|
1922      *#      -Brazeau area from P 1800-19 =[71.751], change to 63.59 ha based on GIS measurements
1923      *#      -JFSA, 2021-01-19 update "W_CLAR_BRAZ" to 73.29 ha based on GIS measurements
1924      *#      -JFSA, 2021-01-22 Brazeau ("MS_P10"+"P10-OVF")brazeau pond discharges directly
1925      to the jock river through a road side ditch on the west side of Borrisokane road
1926      (station 6016)

```

```

1925 CONTINUOUS STANDHYD NHYD=[ "W_CLAR_BRAZ" ], DT=[ 1 ] mi n, AREA=[ 73.29 ] ( ha ),
1926 XI MP=[ 0.6 ], TI MP=[ 0.65 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
1927 SCS curve number CN=[ 77 ],
1928 Pervious surfaces: I A per=[ 4.67 ] ( mm ), SLPP=[ 1 ] ( % ),
1929 LGP=[ 40 ] ( m ), MNP=[ 0.25 ], SCP=[ 0 ] ( mi n ),
1930 Impervious surfaces: I A i mp=[ 1.57 ] ( mm ), SLPI=[ 0.5 ] ( % ),
1931 LGI=[ 699.00 ] ( m ), MNI=[ 0.013 ], SCI=[ 0 ] ( mi n ),
1932 Continuous simulation parameters:
1933 I a REC per=[ 4 ] ( hr s ), I a REC i mp=[ 4 ] ( hr s ),
1934 SM N=[ -1 ] ( mm ), SMAX=[ -1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
1935 Inter Event Ti me=[ 18 ] ( hr s ), END=- 1
1936 *%-----|-----|
1937 * 2020-12-01 correct pond curve values
1938 ROUTE RESERVOIR NHYDout=[ "MS_P10" ], NHYDin=[ "W_CLAR_BRAZ" ],
1939 RDT=[ 1 ] ( mi n ),
1940 TABLE of ( OUTFLOW STORAGE ) values
1941 ( cms ) - ( ha - m )
1942 [ 0.0 , 0.0 ]
1943 [ 0.068 , 0.001 ]
1944 [ 0.271 , 0.022 ]
1945 [ 0.379 , 0.051 ]
1946 [ 0.48 , 0.091 ]
1947 [ 0.853 , 0.341 ]
1948 [ 1.005 , 0.61 ]
1949 [ 1.128 , 1.231 ]
1950 [ 1.155 , 1.592 ]
1951 [ 1.194 , 1.876 ]
1952 [ 1.2 , 1.921 ]
1953 [ 1.259 , 2.369 ]
1954 [ 1.3 , 2.665 ]
1955 [ 1.349 , 2.813 ]
1956 [ -1 , -1 ] (max twenty pts)
1957 NHYDovf=[ "P10-OVF" ]
1958 *%-----|-----|
1959 * -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
1960 CONTINUOUS STANDHYD NHYD=[ "S-1-FO-D2" ], DT=[ 1 ] mi n, AREA=[ 4.94 ] ( ha ),
1961 XI MP=[ 0.55 ], TI MP=[ 0.55 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
1962 SCS curve number CN=[ 74 ],
1963 Pervious surfaces: I A per=[ 4.67 ] ( mm ), SLPP=[ 0.5 ] ( % ),
1964 LGP=[ 40 ] ( m ), MNP=[ 0.25 ], SCP=[ 0 ] ( mi n ),
1965 Impervious surfaces: I A i mp=[ 1.57 ] ( mm ), SLPI=[ 0.5 ] ( % ),
1966 LGI=[ 181.475 ] ( m ), MNI=[ 0.013 ], SCI=[ 0 ] ( mi n ),
1967 Continuous simulation parameters:
1968 I a REC per=[ 4 ] ( hr s ), I a REC i mp=[ 4 ] ( hr s ),
1969 SM N=[ -1 ] ( mm ), SMAX=[ -1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
1970 Inter Event Ti me=[ 18 ] ( hr s ), END=- 1
1971 *%-----|-----|
1972 *CONTINUOUS NASHYD NHYD=[ "S-1-FO-D2" ], DT=[ 1 ] mi n, AREA=[ 4.94 ] ( ha ),
1973 * DWF=[ 0 ] ( cms ), CNV C=[ 77 ], I A=[ 4.67 ] ( mm ),
1974 * N=[ 3 ], TP=[ 1.10 ] hr s,
1975 * Continuous simulation parameters:
1976 * I a REC per=[ 4 ] ( hr s ),
1977 * SM N=[ -1 ] ( mm ), SMAX=[ -1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
1978 * Inter Event Ti me=[ 12 ] ( hr s )
1979 * Baseflow simulation parameters:
1980 * BaseFlowOption=[ 1 ] ,
1981 * I ni t GWRes Vol =[ 50 ] ( mm ), GWRes K=[ 0.96 ] ( mm / day / mm )
1982 * VHydCond=[ 0.055 ] ( mm / hr ), END=- 1
1983 *%-----|-----|
1984 *COMPUTE DUALHYD NHYDin=[ "S-1-FO-D2" ], CI NLET=[ 0.508 ] ( cms ), NI NLET=[ 1 ],
1985 * M a j NHYD=[ "S-1-FO-D2J" ]
1986 * M nNHYD=[ "S-1-FO-D2N" ]
1987 * TM I STO=[ 9999999 ] ( cu - m )
1988 *%-----|-----|
1989 *ADD HYD NHYDs um=[ "S-1-FO-D2S" ], NHYDs to add=[ "S-1-FO-D2J"+"S-1-FO-D2N" ]

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```

1990 *%-----|-----|
1991 *ROUTE RESERVOIR      NHYDout=["S-1-FO-D2R"] , NHYDin=["S-1-FO-D2S"] ,
1992 *                      RDT=[1](min),
1993 *                      TABLE of ( OUTFLOW STORAGE ) values
1994 *                      (cms) - (ha-m)
1995 *                      [ 0.0      , 0.0 ]
1996 *                      [ 0.0590, 0.1970 ]
1997 *                      [   -1   ,  -1   ] (max twenty pts)
1998 *                      NHYDovf=["S-1FOD2ovf"]
1999 *%-----|-----|
2000 ADD HYD              NHYDsum=["980"], NHYDsto add=["FOSTER-OUT"+"S-1-FO-D2"]
2001 *%-----|-----|
2002 SAVE HYD            NHYD=["980"], # OF PCYCLES=[-1], ICASEsh=[1]
2003                    HYD_COMMENT=["Total Flows at Station 980 on Foster Drain"]
2004 *%-----|-----|
2005 *#
2006 *# Hydrograph from Node Foster SWM (Station 980) to Node at station 520
2007 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 980
2008 *#
2009 ROUTE CHANNEL      NHYDout=["980-out"] , NHYDin=["980"] ,
2010                    RDT=[1](min),
2011                    CHLGTH=[460](m),  CHSLOPE=[0.04348](%),
2012                    FPSLOPE=[0.04348](%),
2013                    SECNUM=[1.0],    NSEG=[3]
2014                    ( SEGROUGH, SEGDI ST (m))=
2015                    [ 0.050, 45.90
2016                    -0.035, 53.30
2017                    0.050, 100] NSEG times
2018                    ( DISTANCE (m), ELEVATION (m))=
2019                    [ 0, 91.75 ]
2020                    [ 42.4, 92.18 ]
2021                    [ 43.5, 92.16 ]
2022                    [ 44.1, 92.1 ]
2023                    [ 44.6, 92 ]
2024                    [ 44.8, 91.86 ]
2025                    [ 45.9, 91.04 ]
2026                    [ 46.4, 90.65 ]
2027                    [ 46.8, 90.36 ]
2028                    [ 47.9, 90.32 ]
2029                    [ 48.7, 90.35 ]
2030                    [ 50.7, 90.33 ]
2031                    [ 52.2, 90.38 ]
2032                    [ 52.5, 90.59 ]
2033                    [ 53.3, 91.28 ]
2034                    [ 54, 91.83 ]
2035                    [ 54.3, 92 ]
2036                    [ 54.8, 92.08 ]
2037                    [ 55.4, 92.12 ]
2038                    [ 100, 91.84 ]
2039 *%-----|-----|
2040 * -JFSA 2021-02-26 "S-1-FO-DI" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2041 CONTINUOUS STANDHYD NHYD=["S-1-FO-DI"], DT=[1]min, AREA=[5.11](ha),
2042                    XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
2043                    SCS curve number CN=[74],
2044                    Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
2045                    LGP=[40](m), MNP=[0.25], SCP=[0](min),
2046                    Impervious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
2047                    LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2048                    Continuous simulation parameters:
2049                    I a REC per=[4](hrs), I a REC i mp=[4](hrs),
2050                    SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2051                    Inter Event Time=[18](hrs), END=-1
2052 *%-----|-----|
2053 *COMPUTE DUALHYD    NHYDin=["S-1-FO-DI"], CNILET=[0.605](cms), NI NLET=[1],
2054 *                    Mij NHYD=["S-1-FO-DIJ"]

```



```

2055 * M nNHYD=[ "S- 1- FO- DIN" ]
2056 * TM STO=[ 9999999 ] ( cu- m)
2057 *%-----|-----|
2058 *ADD HYD NHYDsum=[ "S- 1- FO- DIS" ], NHYDs to add=[ "S- 1- FO- DIN" + "S- 1- FO- DIJ" ]
2059 *%-----|-----|
2060 *ROUTE RESERVOIR NHYDout=[ "S- 1- FO- DIR" ] , NHYDin=[ "S- 1- FO- DIS" ] ,
2061 * RDT=[ 1 ] ( mi n) ,
2062 * TABLE of ( OUTFLOW STORAGE ) values
2063 * ( cms ) - ( ha- m)
2064 * [ 0. 0 , 0. 0 ]
2065 * [ 0. 0611, 0. 2038 ]
2066 * [ -1 , -1 ] (max twenty pts)
2067 * NHYDovf=[ "S- 1FODlovf" ]
2068 *%-----|-----|
2069 ADD HYD NHYDsum=[ "520" ], NHYDs to add=[ "980- out " + "S- 1- FO- DI" ]
2070 *%-----|-----|
2071 SAVE HYD NHYD=[ "520" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
2072 HYD_COMMENT=[ "Total Flows at Sation 520 on Foster Drain" ]
2073 *%-----|-----|
2074 *# Hydrograph from Node at Station 520 (Foster Drain) to Node at station 6016 (Jock
River)
2075 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 520
2076 *#
2077 ROUTE CHANNEL NHYDout = [ "520- out " ] , NHYDin = [ "520" ] ,
2078 RDT = [ 1 ] ( mi n) ,
2079 CHLGTH = [ 860 ] ( m) , CHSLOPE = [ 0. 5872 ] ( % ) ,
2080 FPSLOPE = [ 0. 5872 ] ( % ) ,
2081 SECNUM = [ 1. 0 ] , NSEG = [ 3 ]
2082 ( SEGROUGH, SEGDI ST ( m) ) =
2083 [ 0. 050, 45. 90
2084 - 0. 035, 54. 3
2085 0. 050, 100. 1097 ] NSEG times
2086 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
2087 [ 0 , 91. 26 ]
2088 [ 44. 9 , 91. 46 ]
2089 [ 45. 1 , 91. 37 ]
2090 [ 45. 9 , 90. 84 ]
2091 [ 47 , 90. 32 ]
2092 [ 47. 5 , 90. 22 ]
2093 [ 48 , 90. 17 ]
2094 [ 50. 7 , 90. 19 ]
2095 [ 51. 5 , 90. 17 ]
2096 [ 52. 2 , 90. 13 ]
2097 [ 52. 7 , 90. 12 ]
2098 [ 53. 3 , 90. 14 ]
2099 [ 53. 5 , 90. 31 ]
2100 [ 53. 9 , 90. 59 ]
2101 [ 54. 3 , 90. 87 ]
2102 [ 54. 7 , 91. 04 ]
2103 [ 55. 3 , 91. 24 ]
2104 [ 55. 5 , 91. 26 ]
2105 [ 63. 7 , 91. 37 ]
2106 [ 100. 1097 , 91. 43 ]
2107 *%-----|-----|
2108 * -JFSA 2021-02-26 "S- 1- FO- F- D" is a part of S- 1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2109 CONTINUOUS STANDHYD NHYD=[ "S- 1- FO- F- D" ], DT=[ 1 ] mi n, AREA=[ 14. 96 ] ( ha) ,
2110 XI MP=[ 0. 65 ] , TI MP=[ 0. 65 ] , DWF=[ 0 ] ( cms ) , LOSS=[ 2 ] ,
2111 SCS curve number CN=[ 74 ] ,
2112 Pervious surfaces: I A per = [ 4. 67 ] ( mm) , SLPP = [ 0. 5 ] ( % ) ,
2113 LGP = [ 40 ] ( m) , MNP = [ 0. 25 ] , SCP = [ 0 ] ( mi n) ,
2114 Impervious surfaces: I A i mp = [ 1. 57 ] ( mm) , SLPI = [ 0. 5 ] ( % ) ,
2115 LGI = [ 315. 806 ] ( m) , MNI = [ 0. 013 ] , SCI = [ 0 ] ( mi n) ,
2116 Continuous simulation parameters:
2117 I a REC per = [ 4 ] ( hr s) , I a REC i mp = [ 4 ] ( hr s) ,
2118 SM N = [ - 1 ] ( mm) , SMAX = [ - 1 ] ( mm) , SK = [ 0. 010 ] / ( mm) ,

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2119      InterEventTime=[ 18](hrs),      END=- 1
2120  *%-----|-----|
2121  *CONTINUOUS NASHYD  NHYD=["S-1-FO-F-D"], DT=[ 1]min, AREA=[ 14.96](ha),
2122  *                   DWF=[ 0](cms), CNVC=[ 77], IA=[ 4.67](mm),
2123  *                   N=[ 3], TP=[ 1.007]hrs,
2124  *                   Continuous simulation parameters:
2125  *                   IaRECper=[ 4](hrs),
2126  *                   SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2127  *                   InterEventTime=[ 12](hrs)
2128  *                   Baseflow simulation parameters:
2129  *                   BaseFlowOption=[ 1],
2130  *                   InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2131  *                   VHydCond=[ 0.055](mm/hr),      END=- 1
2132  *%-----|-----|
2133  *COMPUTE DUALHYD   NHYDin=["S-1-FO-F-D"], CINLET=[ 1.749](cms), NINLET=[ 1],
2134  *                   MajNHYD=["S-1FO-F-DJ"]
2135  *                   MnNHYD=["S-1FO-F-DN"]
2136  *                   TMSSTO=[ 9999999](cu-m)
2137  *%-----|-----|
2138  *ADD HYD            NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2139  *%-----|-----|
2140  *ROUTE RESERVOIR  NHYDout=["S-1FO-F-DR"], NHYDin=["S-1FO-F-DS"],
2141  *                   RDT=[ 1](min),
2142  *                   TABLE of ( OUTFLOW STORAGE ) values
2143  *                   ( cms ) - ( ha-m )
2144  *                   [ 0.0      , 0.0 ]
2145  *                   [ 0.1788, 0.5966 ]
2146  *                   [ -1      , -1 ] (max twenty pts)
2147  *                   NHYDovf=["S-1FoFDovf"]
2148  *%-----|-----|
2149  * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2150  CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[ 1]min, AREA=[ 5.27](ha), XI MP=[ 0.325],
TI MP=[ 0.65], DWF=[ 0](cms), LOSS=[ 1]:
2151  Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
2152  Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](min),
2153  Impervious areas: IAimp=[ 0.785](mm), SLPI=[ 0.75](%),
LGI=[ 187.439](m), MNI=[ 0.013], SCI=[ 0](min),
2154  Continuous simulation parameters:
2155  IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
END=- 1
2156  *%-----|-----|
2157  *CONTINUOUS NASHYD  NHYD=["S-1-D8"], DT=[ 1]min, AREA=[ 5.27](ha),
2158  *                   DWF=[ 0](cms), CNVC=[ 77], IA=[ 4.67](mm),
2159  *                   N=[ 3], TP=[ 1.10]hrs,
2160  *                   Continuous simulation parameters:
2161  *                   IaRECper=[ 4](hrs),
2162  *                   SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2163  *                   InterEventTime=[ 12](hrs)
2164  *                   Baseflow simulation parameters:
2165  *                   BaseFlowOption=[ 1],
2166  *                   InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2167  *                   VHydCond=[ 0.055](mm/hr),      END=- 1
2168  *%-----|-----|
2169  *COMPUTE DUALHYD   NHYDin=["S-1-D8"], CINLET=[ 2.279](cms), NINLET=[ 1],
2170  *                   MajNHYD=["S-1-D8J"]
2171  *                   MnNHYD=["S-1-D8N"]
2172  *                   TMSSTO=[ 9999999](cu-m)
2173  *%-----|-----|
2174  *ADD HYD            NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2175  *%-----|-----|
2176  *ADD HYD            NHYDsum=["S-1-D"], NHYDs to add=["S-1-Okeefe"+"S-1"+"S-1-Fost"]
2177  *%-----|-----|
2178  *COMPUTE DUALHYD   NHYDin=["S-1-D"], CINLET=[ 11.616](cms), NINLET=[ 1],

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2179 *           Mj NHYD=[ " S- 1- D- M " ]
2180 *           MnNHYD=[ " S- 1- D- MN" ]
2181 *           TM STO=[ 5974 ] ( cu- m)
2182 *%-----|-----
2183 *ADD HYD           NHYDs um=[ " S- 1- DEV" ] , NHYDs t o add=[ " S- 1- D- M" +" S- 1- D- MN" ]
2184 *%-----|-----
2185 *ROUTE RESERVOIR NHYDout =[ " S- 1- D8R" ] , NHYDin =[ " S- 1- D8S" ] ,
2186 *           RDT=[ 1 ] ( mi n) ,
2187 *           TABLE of ( OUTFLOW STORAGE ) values
2188 *                   ( cms ) - ( ha- m)
2189 *                   [ 0. 0      , 0. 0 ]
2190 *                   [ 0. 0630, 0. 2102 ]
2191 *                   [      -1 ,      -1 ] (max twenty pts)
2192 *           NHYDovf=[ " S- 1- D8Rovf" ]
2193 *%-----|-----
2194 * -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2195 CONTINUOUS NASHYD NHYD=[ " S- 1- A" ] , DT=[ 1 ] mi n, AREA=[ 75. 88 ] ( ha) ,
2196 DWF=[ 0 ] ( cms ) , CN C=[ 77 ] , IA=[ 4. 67 ] ( mm) ,
2197 N=[ 3 ] , TP=[ 0. 619 ] hr s ,
2198 Continuous simulation parameters:
2199 IaRECper=[ 4 ] ( hr s) ,
2200 SM N=[ - 1 ] ( mm) , SMAX=[ - 1 ] ( mm) , SK=[ 0. 010 ] / ( mm) ,
2201 InterEvent Time=[ 12 ] ( hr s)
2202 Baseflow simulation parameters:
2203 BaseFlowOption=[ 1 ] ,
2204 Init GWRes Vol =[ 50 ] ( mm) , GWRes K=[ 0. 96 ] ( mm/ day/ mm)
2205 VHydCond=[ 0. 055 ] ( mm/ hr) , END=- 1
2206 *%-----|-----
2207 * -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2208 CONTINUOUS NASHYD NHYD=[ " W_CLAR_UNDE" ] , DT=[ 1 ] mi n, AREA=[ 35. 65 ] ( ha) ,
2209 DWF=[ 0 ] ( cms ) , CN C=[ 77 ] , IA=[ 4. 67 ] ( mm) ,
2210 N=[ 3 ] , TP=[ 1. 10 ] hr s ,
2211 Continuous simulation parameters:
2212 IaRECper=[ 4 ] ( hr s) ,
2213 SM N=[ - 1 ] ( mm) , SMAX=[ - 1 ] ( mm) , SK=[ 0. 010 ] / ( mm) ,
2214 InterEvent Time=[ 12 ] ( hr s)
2215 Baseflow simulation parameters:
2216 BaseFlowOption=[ 1 ] ,
2217 Init GWRes Vol =[ 50 ] ( mm) , GWRes K=[ 0. 96 ] ( mm/ day/ mm)
2218 VHydCond=[ 0. 055 ] ( mm/ hr) , END=- 1
2219 *%-----|-----
2220 ADD HYD           NHYDs um=[ " SN_FO" ] , NHYDs t o
add=[ " N_FO" +" 520- out " +" MS_P10" +" P10- OVF" +" W_CLAR_UNDE" +" S- 1- FO- F- D" +" S- 1- D8" +" S- 1- A" ]
2221 *%-----|-----
2222 SAVE HYD          NHYD=[ " SN_FO" ] , # OF PCYCLES=[ - 1 ] , I CASEs h=[ 1 ]
2223 HYD_COMMENT=[ " Total Flows at Foster Drain" ]
2224 *%-----|-----
2225 *# Hydrograph from Node Foster routed to Node at Cedarview Road
2226 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
2227 *#
2228 ROUTE CHANNEL    NHYDout =[ " N_CE" ] , NHYDin =[ " SN_FO" ] ,
2229 RDT=[ 1 ] ( mi n) ,
2230 CHLGTH=[ 159 ] ( m) , CHSLOPE=[ 0. 0818 ] ( % ) ,
2231 FPSLOPE=[ 0. 0818 ] ( % ) ,
2232 SECNUM=[ 1. 0 ] , NSEG=[ 3 ]
2233 ( SEGROUGH, SEGDI ST ( m) ) =
2234 [ 0. 050, - 15. 46
2235 - 0. 035, 26. 55
2236 0. 050, 116. 76 ] NSEG times
2237 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
2238 [ - 645. 23, 91. 50 ]
2239 [ - 391. 20, 91. 50 ]
2240 [ - 91. 00, 91. 50 ]

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2241          [- 85.52, 91.50]
2242          [- 15.46, 89.40]
2243          [- 9.79, 89.31]
2244          [- 3.22, 86.24]
2245          [ 3.22, 85.07]
2246          [ 10.96, 85.79]
2247          [ 16.44, 86.49]
2248          [ 26.55, 89.45]
2249          [ 29.03, 90.27]
2250          [ 35.76, 90.67]
2251          [ 36.67, 91.00]
2252          [ 108.08, 91.00]
2253          [ 109.82, 90.50]
2254          [ 112.04, 90.50]
2255          [ 114.62, 91.00]
2256          [ 116.76, 91.50]
2257  *%-----|-----|
2258  *#*****|
2259  *#      Catchment S-1
2260  *#      - To Jock River (north and south of Jock)
2261  *#      - Primarily agricultural fields; portion of sand quarry
2262  *%-----|-----|
2263  *%      -2020-12-17 "S-1-Undev" and "S-1-Fost" was a part of Foster drain, they are below
the foster pond. Now they are added to S-1 subcatchment based on Project 1474-BCDC,
JFSA, Nov. 2020
2264  *%      -2020-12-17 Change O-14 (it was part of Okeefe drain) to "S-1-Okeefe" and add it
to S-1 subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
2265  *%      -2020-12-17 Add "S-1-BCDC" as NASHYD
2266  *%      -2020-12-17 all other S-1 subcatchment as STANDHYD with DUALHYD and ROUTE RESERVOIR
2267  *%-----|-----|
2268  *#*****|
2269  *      -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2270  *CONTINUOUS NASHYD  NHYD=["S-1-A"], DT=[ 1] mi n, AREA=[ 75.88] (ha),
2271  *                   DWF=[ 0] (cms), CNVC=[ 77], IA=[ 4.67] (mm),
2272  *                   N=[ 3], TP=[ 0.619] hrs,
2273  *                   Continuous simulation parameters:
2274  *                   IaRECper=[ 4] (hrs),
2275  *                   SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
2276  *                   InterEventTime=[ 12] (hrs)
2277  *                   Baseflow simulation parameters:
2278  *                   BaseFlowOption=[ 1] ,
2279  *                   InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
2280  *                   VHydCond=[ 0.055] (mm/ hr), END=- 1
2281  *%-----|-----|
2282  CONTINUOUS NASHYD  NHYD=["S-1-B"], DT=[ 1] mi n, AREA=[ 55.36] (ha),
2283  *                   DWF=[ 0] (cms), CNVC=[ 77], IA=[ 4.67] (mm),
2284  *                   N=[ 3], TP=[ 0.451] hrs,
2285  *                   Continuous simulation parameters:
2286  *                   IaRECper=[ 4] (hrs),
2287  *                   SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
2288  *                   InterEventTime=[ 12] (hrs)
2289  *                   Baseflow simulation parameters:
2290  *                   BaseFlowOption=[ 1] ,
2291  *                   InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
2292  *                   VHydCond=[ 0.055] (mm/ hr), END=- 1
2293  *%-----|-----|
2294  *#      - JFSA 2021-02-24 change the name from S-1-BCDC to S-1-A and S-1-B. Change their
TP values based on the new areas compared to the old ones.
2295  *CONTINUOUS NASHYD  NHYD=["S-1-BCDC"], DT=[ 1] mi n, AREA=[ 134.9] (ha),
2296  *                   DWF=[ 0] (cms), CNVC=[ 77], IA=[ 4.67] (mm),
2297  *                   N=[ 3], TP=[ 1.10] hrs,
2298  *                   Continuous simulation parameters:
2299  *                   IaRECper=[ 4] (hrs),
2300  *                   SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
2301  *                   InterEventTime=[ 12] (hrs)

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2302 *          Baseflow simulation parameters:
2303 *          BaseFlowOption=[ 1] ,
2304 *          InitGWResVol =[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2305 *          VHydCond=[ 0.055](mm/hr) ,    END=- 1
2306 *%-----|-----
2307 *# - JFSA 2021-02-24 "S-1-BCDC-1" and "S-1-BCDC-2" are not existing anymore.
      "S-1-BCDC-1" is part of "S-1-FO-D2" and "S-1-BCDC-2" is part of "S-1-D2" and "S-1-D3"
2308 *CONTINUOUS NASHYD  NHYD=["S-1-BCDC-1"], DT=[ 1]min, AREA=[ 0.3](ha),
2309 *          DWF=[ 0](cms) , CN/C=[ 77] , IA=[ 4.67](mm) ,
2310 *          N=[ 3] , TP=[ 1.10]hrs ,
2311 *          Continuous simulation parameters:
2312 *          IaRECper=[ 4](hrs) ,
2313 *          SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2314 *          InterEventTime=[ 12](hrs)
2315 *          Baseflow simulation parameters:
2316 *          BaseFlowOption=[ 1] ,
2317 *          InitGWResVol =[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2318 *          VHydCond=[ 0.055](mm/hr) ,    END=- 1
2319 *%-----|-----
2320 *CONTINUOUS NASHYD  NHYD=["S-1-BCDC-2"], DT=[ 1]min, AREA=[ 1.3](ha),
2321 *          DWF=[ 0](cms) , CN/C=[ 77] , IA=[ 4.67](mm) ,
2322 *          N=[ 3] , TP=[ 1.10]hrs ,
2323 *          Continuous simulation parameters:
2324 *          IaRECper=[ 4](hrs) ,
2325 *          SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2326 *          InterEventTime=[ 12](hrs)
2327 *          Baseflow simulation parameters:
2328 *          BaseFlowOption=[ 1] ,
2329 *          InitGWResVol =[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2330 *          VHydCond=[ 0.055](mm/hr) ,    END=- 1
2331 *%-----|-----
2332 *# - JFSA 2021-01-19, after adding Greenbank pond, "S-1-BCDC-3" is not existing
      anymore
2333 *CONTINUOUS NASHYD  NHYD=["S-1-BCDC-3"], DT=[ 1]min, AREA=[ 3.9](ha),
2334 *          DWF=[ 0](cms) , CN/C=[ 77] , IA=[ 4.67](mm) ,
2335 *          N=[ 3] , TP=[ 1.10]hrs ,
2336 *          Continuous simulation parameters:
2337 *          IaRECper=[ 4](hrs) ,
2338 *          SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2339 *          InterEventTime=[ 12](hrs)
2340 *          Baseflow simulation parameters:
2341 *          BaseFlowOption=[ 1] ,
2342 *          InitGWResVol =[ 50](mm) , GWResK=[ 0.96](mm/day/mm)
2343 *          VHydCond=[ 0.055](mm/hr) ,    END=- 1
2344 *%-----|-----
2345 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
      before station 7245 on Jock River
2346 *CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[ 1](min) , AREA=[ 44.93](ha) , XI MP=[ 0.65] ,
      TI MP=[ 0.65] , DWF=[ 0](cms) ,
2347 *          LOSS=[ 2] , SCS curve number CN=[ 75] , Pervious surfaces:
      IAper=[ 4.67](mm) , SLPP=[ 2.0](%) ,
2348 *          LGP=[ 40](m) , MNP=[ 0.25] , SCP=[ 0](min) , Impervious surfaces:
      IAimp=[ 1.57](mm) , SLPI=[ 0.75](%) ,
2349 *          LGI=[ 547.296](m) , MNI=[ 0.013] , SCI=[ 0](min) ,
2350 *          Continuous simulation parameters:
2351 *          IaRECper=[ 4](hrs) , IaRECimp=[ 4](hrs) ,
2352 *          SMN=[ -1](mm) , SMAX=[ -1](mm) , SK=[ 0.010]/(mm) ,
2353 *          InterEventTime=[ 12](hrs) ,    END=- 1
2354 *%-----|-----
2355 *COMPUTE DUALHYD  NHYDin=["S-1-Okeefe"], CINLET=[ 4.796](cms) , NINLET=[ 1] ,
2356 *          MajNHYD=["S-1-OkM"]
2357 *          MnNHYD=["S-1-OkMN"]
2358 *          TMS TO=[ 9999999](cu-m)
2359 *%-----|-----
2360 *ADD HYD          NHYDs um=["S-1-OkS"] , NHYDs to add=["S-1-OkM"+"S-1-OkMN"]
2361 *%-----|-----

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2362 *ROUTE RESERVOIR      NHYDout=[ "S-1-OkSR" ] , NHYDin=[ "S-1-OkS" ] ,
2363 *                      RDT=[ 1 ] (mi n),
2364 *                      TABLE of ( OUTFLOW STORAGE ) values
2365 *                      ( cms ) - ( ha-m)
2366 *                      [ 0.0      , 0.0 ]
2367 *                      [ 0.5370, 1.7917 ]
2368 *                      [ -1      , -1      ] (max twenty pts)
2369 *                      NHYDovf=[ "S-1-OkSovf" ]
2370 *%-----|-----
2371 *CONTINUOUS NASHYD     NHYD=[ "S-1-Okeefe" ], DT=[ 1 ] mi n, AREA=[ 44.93 ] (ha),
2372 *                      DWF=[ 0 ] (cms), CN C=[ 77 ], IA=[ 4.67 ] (mm),
2373 *                      N=[ 3 ], TP=[ 1.049 ] hrs,
2374 *                      Continuous simulation parameters:
2375 *                      IaRECper=[ 4 ] (hrs),
2376 *                      SMN=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2377 *                      InterEventTime=[ 12 ] (hrs)
2378 *                      Baseflow simulation parameters:
2379 *                      BaseFlowOption=[ 1 ] ,
2380 *                      InitGWResVol=[ 50 ] (mm), GWResK=[ 0.96 ] (mm/ day/ mm)
2381 *                      VHydCond=[ 0.055 ] (mm/ hr), END=- 1
2382 *%-----|-----
2383 * -JFSA 2021-02-26 "S-1-FO-DI" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2384 *CONTINUOUS STANDHYD NHYD=[ "S-1-FO-DI" ], DT=[ 1 ] mi n, AREA=[ 5.11 ] (ha),
2385 *                      XI MP=[ 0.65 ], TI MP=[ 0.65 ], DWF=[ 0 ] (cms), LOSS=[ 2 ],
2386 *                      SCS curve number CN=[ 74 ],
2387 *                      Pervious surfaces: IAper=[ 4.67 ] (mm), SLPP=[ 0.5 ] ( % ),
2388 *                      LGP=[ 40 ] (m), MNP=[ 0.25 ], SCP=[ 0 ] (mi n),
2389 *                      Impervious surfaces: IAimp=[ 1.57 ] (mm), SLPI=[ 0.5 ] ( % ),
2390 *                      LGI=[ 184.572 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),
2391 *                      Continuous simulation parameters:
2392 *                      IaRECper=[ 4 ] (hrs), IaRECImp=[ 4 ] (hrs),
2393 *                      SMN=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2394 *                      InterEventTime=[ 18 ] (hrs), END=- 1
2395 *%-----|-----
2396 *COMPUTE DUALHYD      NHYDin=[ "S-1-FO-DI" ], CINLET=[ 0.605 ] (cms), NINLET=[ 1 ],
2397 *                      MajNHYD=[ "S-1-FO-DIJ" ]
2398 *                      MnNHYD=[ "S-1-FO-DIN" ]
2399 *                      TMSTO=[ 9999999 ] (cu-m)
2400 *%-----|-----
2401 *ADD HYD               NHYDsum=[ "S-1-FO-DIS" ], NHYDsto add=[ "S-1-FO-DIN"+"S-1-FO-DIJ" ]
2402 *%-----|-----
2403 *ROUTE RESERVOIR      NHYDout=[ "S-1-FO-DIR" ] , NHYDin=[ "S-1-FO-DIS" ] ,
2404 *                      RDT=[ 1 ] (mi n),
2405 *                      TABLE of ( OUTFLOW STORAGE ) values
2406 *                      ( cms ) - ( ha-m)
2407 *                      [ 0.0      , 0.0 ]
2408 *                      [ 0.0611, 0.2038 ]
2409 *                      [ -1      , -1      ] (max twenty pts)
2410 *                      NHYDovf=[ "S-1FODlovf" ]
2411 *%-----|-----
2412 *CONTINUOUS NASHYD     NHYD=[ "S-1-FO-DI" ], DT=[ 1 ] mi n, AREA=[ 5.11 ] (ha),
2413 *                      DWF=[ 0 ] (cms), CN C=[ 77 ], IA=[ 4.67 ] (mm),
2414 *                      N=[ 3 ], TP=[ 1.10 ] hrs,
2415 *                      Continuous simulation parameters:
2416 *                      IaRECper=[ 4 ] (hrs),
2417 *                      SMN=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2418 *                      InterEventTime=[ 12 ] (hrs)
2419 *                      Baseflow simulation parameters:
2420 *                      BaseFlowOption=[ 1 ] ,
2421 *                      InitGWResVol=[ 50 ] (mm), GWResK=[ 0.96 ] (mm/ day/ mm)
2422 *                      VHydCond=[ 0.055 ] (mm/ hr), END=- 1
2423 *%-----|-----
2424 * -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
2425 *CONTINUOUS STANDHYD NHYD=[ "S-1-FO-D2" ], DT=[ 1 ] mi n, AREA=[ 4.94 ] (ha),

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2426 * XI MP=[ 0.55], TI MP=[ 0.55], DWF=[ 0](cms), LOSS=[ 2],
2427 * SCS curve number CN=[ 74],
2428 * Previous surfaces: IAper=[ 4.67](mm), SLPP=[ 0.5](%),
2429 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
2430 * Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 0.5](%),
2431 * LGI=[ 181.475](m), MNI=[ 0.013], SCI=[ 0](min),
2432 * Continuous simulation parameters:
2433 * IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
2434 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2435 * InterEventTime=[ 18](hrs), END=- 1
2436 %-----|-----
2437 *CONTINUOUS NASHYD NHYD=["S-1-FO-D2"], DT=[ 1]min, AREA=[ 4.94](ha),
2438 * DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
2439 * N=[ 3], TP=[ 1.10]hrs,
2440 * Continuous simulation parameters:
2441 * IaRECper=[ 4](hrs),
2442 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2443 * InterEventTime=[ 12](hrs)
2444 * Baseflow simulation parameters:
2445 * BaseFlowOption=[ 1],
2446 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2447 * VHydCond=[ 0.055](mm/hr), END=- 1
2448 %-----|-----
2449 *COMPUTE DUALHYD NHYDin=["S-1-FO-D2"], CINLET=[ 0.508](cms), NINLET=[ 1],
2450 * MajNHYD=["S-1-FO-D2J"]
2451 * MnNHYD=["S-1-FO-D2N"]
2452 * TMS TO=[ 9999999](cu-m)
2453 %-----|-----
2454 *ADD HYD NHYDsum=["S-1-FO-D2S"], NHYDsto add=["S-1-FO-D2J"+"S-1-FO-D2N"]
2455 %-----|-----
2456 *ROUTE RESERVOIR NHYDout=["S-1-FO-D2R"], NHYDin=["S-1-FO-D2S"],
2457 * RDT=[ 1](min),
2458 * TABLE of ( OUTFLOW STORAGE ) values
2459 * (cms) - (ha-m)
2460 * [ 0.0 , 0.0 ]
2461 * [ 0.0590, 0.1970 ]
2462 * [ -1 , -1 ] (max twenty pts)
2463 * NHYDovf=["S-1FOD2ovf"]
2464 %-----|-----
2465 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2466 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[ 1]min, AREA=[ 14.96](ha),
2467 * XI MP=[ 0.65], TI MP=[ 0.65], DWF=[ 0](cms), LOSS=[ 2],
2468 * SCS curve number CN=[ 74],
2469 * Previous surfaces: IAper=[ 4.67](mm), SLPP=[ 0.5](%),
2470 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
2471 * Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 0.5](%),
2472 * LGI=[ 315.806](m), MNI=[ 0.013], SCI=[ 0](min),
2473 * Continuous simulation parameters:
2474 * IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
2475 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2476 * InterEventTime=[ 18](hrs), END=- 1
2477 %-----|-----
2478 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[ 1]min, AREA=[ 14.96](ha),
2479 * DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
2480 * N=[ 3], TP=[ 1.007]hrs,
2481 * Continuous simulation parameters:
2482 * IaRECper=[ 4](hrs),
2483 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2484 * InterEventTime=[ 12](hrs)
2485 * Baseflow simulation parameters:
2486 * BaseFlowOption=[ 1],
2487 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2488 * VHydCond=[ 0.055](mm/hr), END=- 1
2489 %-----|-----
2490 *COMPUTE DUALHYD NHYDin=["S-1-FO-F-D"], CINLET=[ 1.749](cms), NINLET=[ 1],

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2491 *           Mj NHYD=[ "S- 1FO- F- DJ" ]
2492 *           MnNHYD=[ "S- 1FO- F- DN" ]
2493 *           TMI STO=[ 9999999 ] ( cu- m)
2494 *%-----|-----|
2495 *ADD HYD           NHYDs um=[ "S- 1FO- F- DS" ], NHYDs to add=[ "S- 1FO- F- DJ"+"S- 1FO- F- DN" ]
2496 *%-----|-----|
2497 *ROUTE RESERVOIR NHYDout=[ "S- 1FO- F- DR" ] , NHYDin=[ "S- 1FO- F- DS" ] ,
2498 *           RDT=[ 1 ] ( mi n) ,
2499 *           TABLE of ( OUTFLOW STORAGE ) values
2500 *                   ( cms ) - ( ha- m)
2501 *                   [ 0.0      , 0.0 ]
2502 *                   [ 0.1788, 0.5966 ]
2503 *                   [   -1   ,  -1   ] (max twenty pts)
2504 *           NHYDovf=[ "S- 1FoFDovf" ]
2505 *%-----|-----|
2506 *CONTINUOUS STANDHYD NHYD=[ "S- 1- D1" ], DT=[ 1 ] ( mi n) , AREA=[ 21.67 ] ( ha) , XI MP=[ 0.65 ] ,
TI MP=[ 0.65 ] , DWF=[ 0 ] ( cms) ,
2507 *           LOSS=[ 2 ] , SCS curve number CN=[ 75 ] , Pervious surfaces:
I Aper=[ 4.67 ] ( mm) , SLPP=[ 2.0 ] ( % ) ,
2508 *           LGP=[ 40 ] ( m) , MNP=[ 0.25 ] , SCP=[ 0 ] ( mi n) , Impervious surfaces:
I Ai mp=[ 1.57 ] ( mm) , SLPI=[ 0.75 ] ( % ) ,
2509 *           LGI=[ 380.088 ] ( m) , MNI=[ 0.013 ] , SCI=[ 0 ] ( mi n) ,
2510 *           Continuous simulation parameters:
2511 *           IaRECper=[ 4 ] ( hr s) , IaRECI mp=[ 4 ] ( hr s) ,
2512 *           SMN=[ -1 ] ( mm) , SMAX=[ -1 ] ( mm) , SK=[ 0.010 ] / ( mm) ,
2513 *           InterEvent Time=[ 12 ] ( hr s) , END=- 1
2514 *%-----|-----|
2515 *CONTINUOUS NASHYD NHYD=[ "S- 1- D1" ], DT=[ 1 ] mi n , AREA=[ 21.67 ] ( ha) ,
2516 *           DWF=[ 0 ] ( cms) , CN C=[ 77 ] , IA=[ 4.67 ] ( mm) ,
2517 *           N=[ 3 ] , TP=[ 1.066 ] hr s ,
2518 *           Continuous simulation parameters:
2519 *           IaRECper=[ 4 ] ( hr s) ,
2520 *           SMN=[ -1 ] ( mm) , SMAX=[ -1 ] ( mm) , SK=[ 0.010 ] / ( mm) ,
2521 *           InterEvent Time=[ 12 ] ( hr s)
2522 *           Baseflow simulation parameters:
2523 *           BaseFlowOption=[ 1 ] ,
2524 *           InitGWRes Vol=[ 50 ] ( mm) , GWRes K=[ 0.96 ] ( mm/ day/ mm)
2525 *           VHydCond=[ 0.055 ] ( mm/ hr ) , END=- 1
2526 *%-----|-----|
2527 *COMPUTE DUALHYD NHYDin=[ "S- 1- D1" ], CINLET=[ 2.482 ] ( cms) , NI NLET=[ 1 ] ,
2528 *           Mj NHYD=[ "S- 1- DIJ" ]
2529 *           MnNHYD=[ "S- 1- DIN" ]
2530 *           TMI STO=[ 9999999 ] ( cu- m)
2531 *%-----|-----|
2532 *ADD HYD           NHYDs um=[ "S- 1- DIS" ], NHYDs to add=[ "S- 1- DIJ"+"S- 1- DIN" ]
2533 *%-----|-----|
2534 *ROUTE RESERVOIR NHYDout=[ "S- 1- DIR" ] , NHYDin=[ "S- 1- DIS" ] ,
2535 *           RDT=[ 1 ] ( mi n) ,
2536 *           TABLE of ( OUTFLOW STORAGE ) values
2537 *                   ( cms ) - ( ha- m)
2538 *                   [ 0.0      , 0.0 ]
2539 *                   [ 0.2590, 0.8642 ]
2540 *                   [   -1   ,  -1   ] (max twenty pts)
2541 *           NHYDovf=[ "S- 1- DIRovf" ]
2542 *%-----|-----|
2543 * -JFSA 2021-02-25 "S- 1- D2" and "S- 1- D3" are part of S- 1 sub-catchment. They are
moved to drain before station 6215 on Jock River
2544 *CONTINUOUS STANDHYD NHYD=[ "S- 1- D2" ], DT=[ 1 ] ( mi n) , AREA=[ 18.67 ] ( ha) , XI MP=[ 0.65 ] ,
TI MP=[ 0.65 ] , DWF=[ 0 ] ( cms) ,
2545 *           LOSS=[ 2 ] , SCS curve number CN=[ 75 ] , Pervious surfaces:
I Aper=[ 4.67 ] ( mm) , SLPP=[ 2.0 ] ( % ) ,
2546 *           LGP=[ 40 ] ( m) , MNP=[ 0.25 ] , SCP=[ 0 ] ( mi n) , Impervious surfaces:
I Ai mp=[ 1.57 ] ( mm) , SLPI=[ 0.75 ] ( % ) ,
2547 *           LGI=[ 352.798 ] ( m) , MNI=[ 0.013 ] , SCI=[ 0 ] ( mi n) ,
2548 *           Continuous simulation parameters:
2549 *           IaRECper=[ 4 ] ( hr s) , IaRECI mp=[ 4 ] ( hr s) ,

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2550 * SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
2551 * InterEventTime=[ 12 ] ( hr s ) , END=- 1
2552 *%-----|-----
2553 *CONTINUOUS NASHYD NHYD=[ " S- 1- D2" ] , DT=[ 1 ] mi n , AREA=[ 18. 67 ] ( ha ) ,
2554 * DWF=[ 0 ] ( cms ) , CNVC=[ 77 ] , IA=[ 4. 67 ] ( mm ) ,
2555 * N=[ 3 ] , TP=[ 1. 120 ] hr s ,
2556 * Continuous simulation parameters:
2557 * IaRECper=[ 4 ] ( hr s ) ,
2558 * SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
2559 * InterEventTime=[ 12 ] ( hr s )
2560 * Baseflow simulation parameters:
2561 * BaseFlowOption=[ 1 ] ,
2562 * InitGWResVol=[ 50 ] ( mm ) , GWResK=[ 0. 96 ] ( mm/ day/ mm)
2563 * VHydCond=[ 0. 055 ] ( mm/ hr ) , END=- 1
2564 *%-----|-----
2565 *COMPUTE DUALHYD NHYDin=[ " S- 1- D2" ] , CINLET=[ 2. 062 ] ( cms ) , NINLET=[ 1 ] ,
2566 * MajNHYD=[ " S- 1- D2J" ]
2567 * MnNHYD=[ " S- 1- D2N" ]
2568 * TMSTO=[ 9999999 ] ( cu- m)
2569 *%-----|-----
2570 *ADD HYD NHYDsum=[ " S- 1- D2S" ] , NHYDsto add=[ " S- 1- D2J" + " S- 1- D2N" ]
2571 *%-----|-----
2572 *ROUTE RESERVOIR NHYDout=[ " S- 1- D2R" ] , NHYDin=[ " S- 1- D2S" ] ,
2573 * RDT=[ 1 ] ( mi n ) ,
2574 * TABLE of ( OUTFLOW STORAGE ) values
2575 * ( cms ) - ( ha- m)
2576 * [ 0. 0 , 0. 0 ]
2577 * [ 0. 2231 , 0. 7445 ]
2578 * [ - 1 , - 1 ] ( max twenty pts)
2579 * NHYDovf=[ " S- 1- D2Rovf" ]
2580 *%-----|-----
2581 *CONTINUOUS STANDHYD NHYD=[ " S- 1- D3" ] , DT=[ 1 ] ( mi n ) , AREA=[ 6. 79 ] ( ha ) , XI MP=[ 0. 65 ] ,
TI MP=[ 0. 65 ] , DWF=[ 0 ] ( cms ) ,
2582 * LOSS=[ 2 ] , SCS curve number CN=[ 75 ] , Pervious surfaces:
I Aper=[ 4. 67 ] ( mm ) , SLPP=[ 2. 0 ] ( % ) ,
2583 * LGP=[ 40 ] ( m ) , MNP=[ 0. 25 ] , SCP=[ 0 ] ( mi n ) , Impervious surfaces:
I Ai mp=[ 1. 57 ] ( mm ) , SLPI=[ 0. 75 ] ( % ) ,
2584 * LGI=[ 212. 760 ] ( m ) , MNI=[ 0. 013 ] , SCI=[ 0 ] ( mi n ) ,
2585 * Continuous simulation parameters:
2586 * IaRECper=[ 4 ] ( hr s ) , IaRECi mp=[ 4 ] ( hr s ) ,
2587 * SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
2588 * InterEventTime=[ 12 ] ( hr s ) , END=- 1
2589 *%-----|-----
2590 *CONTINUOUS NASHYD NHYD=[ " S- 1- D3" ] , DT=[ 1 ] mi n , AREA=[ 6. 79 ] ( ha ) ,
2591 * DWF=[ 0 ] ( cms ) , CNVC=[ 77 ] , IA=[ 4. 67 ] ( mm ) ,
2592 * N=[ 3 ] , TP=[ 1. 281 ] hr s ,
2593 * Continuous simulation parameters:
2594 * IaRECper=[ 4 ] ( hr s ) ,
2595 * SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
2596 * InterEventTime=[ 12 ] ( hr s )
2597 * Baseflow simulation parameters:
2598 * BaseFlowOption=[ 1 ] ,
2599 * InitGWResVol=[ 50 ] ( mm ) , GWResK=[ 0. 96 ] ( mm/ day/ mm)
2600 * VHydCond=[ 0. 055 ] ( mm/ hr ) , END=- 1
2601 *%-----|-----
2602 *COMPUTE DUALHYD NHYDin=[ " S- 1- D3" ] , CINLET=[ 0. 719 ] ( cms ) , NINLET=[ 1 ] ,
2603 * MajNHYD=[ " S- 1- D3J" ]
2604 * MnNHYD=[ " S- 1- D3N" ]
2605 * TMSTO=[ 9999999 ] ( cu- m)
2606 *%-----|-----
2607 *ADD HYD NHYDsum=[ " S- 1- D3S" ] , NHYDsto add=[ " S- 1- D3J" + " S- 1- D3N" ]
2608 *%-----|-----
2609 *ROUTE RESERVOIR NHYDout=[ " S- 1- D3R" ] , NHYDin=[ " S- 1- D3S" ] ,
2610 * RDT=[ 1 ] ( mi n ) ,
2611 * TABLE of ( OUTFLOW STORAGE ) values
2612 * ( cms ) - ( ha- m)

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2613 * [ 0.0 , 0.0 ]
2614 * [ 0.0811, 0.2708 ]
2615 * [ -1 , -1 ] (max twenty pts)
2616 * NHYDovf=[ "S-1-D3Rovf" ]
2617 *%-----|-----
2618 CONTINUOUS STANDHYD NHYD=[ "S-1-D4" ], DT=[ 1 ](mi n), AREA=[ 3.28 ](ha), XI MP=[ 0.65 ],
TI MP=[ 0.65 ], DWF=[ 0 ](cms),
2619 LOSS=[ 2 ], SCS curve number CN=[ 75 ], Pervious surfaces:
I Aper=[ 4.67 ](mm), SLPP=[ 2.0 ]( % ),
2620 LGP=[ 40 ](m), MNP=[ 0.25 ], SCP=[ 0 ](mi n), Impervious surfaces:
I Ai mp=[ 1.57 ](mm), SLPI=[ 0.75 ]( % ),
2621 LGI=[ 147.874 ](m), MNI=[ 0.013 ], SCI=[ 0 ](mi n),
2622 Continuous simulation parameters:
I aRECper=[ 4 ](hrs), I aRECI mp=[ 4 ](hrs),
2623 SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2624 SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2625 InterEventTime=[ 12 ](hrs), END=- 1
2626 *%-----|-----
2627 *CONTINUOUS NASHYD NHYD=[ "S-1-D4" ], DT=[ 1 ]mi n, AREA=[ 3.28 ](ha),
2628 * DWF=[ 0 ](cms), CN C=[ 77 ], I A=[ 4.67 ](mm),
2629 * N=[ 3 ], TP=[ 1.10 ]hrs,
2630 * Continuous simulation parameters:
2631 * I aRECper=[ 4 ](hrs),
2632 * SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2633 * InterEventTime=[ 12 ](hrs)
2634 * Baseflow simulation parameters:
2635 * BaseFlowOption=[ 1 ],
2636 * I nitGWResVol=[ 50 ](mm), GWResK=[ 0.96 ](mm/day/mm)
2637 * VHydCond=[ 0.055 ](mm/hr), END=- 1
2638 *%-----|-----
2639 *COMPUTE DUALHYD NHYDi n=[ "S-1-D4" ], CI NLET=[ 0.373 ](cms), NI NLET=[ 1 ],
2640 * Mi j NHYD=[ "S-1-D4J" ]
2641 * M nNHYD=[ "S-1-D4N" ]
2642 * TM I STO=[ 9999999 ](cu-m)
2643 *%-----|-----
2644 *ADD HYD NHYDs um=[ "S-1-D4S" ], NHYDs to add=[ "S-1-D4J"+"S-1-D4N" ]
2645 *%-----|-----
2646 *ROUTE RESERVOIR NHYDout=[ "S-1-D4R" ], NHYDi n=[ "S-1-D4S" ],
2647 * RDT=[ 1 ](mi n),
2648 * TABLE of ( OUTFLOW STORAGE ) values
2649 * (cms) - (ha-m)
2650 * [ 0.0 , 0.0 ]
2651 * [ 0.0392, 0.1308 ]
2652 * [ -1 , -1 ] (max twenty pts)
2653 * NHYDovf=[ "S-1-D4Rovf" ]
2654 *%-----|-----
2655 CONTINUOUS STANDHYD NHYD=[ "S-1-D5" ], DT=[ 1 ](mi n), AREA=[ 12.84 ](ha), XI MP=[ 0.65 ],
TI MP=[ 0.65 ], DWF=[ 0 ](cms),
2656 LOSS=[ 2 ], SCS curve number CN=[ 75 ], Pervious surfaces:
I Aper=[ 4.67 ](mm), SLPP=[ 2.0 ]( % ),
2657 LGP=[ 40 ](m), MNP=[ 0.25 ], SCP=[ 0 ](mi n), Impervious surfaces:
I Ai mp=[ 1.57 ](mm), SLPI=[ 0.75 ]( % ),
2658 LGI=[ 292.57 ](m), MNI=[ 0.013 ], SCI=[ 0 ](mi n),
2659 Continuous simulation parameters:
I aRECper=[ 4 ](hrs), I aRECI mp=[ 4 ](hrs),
2660 SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2661 SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2662 InterEventTime=[ 12 ](hrs), END=- 1
2663 *%-----|-----
2664 *CONTINUOUS NASHYD NHYD=[ "S-1-D5" ], DT=[ 1 ]mi n, AREA=[ 12.84 ](ha),
2665 * DWF=[ 0 ](cms), CN C=[ 77 ], I A=[ 4.67 ](mm),
2666 * N=[ 3 ], TP=[ 1.10 ]hrs,
2667 * Continuous simulation parameters:
2668 * I aRECper=[ 4 ](hrs),
2669 * SM N=[ -1 ](mm), SMAX=[ -1 ](mm), SK=[ 0.010 ]/(mm),
2670 * InterEventTime=[ 12 ](hrs)
2671 * Baseflow simulation parameters:
2672 * BaseFlowOption=[ 1 ],

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2673 *          I n i t G W R e s V o l =[ 50 ] ( m m ) ,   G W R e s K =[ 0. 96 ] ( m m / d a y / m m )
2674 *          V H y d C o n d =[ 0. 055 ] ( m m / h r ) ,   E N D = - 1
2675 *%-----|-----
2676 *COMPUTE DUALHYD  N H Y D i n =[ " S - 1 - D 5 " ] ,   C I N L E T =[ 1. 395 ] ( c m s ) ,   N I N L E T =[ 1 ] ,
2677 *          M a j N H Y D =[ " S - 1 - D 5 J " ]
2678 *          M n N H Y D =[ " S - 1 - D 5 N " ]
2679 *          T M I S T O =[ 9999999 ] ( c u - m )
2680 *%-----|-----
2681 *ADD HYD          N H Y D s u m =[ " S - 1 - D 5 S " ] ,   N H Y D s t o a d d =[ " S - 1 - D 5 J " + " S - 1 - D 5 N " ]
2682 *%-----|-----
2683 *ROUTE RESERVOIR N H Y D o u t =[ " S - 1 - D 5 R " ] ,   N H Y D i n =[ " S - 1 - D 5 S " ] ,
2684 *          R D T =[ 1 ] ( m i n ) ,
2685 *          T A B L E o f ( O U T F L O W S T O R A G E ) v a l u e s
2686 *          ( c m s ) - ( h a - m )
2687 *          [ 0. 0      , 0. 0 ]
2688 *          [ 0. 1535, 0. 5120 ]
2689 *          [ - 1      , - 1      ] ( m a x t w e n t y p t s )
2690 *          N H Y D o v f =[ " S - 1 - D 5 R o v f " ]
2691 *%-----|-----
2692 CONTINUOUS STANDHYD N H Y D =[ " S - 1 - D 6 " ] ,   D T =[ 1 ] ( m i n ) ,   A R E A =[ 1. 75 ] ( h a ) ,   X I M P =[ 0. 65 ] ,
T I M P =[ 0. 65 ] ,   D W F =[ 0 ] ( c m s ) ,
2693 LOSS =[ 2 ] ,   S C S c u r v e n u m b e r C N =[ 75 ] ,   P e r v i o u s s u r f a c e s :
I A p e r =[ 4. 67 ] ( m m ) ,   S L P P =[ 2. 0 ] ( % ) ,
2694 L G P =[ 40 ] ( m ) ,   M N P =[ 0. 25 ] ,   S C P =[ 0 ] ( m i n ) ,   I m p e r v i o u s s u r f a c e s :
I A i m p =[ 1. 57 ] ( m m ) ,   S L P I =[ 0. 75 ] ( % ) ,
2695 L G I =[ 108. 01 ] ( m ) ,   M N I =[ 0. 013 ] ,   S C I =[ 0 ] ( m i n ) ,
2696 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
2697 I a R E C p e r =[ 4 ] ( h r s ) ,   I a R E C i m p =[ 4 ] ( h r s ) ,
2698 S M N =[ - 1 ] ( m m ) ,   S M A X =[ - 1 ] ( m m ) ,   S K =[ 0. 010 ] / ( m m ) ,
2699 I n t e r E v e n t T i m e =[ 12 ] ( h r s ) ,   E N D = - 1
2700 *%-----|-----
2701 *CONTINUOUS NASHYD  N H Y D =[ " S - 1 - D 6 " ] ,   D T =[ 1 ] m i n ,   A R E A =[ 1. 75 ] ( h a ) ,
2702 *          D W F =[ 0 ] ( c m s ) ,   C N C =[ 77 ] ,   I A =[ 4. 67 ] ( m m ) ,
2703 *          N =[ 3 ] ,   T P =[ 1. 10 ] h r s ,
2704 *          C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
2705 *          I a R E C p e r =[ 4 ] ( h r s ) ,
2706 *          S M N =[ - 1 ] ( m m ) ,   S M A X =[ - 1 ] ( m m ) ,   S K =[ 0. 010 ] / ( m m ) ,
2707 *          I n t e r E v e n t T i m e =[ 12 ] ( h r s )
2708 *          B a s e f l o w s i m u l a t i o n p a r a m e t e r s :
2709 *          B a s e F l o w O p t i o n =[ 1 ] ,
2710 *          I n i t G W R e s V o l =[ 50 ] ( m m ) ,   G W R e s K =[ 0. 96 ] ( m m / d a y / m m )
2711 *          V H y d C o n d =[ 0. 055 ] ( m m / h r ) ,   E N D = - 1
2712 *%-----|-----
2713 *COMPUTE DUALHYD  N H Y D i n =[ " S - 1 - D 6 " ] ,   C I N L E T =[ 0. 218 ] ( c m s ) ,   N I N L E T =[ 1 ] ,
2714 *          M a j N H Y D =[ " S - 1 - D 6 J " ]
2715 *          M n N H Y D =[ " S - 1 - D 6 N " ]
2716 *          T M I S T O =[ 9999999 ] ( c u - m )
2717 *%-----|-----
2718 *ADD HYD          N H Y D s u m =[ " S - 1 - D 6 S " ] ,   N H Y D s t o a d d =[ " S - 1 - D 6 J " + " S - 1 - D 6 N " ]
2719 *%-----|-----
2720 *ROUTE RESERVOIR N H Y D o u t =[ " S - 1 - D 6 R " ] ,   N H Y D i n =[ " S - 1 - D 6 S " ] ,
2721 *          R D T =[ 1 ] ( m i n ) ,
2722 *          T A B L E o f ( O U T F L O W S T O R A G E ) v a l u e s
2723 *          ( c m s ) - ( h a - m )
2724 *          [ 0. 0      , 0. 0 ]
2725 *          [ 0. 0209, 0. 0698 ]
2726 *          [ - 1      , - 1      ] ( m a x t w e n t y p t s )
2727 *          N H Y D o v f =[ " S - 1 - D 6 R o v f " ]
2728 *%-----|-----
2729 CONTINUOUS STANDHYD N H Y D =[ " S - 1 - D 7 " ] ,   D T =[ 1 ] ( m i n ) ,   A R E A =[ 2. 03 ] ( h a ) ,   X I M P =[ 0. 65 ] ,
T I M P =[ 0. 65 ] ,   D W F =[ 0 ] ( c m s ) ,
2730 LOSS =[ 2 ] ,   S C S c u r v e n u m b e r C N =[ 75 ] ,   P e r v i o u s s u r f a c e s :
I A p e r =[ 4. 67 ] ( m m ) ,   S L P P =[ 2. 0 ] ( % ) ,
2731 L G P =[ 40 ] ( m ) ,   M N P =[ 0. 25 ] ,   S C P =[ 0 ] ( m i n ) ,   I m p e r v i o u s s u r f a c e s :
I A i m p =[ 1. 57 ] ( m m ) ,   S L P I =[ 0. 75 ] ( % ) ,
2732 L G I =[ 116. 33 ] ( m ) ,   M N I =[ 0. 013 ] ,   S C I =[ 0 ] ( m i n ) ,

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2733 Continuous simulation parameters:
2734 IaRECper=[ 4](hr s), IaRECImp=[ 4](hr s),
2735 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2736 InterEventTime=[ 12](hr s), END=- 1
2737 *%-----|-----
2738 *CONTINUOUS NASHYD NHYD=["S-1-D7"], DT=[ 1]min, AREA=[ 2.03](ha),
2739 * DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
2740 * N=[ 3], TP=[ 1.10]hr s,
2741 * Continuous simulation parameters:
2742 * IaRECper=[ 4](hr s),
2743 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2744 * InterEventTime=[ 12](hr s)
2745 * Baseflow simulation parameters:
2746 * BaseFlowOption=[ 1],
2747 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2748 * VHydCond=[ 0.055](mm/hr), END=- 1
2749 *%-----|-----
2750 *COMPUTE DUALHYD NHYDin=["S-1-D7"], CINLET=[ 2.279](cms), NINLET=[ 1],
2751 * MjNHYD=["S-1-D7J"]
2752 * MnNHYD=["S-1-D7N"]
2753 * TMS TO=[ 9999999](cu-m)
2754 *%-----|-----
2755 *ADD HYD NHYDsum=["S-1-D7S"], NHYDs to add=["S-1-D7J"+"S-1-D7N"]
2756 *%-----|-----
2757 *ROUTE RESERVOIR NHYDout=["S-1-D7R"], NHYDin=["S-1-D7S"],
2758 * RDT=[ 1](min),
2759 * TABLE of ( OUTFLOW STORAGE ) values
2760 * (cms) - (ha-m)
2761 * [ 0.0 , 0.0 ]
2762 * [ 0.0243, 0.0810 ]
2763 * [ -1 , -1 ] (max twenty pts)
2764 * NHYDovf=["S-1-D8Rovf"]
2765 *%-----|-----
2766 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2767 *CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[ 1](min), AREA=[ 5.27](ha), XI MP=[ 0.65],
TI MP=[ 0.65], DWF=[ 0](cms),
2768 * LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
I Aper=[ 4.67](mm), SLPP=[ 2.0](%),
2769 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min), Impervious surfaces:
I Ai mp=[ 1.57](mm), SLPI=[ 0.75](%),
2770 * LGI=[ 187.439](m), MNI=[ 0.013], SCI=[ 0](min),
2771 * Continuous simulation parameters:
2772 * IaRECper=[ 4](hr s), IaRECImp=[ 4](hr s),
2773 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2774 * InterEventTime=[ 12](hr s), END=- 1
2775 *%-----|-----
2776 *CONTINUOUS NASHYD NHYD=["S-1-D8"], DT=[ 1]min, AREA=[ 5.27](ha),
2777 * DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
2778 * N=[ 3], TP=[ 1.10]hr s,
2779 * Continuous simulation parameters:
2780 * IaRECper=[ 4](hr s),
2781 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2782 * InterEventTime=[ 12](hr s)
2783 * Baseflow simulation parameters:
2784 * BaseFlowOption=[ 1],
2785 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2786 * VHydCond=[ 0.055](mm/hr), END=- 1
2787 *%-----|-----
2788 *COMPUTE DUALHYD NHYDin=["S-1-D8"], CINLET=[ 2.279](cms), NINLET=[ 1],
2789 * MjNHYD=["S-1-D8J"]
2790 * MnNHYD=["S-1-D8N"]
2791 * TMS TO=[ 9999999](cu-m)
2792 *%-----|-----
2793 *ADD HYD NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2794 *%-----|-----

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2795 *ADD HYD NHYDs um=[ "S- 1- D" ], NHYDs to add=[ "S- 1- Okeefe"+"S- 1"+"S- 1- Fost" ]
2796 *%-----|-----|
2797 *COMPUTE DUALHYD NHYDi n=[ "S- 1- D" ], CI NLET=[ 11. 616 ] (cms), NI NLET=[ 1 ],
2798 * M j NHYD=[ "S- 1- D- M" ]
2799 * M nNHYD=[ "S- 1- D- MN" ]
2800 * T M STO=[ 5974 ] (cu- m)
2801 *%-----|-----|
2802 *ADD HYD NHYDs um=[ "S- 1- DEV" ], NHYDs to add=[ "S- 1- D- M"+"S- 1- D- MN" ]
2803 *%-----|-----|
2804 *ROUTE RESERVOIR NHYDout=[ "S- 1- D&R" ] , NHYDi n=[ "S- 1- D&S" ] ,
2805 * RDT=[ 1 ] (mi n),
2806 * TABLE of ( OUTFLOW STORAGE ) values
2807 * (cms) - (ha- m)
2808 * [ 0. 0 , 0. 0 ]
2809 * [ 0. 0630, 0. 2102 ]
2810 * [ -1 , -1 ] (max twenty pts)
2811 * NHYDovf=[ "S- 1- D&Rovf" ]
2812 *%-----|-----|
2813 *%-----|-----|
2814 * - JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
2815 *# Catchment W_CLAR
2816 *# - To West Clarke Drain (south of the Jock)
2817 *# - Subdivision with 43% imp. as per Barrhaven South MS
2818 *# - 2020-11-30 update CLARKE Tributary Drainage Area to = 121 ha based on
P598(04)-11
2819 *# - 2020-11-30 split CLARKE Drainage Area to MAJOR and ALL
2820 *#*****
2821 CONTINUOUS STANDHYD NHYD=[ "W_CLAR_M" ], DT=[ 1 ] mi n, AREA=[ 1. 772 ] (ha),
2822 XI MP=[ 0. 46 ], TI MP=[ 0. 59 ], DWF=[ 0 ] (cms), LOSS=[ 2 ],
2823 SCS curve number CN=[ 77 ],
2824 Pervious surfaces: I A per=[ 4. 67 ] (mm), SLPP=[ 1 ] ( % ),
2825 LGP=[ 40 ] (m), MNP=[ 0. 25 ], SCP=[ 0 ] (mi n),
2826 Impervious surfaces: I A i mp=[ 1. 57 ] (mm), SLPI=[ 1 ] ( % ),
2827 LGI=[ 109 ] (m), MNI=[ 0. 013 ], SCI=[ 0 ] (mi n),
2828 Continuous simulation parameters:
2829 I a REC per=[ 4 ] (hrs), I a REC i mp=[ 4 ] (hrs),
2830 SM N=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0. 010 ] / (mm),
2831 Inter Event Ti me=[ 18 ] (hrs), END=- 1
2832 *%-----|-----|
2833 *COMPUTE DUALHYD NHYDi n=[ "W_CLAR_M" ], CI NLET=[ 0. 213 ] (cms), NI NLET=[ 1 ],
2834 * M j NHYD=[ "W_CLAR_M j" ]
2835 * M nNHYD=[ "W_CLAR_M n" ]
2836 * T M STO=[ 0. 1 ] (cu- m)
2837 *%-----|-----|
2838 *# 5- Year + 12% Capture
2839 ROUTE RESERVOIR NHYDout=[ "W_CLAR_M n" ] , NHYDi n=[ "W_CLAR_M" ] ,
2840 * RDT=[ 1 ] (mi n),
2841 * TABLE of ( OUTFLOW STORAGE ) values
2842 * (cms) - (ha- m)
2843 * [ 0. 0 , 0. 0 ]
2844 * [ 0. 213 , 0. 0001 ]
2845 * [ -1 , -1 ] (max twenty pts)
2846 * NHYDovf=[ "W_CLAR_M j" ] ,
2847 *%-----|-----|
2848 * - Clarke_All area from P 598(04)-11 = 120.207 ha, change to 127.298 ha based on
GIS measurements,
2849 * - JFSA 2021-01-19 update W_CLAR_ALL to (121.17-1.772=119.398) ha based on GIS
measurements W_CLAR is 121.17 ha and W_CLAR_M is 1.772 ha
2850 CONTINUOUS STANDHYD NHYD=[ "W_CLAR_ALL" ], DT=[ 1 ] mi n, AREA=[ 119. 398 ] (ha),
2851 XI MP=[ 0. 60 ], TI MP=[ 0. 65 ], DWF=[ 0 ] (cms), LOSS=[ 2 ],
2852 SCS curve number CN=[ 77 ],
2853 Pervious surfaces: I A per=[ 4. 67 ] (mm), SLPP=[ 1 ] ( % ),
2854 LGP=[ 40 ] (m), MNP=[ 0. 25 ], SCP=[ 0 ] (mi n),
2855 Impervious surfaces: I A i mp=[ 1. 57 ] (mm), SLPI=[ 1 ] ( % ),
2856 LGI=[ 892. 18 ] (m), MNI=[ 0. 013 ], SCI=[ 0 ] (mi n),

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2857 Continuous simulation parameters:
2858 IaRECper=[ 4](hrs), IaREClmp=[ 4](hrs),
2859 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2860 InterEventTime=[ 18](hrs), END=- 1
2861 *%-----|-----|
2862 ADD HYD NHYDsum=[ "W_CLAR" ], NHYDsto add=[ "W_CLAR_ALL"+"W_CLAR_Mj" ]
2863 *%-----|-----|
2864 SAVE HYD NHYD=[ "W_CLAR" ], # OF PCYCLES=[ -1 ], ICASEsh=[ 1]
2865 HYD_COMMENT=[ "Total Flows to West Clarke" ]
2866 *#*****
2867 *# West Clarke Pond 2
2868 *# - Rating curve obtained from Barrhaven South M&S modeling
2869 *# - Tributary Drainage Area to M&S Pond 2 = 241 ha
2870 *#*****
2871 ROUTE RESERVOIR NHYDout=[ "MS_P2" ], NHYDin=[ "W_CLAR" ],
2872 RDT=[ 1](min),
2873 TABLE of ( OUTFLOW STORAGE ) values
2874 (cms) - (ha-m)
2875 [ 0.0 , 0.0 ]
2876 [ 0.128 , 0.161 ]
2877 [ 0.138 , 0.409 ]
2878 [ 0.148 , 0.68 ]
2879 [ 0.227 , 0.931 ]
2880 [ 0.354 , 1.223 ]
2881 [ 0.505 , 1.52 ]
2882 [ 0.666 , 1.821 ]
2883 [ 0.831 , 2.123 ]
2884 [ 0.995 , 2.434 ]
2885 [ 1.069 , 2.583 ]
2886 [ 1.51 , 2.647 ]
2887 [ 4.904 , 2.861 ]
2888 [ 13.048 , 3.188 ]
2889 [ 23.745 , 3.523 ]
2890 [ 36.474 , 3.871 ]
2891 [ 45.938 , 4.127 ]
2892 [ 61.652 , 4.539 ]
2893 [ -1 , -1 ] (max twenty pts)
2894 NHYDovf=[ "P2- OVF" ]
2895 *%-----|-----|
2896 *#*****
2897 * -JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2898 *CONTINUOUS NASHYD NHYD=[ "W_CLAR_UNDE" ], DT=[ 1]min, AREA=[ 35.65](ha),
2899 * DWF=[ 0](cms), CNVC=[ 77 ], IA=[ 4.67](mm),
2900 * N=[ 3 ], TP=[ 1.10]hrs,
2901 * Continuous simulation parameters:
2902 * IaRECper=[ 4](hrs),
2903 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2904 * InterEventTime=[ 12](hrs)
2905 * Baseflow simulation parameters:
2906 * BaseFlowOption=[ 1 ],
2907 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2908 * VHydCond=[ 0.055](mm/hr), END=- 1
2909 *%-----|-----|
2910 ADD HYD NHYDsum=[ "SN_CE" ], NHYDsto
add=[ "N_CE"+"S-1-D4"+"S-1-D5"+"MS_P2"+"P2- OVF" ]
2911 *%-----|-----|
2912 SAVE HYD NHYD=[ "SN_CE" ], # OF PCYCLES=[ -1 ], ICASEsh=[ 1]
2913 HYD_COMMENT=[ "Total Flows before Station 5737 on Jock River" ]
2914 *%-----|-----|
2915 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5737
2916 *# 2021-02-25 add station 5737 before station 5002. Station 5737 was extracted from the
HEC-RAS model
T:\PROJ\1474-16\Design\20201026-QuantityControlAnalysis\HEC-RAS\JockLidar2005
2917 *# JFSA 2021-03-02 change the slope to 0.0175% instead of 0.02593 to stabilize the model

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2918 ROUTE CHANNEL      NHYDout =[" 5737" ] , NHYDin=[" SN_CE" ] ,
2919                      RDT=[ 1 ]( m i n ) ,
2920                      CHLGTH=[ 270 ]( m ) ,    CHSLOPE=[ 0. 0175 ]( % ) ,
2921                      FPSLOPE=[ 0. 0175 ]( % ) ,
2922                      SECNUM=[ 1. 0 ] ,          NSEG=[ 3 ]
2923                      ( SEGROUGH, SEGDI ST ( m ) ) =
2924                      [ 0. 050, - 24. 04
2925                      - 0. 035, 23. 92
2926                      0. 050, 1130. 8 ] NSEG t i m e s
2927                      ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
2928                      [ - 1060. 52, 94 ]
2929                      [ - 268. 6, 91. 5 ]
2930                      [ - 259. 43, 91. 5 ]
2931                      [ - 179. 48, 91. 5 ]
2932                      [ - 67. 9, 91. 5 ]
2933                      [ - 59. 21, 91. 5 ]
2934                      [ - 33. 19, 91 ]
2935                      [ - 26. 08, 90. 5 ]
2936                      [ - 24. 04, 90 ]
2937                      [ - 13. 14, 86. 77 ]
2938                      [ 0, 85 ]
2939                      [ 14. 68, 86. 74 ]
2940                      [ 23. 92, 90 ]
2941                      [ 25. 78, 90. 5 ]
2942                      [ 31. 91, 91 ]
2943                      [ 91. 95, 91. 5 ]
2944                      [ 772. 15, 92 ]
2945                      [ 961. 49, 92. 5 ]
2946                      [ 1044. 69, 93 ]
2947                      [ 1130. 8, 95 ]
2948 *%-----|-----|
2949 ADD HYD            NHYDs um=[" 5002" ] , NHYDs t o add=[" 5737"+"S- 1- D1"+"S- 1- D6"+"S- 1- D7" ]
2950 *%-----|-----|
2951 SAVE HYD          NHYD=[" 5002" ] ,    # OF PCYCLES=[ - 1 ] ,    I CASEs h=[ 1 ]
2952                      HYD_ COMMENT=[" Total Flows before Station 5002 on Jock Ri ver" ]
2953 *%-----|-----|
2954 *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
2955 *# Channel X- Section obtained from RVCA Hydraulic Model - Station 5002
2956 *# JFSA 2021-02-19 Change the slope from 0.01 % (as per Stantec Report 2007) to 0.0255
% so the model will be more stable and give reasonable results. It is justifiable as
ROUTE CHANNELs aren't well suited to really flat slopes.
2957 *# JFSA 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one
with 825 m length so the model will be more stable
2958 *# JFSA 2021-02-26 change the length of 5002 route channel from 825 m to 736 m That is
because of adding station 5737 between station 6016 and station 5002. Then the length
from station 5737 to station 5002 is 736 m Change the slope from 0.0255 % to 0.09511 %
2959 *
2960 ROUTE CHANNEL      NHYDout =[" N_ WCa" ] , NHYDin=[" 5002" ] ,
2961                      RDT=[ 1 ]( m i n ) ,
2962                      CHLGTH=[ 245. 33333 ]( m ) ,    CHSLOPE=[ 0. 09511 ]( % ) ,
2963                      FPSLOPE=[ 0. 09511 ]( % ) ,
2964                      SECNUM=[ 1. 0 ] ,          NSEG=[ 3 ]
2965                      ( SEGROUGH, SEGDI ST ( m ) ) =
2966                      [ 0. 050, - 37. 5
2967                      - 0. 035, 37. 50
2968                      0. 050, 157. 05 ] NSEG t i m e s
2969                      ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
2970                      [ - 601. 81, 91. 5 ]
2971                      [ - 37. 50, 90. 00 ]
2972                      [ - 19. 61, 87. 04 ]
2973                      [ 0. 00, 85. 70 ]
2974                      [ 14. 87, 86. 93 ]
2975                      [ 37. 50, 90. 00 ]
2976                      [ 38. 54, 90. 50 ]
2977                      [ 42. 23, 91 ]
2978                      [ 157. 05, 91. 50 ]

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2979 * [ 161.44, 91.50]
2980 * [ 236.48, 93.00]
2981 * [ 385.47, 92.50]
2982 * [ 390.78, 92.50]
2983 *%-----|
2984 ROUTE CHANNEL NHYDout=["N_WCb"] , NHYDin=["N_WCa"] ,
2985 RDT=[ 1]( m n),
2986 CHLGTH=[ 245.33333]( m), CHSLOPE=[ 0.09511]( %),
2987 FPSLOPE=[ 0.09511]( %),
2988 SECNUM=[ 1.0], NSEG=[ 3]
2989 ( SEGROUGH, SEGDI ST ( m))=
2990 [ 0.050, -37.5
2991 -0.035, 37.50
2992 0.050, 157.05] NSEG t i m e s
2993 ( DI STANCE ( m), ELEVATI ON ( m))=
2994 [- 601.81, 91.5]
2995 [- 37.50, 90.00]
2996 [- 19.61, 87.04]
2997 [ 0.00, 85.70]
2998 [ 14.87, 86.93]
2999 [ 37.50, 90.00]
3000 [ 38.54, 90.50]
3001 [ 42.23, 91]
3002 [ 157.05, 91.50]
3003 *%-----|
3004 ROUTE CHANNEL NHYDout=["N_WC"] , NHYDin=["N_WCb"] ,
3005 RDT=[ 1]( m n),
3006 CHLGTH=[ 245.33333]( m), CHSLOPE=[ 0.09511]( %),
3007 FPSLOPE=[ 0.09511]( %),
3008 SECNUM=[ 1.0], NSEG=[ 3]
3009 ( SEGROUGH, SEGDI ST ( m))=
3010 [ 0.050, -37.5
3011 -0.035, 37.50
3012 0.050, 157.05] NSEG t i m e s
3013 ( DI STANCE ( m), ELEVATI ON ( m))=
3014 [- 601.81, 91.5]
3015 [- 37.50, 90.00]
3016 [- 19.61, 87.04]
3017 [ 0.00, 85.70]
3018 [ 14.87, 86.93]
3019 [ 37.50, 90.00]
3020 [ 38.54, 90.50]
3021 [ 42.23, 91]
3022 [ 157.05, 91.50]
3023 ##*****
3024 * -JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
3025 *ADD HYD NHYDsum=["SN_WC"], NHYDs to
add=["MS_P2"+"P2-OVF"+"N_WC"+"W_CLAR_UNDE"]
3026 *%-----|
3027 *SAVE HYD NHYD=["SN_WC"], # OF PCYCLES=[ -1], I CASEs h=[ 1]
3028 * HYD_COMMENT=["Total Flows at West Clarke Pond Outlet"]
3029 *%-----|
3030 *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
3031 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
3032 *#
3033 ROUTE CHANNEL NHYDout=["N_KB"] , NHYDin=["N_WC"] ,
3034 RDT=[ 1]( m n),
3035 CHLGTH=[ 1020]( m), CHSLOPE=[ 0.0498]( %),
3036 FPSLOPE=[ 0.0498]( %),
3037 SECNUM=[ 1.0], NSEG=[ 3]
3038 ( SEGROUGH, SEGDI ST ( m))=
3039 [ 0.050, -23.63
3040 -0.035, 23.63
3041 0.050, 728.3] NSEG t i m e s
3042 ( DI STANCE ( m), ELEVATI ON ( m))=

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3043 [- 1082. 01, 94]
3044 [- 1028. 17, 92. 5]
3045 [- 992. 3, 93. 5]
3046 [- 279. 34, 90]
3047 [- 23. 63, 90]
3048 [- 13. 45, 87. 13]
3049 [- 0. 07, 86. 24]
3050 [ 10. 54, 87. 15]
3051 [ 23. 63, 90]
3052 [ 24. 86, 90. 5]
3053 [ 26. 72, 91]
3054 [ 45. 07, 91. 5]
3055 [ 128. 17, 91. 5]
3056 [ 270. 7, 92. 5]
3057 [ 728. 3, 95]
3058 *%-----|-----|
3059 *#*****|*****|
3060 *# Catchment KEN_BU
3061 *# - To Kennedy-Burnett SWM Facility
3062 *# - Outlets to Fraser-Clarke drain (north of the Jock)
3063 *# - Medium density residential subdivision
3064 * - Add Kennedy Burnett model (Convert PCSWMM from NOVATECH June, 2020 to SWWHYMO)
3065 *#*****|*****|
3066 *CONTINUOUS STANDHYD NHYD=[ "KEN_BU" ], DT=[ 1] mi n, AREA=[ 281] (ha),
3067 * XI MP=[ 0. 55], TI MP=[ 0. 55], DWF=[ 0] (cms), LOSS=[ 2],
3068 * SCS curve number CN=[ 71],
3069 * Pervious surfaces: I A per=[ 4. 67] (mm), SLPP=[ 1] ( %),
3070 * LGP=[ 40] (m), MNP=[ 0. 25], SCP=[ 0] (mi n),
3071 * Impervious surfaces: I A i mp=[ 1. 57] (mm), SLPI =[ 1] ( %),
3072 * LGI =[ 1369] (m), MNI =[ 0. 013], SCI =[ 0] (mi n),
3073 * Continuous simulation parameters:
3074 * I a REC per=[ 4] (hrs), I a REC i mp=[ 4] (hrs),
3075 * SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
3076 * Inter Event Ti me=[ 18] (hrs), END=- 1
3077 *%-----|-----|
3078 *#*****|*****|
3079 *# Existing Kennedy-Burnett SWM Facility
3080 *# - Rating curve obtained from URTKBP
3081 *# - Tributary Drainage Area to Pond = 160 ha
3082 *#*****|*****|
3083 *ROUTE RESERVOIR NHYDout=[ "KEN_P" ], NHYDi n=[ "KEN_BU" ],
3084 * RDT=[ 1] (mi n),
3085 * TABLE of ( OUTFLOW STORAGE ) values
3086 * ( cms ) - ( ha- m)
3087 * [ 0. 0 , 0. 0 ]
3088 * [ 0. 13 , 0. 26]
3089 * [ 0. 43 , 0. 56]
3090 * [ 0. 67 , 0. 90]
3091 * [ 0. 86 , 1. 32]
3092 * [ 1. 01 , 1. 79]
3093 * [ 1. 15 , 2. 33]
3094 * [ - 1 , - 1 ] (max twenty pts)
3095 * NHYDovf=[ "KEN- OV" ]
3096 *%-----|-----|
3097 * -JFSA, 2021-01-19 update all KEN_BU areas based on GIS measurements
3098 CONTINUOUS STANDHYD NHYD=[ "KB- 01A" ], DT=[ 1] mi n, AREA=[ 40. 82] (ha), XI MP=[ 0. 097],
3099 TI MP=[ 0. 4], DWF=[ 0] (cms), LOSS=[ 1]:
3099 Horton: Fo=[ 76. 20] (mm/ hr), Fc=[ 13. 20] (mm/ hr), DCAY=[ 4. 14] (/ hr),
3100 F=[ 0. 00] (mm),
3100 Pervious areas: I A per=[ 4. 67] (mm), SLPP=[ 0. 5] ( %), LGP=[ 40] (m),
3101 MNP=[ 0. 250], SCP=[ 0] (mi n),
3101 Impervious areas: I A i mp=[ 0. 785] (mm), SLPI =[ 0. 5] ( %),
3102 LGI =[ 521. 664] (m), MNI =[ 0. 013], SCI =[ 0] (mi n),
3102 Continuous simulation parameters:
3103 I a REC per=[ 4] (hrs), I a REC i mp=[ 4] (hrs), Inter Event Ti me=[ 12] (hrs),
3103 END=- 1

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3104 *%-----|-----|
3105 COMPUTE DUALHYD NHYDin=[ " KB- 01A" ], CI NLET=[ 3. 6 ] ( cms ), NI NLET=[ 1 ],
3106 Māj NHYD=[ " KB- 01A- M " ]
3107 M nNHYD=[ " KB- 01A- MN" ]
3108 TM STO=[ 4995 ] ( cu- m )
3109 *%-----|-----|
3110 ADD HYD NHYDsum=[ " KB- 01A- S" ], NHYDs to add=[ " KB- 01A- M " + " KB- 01A- MN" ]
3111 *%-----|-----|
3112 CONTINUOUS STANDHYD NHYD=[ " KB- 01B" ], DT=[ 1 ] mi n, AREA=[ 31. 1 ] ( ha ), XI MP=[ 0. 1875 ],
TI MP=[ 0. 375 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3113 Horton: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
3114 Pervious areas: I A per=[ 4. 67 ] ( mm ), SLPP=[ 0. 42 ] ( % ), LGP=[ 40 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
3115 Impervious areas: I A i mp=[ 0. 785 ] ( mm ), SLPI=[ 0. 42 ] ( % ),
LGI=[ 455. 339 ] ( m ), MNI=[ 0. 013 ], SCI=[ 0 ] ( mi n ),
3116 Continuous simulation parameters:
3117 I a REC per=[ 4 ] ( hrs ), I a REC i mp=[ 4 ] ( hrs ), I nter Event Ti me=[ 12 ] ( hrs ),
END=- 1
3118 *%-----|-----|
3119 COMPUTE DUALHYD NHYDin=[ " KB- 01B" ], CI NLET=[ 1. 585 ] ( cms ), NI NLET=[ 1 ],
3120 Māj NHYD=[ " KB- 01B- M " ]
3121 M nNHYD=[ " KB- 01B- MN" ]
3122 TM STO=[ 6075 ] ( cu- m )
3123 *%-----|-----|
3124 ADD HYD NHYDsum=[ " KB- 01B- S" ], NHYDs to add=[ " KB- 01B- M " + " KB- 01B- MN" ]
3125 *%-----|-----|
3126 CONTINUOUS STANDHYD NHYD=[ " KB- 01C" ], DT=[ 1 ] mi n, AREA=[ 13. 78 ] ( ha ), XI MP=[ 0. 2045 ],
TI MP=[ 0. 409 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3127 Horton: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
3128 Pervious areas: I A per=[ 4. 67 ] ( mm ), SLPP=[ 2. 0 ] ( % ), LGP=[ 40 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
3129 Impervious areas: I A i mp=[ 0. 785 ] ( mm ), SLPI=[ 0. 5 ] ( % ),
LGI=[ 303. 095 ] ( m ), MNI=[ 0. 013 ], SCI=[ 0 ] ( mi n ),
3130 Continuous simulation parameters:
3131 I a REC per=[ 4 ] ( hrs ), I a REC i mp=[ 4 ] ( hrs ), I nter Event Ti me=[ 12 ] ( hrs ),
END=- 1
3132 *%-----|-----|
3133 COMPUTE DUALHYD NHYDin=[ " KB- 01C" ], CI NLET=[ 1. 35 ] ( cms ), NI NLET=[ 1 ],
3134 Māj NHYD=[ " KB- 01C- M " ]
3135 M nNHYD=[ " KB- 01C- MN" ]
3136 TM STO=[ 1880 ] ( cu- m )
3137 *%-----|-----|
3138 ADD HYD NHYDsum=[ " KB- 01C- S" ], NHYDs to add=[ " KB- 01C- M " + " KB- 01C- MN" ]
3139 *%-----|-----|
3140 CONTINUOUS STANDHYD NHYD=[ " KB- 03" ], DT=[ 1 ] mi n, AREA=[ 84. 78 ] ( ha ), XI MP=[ 0. 197 ],
TI MP=[ 0. 394 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3141 Horton: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
3142 Pervious areas: I A per=[ 4. 67 ] ( mm ), SLPP=[ 2. 0 ] ( % ), LGP=[ 40 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
3143 Impervious areas: I A i mp=[ 0. 785 ] ( mm ), SLPI=[ 0. 63 ] ( % ),
LGI=[ 751. 798 ] ( m ), MNI=[ 0. 013 ], SCI=[ 0 ] ( mi n ),
3144 Continuous simulation parameters:
3145 I a REC per=[ 4 ] ( hrs ), I a REC i mp=[ 4 ] ( hrs ), I nter Event Ti me=[ 12 ] ( hrs ),
END=- 1
3146 *%-----|-----|
3147 COMPUTE DUALHYD NHYDin=[ " KB- 03" ], CI NLET=[ 5. 27 ] ( cms ), NI NLET=[ 1 ],
3148 Māj NHYD=[ " KB- 03- M " ]
3149 M nNHYD=[ " KB- 03- MN" ]
3150 TM STO=[ 15500 ] ( cu- m )
3151 *%-----|-----|
3152 ADD HYD NHYDsum=[ " KB- 03- S" ], NHYDs to add=[ " KB- 03- M " + " KB- 03- MN" ]
3153 *%-----|-----|
3154 CONTINUOUS STANDHYD NHYD=[ " KB- 04" ], DT=[ 1 ] mi n, AREA=[ 6. 95 ] ( ha ), XI MP=[ 0. 85 ],

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3155 TI MP=[0.85], DWF=[0](cms), LOSS=[1]:
 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
 F=[0.00](mm),
 3156 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
 MNP=[0.250], SCP=[0](min),
 3157 Impervious areas: IAimp=[0.942](mm), SLPI=[0.5](%),
 LGI=[215.252](m), MNI=[0.013], SCI=[0](min),
 3158 Continuous simulation parameters:
 3159 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
 END=- 1
 3160 *%-----|
 3161 COMPUTE DUALHYD NHYDin=["KB- 04"], CINLET=[0.503](cms), NINLET=[1],
 3162 MajNHYD=["KB- 04- M"]
 3163 MnNHYD=["KB- 04- MN"]
 3164 TMSTO=[1972](cu-m)
 3165 *%-----|
 3166 ADD HYD NHYDsum=["KB- 04- S"], NHYDsto add=["KB- 04- M"+"KB- 04- MN"]
 3167 *%-----|
 3168 CONTINUOUS STANDHYD NHYD=["KB- 05"], DT=[1]min, AREA=[5.19](ha), XI MP=[0.93],
 TI MP=[0.93], DWF=[0](cms), LOSS=[1]:
 3169 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
 F=[0.00](mm),
 3170 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
 MNP=[0.250], SCP=[0](min),
 3171 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%),
 LGI=[186.011](m), MNI=[0.013], SCI=[0](min),
 3172 Continuous simulation parameters:
 3173 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
 END=- 1
 3174 *%-----|
 3175 *%-----|
 3176 CONTINUOUS STANDHYD NHYD=["KB- 06"], DT=[1]min, AREA=[12.93](ha), XI MP=[0.873],
 TI MP=[0.873], DWF=[0](cms), LOSS=[1]:
 3177 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
 F=[0.00](mm),
 3178 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
 MNP=[0.250], SCP=[0](min),
 3179 Impervious areas: IAimp=[0.942](mm), SLPI=[4.75](%),
 LGI=[293.598](m), MNI=[0.013], SCI=[0](min),
 3180 Continuous simulation parameters:
 3181 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
 END=- 1
 3182 *%-----|
 3183 COMPUTE DUALHYD NHYDin=["KB- 06"], CINLET=[2.262](cms), NINLET=[1],
 3184 MajNHYD=["KB- 06- M"]
 3185 MnNHYD=["KB- 06- MN"]
 3186 TMSTO=[1950](cu-m)
 3187 *%-----|
 3188 ADD HYD NHYDsum=["KB- 06- S"], NHYDsto add=["KB- 06- M"+"KB- 06- MN"]
 3189 *%-----|
 3190 CONTINUOUS STANDHYD NHYD=["KB- 11"], DT=[1]min, AREA=[4.03](ha), XI MP=[0.675],
 TI MP=[0.675], DWF=[0](cms), LOSS=[1]:
 3191 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
 F=[0.00](mm),
 3192 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
 MNP=[0.250], SCP=[0](min),
 3193 Impervious areas: IAimp=[0.785](mm), SLPI=[2.0](%),
 LGI=[163.911](m), MNI=[0.013], SCI=[0](min),
 3194 Continuous simulation parameters:
 3195 IaRECper=[4](hrs), IaRECImp=[4](hrs), InterEventTime=[12](hrs),
 END=- 1
 3196 *%-----|
 3197 COMPUTE DUALHYD NHYDin=["KB- 11"], CINLET=[0.5773](cms), NINLET=[1],
 3198 MajNHYD=["KB- 11- M"]
 3199 MnNHYD=["KB- 11- MN"]
 3200 TMSTO=[597](cu-m)

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3201  *%-----|-----|
3202  ADD HYD      NHYDsum=["KB-11-S"], NHYDs to add=["KB-11-M"+"KB-11-MN"]
3203  *%-----|-----|
3204  CONTINUOUS STANDHYD NHYD=["S1"], DT=[1] min, AREA=[4.99](ha), XI MP=[0.93], TI MP=[0.93],
DWF=[0](cms), LOSS=[1]:
3205  Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3206  Previous areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3207  ImperVIOUS areas: I A i mp=[1.57](mm), SLPI=[2.0](%),
LGI=[182.392](m), MNI=[0.013], SCI=[0](min),
3208  Continuous simulation parameters:
3209  I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3210  *%-----|-----|
3211  CONTINUOUS STANDHYD NHYD=["KB-15"], DT=[1] min, AREA=[2.15](ha), XI MP=[0.79],
TI MP=[0.79], DWF=[0](cms), LOSS=[1]:
3212  Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3213  Previous areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3214  ImperVIOUS areas: I A i mp=[0.157](mm), SLPI=[0.3](%),
LGI=[119.722](m), MNI=[0.013], SCI=[0](min),
3215  Continuous simulation parameters:
3216  I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3217  *%-----|-----|
3218  *%-----|-----|
3219  ADD HYD      NHYDsum=["KB-P1"], NHYDs to
add=["KB-01A-S"+"KB-01B-S"+"KB-01C-S"+"KB-03-S"+"KB-04-S"+"KB-05"+"KB-06-S"+"KB-11-S"+"KB
-15"+"S1"]
3220  *%-----|-----|
3221  ROUTE RESERVOIR NHYDout=["KB-P1R"], NHYDin=["KB-P1"],
3222  RDT=[1](min),
3223  TABLE of ( OUTFLOW STORAGE ) values
3224  (cms) - (ha-m)
3225  [ 0.0 , 0.0 ]
3226  [0.076,0.003]
3227  [0.088,0.006]
3228  [0.136,0.011]
3229  [0.301,0.017]
3230  [0.454,0.027]
3231  [0.631,0.041]
3232  [1.173,0.068]
3233  [1.91,0.111]
3234  [4.847,0.231]
3235  [9.813,0.436]
3236  [12.134,0.617]
3237  [12.438,0.732]
3238  [12.424,0.811]
3239  [12.425,0.894]
3240  [ -1 , -1 ] (max twenty pts)
3241  NHYDovf=["KB-P1ovf"]
3242  *%-----|-----|
3243  ADD HYD      NHYDsum=["KB-Pond1"], NHYDs to add=["KB-P1R"+"KB-P1ovf"]
3244  *%-----|-----|
3245  SAVE HYD     NHYD=["KB-Pond1"], # OF PCYCLES=[-1], I CASEs h=[1]
3246  HYD_COMMENT=["Total Flows at KB first pond"]
3247  *%-----|-----|
3248  CONTINUOUS STANDHYD NHYD=["KB-07"], DT=[1] min, AREA=[10.86](ha), XI MP=[0.86],
TI MP=[0.86], DWF=[0](cms), LOSS=[1]:
3249  Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3250  Previous areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3251  ImperVIOUS areas: I A i mp=[0.785](mm), SLPI=[2.0](%),

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3252 LGI=[ 269.072](m), MNI=[ 0.013], SCI=[ 0](min),
3253 Continuous simulation parameters:
3253 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3254 END=- 1
3254 *%-----|
3255 COMPUTE DUALHYD NHYDin=["KB-07"], CINLET=[ 2.094](cms), NINLET=[ 1],
3256 MajNHYD=["KB-07-M"]
3257 MnNHYD=["KB-07-MN"]
3258 TMSTO=[ 1378](cu-m)
3259 *%-----|
3260 ADD HYD NHYDsum=["KB-07-S"], NHYDsto add=["KB-07-M"+"KB-07-MN"]
3261 *%-----|
3262 CONTINUOUS STANDHYD NHYD=["KB-08"], DT=[ 1]min, AREA=[ 6.61](ha), XI MP=[ 0.64],
3263 TI MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3263 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3264 F=[ 0.00](mm),
3264 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3265 MNP=[ 0.250], SCP=[ 0](min),
3265 Impervious areas: IAimp=[ 0.785](mm), SLPI=[ 2.0](%),
3266 LGI=[ 209.921](m), MNI=[ 0.013], SCI=[ 0](min),
3266 Continuous simulation parameters:
3267 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3268 END=- 1
3268 *%-----|
3269 COMPUTE DUALHYD NHYDin=["KB-08"], CINLET=[ 1.058](cms), NINLET=[ 1],
3270 MajNHYD=["KB-08-M"]
3271 MnNHYD=["KB-08-MN"]
3272 TMSTO=[ 787](cu-m)
3273 *%-----|
3274 ADD HYD NHYDsum=["KB-08-S"], NHYDsto add=["KB-08-M"+"KB-08-MN"]
3275 *%-----|
3276 CONTINUOUS STANDHYD NHYD=["KB-09"], DT=[ 1]min, AREA=[ 2.6](ha), XI MP=[ 0.86],
3277 TI MP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3277 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3278 F=[ 0.00](mm),
3278 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3279 MNP=[ 0.250], SCP=[ 0](min),
3279 Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 2.0](%),
3280 LGI=[ 131.656](m), MNI=[ 0.013], SCI=[ 0](min),
3280 Continuous simulation parameters:
3281 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3282 END=- 1
3282 *%-----|
3283 *%-----|
3284 CONTINUOUS STANDHYD NHYD=["KB-10_1"], DT=[ 1]min, AREA=[ 2.37](ha), XI MP=[ 0.86],
3285 TI MP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3285 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3286 F=[ 0.00](mm),
3286 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3287 MNP=[ 0.250], SCP=[ 0](min),
3287 Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 2.0](%),
3288 LGI=[ 125.698](m), MNI=[ 0.013], SCI=[ 0](min),
3288 Continuous simulation parameters:
3289 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3290 END=- 1
3290 *%-----|
3291 CONTINUOUS STANDHYD NHYD=["KB-10_2"], DT=[ 1]min, AREA=[ 1.14](ha), XI MP=[ 0.86],
3292 TI MP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3292 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3293 F=[ 0.00](mm),
3293 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3294 MNP=[ 0.250], SCP=[ 0](min),
3294 Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 2.0](%), LGI=[ 87.178](m),
3295 MNI=[ 0.013], SCI=[ 0](min),
3295 Continuous simulation parameters:
3296 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),

```

END=- 1

3297 *%-----|-----|
3298 *%-----|-----|
3299 CONTINUOUS STANDHYD NHYD=[" KB- 12"], DT=[1] mi n, AREA=[4. 86] (ha), XI MP=[0. 79],
TI MP=[0. 79], DWF=[0] (cms), LOSS=[1]:
3300 Horton: Fo=[76. 20] (mm/ hr), Fc=[13. 20] (mm/ hr), DCAY=[4. 14] (/ hr),
F=[0. 00] (mm),
3301 Pervious areas: IAper=[4. 67] (mm), SLPP=[2. 0] (%), LGP=[40] (m),
MNP=[0. 250], SCP=[0] (mi n),
3302 Impervious areas: IAi mp=[1. 099] (mm), SLPI =[2. 0] (%),
LGI =[180. 000] (m), MNI =[0. 013], SCI =[0] (mi n),
3303 Continuous simulation parameters:
3304 IaRECper=[4] (hrs), IaRECI mp=[4] (hrs), InterEvent Time=[12] (hrs),
END=- 1
3305 *%-----|-----|
3306 COMPUTE DUALHYD NHYDi n=[" KB- 12"], CI NLET=[0. 8665] (cms), NI NLET=[1],
3307 Ma j NHYD=[" KB- 12- M"]
3308 M nNHYD=[" KB- 12- MN"]
3309 TM STO=[632] (cu- m)
3310 *%-----|-----|
3311 ADD HYD NHYDs um=[" KB- 12- S"], NHYDs to add=[" KB- 12- M" +" KB- 12- MN"]
3312 *%-----|-----|
3313 CONTINUOUS STANDHYD NHYD=[" KB- 13"], DT=[1] mi n, AREA=[10. 19] (ha), XI MP=[0. 64],
TI MP=[0. 64], DWF=[0] (cms), LOSS=[1]:
3314 Horton: Fo=[76. 20] (mm/ hr), Fc=[13. 20] (mm/ hr), DCAY=[4. 14] (/ hr),
F=[0. 00] (mm),
3315 Pervious areas: IAper=[4. 67] (mm), SLPP=[2. 0] (%), LGP=[40] (m),
MNP=[0. 250], SCP=[0] (mi n),
3316 Impervious areas: IAi mp=[0. 785] (mm), SLPI =[2. 0] (%),
LGI =[260. 640] (m), MNI =[0. 013], SCI =[0] (mi n),
3317 Continuous simulation parameters:
3318 IaRECper=[4] (hrs), IaRECI mp=[4] (hrs), InterEvent Time=[12] (hrs),
END=- 1
3319 *%-----|-----|
3320 COMPUTE DUALHYD NHYDi n=[" KB- 13"], CI NLET=[1. 722] (cms), NI NLET=[1],
3321 Ma j NHYD=[" KB- 13- M"]
3322 M nNHYD=[" KB- 13- MN"]
3323 TM STO=[1077] (cu- m)
3324 *%-----|-----|
3325 ADD HYD NHYDs um=[" KB- 13- S"], NHYDs to add=[" KB- 13- M" +" KB- 13- MN"]
3326 *%-----|-----|
3327 CONTINUOUS STANDHYD NHYD=[" KB- 14"], DT=[1] mi n, AREA=[5. 47] (ha), XI MP=[0. 64],
TI MP=[0. 64], DWF=[0] (cms), LOSS=[1]:
3328 Horton: Fo=[76. 20] (mm/ hr), Fc=[13. 20] (mm/ hr), DCAY=[4. 14] (/ hr),
F=[0. 00] (mm),
3329 Pervious areas: IAper=[4. 67] (mm), SLPP=[2. 0] (%), LGP=[40] (m),
MNP=[0. 250], SCP=[0] (mi n),
3330 Impervious areas: IAi mp=[0. 785] (mm), SLPI =[2. 0] (%),
LGI =[190. 962] (m), MNI =[0. 013], SCI =[0] (mi n),
3331 Continuous simulation parameters:
3332 IaRECper=[4] (hrs), IaRECI mp=[4] (hrs), InterEvent Time=[12] (hrs),
END=- 1
3333 *%-----|-----|
3334 COMPUTE DUALHYD NHYDi n=[" KB- 14"], CI NLET=[0. 8734] (cms), NI NLET=[1],
3335 Ma j NHYD=[" KB- 14- M"]
3336 M nNHYD=[" KB- 14- MN"]
3337 TM STO=[631] (cu- m)
3338 *%-----|-----|
3339 ADD HYD NHYDs um=[" KB- 14- S"], NHYDs to add=[" KB- 14- M" +" KB- 14- MN"]
3340 *%-----|-----|
3341 *%-----|-----|
3342 CONTINUOUS STANDHYD NHYD=[" KB- 16_2"], DT=[1] mi n, AREA=[3. 42] (ha), XI MP=[0. 71],
TI MP=[0. 71], DWF=[0] (cms), LOSS=[1]:
3343 Horton: Fo=[76. 20] (mm/ hr), Fc=[13. 20] (mm/ hr), DCAY=[4. 14] (/ hr),
F=[0. 00] (mm),
3344 Pervious areas: IAper=[4. 67] (mm), SLPP=[2. 0] (%), LGP=[40] (m),

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3345 MNP=[ 0.250], SCP=[ 0]( mi n),
Impervious areas: I Aimp=[ 0.157]( mm), SLPI=[ 0.3]( %),
3346 LGI=[ 150.997]( m), MNI=[ 0.013], SCI=[ 0]( mi n),
3347 Continuous simulation parameters:
IaRECper=[ 4]( hrs), IaRECImp=[ 4]( hrs), InterEventTime=[ 12]( hrs),
END=- 1
3348 *%-----|-----|
3349 ADD HYD NHYDsum=[ "KB- P2"], NHYDs to
add=[ "KB- Pond1"+"KB- 07- S"+"KB- 08- S"+"KB- 09"+"KB- 10_1"+"KB- 10_2"+"KB- 12- S"+"KB- 13- S"+"KB- 1
4- S"+"KB- 16_2"]
3350 *%-----|-----|
3351 ROUTE RESERVOIR NHYDout=[ "KB- P2R"], NHYDin=[ "KB- P2"],
3352 RDT=[ 1]( mi n),
3353 TABLE of ( OUTFLOW STORAGE ) values
3354 ( cms ) - ( ha- m)
3355 [ 0.0 , 0.0 ]
3356 [ 0.053, 0.005]
3357 [ 0.132, 0.009]
3358 [ 0.269, 0.014]
3359 [ 0.455, 0.023]
3360 [ 0.699, 0.037]
3361 [ 0.947, 0.056]
3362 [ 1.853, 0.09]
3363 [ 2.712, 0.146]
3364 [ 6.626, 0.287]
3365 [ 11.228, 0.515]
3366 [ 14.885, 0.738]
3367 [ 16.473, 0.893]
3368 [ 17.311, 0.998]
3369 [ 17.633, 1.063]
3370 [ 17.634, 1.112]
3371 [ -1 , -1 ] (max twenty pts)
3372 NHYDovf=[ "KB- P2ovf"]
3373 *%-----|-----|
3374 ADD HYD NHYDsum=[ "KB- Pond2"], NHYDs to add=[ "KB- P2R"+"KB- P2ovf"]
3375 *%-----|-----|
3376 SAVE HYD NHYD=[ "KB- Pond2"], # OF PCYCLES=[ -1], ICASEsh=[ 1]
3377 HYD_COMMENT=[ "Total Flows at KB second pond"]
3378 *%-----|-----|
3379 CONTINUOUS STANDHYD NHYD=[ "KB- 16_1"], DT=[ 1] mi n, AREA=[ 2.8]( ha), XI MP=[ 0.75],
TI MP=[ 0.75], DWF=[ 0]( cms), LOSS=[ 1]:
3380 Horton: Fo=[ 76.20]( mm/ hr), Fc=[ 13.20]( mm/ hr), DCAY=[ 4.14]( / hr),
F=[ 0.00]( mm),
3381 Pervious areas: I Aper=[ 4.67]( mm), SLPP=[ 2.0]( %), LGP=[ 40]( m),
MNP=[ 0.250], SCP=[ 0]( mi n),
3382 Impervious areas: I Aimp=[ 0.157]( mm), SLPI=[ 0.3]( %),
LGI=[ 136.626]( m), MNI=[ 0.013], SCI=[ 0]( mi n),
3383 Continuous simulation parameters:
3384 IaRECper=[ 4]( hrs), IaRECImp=[ 4]( hrs), InterEventTime=[ 12]( hrs),
END=- 1
3385 *%-----|-----|
3386 ADD HYD NHYDsum=[ "KB- P3"], NHYDs to add=[ "KB- Pond2"+"KB- 16_1"]
3387 *%-----|-----|
3388 *%-----|-----|
3389 * One inflow node from pond 3 is added to the model (ROUTE RESERVOIR)
3390 * Another inflow node from right side of pond 3 is not added to the model
3391 ROUTE RESERVOIR NHYDout=[ "KB- P3R"], NHYDin=[ "KB- P3"],
3392 RDT=[ 1]( mi n),
3393 TABLE of ( OUTFLOW STORAGE ) values
3394 ( cms ) - ( ha- m)
3395 [ 0.0 , 0.0 ]
3396 [ 0.051, 0.002]
3397 [ 0.048, 0.003]
3398 [ 0.057, 0.029]
3399 [ 0.089, 0.045]
3400 [ 0.133, 0.069]

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3401 [ 0.199, 0.106]
3402 [ 0.321, 0.172]
3403 [ 1.029, 0.306]
3404 [ 4.036, 0.527]
3405 [ 8.332, 0.761]
3406 [ 11.727, 0.941]
3407 [ 14.125, 1.067]
3408 [ 15.675, 1.149]
3409 [ 16.555, 1.196]
3410 [ 16.911, 1.214]
3411 [ -1, -1 ] (max twenty pts)
3412 NHYDovf=["KB- P3ovf"]
3413 *%-----|-----|
3414 ADD HYD NHYDsum=["KB- Pond3"], NHYDs to add=["KB- P3R"+"KB- P3ovf"]
3415 *%-----|-----|
3416 SAVE HYD NHYD=["KB- Pond3"], # OF PCYCLES=[-1], ICASEsh=[1]
3417 HYD_COMMENT=["Total Flows at KB third pond"]
3418 *%-----|-----|
3419 *#*****|*****|
3420 *# EXISTING / PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM
3421 Modeling Approach, NOVATECH Report June, 2020)
3422 *# - TO FRASER-CLARKE DRAIN
3423 *#*****|*****|
3424 CONTINUOUS STANDHYD NHYD=["FC-01"], DT=[1]min, AREA=[8.03](ha), XI MP=[0.47],
3425 T I MP=[0.47], DWF=[0](cms), LOSS=[1]:
3426 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3427 F=[0.00](mm),
3428 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3429 MNP=[0.250], SCP=[0](min),
3430 Impervious areas: I A i mp=[1.57](mm), SLPI=[1.0](%),
3431 LGI=[231.373](m), MNI=[0.013], SCI=[0](min),
3432 Continuous simulation parameters:
3433 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
3434 E N D=-1
3435 *%-----|-----|
3436 COMPUTE DUALHYD NHYD i n=["FC-01"], C I N L E T=[0.756](cms), N I N L E T=[1],
3437 M a j N H Y D=["FC-01- M"],
3438 M i n N H Y D=["FC-01- M N"],
3439 T M S T O=[714](cu-m)
3440 *%-----|-----|
3441 ADD HYD NHYDsum=["FC-01-S"], NHYDs to add=["FC-01- M"+"FC-01- M N"]
3442 *%-----|-----|
3443 CONTINUOUS STANDHYD NHYD=["FC-02"], DT=[1]min, AREA=[16.05](ha), XI MP=[0.93],
3444 T I MP=[0.93], DWF=[0](cms), LOSS=[1]:
3445 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3446 F=[0.00](mm),
3447 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
3448 MNP=[0.250], SCP=[0](min),
3449 Impervious areas: I A i mp=[1.57](mm), SLPI=[1.0](%),
3450 LGI=[327.109](m), MNI=[0.013], SCI=[0](min),
3451 Continuous simulation parameters:
3452 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
3453 E N D=-1
3454 *%-----|-----|
3455 COMPUTE DUALHYD NHYD i n=["FC-02"], C I N L E T=[1.159](cms), N I N L E T=[1],
3456 M a j N H Y D=["FC-02- M"],
3457 M i n N H Y D=["FC-02- M N"],
3458 T M S T O=[2385](cu-m)
3459 *%-----|-----|
3460 ADD HYD NHYDsum=["FC-02-S"], NHYDs to add=["FC-02- M"+"FC-02- M N"]
3461 *%-----|-----|
3462 CONTINUOUS STANDHYD NHYD=["FC-03"], DT=[1]min, AREA=[7.37](ha), XI MP=[0.64],
3463 T I MP=[0.64], DWF=[0](cms), LOSS=[1]:
3464 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
3465 F=[0.00](mm),
3466 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),

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3454 MNP=[ 0.250], SCP=[ 0](mi n),
Impervious areas: IAi mp=[ 1.57](mm), SLPI =[ 1.0](%),
LGI =[ 221.660](m), MNI =[ 0.013], SCI =[ 0](mi n),
3455 Continuous simulation parameters:
3456 IaREcper=[ 4](hrs), IaRECi mp=[ 4](hrs), InterEvent Time=[ 12](hrs),
END=- 1
3457 *%-----|-----|
3458 COMPUTE DUALHYD NHYDin=[ "FC-03"], CINLET=[ 0.358](cms), NINLET=[ 1],
3459 Maj NHYD=[ "FC-03-M"]
3460 MnNHYD=[ "FC-03-MN"]
3461 TMI STO=[ 1131](cu-m)
3462 *%-----|-----|
3463 ADD HYD NHYDsum=[ "FC-03-S"], NHYDs to add=[ "FC-03-M"+"FC-03-MN"]
3464 *%-----|-----|
3465 CONTINUOUS STANDHYD NHYD=[ "FC-04"], DT=[ 1]mi n, AREA=[ 12.87](ha), XI MP=[ 0.64],
TI MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3466 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
3467 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](mi n),
3468 Impervious areas: IAi mp=[ 1.57](mm), SLPI =[ 1.0](%),
LGI =[ 292.916](m), MNI =[ 0.013], SCI =[ 0](mi n),
3469 Continuous simulation parameters:
3470 IaREcper=[ 4](hrs), IaRECi mp=[ 4](hrs), InterEvent Time=[ 12](hrs),
END=- 1
3471 *%-----|-----|
3472 COMPUTE DUALHYD NHYDin=[ "FC-04"], CINLET=[ 0.741](cms), NINLET=[ 1],
3473 Maj NHYD=[ "FC-04-M"]
3474 MnNHYD=[ "FC-04-MN"]
3475 TMI STO=[ 1794](cu-m)
3476 *%-----|-----|
3477 ADD HYD NHYDsum=[ "FC-04-S"], NHYDs to add=[ "FC-04-M"+"FC-04-MN"]
3478 *%-----|-----|
3479 *#*****
3480 *# PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM Modeling
Approach, NOVATECH Report June, 2020)
3481 *# - TO JOCK RIVER
3482 *#*****
3483 CONTINUOUS STANDHYD NHYD=[ "JR-01"], DT=[ 1]mi n, AREA=[ 8.24](ha), XI MP=[ 0.64],
TI MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3484 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
3485 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](mi n),
3486 Impervious areas: IAi mp=[ 1.57](mm), SLPI =[ 1.0](%),
LGI =[ 234.379](m), MNI =[ 0.013], SCI =[ 0](mi n),
3487 Continuous simulation parameters:
3488 IaREcper=[ 4](hrs), IaRECi mp=[ 4](hrs), InterEvent Time=[ 12](hrs),
END=- 1
3489 *%-----|-----|
3490 COMPUTE DUALHYD NHYDin=[ "JR-01"], CINLET=[ 0.563](cms), NINLET=[ 1],
3491 Maj NHYD=[ "JR-01-M"]
3492 MnNHYD=[ "JR-01-MN"]
3493 TMI STO=[ 1040](cu-m)
3494 *%-----|-----|
3495 ADD HYD NHYDsum=[ "JR-01-S"], NHYDs to add=[ "JR-01-M"+"JR-01-MN"]
3496 *%-----|-----|
3497 CONTINUOUS STANDHYD NHYD=[ "JR-02"], DT=[ 1]mi n, AREA=[ 1.59](ha), XI MP=[ 0.64],
TI MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3498 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
3499 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](mi n),
3500 Impervious areas: IAi mp=[ 1.57](mm), SLPI =[ 1.0](%),
LGI =[ 102.956](m), MNI =[ 0.013], SCI =[ 0](mi n),
3501 Continuous simulation parameters:

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3502 IaRECper=[ 4]( hrs), IaRECI mp=[ 4]( hrs), InterEventTime=[ 12]( hrs),
END=- 1
3503 *%-----|-----|
3504 COMPUTE DUALHYD NHYDin=["JR-02"], CI NLET=[ 0.153]( cms), NI NLET=[ 1],
3505 Mij NHYD=["JR-02-M"]
3506 MnNHYD=["JR-02-MN"]
3507 TMSTO=[ 153]( cu-m)
3508 *%-----|-----|
3509 ADD HYD NHYDsum=["JR-02-S"], NHYDs to add=["JR-02-M"+"JR-02-MN"]
3510 *%-----|-----|
3511 *#*****|*****|
3512 *# Catchment FRASER
3513 *# - To Fraser-Clarke drain (north of the Jock)
3514 *# - Developed land with assumed 43% imp.
3515 *# - 2020-12-17 Change Fraser area to be 35.1 as measured from QGIS
3516 *# - 2020-12-17 All Fraser is undeveloped (Nashyd)
3517 *#*****|*****|
3518 CONTINUOUS NASHYD NHYD=["FRASER-DRN"], DT=[ 1] min, AREA=[ 13.65]( ha),
3519 DWF=[ 0]( cms), CN/C=[ 77], IA=[ 4.67]( mm),
3520 N=[ 3], TP=[ 0.4258] hrs,
3521 Continuous simulation parameters:
3522 IaRECper=[ 4]( hrs),
3523 SMN=[ -1]( mm), SMAX=[ -1]( mm), SK=[ 0.010]/( mm),
3524 InterEventTime=[ 12]( hrs)
3525 Baseflow simulation parameters:
3526 BaseFlowOption=[ 1],
3527 InitGWResVol=[ 50]( mm), GWResK=[ 0.96]( mm/day/mm)
3528 VHydCond=[ 0.055]( mm/hr), END=- 1
3529 *
3530 CONTINUOUS STANDHYD NHYD=["FRASER-D"], DT=[ 1] min, AREA=[ 21.61]( ha),
3531 XI MP=[ 0.585], TI MP=[ 0.585], DWF=[ 0]( cms), LOSS=[ 2],
3532 SCS curve number CN=[ 80],
3533 Pervious surfaces: IAper=[ 4.67]( mm), SLPP=[ 1]( %),
3534 LGP=[ 40]( m), MNP=[ 0.25], SCP=[ 0]( min),
3535 Impervious surfaces: IAimp=[ 1.57]( mm), SLPI=[ 1]( %),
3536 LGL=[ 379.561]( m), MNI=[ 0.013], SCI=[ 0]( min),
3537 Continuous simulation parameters:
3538 IaRECper=[ 4]( hrs), IaRECI mp=[ 4]( hrs),
3539 SMN=[ -1]( mm), SMAX=[ -1]( mm), SK=[ 0.010]/( mm),
3540 InterEventTime=[ 18]( hrs), END=- 1
3541 *%-----|-----|
3542 COMPUTE DUALHYD NHYDin=["FRASER-D"], CI NLET=[ 2.281]( cms), NI NLET=[ 1],
3543 Mij NHYD=["FRASER-J"]
3544 MnNHYD=["FRASER-N"]
3545 TMSTO=[ 9999999]( cu-m)
3546 *%-----|-----|
3547 ADD HYD NHYDsum=["FRASER-S"], NHYDs to add=["FRASER-J"+"FRASER-N"]
3548 *%-----|-----|
3549 *ROUTE RESERVOIR NHYDout=["MS_P20"], NHYDin=["FRASER"],
3550 * RDT=[ 1]( min),
3551 * TABLE of ( OUTFLOW STORAGE ) values
3552 * ( cms ) - ( ha-m)
3553 * [ 0.0 , 0.0 ]
3554 * [ 0.04 , 0.36 ]
3555 * [ -1 , -1 ] (max twenty pts)
3556 * NHYDovf=["P20-OVF"]
3557 *%-----|-----|
3558 ADD HYD NHYDsum=["4241"], NHYDs to
add=["KB-Pond3"+"S-1-B"+"FRASER-DRN"+"FRASER-S"+"N_KB"+"FC-01-S"+"FC-02-S"+"FC-03-S"]
3559 *%-----|-----|
3560 SAVE HYD NHYD=["4241"], # OF PCYCLES=[ -1], ICASEsh=[ 1]
3561 HYD_COMMENT=["Total Flows at Ken-Burnett Outlet"]
3562 *%-----|-----|
3563 *# Hydrograph from Node Ken-Burnett to station 3633
3564 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4241
3565 *#

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3566 ROUTE CHANNEL      NHYDout=["4241-out"], NHYDin=["4241"], RDT=[1](min),
3567 CHLGTH=[294](m), CHSLOPE=[0.1088](%), FPSLOPE=[0.1088](%),
3568 SECNUM=[1.0], NSEG=[3]
3569 ( SEGROUGH, SEGDI ST (m))=[0.05, -20.12
3570 -0.035, 45.26
3571 0.05, 403.84] NSEG times
3572 ( DI STANCE (m), ELEVATI ON (m))=[
3573 [-909.72, 95 ]
3574 [-907.09, 94.5 ]
3575 [-904.65, 94 ]
3576 [-902.26, 93.5 ]
3577 [-44.51, 91.5 ]
3578 [-25.1, 91.5 ]
3579 [-20.98, 91 ]
3580 [-20.61, 90.5 ]
3581 [-20.12, 90 ]
3582 [-6.13, 87.26 ]
3583 [17.51, 86.56 ]
3584 [31.37, 87.2 ]
3585 [45.26, 90 ]
3586 [50.41, 90.5 ]
3587 [63.06, 91 ]
3588 [134.5, 91.5 ]
3589 [190.63, 92 ]
3590 [251.98, 92.5 ]
3591 [321.32, 93.5 ]
3592 [403.84, 95 ]
3593 *%-----|-----|
3594 ADD HYD      NHYDsum=["SN_KB"], NHYDs to
add=["4241-out"+"FC-04-S"+"JR-01-S"+"JR-02-S"]
3595 *%-----|-----|
3596 SAVE HYD     NHYD=["SN_KB"], # OF PCYCLES=[-1], ICASEsh=[1]
3597 HYD_COMMENT=["Total Flows before Station 3633"]
3598 *%-----|-----|
3599 *# Hydrograph from Station 3633 to Node Todd
3600 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
3601 *# JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and
change the slope from 0.0498% to 0.24671% That is because of adding station 4241
between station 4534 and station 3633
*#
3602 ROUTE CHANNEL      NHYDout=["N_TO"], NHYDin=["SN_KB"], RDT=[1](min),
3603 CHLGTH=[608](m), CHSLOPE=[0.24671](%), FPSLOPE=[0.24671](%),
3604 SECNUM=[1.0], NSEG=[3]
3605 ( SEGROUGH, SEGDI ST (m))=[0.05, -23.74
3606 -0.035, 23.74
3607 0.05, 26.50] NSEG times
3608 ( DI STANCE (m), ELEVATI ON (m))=[
3609 -29.24, 91.0
3610 -27.41, 90.5
3611 -25.64, 90
3612 -23.74, 89.5
3613 -22, 89.26
3614 -20, 88.51
3615 -19, 88.32
3616 -15, 88.1
3617 -10, 88.11
3618 -5, 88.17
3619 0, 88.27
3620 5, 88.19
3621 10, 88.06
3622 15, 88.48
3623 16, 88.7
3624 23.74, 89.5
3625 24.68, 90
3626 25.57, 90.5
3627 26.50, 91.0

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3629 * [-29.24, 91]
3630 * [-27.41, 90.5]
3631 * [-25.64, 90]
3632 * [-23.74, 89.5]
3633 * [-22, 89.26]
3634 * [-20, 88.51]
3635 * [-19, 88.32]
3636 * [-15, 88.1]
3637 * [-10, 88.11]
3638 * [-5, 88.17]
3639 * [0, 88.27]
3640 * [5, 88.19]
3641 * [10, 88.06]
3642 * [15, 88.48]
3643 * [16, 88.7]
3644 * [23.74, 89.5]
3645 * [24.68, 90]
3646 * [25.57, 90.5]
3647 *%-----|-----|
3648 *#*****|
3649 *# Catchment Greenbank
3650 *# - To Greenbank Drain (south of the Jock)
3651 *# - JFSA 2021-01-18 add Greenbank pond as per JFSA, P598(06)-15, June 2016
3652 *# - JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
3653 *#*****|
3654 CONTINUOUS STANDHYD NHYD=["Greenbank"], DT=[1] min, AREA=[36.6] (ha),
3655 XI MP=[0.639], TI MP=[0.682], DWF=[0] (cms), LOSS=[2],
3656 SCS curve number CN=[77],
3657 Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
3658 LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
3659 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
3660 LGI=[493.96] (m), MNI=[0.013], SCI=[0] (min),
3661 Continuous simulation parameters:
3662 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
3663 S M N=[-1] (mm), S M A X=[-1] (mm), S K=[0.010] / (mm),
3664 I n t e r E v e n t T i m e=[18] (hrs), E N D=-1
3665 *%-----|-----|
3666 ROUTE RESERVOIR NHYDout=["GreenB_MN"], NHYDin=["Greenbank"],
3667 RDT=[1] (min),
3668 TABLE of ( OUTFLOW STORAGE ) values
3669 ( cms ) - ( ha-m )
3670 [ 0.0 , 0.0 ]
3671 [ 0.033 , 0.084 ]
3672 [ 0.039 , 0.201 ]
3673 [ 0.113 , 0.292 ]
3674 [ 0.237 , 0.386 ]
3675 [ 0.382 , 0.484 ]
3676 [ 0.539 , 0.585 ]
3677 [ 0.7 , 0.692 ]
3678 [ 0.86 , 0.804 ]
3679 [ 4.684 , 0.922 ]
3680 [ 11.539 , 1.052 ]
3681 [ 20.867 , 1.168 ]
3682 [ 103.616 , 1.974 ]
3683 [ -1 , -1 ] (max twenty pts)
3684 NHYDovf=["GreenB_M"],
3685 *%-----|-----|
3686 *%-----|-----|
3687 ADD HYD NHYDsum=["GreenB"], NHYDs to add=["N_TO"+"GreenB_M"+"GreenB_MN"]
3688 *%-----|-----|
3689 SAVE HYD NHYD=["GreenB"], # OF PCYCLES=[-1], I CASEs h=[1]
3690 HYD_COMMENT=["Total Flows at Greenbank Drain"]
3691 *%-----|-----|
3692 *#*****|
3693 *# Catchment TODD
3694 *# - To Todd Drain (south of the Jock)

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3695 *# - Subdivision with 43% imp. as per Barrhaven South M5S
3696 *# - 2020-11-30 increase imp. based on P598(04)-11
3697 *# - 2020-11-30 update TODD Tributary Drainage Area to = 146.015 ha based on
P598(04)-11
3698 *# - 2020-11-30 split TODD Drainage Area to MAJOR, MINOR, POND and ALL
3699 *#*****
3700 *# - JFSA 2021-01-19 add "TODD_MN1" as part of Clarke("W_CLAR_M") and remove it
from Todd
3701 *CONTINUOUS STANDHYD NHYD=["TODD_MN1"], DT=[1] min, AREA=[1.772](ha),
3702 * XI MP=[0.53], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3703 * SCS curve number CN=[77],
3704 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3705 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3706 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3707 * LGI=[108.689](m), MNI=[0.013], SCI=[0](min),
3708 * Continuous simulation parameters:
3709 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3710 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3711 * InterEventTime=[18](hrs), END=-1
3712 *%-----|-----
3713 CONTINUOUS STANDHYD NHYD=["TODD_MN2"], DT=[1] min, AREA=[2.1](ha),
3714 * XI MP=[0.53], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3715 * SCS curve number CN=[77],
3716 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3717 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3718 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3719 * LGI=[118.322](m), MNI=[0.013], SCI=[0](min),
3720 * Continuous simulation parameters:
3721 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3722 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3723 * InterEventTime=[18](hrs), END=-1
3724 *%-----|-----
3725 CONTINUOUS STANDHYD NHYD=["TODD_MN3"], DT=[1] min, AREA=[0.117](ha),
3726 * XI MP=[0.53], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3727 * SCS curve number CN=[77],
3728 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3729 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3730 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3731 * LGI=[27.928](m), MNI=[0.013], SCI=[0](min),
3732 * Continuous simulation parameters:
3733 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3734 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3735 * InterEventTime=[18](hrs), END=-1
3736 *%-----|-----
3737 CONTINUOUS STANDHYD NHYD=["TODD_M"], DT=[1] min, AREA=[30.230](ha),
3738 * XI MP=[0.52], TI MP=[0.64], DWF=[0](cms), LOSS=[2],
3739 * SCS curve number CN=[77],
3740 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3741 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3742 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3743 * LGI=[448.925](m), MNI=[0.013], SCI=[0](min),
3744 * Continuous simulation parameters:
3745 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3746 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3747 * InterEventTime=[18](hrs), END=-1
3748 *%-----|-----
3749 * -JFSA 2021-01-19 update "TODD_ALL" area from 108.741 ha to 112.908 ha based on
GIS measurements (148.41-30.23-0.117-2.1-3.055=112.908 ha)
3750 CONTINUOUS STANDHYD NHYD=["TODD_ALL"], DT=[1] min, AREA=[112.908](ha),
3751 * XI MP=[0.52], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3752 * SCS curve number CN=[77],
3753 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3754 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3755 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3756 * LGI=[867.594](m), MNI=[0.013], SCI=[0](min),
3757 * Continuous simulation parameters:

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3758 IaREcper=[ 4](hrs), IaREcImp=[ 4](hrs),
3759 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3760 InterEventTime=[ 18](hrs), END=- 1
3761 *%-----|-----
3762 CONTINUOUS STANDHYD NHYD=["TODD_P"], DT=[ 1]min, AREA=[ 3.055](ha),
3763 XI MP=[ 0.63], TI MP=[ 0.63], DWF=[ 0](cms), LOSS=[ 2],
3764 SCS curve number CN=[ 77],
3765 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
3766 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
3767 Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
3768 LGI=[ 142.712](m), MNI=[ 0.013], SCI=[ 0](min),
3769 Continuous simulation parameters:
3770 IaREcper=[ 4](hrs), IaREcImp=[ 4](hrs),
3771 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3772 InterEventTime=[ 18](hrs), END=- 1
3773 *%-----|-----
3774 *%-----|-----
3775 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3776 *CONTINUOUS STANDHYD NHYD=["TODD_DEVL"], DT=[ 1]min, AREA=[ 15.87](ha),
3777 * XI MP=[ 0.63], TI MP=[ 0.63], DWF=[ 0](cms), LOSS=[ 2],
3778 * SCS curve number CN=[ 77],
3779 * Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
3780 * LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
3781 * Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
3782 * LGI=[ 325.27](m), MNI=[ 0.013], SCI=[ 0](min),
3783 * Continuous simulation parameters:
3784 * IaREcper=[ 4](hrs), IaREcImp=[ 4](hrs),
3785 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3786 * InterEventTime=[ 18](hrs), END=- 1
3787 *%-----|-----
3788 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3789 *CONTINUOUS NASHYD NHYD=["TODD_UnD"], DT=[ 1]min, AREA=[ 12.47](ha),
3790 * DWF=[ 0](cms), CN C=[ 77], IA=[ 4.67](mm),
3791 * N=[ 3], TP=[ 1.10]hrs,
3792 * Continuous simulation parameters:
3793 * IaREcper=[ 4](hrs),
3794 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
3795 * InterEventTime=[ 12](hrs)
3796 * Baseflow simulation parameters:
3797 * BaseFlowOption=[ 1],
3798 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
3799 * VHydCond=[ 0.055](mm/hr), END=- 1
3800 *%-----|-----
3801 *# 5- Year + 12% Capture
3802 *COMPUTE DUALHYD NHYDin=["TODD_Mj"], CINLET=[ 3.314](cms), NINLET=[ 1],
3803 * MjNHYD=["TODD_Mj"]
3804 * MnNHYD=["TODD_Mn"]
3805 * TMSTO=[ 0.1](cu-m)
3806 ROUTE RESERVOIR NHYDout=["TODD_Mn"], NHYDin=["TODD_Mj"],
3807 RDT=[ 1](min),
3808 TABLE of ( OUTFLOW STORAGE ) values
3809 ( cms ) - ( ha-m)
3810 [ 0.0 , 0.0 ]
3811 [ 3.314 , 0.0001 ]
3812 [ -1 , -1 ] (max twenty pts)
3813 NHYDovf=["TODD_Mj"],
3814 *%-----|-----
3815 *# 5- Year + 12% Capture
3816 *COMPUTE DUALHYD NHYDin=["TODD_MN1"], CINLET=[ 0.227](cms), NINLET=[ 1],
3817 * MjNHYD=["TODD_MN1j"]
3818 * MnNHYD=["TODD_MN1n"]
3819 * TMSTO=[ 0.1](cu-m)
3820 *ROUTE RESERVOIR NHYDout=["TODD_MN1n"], NHYDin=["TODD_MN1"],

```

```

3821 *          RDT=[ 1](mi n),
3822 *          TABLE of ( OUTFLOW STORAGE ) values
3823 *                ( cms ) - ( ha- m)
3824 *                [ 0.0 , 0.0 ]
3825 *                [ 0.227 , 0.0001 ]
3826 *                [ -1 , -1 ] (max t went y pts)
3827 *          NHYDovf=[ "TODD_MN1j" ] ,
3828 *%-----|-----|
3829 *COMPUTE DUALHYD  NHYDi n=[ "TODD_MN2" ], CI NLET=[ 0.268]( cms ), NI NLET=[ 1],
3830 *                Mj NHYD=[ "TODD_MN2j" ]
3831 *                MnNHYD=[ "TODD_MN2n" ]
3832 *                TM STO=[ 0.1]( cu- m)
3833 ROUTE RESERVOIR  NHYDout =[ "TODD_MN2n" ] , NHYDi n=[ "TODD_MN2" ] ,
3834 RDT=[ 1](mi n),
3835          TABLE of ( OUTFLOW STORAGE ) values
3836                ( cms ) - ( ha- m)
3837                [ 0.0 , 0.0 ]
3838                [ 0.268 , 0.0001 ]
3839                [ -1 , -1 ] (max t went y pts)
3840          NHYDovf=[ "TODD_MN2j" ] ,
3841 *%-----|-----|
3842 *COMPUTE DUALHYD  NHYDi n=[ "TODD_MN3" ], CI NLET=[ 0.016]( cms ), NI NLET=[ 1],
3843 *                Mj NHYD=[ "TODD_MN3j" ]
3844 *                MnNHYD=[ "TODD_MN3n" ]
3845 *                TM STO=[ 0.1]( cu- m)
3846 ROUTE RESERVOIR  NHYDout =[ "TODD_MN3n" ] , NHYDi n=[ "TODD_MN3" ] ,
3847 RDT=[ 1](mi n),
3848          TABLE of ( OUTFLOW STORAGE ) values
3849                ( cms ) - ( ha- m)
3850                [ 0.0 , 0.0 ]
3851                [ 0.016 , 0.0001 ]
3852                [ -1 , -1 ] (max t went y pts)
3853          NHYDovf=[ "TODD_MN3j" ] ,
3854 *%-----|-----|
3855 *          -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
3856 CONTINUOUS STANDHYD NHYD=[ "A2" ], DT=[ 1]mi n, AREA=[ 25.5]( ha),
3857 XI MP=[ 0.42], TI MP=[ 0.52], DWF=[ 0]( cms ), LOSS=[ 2],
3858 SCS curve number CN=[ 75],
3859 Pervious surfaces: I Aper=[ 4.67]( mm), SLPP=[ 1]( %),
3860                LGP=[ 40]( m), MNP=[ 0.25], SCP=[ 0]( mi n),
3861 Impervious surfaces: I Ai mp=[ 1.57]( mm), SLPI =[ 1]( %),
3862                LGI =[ 566]( m), MNI =[ 0.013], SCI =[ 0]( mi n),
3863 Continuous simulation parameters:
3864 I aRECper=[ 4]( hrs), I aRECI mp=[ 4]( hrs),
3865 SM N=[ -1]( mm), SMAX=[ -1]( mm), SK=[ 0.010]/( mm),
3866 I nterEvent Ti me=[ 18]( hrs), END=- 1
3867 *%-----|-----|
3868 COMPUTE DUALHYD  NHYDi n=[ "A2" ], CI NLET=[ 1.818]( cms ), NI NLET=[ 1],
3869 Mj NHYD=[ "A2- M" ]
3870 MnNHYD=[ "A2- MN" ]
3871 TM STO=[ 924]( cu- m)
3872 *%-----|-----|
3873 ADD HYD          NHYDs um=[ "TODD" ], NHYDs to
add=[ "TODD_MN2n"+"TODD_MN3n"+"TODD_Mj"+"TODD_P"+"TODD_ALL"+"W_CLAR_Mn" ]
3874 *%-----|-----|
3875 SAVE HYD        NHYD=[ "TODD" ], # OF PCYCLES=[ -1], I CASEs h=[ 1]
3876 HYD_COMMENT=[ "Tot al Fl ows at Todd Dr ai n" ]
3877 *%-----|-----|
3878 *#*****|*****|
3879 *# Todd Pond 3
3880 *# - Rating curve obtained from Barrhaven South M5S modeling
3881 *# - stantec 2007, Tributary Drainage Area to M5S Pond 3 = 193 ha
3882 *#*****|*****|
3883 ROUTE RESERVOIR  NHYDout =[ "M5_P3" ], NHYDi n=[ "TODD" ],
3884 RDT=[ 1](mi n),

```

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3885                                     TABLE of ( OUTFLOW STORAGE ) values
3886                                     ( cms ) - ( ha - m )
3887                                     [ 0.0 , 0.0 ]
3888                                     [ 0.014 , 0.155 ]
3889                                     [ 0.048 , 0.394 ]
3890                                     [ 0.061 , 0.56 ]
3891                                     [ 0.08 , 0.909 ]
3892                                     [ 0.088 , 1.089 ]
3893                                     [ 0.109 , 1.652 ]
3894                                     [ 0.118 , 1.952 ]
3895                                     [ 0.122 , 2.099 ]
3896                                     [ 1.972 , 2.269 ]
3897                                     [ 9.135 , 2.598 ]
3898                                     [ 15.608 , 2.826 ]
3899                                     [ 19.256 , 2.942 ]
3900                                     [ 27.282 , 3.181 ]
3901                                     [ 40.957 , 3.55 ]
3902                                     [ 56.372 , 3.929 ]
3903                                     [ 73.349 , 4.317 ]
3904                                     [ 85.469 , 4.579 ]
3905                                     [ 104.771 , 4.977 ]
3906                                     [ -1 , -1 ] (max twenty pts)
3907                                     NHYDovf=[ "P3- OVF" ]
3908 *%-----|-----|
3909 ADD HYD                                     NHYDs um=[ "SN_TO" ], NHYDs to
add=[ "GreenB"+"MS_P3"+"P3- OVF"+"TODD_MN2j"+"A2- M" ]
3910 *%-----|-----|
3911 SAVE HYD                                    NHYD=[ "SN_TO" ], # OF PCYCLES=[ -1 ], I CASEs h=[ 1 ]
3912                                     HYD_COMMENT=[ "Total Flows at Todd Drain" ]
3913 *%-----|-----|
3914 *#
3915 *# Hydrograph from Todd Drain routed to Corrigan Drain
3916 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
3917 *# 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so
the model will be more stable and give reasonable results. It is justifiable as ROUTE
CHANNELs aren't well suited to really flat slopes.
3918 *
3919 ROUTE CHANNEL                               NHYDout = [ "N_TO" ] , NHYDin = [ "SN_TO" ] ,
3920 RDT=[ 1 ] ( min ),
3921 CHLGTH=[ 280 ] ( m ) , CHSLOPE=[ 0.05 ] ( % ) ,
3922                                     FPSLOPE=[ 0.05 ] ( % ) ,
3923 SECNUM=[ 1.0 ] , NSEG=[ 3 ]
3924 ( SEGROUGH, SEGDI ST ( m ) ) =
3925 [ 0.075, -17.72
3926 -0.045, 17.72
3927 0.075, 80.62 ] NSEG times
3928 ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
3929 [ -83.32, 90.00 ]
3930 [ -81.36, 89.50 ]
3931 [ -79.12, 89.00 ]
3932 [ -76.13, 88.50 ]
3933 [ -20.46, 88.00 ]
3934 [ -19.36, 87.50 ]
3935 [ -18.51, 87.00 ]
3936 [ -17.72, 86.50 ]
3937 [ -11.95, 85.24 ]
3938 [ -0.11, 85.12 ]
3939 [ 11.49, 85.20 ]
3940 [ 17.72, 86.50 ]
3941 [ 19.74, 87.00 ]
3942 [ 21.22, 87.50 ]
3943 [ 22.68, 88.00 ]
3944 [ 24.28, 88.50 ]
3945 [ 26.79, 89.00 ]
3946 [ 71.98, 90.00 ]
3947 [ 80.62, 90.50 ]

```



```

3948 *%-----|-----|
3949 SAVE HYD      NHYD=["N_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3950              HYD_COMMENT=["Total inflows at Station 2462"]
3951 *%-----|-----|
3952 *#*****|-----|
3953 *#    Catchment CORRIG
3954 *#    - To Corrigan Drain (south of the Jock)
3955 *#    - Primarily Developed (medium density)
3956 *#    - JFSA JAN 2021, add Corrigan subcatchments as per IBI, July 2008
3957 *#*****|-----|
3958 *ROUTE RESERVOIR  NHYDout=["MS_P1"], NHYDin=["CORRIG"],
3959 *              RDT=[1](min),
3960 *              TABLE of ( OUTFLOW STORAGE ) values
3961 *              (cms) - (ha-m)
3962 *              [ 0.0 , 0.0 ]
3963 *              [ 0.06 , 0.58]
3964 *              [ -1 , -1 ] (max twenty pts)
3965 *              NHYDovf=["P1-OVF"]
3966 *%-----|-----|
3967 *ADD HYD      NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
3968 *%-----|-----|
3969 *SAVE HYD      NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
3970 *              HYD_COMMENT=["Total Flows at Corrigan Drain"]
3971 *%-----|-----|
3972 *    -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.

3973 CONTINUOUS STANDHYD NHYD=["corr1"], DT=[1]min, AREA=[15.87](ha),
3974 XI MP=[0.63], TI MP=[0.63], DWF=[0](cms), LOSS=[2],
3975 SCS curve number CN=[77],
3976 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3977 LGP=[40](m), MNP=[0.25], SCP=[0](min),
3978 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3979 LGI=[325.27](m), MNI=[0.013], SCI=[0](min),
3980 Continuous simulation parameters:
3981 IARECper=[4](hrs), IARECimp=[4](hrs),
3982 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3983 InterEventTime=[18](hrs), END=-1
3984 *%-----|-----|
3985 *    -JFSA 2021-02-23 add DUALHYD for "corr1". "corr1" DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to "corr1".
3986 *    At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
3987 COMPUTE DUALHYD NHYDin=["corr1"], CNLET=[1.818](cms), NI NLET=[1],
3988 Maj NHYD=["corr1-M"]
3989 MnNHYD=["corr1-MN"]
3990 TM STO=[924](cu-m)
3991 *%-----|-----|
3992 *    -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.

3993 CONTINUOUS NASHYD NHYD=["corr2"], DT=[1]min, AREA=[12.47](ha),
3994 DWF=[0](cms), CN C=[77], IA=[4.67](mm),
3995 N=[3], TP=[1.10]hrs,
3996 Continuous simulation parameters:
3997 IARECper=[4](hrs),
3998 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3999 InterEventTime=[12](hrs)
4000 Baseflow simulation parameters:
4001 BaseFlowOption=[1],
4002 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4003 VHydCond=[0.055](mm/hr), END=-1
4004 *%-----|-----|
4005 *    -JFSA 2021-01-19 change Al-Corrig to be developed as per geottawa website and
apply the parameters of A2, the nearest sub-catchment to Al-Corrig, LGI is calculated
based on Al-Corrig area
4006 *    -JFSA 2021-01-19 update all Corrigan areas based on GIS measurements, and keep

```

LGI as it is from Corrigan Report, IBI Group, 2008 because LGI calculated is less than LGI from the Corrigan Report

```
4007 CONTINUOUS STANDHYD NHYD=["Al-Corrig"], DT=[1] min, AREA=[15.75](ha),
4008 XI MP=[0.42], TI MP=[0.52], DWF=[0](cms), LOSS=[2],
4009 SCS curve number CN=[75],
4010 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4011 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4012 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4013 LGI=[324.037](m), MNI=[0.013], SCI=[0](min),
4014 Continuous simulation parameters:
4015 IaRECper=[4](hrs), IaRECImp=[4](hrs),
4016 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4017 InterEventTime=[18](hrs), END=-1
4018 *
4019 * -JFSA 2021-01-25 add DUALHYD for Al-Corrig. Al-Corrig DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to Al-Corrig.
4020 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
4021 COMPUTE DUALHYD NHYDin=["Al-Corrig"], CINLET=[1.818](cms), NINLET=[1],
4022 Mj NHYD=["Al-M"]
4023 MnNHYD=["Al-MN"]
4024 TMS TO=[924](cu-m)
4025 *%-----|-----|
4026 *CONTINUOUS NASHYD NHYD=["Al-Corrig"], DT=[1] min, AREA=[15.75](ha),
4027 * DWF=[0](cms), CN C=[66], IA=[2.5](mm),
4028 * N=[3.0], TP=[0.36]hrs,
4029 * Continuous simulation parameters:
4030 * IaRECper=[4](hrs),
4031 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4032 * InterEventTime=[12](hrs)
4033 * Baseflow simulation parameters:
4034 * BaseFlowOption=[1],
4035 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4036 * VHydCond=[0.055](mm/hr), END=-1
4037 *%-----|-----|
4038 CONTINUOUS NASHYD NHYD=["B1"], DT=[1] min, AREA=[2.77](ha),
4039 DWF=[0](cms), CN C=[56], IA=[2.5](mm),
4040 N=[3.0], TP=[0.23]hrs,
4041 Continuous simulation parameters:
4042 IaRECper=[4](hrs),
4043 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4044 InterEventTime=[12](hrs)
4045 Baseflow simulation parameters:
4046 BaseFlowOption=[1],
4047 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
4048 VHydCond=[0.055](mm/hr), END=-1
4049 *%-----|-----|
4050 CONTINUOUS STANDHYD NHYD=["A4"], DT=[1] min, AREA=[1.27](ha),
4051 XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
4052 SCS curve number CN=[75],
4053 Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
4054 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4055 Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
4056 LGI=[253](m), MNI=[0.013], SCI=[0](min),
4057 Continuous simulation parameters:
4058 IaRECper=[4](hrs), IaRECImp=[4](hrs),
4059 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4060 InterEventTime=[18](hrs), END=-1
4061 *%-----|-----|
4062 COMPUTE DUALHYD NHYDin=["A4"], CINLET=[0.405](cms), NINLET=[1],
4063 Mj NHYD=["A4-M"]
4064 MnNHYD=["A4-MN"]
4065 TMS TO=[68](cu-m)
4066 *%-----|-----|
4067 ADD HYD NHYDs um=["MH101"], NHYDs to
add=["Al-M"+"Al-MN"+"corr1-M"+"corr1-MN"+"corr2"+"B1"+"A4-MN"]
```

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4068 *%-----|-----|
4069 SAVE HYD      NHYD=["MHI01"], # OF PCYCLES=[-1], I CASEs h=[1]
4070              HYD_COMMENT=["Total Flows at MHI01"]
4071 *%-----|-----|
4072 ROUTE PIPE    PTYPE=[1] circ, NHYDout=["101-102"], RNUMBER=[1.0], PDI AM=[1050] (mm),
4073              PLNGTH=[368] (m), PROUGH=[0.013], PSLOPE=[0.0054] (m/m),
              NHYDin=["MHI01"], RDT=[1]
4074 *%-----|-----|
4075 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
4076 *CONTINUOUS STANDHYD NHYD=["A2"], DT=[1] min, AREA=[25.5] (ha),
4077 *              XI MP=[0.42], TI MP=[0.52], DWF=[0] (cms), LOSS=[2],
4078 *              SCS curve number CN=[75],
4079 *              Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
4080 *              LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
4081 *              Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
4082 *              LGI=[566] (m), MNI=[0.013], SCI=[0] (min),
4083 *              Continuous simulation parameters:
4084 *              I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
4085 *              SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010] / (mm),
4086 *              Inter Event Ti me=[18] (hrs), END=-1
4087 *%-----|-----|
4088 *COMPUTE DUALHYD NHYDin=["A2"], CI NLET=[1.818] (cms), NI NLET=[1],
4089 *              M a j NHYD=["A2-M"]
4090 *              M n NHYD=["A2-MN"]
4091 *              TM STO=[924] (cu-m)
4092 *%-----|-----|
4093 ADD HYD       NHYDs um=["MHI02"], NHYDs to add=["A2-MN"+"101-102"]
4094 *%-----|-----|
4095 SAVE HYD      NHYD=["MHI02"], # OF PCYCLES=[-1], I CASEs h=[1]
4096              HYD_COMMENT=["Total Flows at MHI02"]
4097 *%-----|-----|
4098 CONTINUOUS STANDHYD NHYD=["A5"], DT=[1] min, AREA=[1.6] (ha),
4099 *              XI MP=[0.71], TI MP=[0.71], DWF=[0] (cms), LOSS=[2],
4100 *              SCS curve number CN=[75],
4101 *              Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
4102 *              LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
4103 *              Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
4104 *              LGI=[300] (m), MNI=[0.013], SCI=[0] (min),
4105 *              Continuous simulation parameters:
4106 *              I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
4107 *              SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010] / (mm),
4108 *              Inter Event Ti me=[18] (hrs), END=-1
4109 *%-----|-----|
4110 ADD HYD       NHYDs um=["A5T"], NHYDs to add=["A4-M"+"A5"]
4111 *%-----|-----|
4112 COMPUTE DUALHYD NHYDin=["A5T"], CI NLET=[0.357] (cms), NI NLET=[1],
4113 *              M a j NHYD=["A5-M"]
4114 *              M n NHYD=["A5-MN"]
4115 *              TM STO=[60] (cu-m)
4116 *%-----|-----|
4117 * -JFSA Jan. 2021, A3 is a part of Todd so it is removed
4118 * -JFSA Jan. 2021, "A2-M" added to "Todd"
4119 *CONTINUOUS STANDHYD NHYD=["A3"], DT=[1] min, AREA=[18.4] (ha),
4120 *              XI MP=[0.58], TI MP=[0.65], DWF=[0] (cms), LOSS=[2],
4121 *              SCS curve number CN=[75],
4122 *              Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
4123 *              LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
4124 *              Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
4125 *              LGI=[450] (m), MNI=[0.013], SCI=[0] (min),
4126 *              Continuous simulation parameters:
4127 *              I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
4128 *              SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010] / (mm),
4129 *              Inter Event Ti me=[18] (hrs), END=-1
4130 *%-----|-----|
4131 *ADD HYD       NHYDs um=["A3-A2M"], NHYDs to add=["A2-M"+"A3"]

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4132 *%-----|-----|
4133 *COMPUTE DUALHYD   NHYDin=[" A3- A2M"], CINLET=[ 2.208](cms), NINLET=[ 1],
4134 *                   MjNHYD=[" A3R- M"]
4135 *                   MnNHYD=[" A3R- MN"]
4136 *                   TMSSTO=[ 908](cu-m)
4137 *%-----|-----|
4138 ROUTE PIPE         PTYPE=[ 1]circ, NHYDout=[" 102- 103"], RNUMBER=[ 1.0], PDIAM=[ 1500](mm),
4139                   PLNGTH=[ 504](m), PROUGH=[ 0.013], PSLOPE=[ 0.0028](m/m),
                   NHYDin=[" MH102"], RDT=[ 1]
4140 *%-----|-----|
4141 ADD HYD            NHYDsum=[" MH103"], NHYDs to add=[" 102- 103"+" A5- MN"]
4142 *%-----|-----|
4143 SAVE HYD           NHYD=[" MH103"], # OF PCYCLES=[ - 1], ICASEsh=[ 1]
4144                   HYD_COMMENT=[" Total Flows at MH103"]
4145 *%-----|-----|
4146 ROUTE PIPE         PTYPE=[ 1]circ, NHYDout=[" 103- 104"], RNUMBER=[ 1.0], PDIAM=[ 1650](mm),
4147                   PLNGTH=[ 438](m), PROUGH=[ 0.013], PSLOPE=[ 0.0046](m/m),
                   NHYDin=[" MH103"], RDT=[ 1]
4148 *%-----|-----|
4149 CONTINUOUS STANDHYD NHYD=[" A6"], DT=[ 1]min, AREA=[ 1.56](ha),
4150                   XI MP=[ 0.71], TI MP=[ 0.71], DWF=[ 0](cms), LOSS=[ 2],
4151                   SCS curve number CN=[ 75],
4152                   Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4153                   LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4154                   Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4155                   LGI=[ 280](m), MNI=[ 0.013], SCI=[ 0](min),
4156                   Continuous simulation parameters:
4157                   IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4158                   SMN=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0.010]/(mm),
4159                   InterEventTime=[ 18](hrs), END=- 1
4160 *%-----|-----|
4161 ADD HYD            NHYDsum=[" A6T"], NHYDs to add=[" A5- M"+" A6"]
4162 *%-----|-----|
4163 COMPUTE DUALHYD   NHYDin=[" A6T"], CINLET=[ 0.357](cms), NINLET=[ 1],
4164 *                   MjNHYD=[" A6- M"]
4165 *                   MnNHYD=[" A6- MN"]
4166 *                   TMSSTO=[ 60](cu-m)
4167 *%-----|-----|
4168 * -JFSA Jan. 2021, A7-corrig is a part of Todd so it is removed
4169 *CONTINUOUS STANDHYD NHYD=[" A7-corrig"], DT=[ 1]min, AREA=[ 11.8](ha),
4170 *                   XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4171 *                   SCS curve number CN=[ 75],
4172 *                   Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4173 *                   LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4174 *                   Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4175 *                   LGI=[ 438](m), MNI=[ 0.013], SCI=[ 0](min),
4176 *                   Continuous simulation parameters:
4177 *                   IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
4178 *                   SMN=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0.010]/(mm),
4179 *                   InterEventTime=[ 18](hrs), END=- 1
4180 *%-----|-----|
4181 *ADD HYD            NHYDsum=[" A7- A3RM"], NHYDs to add=[" A3R- M"+" A7-corrig"]
4182 *%-----|-----|
4183 *COMPUTE DUALHYD   NHYDin=[" A7- A3RM"], CINLET=[ 1.003](cms), NINLET=[ 1],
4184 *                   MjNHYD=[" A7R- M"]
4185 *                   MnNHYD=[" A7R- MN"]
4186 *                   TMSSTO=[ 496](cu-m)
4187 *%-----|-----|
4188 ADD HYD            NHYDsum=[" MH104"], NHYDs to add=[" A6- MN"+" 103- 104"+" TODD_Mn"]
4189 *%-----|-----|
4190 SAVE HYD           NHYD=[" MH104"], # OF PCYCLES=[ - 1], ICASEsh=[ 1]
4191                   HYD_COMMENT=[" Total Flows at MH104"]
4192 *%-----|-----|
4193 CONTINUOUS STANDHYD NHYD=[" B2"], DT=[ 1]min, AREA=[ 12.31](ha),
4194                   XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4195                   SCS curve number CN=[ 75],

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4196          Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4197                LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
4198          Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 1](%),
4199                LGI=[ 417](m), MNI=[ 0.013], SCI=[ 0](mi n),
4200          Continuous simulation parameters:
4201          I a RE C per=[ 4](hrs), I a RE C i mp=[ 4](hrs),
4202          SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4203          Inter Event Ti me=[ 18](hrs), END=- 1
4204          *%-----|
4205          COMPUTE DUALHYD          NHYD i n=[ "B2"], CI NLET=[ 1.029](cms), NI NLET=[ 1],
4206          Major NHYD=[ "B2- M"]
4207          Minor NHYD=[ "B2- MN"]
4208          TM S TO=[ 508](cu-m)
4209          *%-----|
4210          ROUTE PI PE              PTYPE=[ 1]circ, NHYDout=[ "315-333"], RNUMBER=[ 1.0], PDI AM=[ 1200](mm),
4211          PLNGTH=[ 254](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
          NHYD i n=[ "B2- MN"], RDT=[ 1]
4212          *%-----|
4213          CONTINUOUS STANDHYD     NHYD=[ "B3"], DT=[ 1]mi n, AREA=[ 5.59](ha),
4214          XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4215          SCS curve number CN=[ 75],
4216          Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4217                LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
4218          Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 1](%),
4219                LGI=[ 345](m), MNI=[ 0.013], SCI=[ 0](mi n),
4220          Continuous simulation parameters:
4221          I a RE C per=[ 4](hrs), I a RE C i mp=[ 4](hrs),
4222          SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4223          Inter Event Ti me=[ 18](hrs), END=- 1
4224          *%-----|
4225          COMPUTE DUALHYD          NHYD i n=[ "B3"], CI NLET=[ 0.459](cms), NI NLET=[ 1],
4226          Major NHYD=[ "B3- M"]
4227          Minor NHYD=[ "B3- MN"]
4228          TM S TO=[ 227](cu-m)
4229          *%-----|
4230          ADD HYD                  NHYDs um=[ "MH333"], NHYDs to add=[ "B3- MN"+"315-333"]
4231          *%-----|
4232          SAVE HYD                 NHYD=[ "MH333"], # OF PCYCLES=[ -1], I CASEs h=[ 1]
4233          HYD_COMMENT=[ "Total Flows at MH333"]
4234          *%-----|
4235          ROUTE PI PE              PTYPE=[ 1]circ, NHYDout=[ "333-335"], RNUMBER=[ 1.0], PDI AM=[ 1200](mm),
4236          PLNGTH=[ 251](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
          NHYD i n=[ "MH333"], RDT=[ 1]
4237          *%-----|
4238          ROUTE PI PE              PTYPE=[ 1]circ, NHYDout=[ "335-338"], RNUMBER=[ 1.0], PDI AM=[ 1200](mm),
4239          PLNGTH=[ 185](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
          NHYD i n=[ "333-335"], RDT=[ 1]
4240          *%-----|
4241          ROUTE PI PE              PTYPE=[ 1]circ, NHYDout=[ "338-340"], RNUMBER=[ 1.0], PDI AM=[ 1350](mm),
4242          PLNGTH=[ 233](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
          NHYD i n=[ "335-338"], RDT=[ 1]
4243          *%-----|
4244          CONTINUOUS STANDHYD     NHYD=[ "B4"], DT=[ 1]mi n, AREA=[ 7.6](ha),
4245          XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4246          SCS curve number CN=[ 75],
4247          Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4248                LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
4249          Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 1](%),
4250                LGI=[ 388](m), MNI=[ 0.013], SCI=[ 0](mi n),
4251          Continuous simulation parameters:
4252          I a RE C per=[ 4](hrs), I a RE C i mp=[ 4](hrs),
4253          SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4254          Inter Event Ti me=[ 18](hrs), END=- 1
4255          *%-----|
4256          COMPUTE DUALHYD          NHYD i n=[ "B4"], CI NLET=[ 0.655](cms), NI NLET=[ 1],
4257          Major NHYD=[ "B4- M"]

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4258 M nNHYD=[ " B4- MN" ]
4259 TM STO=[ 323] ( cu- m)
4260 *%-----|-----|
4261 ADD HYD NHYDs um=[ " M340" ], NHYDs to add=[ " 338- 340" + " B4- MN" ]
4262 *%-----|-----|
4263 SAVE HYD NHYD=[ " M340" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
4264 HYD_ COMMENT=[ " Tot al Fl ows at M340" ]
4265 *%-----|-----|
4266 ROUTE PI PE PTYPE=[ 1] circ, NHYDout=[ " 340- 104" ], RNUMBER=[ 1. 0 ], PDI AM=[ 1650] ( mm ),
4267 PLNGTH=[ 240] ( m ), PROUGH=[ 0. 013 ], PSLOPE=[ 0. 0015] ( m / m ),
NHYDin=[ " M340" ], RDT=[ 1 ]
4268 *%-----|-----|
4269 ADD HYD NHYDs um=[ " M104T" ], NHYDs to add=[ " 340- 104" + " M104" ]
4270 *%-----|-----|
4271 ROUTE PI PE PTYPE=[ 2] rect, NHYDout=[ " 104- 105" ], RNUMBER=[ 1. 0 ],
4272 PW DTH=[ 2400] ( mm ) by PHEI GHT=[ 2100] ( mm ),
PLNGTH=[ 380] ( m ), PROUGH=[ 0. 013 ], PSLOPE=[ 0. 001] ( m / m ),
NHYDin=[ " M104T" ], RDT=[ 1 ]
4273 *%-----|-----|
4274 CONTINUOUS STANDHYD NHYD=[ " B5" ], DT=[ 1] mi n, AREA=[ 2. 2] ( ha ),
4275 XI MP=[ 0. 57 ], TI MP=[ 0. 57 ], DWF=[ 0] ( cms ), LOSS=[ 2 ],
4276 SCS curve number CN=[ 75 ],
4277 Pervious surfaces: IAper=[ 4. 67] ( mm ), SLPP=[ 1] ( % ),
4278 LGP=[ 40] ( m ), MNP=[ 0. 25 ], SCP=[ 0] ( mi n ),
4279 Impervious surfaces: IAi mp=[ 1. 57] ( mm ), SLPI =[ 1] ( % ),
4280 LGI =[ 187] ( m ), MNI =[ 0. 013 ], SCI =[ 0] ( mi n ),
4281 Continuous simulation parameters:
4282 IaREcper=[ 4] ( hrs ), IaRECi mp=[ 4] ( hrs ),
4283 SM N=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
4284 Inter Event Ti me=[ 18] ( hrs ), END=- 1
4285 *%-----|-----|
4286 COMPUTE DUALHYD NHYDin=[ " B5" ], CI NLET=[ 0. 260] ( cms ), NI NLET=[ 1 ],
4287 MAj NHYD=[ " B5- M" ]
4288 M nNHYD=[ " B5- MN" ]
4289 TM STO=[ 250] ( cu- m)
4290 *%-----|-----|
4291 CONTINUOUS STANDHYD NHYD=[ " A8" ], DT=[ 1] mi n, AREA=[ 0. 96] ( ha ),
4292 XI MP=[ 0. 71 ], TI MP=[ 0. 71 ], DWF=[ 0] ( cms ), LOSS=[ 2 ],
4293 SCS curve number CN=[ 75 ],
4294 Pervious surfaces: IAper=[ 4. 67] ( mm ), SLPP=[ 1] ( % ),
4295 LGP=[ 40] ( m ), MNP=[ 0. 25 ], SCP=[ 0] ( mi n ),
4296 Impervious surfaces: IAi mp=[ 1. 57] ( mm ), SLPI =[ 1] ( % ),
4297 LGI =[ 186] ( m ), MNI =[ 0. 013 ], SCI =[ 0] ( mi n ),
4298 Continuous simulation parameters:
4299 IaREcper=[ 4] ( hrs ), IaRECi mp=[ 4] ( hrs ),
4300 SM N=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
4301 Inter Event Ti me=[ 18] ( hrs ), END=- 1
4302 *%-----|-----|
4303 ADD HYD NHYDs um=[ " A8T" ], NHYDs to add=[ " A6- M" + " A8" ]
4304 *%-----|-----|
4305 COMPUTE DUALHYD NHYDin=[ " A8T" ], CI NLET=[ 0. 238] ( cms ), NI NLET=[ 1 ],
4306 MAj NHYD=[ " A8- M" ]
4307 M nNHYD=[ " A8- MN" ]
4308 TM STO=[ 40] ( cu- m)
4309 *%-----|-----|
4310 ADD HYD NHYDs um=[ " M105" ], NHYDs to
4311 add=[ " 104- 105" + " B5- MN" + " A8- MN" + " TODD_ MN3j " ]
4312 *%-----|-----|
4313 SAVE HYD NHYD=[ " M105" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
4314 HYD_ COMMENT=[ " Tot al Fl ows at M105" ]
4315 *%-----|-----|
4316 DI VERT HYD NHYDin=[ " A8- M" ] NI Dout=[ 2] max five,
4317 out flow hydr ographs ( NHYDs )=[ " A8- M- J R" " A8- M- B6" ]
4318 flow distribution table: ( modify as necessary )
Note: all flows are in ( cms )

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4319          QI Di + QI Di i = QTOTAL
4320          [ 0 + 0 = 0 ]
4321          [ 50 + 50 = 100 ] end
4322  *%-----|-----
4323  DI VERT HYD      NHYDi n=[ "MHI05" ] NI Dout=[ 2 ] max five,
4324                  outflow hydrographs (NHYDs)=[ "MHI05- JR" "MHI05- B6" ]
4325                  flow distribution table: (modify as necessary)
4326                  Note: all flows are in (cms)
4327                  QI Di + QI Di i = QTOTAL
4328                  [ 0 + 0 = 0 ]
4329                  [ 0 + 3.0 = 3.0 ]
4330                  [ 96.9+ 3.1 = 100 ] end
4331  *%-----|-----
4332  CONTINUOUS STANDHYD NHYD=[ "B7" ], DT=[ 1 ] mi n, AREA=[ 7.19 ] ( ha ),
4333                  XI MP=[ 0.41 ], TI MP=[ 0.54 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
4334                  SCS curve number CN=[ 75 ],
4335                  Pervious surfaces: I Aper=[ 4.67 ] ( mm ), SLPP=[ 1 ] ( % ),
4336                  LGP=[ 40 ] ( m ), MNP=[ 0.25 ], SCP=[ 0 ] ( mi n ),
4337                  Impervious surfaces: I Ai mp=[ 1.57 ] ( mm ), SLPI=[ 1 ] ( % ),
4338                  LGI=[ 211 ] ( m ), MNI=[ 0.013 ], SCI=[ 0 ] ( mi n ),
4339                  Continuous simulation parameters:
4340                  I a RECper=[ 4 ] ( hr s ), I a RECi mp=[ 4 ] ( hr s ),
4341                  SM N=[ -1 ] ( mm ), SMAX=[ -1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
4342                  Inter Event Ti me=[ 18 ] ( hr s ), END=- 1
4343  *%-----|-----
4344  ADD HYD          NHYDs um=[ "B7- B4M" ], NHYDs to add=[ "B4- M" + "B7" ]
4345  *%-----|-----
4346  COMPUTE DUALHYD NHYDi n=[ "B7- B4M" ], CI NLET=[ 0.629 ] ( cms ), NI NLET=[ 1 ],
4347                  M ij NHYD=[ "B7R- M" ]
4348                  M nNHYD=[ "B7R- MN" ]
4349                  TM STO=[ 311 ] ( cu- m )
4350  *%-----|-----
4351  ROUTE PIPE      PTYPE=[ 1 ] circ, NHYDout=[ "360- 106A" ], RNUMBER=[ 1.0 ], PDI AM=[ 1050 ] ( mm ),
4352                  PLNGTH=[ 167 ] ( m ), PROUGH=[ 0.013 ], PSLOPE=[ 0.001 ] ( m/ m ),
4353                  NHYDi n=[ "B7R- MN" ], RDT=[ 1 ]
4354  *%-----|-----
4355  * -JFSA 2021-01-19 change B6 to be developed as per geottawa website and apply the
4356  parameters of A7, the nearest sub-catchment to B6, LGI is calculated based on B6 area
4357  CONTINUOUS STANDHYD NHYD=[ "B6" ], DT=[ 1 ] mi n, AREA=[ 3.29 ] ( ha ),
4358                  XI MP=[ 0.41 ], TI MP=[ 0.54 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
4359                  SCS curve number CN=[ 75 ],
4360                  Pervious surfaces: I Aper=[ 4.67 ] ( mm ), SLPP=[ 1 ] ( % ),
4361                  LGP=[ 40 ] ( m ), MNP=[ 0.25 ], SCP=[ 0 ] ( mi n ),
4362                  Impervious surfaces: I Ai mp=[ 1.57 ] ( mm ), SLPI=[ 1 ] ( % ),
4363                  LGI=[ 148.099 ] ( m ), MNI=[ 0.013 ], SCI=[ 0 ] ( mi n ),
4364                  Continuous simulation parameters:
4365                  I a RECper=[ 4 ] ( hr s ), I a RECi mp=[ 4 ] ( hr s ),
4366                  SM N=[ -1 ] ( mm ), SMAX=[ -1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
4367                  Inter Event Ti me=[ 18 ] ( hr s ), END=- 1
4368  *%-----|-----
4369  * -JFSA 2021-01-25 add B1 DUALHYD as per Corrigan Report, IBI Group, 2008
4370  COMPUTE DUALHYD NHYDi n=[ "B6" ], CI NLET=[ 0.064 ] ( cms ), NI NLET=[ 1 ],
4371                  M ij NHYD=[ "B6- M" ]
4372                  M nNHYD=[ "B6- MN" ]
4373                  TM STO=[ 5484 ] ( cu- m )
4374  *%-----|-----
4375  * CONTINUOUS NASHHYD NHYD=[ "B6" ], DT=[ 1 ] mi n, AREA=[ 3.29 ] ( ha ),
4376  * DWF=[ 0 ] ( cms ), CN C=[ 75 ], I A=[ 2.5 ] ( mm ),
4377  * N=[ 3.0 ], TP=[ 0.36 ] hr s,
4378  * Continuous simulation parameters:
4379  * I a RECper=[ 4 ] ( hr s ),
4380  * SM N=[ -1 ] ( mm ), SMAX=[ -1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
4381  * Inter Event Ti me=[ 12 ] ( hr s )
4382  * Baseflow simulation parameters:

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4381 * BaseFlowOption=[ 1 ] ,
4382 * InitGWResVol =[ 50 ] ( mm ) , GWResK=[ 0.96 ] ( mm/ day/ mm)
4383 * VHydCond=[ 0.055 ] ( mm/ hr ) , END=- 1
4384 *%-----|-----|
4385 *% -EX-LAND is external land. It is a part of JOCKVA sub-catchment as per Corrigan
Report, IBI Group, 2008
4386 CONTINUOUS STANDHYD NHYD=[ "EX-LAND" ] , DT=[ 1 ] mi n , AREA=[ 32.5 ] ( ha ) ,
4387 XI MP=[ 0.50 ] , TI MP=[ 0.50 ] , DWF=[ 0 ] ( cms ) , LOSS=[ 2 ] ,
4388 SCS curve number CN=[ 74 ] ,
4389 Pervious surfaces: I A per =[ 4.67 ] ( mm ) , SLPP=[ 1 ] ( % ) ,
4390 LGP=[ 40 ] ( m ) , MNP=[ 0.25 ] , SCP=[ 0 ] ( mi n ) ,
4391 Impervious surfaces: I A i mp =[ 1.57 ] ( mm ) , SLPI =[ 1 ] ( % ) ,
4392 LGI =[ 465.475 ] ( m ) , MNI =[ 0.013 ] , SCI =[ 0 ] ( mi n ) ,
4393 Continuous simulation parameters:
4394 I a REC per =[ 4 ] ( hrs ) , I a REC i mp =[ 4 ] ( hrs ) ,
4395 SM N =[ - 1 ] ( mm ) , SMAX =[ - 1 ] ( mm ) , SK =[ 0.010 ] / ( mm ) ,
4396 InterEventTi me =[ 18 ] ( hrs ) , END=- 1
4397 *%-----|-----|
4398 COMPUTE DUALHYD NHYD i n =[ "EX-LAND" ] , CI NLET =[ 2.275 ] ( cms ) , NI NLET =[ 1 ] ,
4399 M i j NHYD =[ "EX-LAND- M " ]
4400 M n NHYD =[ "EX-LAND- MN" ]
4401 TM S TO =[ 1365 ] ( cu- m)
4402 *%-----|-----|
4403 ADD HYD NHYDs um =[ " B6- B7ExM " ] , NHYDs t o
add =[ " B7R- M " + " EX-LAND- M " + " B5- M " + " B6- M " + " B6- MN" + " A8- M - B6 " ]
4404 *%-----|-----|
4405 COMPUTE DUALHYD NHYD i n =[ " B6- B7ExM " ] , CI NLET =[ 0.064 ] ( cms ) , NI NLET =[ 1 ] ,
4406 M i j NHYD =[ " B6R- M " ]
4407 M n NHYD =[ " B6R- MN" ]
4408 TM S TO =[ 5484 ] ( cu- m)
4409 *%-----|-----|
4410 ROUTE PI PE PTYPE =[ 1 ] circ , NHYDout =[ " 105- 106A " ] , RNUMBER =[ 1.0 ] , PDI AM =[ 1800 ] ( mm ) ,
4411 PLNGTH =[ 208 ] ( m ) , PROUGH =[ 0.013 ] , PSLOPE =[ 0.001 ] ( m/ m ) ,
NHYD i n =[ " MHI 05- B6 " ] , RDT =[ 1 ]
4412 *%-----|-----|
4413 ADD HYD NHYDs um =[ " MHI 06A " ] , NHYDs t o
add =[ " 360- 106A " + " 105- 106A " + " B6R- MN" + " B6R- M " ]
4414 *%-----|-----|
4415 SAVE HYD NHYD =[ " MHI 06A " ] , # OF PCYCLES =[ - 1 ] , I CASEs h =[ 1 ]
4416 HYD_COMMENT =[ " Total Flows at MHI 06A " ]
4417 *%-----|-----|
4418 *% -JFSA 2021-01-12 THE MANHOLE MHI06 is called MHI17/106 in Corrigan Report, IBI
Group, July 2008
4419 *%
4420 ROUTE PI PE PTYPE =[ 1 ] circ , NHYDout =[ " 106A- 106 " ] , RNUMBER =[ 1.0 ] , PDI AM =[ 1800 ] ( mm ) ,
4421 PLNGTH =[ 190 ] ( m ) , PROUGH =[ 0.013 ] , PSLOPE =[ 0.001 ] ( m/ m ) ,
NHYD i n =[ " MHI 06A " ] , RDT =[ 1 ]
4422 *%-----|-----|
4423 CONTINUOUS STANDHYD NHYD=[ " A9 " ] , DT=[ 1 ] mi n , AREA=[ 2.44 ] ( ha ) ,
4424 XI MP=[ 0.71 ] , TI MP=[ 0.71 ] , DWF=[ 0 ] ( cms ) , LOSS=[ 2 ] ,
4425 SCS curve number CN=[ 75 ] ,
4426 Pervious surfaces: I A per =[ 4.67 ] ( mm ) , SLPP=[ 1 ] ( % ) ,
4427 LGP=[ 40 ] ( m ) , MNP=[ 0.25 ] , SCP=[ 0 ] ( mi n ) ,
4428 Impervious surfaces: I A i mp =[ 1.57 ] ( mm ) , SLPI =[ 1 ] ( % ) ,
4429 LGI =[ 262 ] ( m ) , MNI =[ 0.013 ] , SCI =[ 0 ] ( mi n ) ,
4430 Continuous simulation parameters:
4431 I a REC per =[ 4 ] ( hrs ) , I a REC i mp =[ 4 ] ( hrs ) ,
4432 SM N =[ - 1 ] ( mm ) , SMAX =[ - 1 ] ( mm ) , SK =[ 0.010 ] / ( mm ) ,
4433 InterEventTi me =[ 18 ] ( hrs ) , END=- 1
4434 *%-----|-----|
4435 COMPUTE DUALHYD NHYD i n =[ " A9 " ] , CI NLET =[ 0.547 ] ( cms ) , NI NLET =[ 1 ] ,
4436 M i j NHYD =[ " A9- M " ]
4437 M n NHYD =[ " A9- MN" ]
4438 TM S TO =[ 0 ] ( cu- m)
4439 *%-----|-----|
4440 ADD HYD NHYDs um =[ " MHI 06 " ] , NHYDs t o add =[ " 106A- 106 " + " A9- MN" ]

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4441  *%-----|-----|
4442  SAVE HYD      NHYD=["MHI06"], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
4443              HYD_COMMENT=["Tot al Fl ows at MHI06"]
4444  *%-----|-----|
4445  *% -JFSA 2021-01-12 THE MANHOLE MHI07 is called MHI18/107 in Corrigan Report, IBI
Group, July 2008
4446  *%
4447  ROUTE PIPE    PTYPE=[ 1]circ, NHYDout=["106-107"], RNUMBER=[ 1.0], PDI AM=[ 1800](mm),
4448              PLNGTH=[ 122.5](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
              NHYDin=["MHI06"], RDT=[ 1]
4449  *%-----|-----|
4450  CONTINUOUS STANDHYD NHYD=["A10"], DT=[ 1]mi n, AREA=[ 4.14](ha),
4451              XI MP=[ 0.35], TI MP=[ 0.47], DWF=[ 0](cms), LOSS=[ 2],
4452              SCS curve number CN=[ 75],
4453              Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4454              LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
4455              Impervious surfaces: I Ai mp=[ 1.57](mm), SLPI=[ 1](%),
4456              LGI=[ 183](m), MNI =[ 0.013], SCI =[ 0](mi n),
4457              Continuous simulation parameters:
4458              I a REC per=[ 4](hrs), I a REC i mp=[ 4](hrs),
4459              SM N=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0.010]/(mm),
4460              Inter Event Ti me=[ 18](hrs), END=- 1
4461  *%-----|-----|
4462  COMPUTE DUALHYD NHYDin=["A10"], CI NLET=[ 0.310](cms), NI NLET=[ 1],
4463              M a j NHYD=["A10- M"]
4464              M nNHYD=["A10- MN"]
4465              TM I STO=[ 228](cu- m)
4466  *%-----|-----|
4467  CONTINUOUS STANDHYD NHYD=["A11"], DT=[ 1]mi n, AREA=[ 10.61](ha),
4468              XI MP=[ 0.53], TI MP=[ 0.62], DWF=[ 0](cms), LOSS=[ 2],
4469              SCS curve number CN=[ 75],
4470              Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4471              LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
4472              Impervious surfaces: I Ai mp=[ 1.57](mm), SLPI=[ 1](%),
4473              LGI=[ 379](m), MNI =[ 0.013], SCI =[ 0](mi n),
4474              Continuous simulation parameters:
4475              I a REC per=[ 4](hrs), I a REC i mp=[ 4](hrs),
4476              SM N=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0.010]/(mm),
4477              Inter Event Ti me=[ 18](hrs), END=- 1
4478  *%-----|-----|
4479  COMPUTE DUALHYD NHYDin=["A11"], CI NLET=[ 0.993](cms), NI NLET=[ 1],
4480              M a j NHYD=["A11- M"]
4481              M nNHYD=["A11- MN"]
4482              TM I STO=[ 556](cu- m)
4483  *%-----|-----|
4484  ADD HYD       NHYDs um=["MHI07"], NHYDs to add=["106-107"+"A10- MN"+"A11- MN"]
4485  *%-----|-----|
4486  SAVE HYD      NHYD=["MHI07"], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
4487              HYD_COMMENT=["Tot al Fl ows at MHI07"]
4488  *%-----|-----|
4489  ROUTE PIPE    PTYPE=[ 1]circ, NHYDout=["107-119"], RNUMBER=[ 1.0], PDI AM=[ 1800](mm),
4490              PLNGTH=[ 114](m), PROUGH=[ 0.013], PSLOPE=[ 0.0012](m/m),
              NHYDin=["MHI07"], RDT=[ 1]
4491  *%-----|-----|
4492  *% -JFSA 2021-01-12 THE MANHOLE MHI08 is called MHI20/108 in Corrigan Report, IBI
Group, July 2008
4493  *%
4494  ROUTE PIPE    PTYPE=[ 1]circ, NHYDout=["119-108"], RNUMBER=[ 1.0], PDI AM=[ 1800](mm),
4495              PLNGTH=[ 65.8](m), PROUGH=[ 0.013], PSLOPE=[ 0.0012](m/m),
              NHYDin=["107-119"], RDT=[ 1]
4496  *%-----|-----|
4497  CONTINUOUS STANDHYD NHYD=["A12"], DT=[ 1]mi n, AREA=[ 12.29](ha),
4498              XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4499              SCS curve number CN=[ 75],
4500              Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4501              LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),

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4502      Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 1](%),
4503      LGI=[ 183](m), MNI=[ 0.013], SCI=[ 0](mi n),
4504      Continuous simulation parameters:
4505      I a RE C per=[ 4](hrs), I a RE C i mp=[ 4](hrs),
4506      SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4507      Inter Event Ti me=[ 18](hrs), END=- 1
4508 *%-----|
4509 COMPUTE DUALHYD      NHYD i n=[ "A12"], CI NLET=[ 1.029](cms), NI NLET=[ 1],
4510      M i j NHYD=[ "A12- M "]
4511      M n NHYD=[ "A12- MN"]
4512      TM I STO=[ 672](cu- m)
4513 *%-----|
4514 CONTINUOUS STANDHYD      NHYD=[ "A13"], DT=[ 1]mi n, AREA=[ 2.59](ha),
4515      XI MP=[ 0.71], TI MP=[ 0.71], DWF=[ 0](cms), LOSS=[ 2],
4516      SCS curve number CN=[ 75],
4517      Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4518      LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
4519      Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 1](%),
4520      LGI=[ 379](m), MNI=[ 0.013], SCI=[ 0](mi n),
4521      Continuous simulation parameters:
4522      I a RE C per=[ 4](hrs), I a RE C i mp=[ 4](hrs),
4523      SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4524      Inter Event Ti me=[ 18](hrs), END=- 1
4525 *%-----|
4526 COMPUTE DUALHYD      NHYD i n=[ "A13"], CI NLET=[ 0.571](cms), NI NLET=[ 1],
4527      M i j NHYD=[ "A13- M "]
4528      M n NHYD=[ "A13- MN"]
4529      TM I STO=[ 0](cu- m)
4530 *%-----|
4531 * -JFSA 2021-01-22 add the Corrigan pond area ("Pond-Block")
4532 CONTINUOUS STANDHYD      NHYD=[ "Pond- Bl ock"], DT=[ 1]mi n, AREA=[ 2.94](ha),
4533      XI MP=[ 0.415], TI MP=[ 0.415], DWF=[ 0](cms), LOSS=[ 2],
4534      SCS curve number CN=[ 75],
4535      Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
4536      LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](mi n),
4537      Impervious surfaces: I A i mp=[ 1.57](mm), SLPI=[ 1](%),
4538      LGI=[ 183](m), MNI=[ 0.013], SCI=[ 0](mi n),
4539      Continuous simulation parameters:
4540      I a RE C per=[ 4](hrs), I a RE C i mp=[ 4](hrs),
4541      SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
4542      Inter Event Ti me=[ 18](hrs), END=- 1
4543 *%-----|
4544 ADD HYD      NHYDs um=[ "MH108"], NHYDs to add=[ "119-108"+"A13- MN"+"A12- MN"]
4545 *%-----|
4546 SAVE HYD      NHYD=[ "MH108"], # OF PCYCLES=[ -1], I CASEs h=[ 1]
4547      HYD_ COMMENT=[ "Tot al Fl ows at MH108"]
4548 *%-----|
4549 ROUTE PIPE      PTYPE=[ 1]circ, NHYDout=[ "108-116"], RNUMBER=[ 1.0], PDI AM=[ 1800](mm),
4550      PLNGTH=[ 76.6](m), PROUGH=[ 0.013], PSLOPE=[ 0.0013](m/m),
4551      NHYD i n=[ "MH108"], RDT=[ 1]
4552 *%-----|
4553 ROUTE PIPE      PTYPE=[ 1]circ, NHYDout=[ "116-corrig"], RNUMBER=[ 1.0],
4554      PDI AM=[ 1800](mm),
4555      PLNGTH=[ 79.5](m), PROUGH=[ 0.013], PSLOPE=[ 0.0013](m/m),
4556      NHYD i n=[ "108-116"], RDT=[ 1]
4557 *%-----|
4558 ADD HYD      NHYDs um=[ "Corrigan"], NHYDs to add=[ "116-corrig"+"Pond- Bl ock"]
4559 *%-----|
4560 SAVE HYD      NHYD=[ "Corrigan"], # OF PCYCLES=[ -1], I CASEs h=[ 1]
4561      HYD_ COMMENT=[ "Tot al Fl ows at Corrigan Pond"]
4562 *%-----|
4563 ROUTE RESERVOIR      NHYDout=[ "Co- P"], NHYD i n=[ "Corrigan"],
4564      RDT=[ 1](mi n),
4565      TABLE of ( OUTFLOW STORAGE ) values
4566      (cms) - (ha- m)
4567      [ 0.0 , 0.0 ]

```

```

4565 [ 0.015 , 0.04118]
4566 [ 0.030 , 0.08297]
4567 [ 0.045 , 0.12537]
4568 [ 0.060 , 0.16837]
4569 [ 0.075 , 0.21199]
4570 [ 0.090 , 0.27545]
4571 [ 0.105 , 0.34650]
4572 [ 0.120 , 0.42049]
4573 [ 0.135 , 0.50188]
4574 [ 0.186 , 0.60307]
4575 [ 2.110 , 0.79083]
4576 [ 5.874 , 1.00271]
4577 [ 11.395 , 1.29643]
4578 [ 18.770 , 1.62054]
4579 [ 28.143 , 1.97516]
4580 [ -1 , -1 ] (max twenty pts)
4581 NHYDovf=[ "Co- P- OVF" ]
4582 *%-----|-----|
4583 ADD HYD NHYDsum=[ "corrig" ], NHYDs to
add=[ "Co- P- OVF" + "Co- P" + "N_TO" + "M105- JR" + "A8- M- JR" + "A9- M" + "A10- M" + "A11- M" + "A12- M" + "A
13- M" ]
4584 *%-----|-----|
4585 SAVE HYD NHYD=[ "corrig" ], # OF PCYCLES=[ -1 ], ICASEsh=[ 1]
4586 HYD_COMMENT=[ "Total Flows at Corrigan Pond" ]
4587 *%-----|-----|
4588 *#*****|
4589 *# Corrigan Pond 1
4590 *# - Rating curve obtained from Barrhaven South M&S modeling
4591 *# - Tributary Drainage Area to M&S Pond 1 = 145 ha
4592 *#*****|
4593 *ROUTE RESERVOIR NHYDout=[ "MS_P1" ], NHYDin=[ "CORRIG" ],
4594 * RDT=[ 1 ] (min),
4595 * TABLE of ( OUTFLOW STORAGE ) values
4596 * (cms) - (ha-m)
4597 * [ 0.0 , 0.0 ]
4598 * [ 0.06 , 0.58]
4599 * [ -1 , -1 ] (max twenty pts)
4600 * NHYDovf=[ "P1- OVF" ]
4601 *%-----|-----|
4602 *ADD HYD NHYDsum=[ "SN_CO" ], NHYDs to add=[ "N_TO" + "P1- OVF" + "MS_P1" ]
4603 *%-----|-----|
4604 *SAVE HYD NHYD=[ "SN_CO" ], # OF PCYCLES=[ -1 ], ICASEsh=[ 1]
4605 * HYD_COMMENT=[ "Total Flows at Corrigan Drain" ]
4606 *%-----|-----|
4607 *#
4608 *# Hydrograph from Corrigan Drain routed to Jockvale Road
4609 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
4610 *#
4611 ROUTE CHANNEL NHYDout=[ "N_M" ], NHYDin=[ "corrig" ],
4612 RDT=[ 1 ] (min),
4613 CHLGTH=[ 580 ] (m), CHSLOPE=[ 0.4448 ] ( % ),
4614 FPSLOPE=[ 0.4448 ] ( % ),
4615 SECNUM=[ 1.0 ], NSEG=[ 3]
4616 ( SEGROUGH, SEGDI ST (m) ) =
4617 [ 0.075, -17.72
4618 -0.045, 17.72
4619 0.075, 80.62 ] NSEG times
4620 ( DI STANCE (m), ELEVATI ON (m) ) =
4621 [ -83.32, 90.00]
4622 [ -81.36, 89.50]
4623 [ -79.12, 89.00]
4624 [ -76.13, 88.50]
4625 [ -20.46, 88.00]
4626 [ -19.36, 87.50]
4627 [ -18.51, 87.00]
4628 [ -17.72, 86.50]

```

4629 [- 11.95, 85.24]
 4630 [- 0.11, 85.12]
 4631 [11.49, 85.20]
 4632 [17.72, 86.50]
 4633 [19.74, 87.00]
 4634 [21.22, 87.50]
 4635 [22.68, 88.00]
 4636 [24.28, 88.50]
 4637 [26.79, 89.00]
 4638 [71.98, 90.00]
 4639 [80.62, 90.50]

*%-----|-----|
 *#*****|

4642 *# Catchment MLLS
 4643 *# - To SWM Facility north of the Jock
 4644 *# - Primarily residential development

4645 *#*****|

4646 CONTINUOUS STANDHYD NHYD=["MLLS"], DT=[1]min, AREA=[175.99](ha),
 4647 XI MP=[0.38], TI MP=[0.38], DWF=[0](cms), LOSS=[2],
 4648 SCS curve number CN=[74],
 4649 Per vious surfaces: I A per=[4.67](mm), SLPP=[1](%),
 4650 LGP=[40](m), MNP=[0.25], SCP=[0](min),
 4651 Imper vious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
 4652 LGI=[1118.123](m), MNI=[0.013], SCI=[0](min),
 4653 Continuous simulation parameters:
 4654 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
 4655 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
 4656 Inter Event Ti me=[18](hrs), END=-1

4657 *%-----|-----|
 4658 *#*****|

4659 *# Chapman Mills SWM Pond
 4660 *# - Rating curve obtained from CCL hydraulic modeling

4661 *#*****|

4662 ROUTE RESERVOIR NHYDout=["MLL_P"], NHYDin=["MLLS"],
 4663 RDT=[1](min),
 4664 TABLE of (OUTFLOW STORAGE) values

	(cms) - (ha - m)
4665	[0.0 , 0.0]
4666	[0.01 , 0.01]
4667	[0.05 , 0.06]
4668	[0.09 , 0.11]
4669	[0.13 , 0.15]
4670	[0.18 , 0.19]
4671	[0.28 , 0.28]
4672	[0.37 , 0.34]
4673	[0.45 , 0.40]
4674	[0.51 , 0.44]
4675	[0.56 , 0.47]
4676	[0.64 , 0.52]
4677	[0.76 , 0.59]
4678	[0.86 , 0.65]
4679	[1.09 , 0.78]
4680	[1.44 , 0.96]
4681	[3.18 , 1.84]
4682	[4.05 , 2.31]
4683	[-1 , -1] (max twenty pts)

4684 NHYDovf=["ML-OV"]

4685 *%-----|-----|
 4686 *#*****|

4687 ADD HYD NHYDsum=["SN_M"], NHYDs to add=["N_M"+"ML-OV"+"MLL_P"]

4688 *%-----|-----|

4689 SAVE HYD NHYD=["SN_M"], # OF PCYCLES=[-1], I CASEs h=[1]
 4690 HYD_COMMENT=["Total Flows at Jockvale Road"]

4691 *%-----|-----|

4692 *#
 4693 *# Hydrograph from Jockvale Road routed to Heart's Desire
 4694 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689

```

4695 *#
4696 ROUTE CHANNEL NHYDout=["N_DE"], NHYDin=["SN_M"],
4697 RDT=[1](min),
4698 CHLGTH=[1962](m), CHSLOPE=[0.2227](%),
4699 FPSLOPE=[0.2227](%),
4700 SECNUM=[1.0], NSEG=[3]
4701 (SEGROUGH, SEGDIST(m))=
4702 [0.075, -17.56
4703 -0.045, 18.27
4704 0.075, 32.51] NSEG times
4705 (DISTANCE(m), ELEVATION(m))=
4706 [-54.07, 85.00]
4707 [-39.43, 84.50]
4708 [-28.30, 84.00]
4709 [-24.12, 83.50]
4710 [-22.30, 83.00]
4711 [-20.55, 82.50]
4712 [-17.56, 82.00]
4713 [-12.63, 81.22]
4714 [-0.11, 80.75]
4715 [11.55, 81.22]
4716 [18.27, 82.00]
4717 [19.82, 82.50]
4718 [22.48, 83.00]
4719 [27.90, 83.50]
4720 [29.31, 84.00]
4721 [30.81, 84.50]
4722 [32.51, 85.00]
4723 *%-----|-----
4724 *#*****
4725 *# Catchment DESIRE
4726 *# - To Jock River (north of the Jock)
4727 *# - Rural-estate subdivision (Heart's Desire Community)
4728 *#*****
4729 CONTINUOUS STANDHYD NHYD=["DESIRE"], DT=[1]min, AREA=[23.78](ha),
4730 XI MP=[0.25], TI MP=[0.25], DWF=[0](cms), LOSS=[2],
4731 SCS curve number CN=[77],
4732 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4733 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4734 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4735 LGI=[400](m), MNI=[0.013], SCI=[0](min),
4736 Continuous simulation parameters:
4737 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
4738 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4739 I n t e r E v e n t T i m e=[18](hrs), E N D=-1
4740 *%-----|-----
4741 *#*****
4742 *# Catchment JOCKVA
4743 *# - To Jockvale SWM Facility
4744 *# - Residential development & golf course
4745 *# - JFSA 2021-01-11 update JOCKVA after updating CORRIG as per IBI GROUP, July 2008.
4746 *# JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two
areas JOCKVA and EX-LAND 32.5 ha as per IBI GROUP, July 2008.
4747 *#*****
4748 CONTINUOUS STANDHYD NHYD=["JOCKVA"], DT=[1]min, AREA=[225.13](ha),
4749 XI MP=[0.50], TI MP=[0.50], DWF=[0](cms), LOSS=[2],
4750 SCS curve number CN=[74],
4751 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4752 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4753 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4754 LGI=[1310.55](m), MNI=[0.013], SCI=[0](min),
4755 Continuous simulation parameters:
4756 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
4757 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4758 I n t e r E v e n t T i m e=[18](hrs), E N D=-1
4759 *%-----|-----

```

```

4760 ADD HYD          NHYDs um=[" J OCKVA- TO' ], NHYDs t o
add=[" EX- LAND- MN" +" J OCKVA" +" B2- M" +" B3- M" ]
4761 *%-----|-----|
4762 SAVE HYD         NHYD=[" J OCKVA- TO' ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
4763 HYD_COMMENT=[" Total Flows at KB first pond" ]
4764 *%-----|-----|
4765 *#*****|-----|
4766 *#   Jockvale SWM Facility
4767 *#   - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
4768 *#*****|-----|
4769 ROUTE RESERVOIR  NHYDout =[" J OCK_P" ], NHYDi n=[" J OCKVA- TO' ],
4770 RDT=[ 1 ] ( mi n ),
4771             TABLE of ( OUTFLOW STORAGE ) values
4772             ( cms ) - ( ha- m )
4773             [ 0.0 , 0.0 ]
4774             [ 0.27 , 0.03 ]
4775             [ 0.28 , 0.55 ]
4776             [ 0.29 , 1.14 ]
4777             [ 0.30 , 1.80 ]
4778             [ 0.31 , 2.32 ]
4779             [ 1.12 , 2.87 ]
4780             [ 2.92 , 3.45 ]
4781             [ 4.64 , 4.07 ]
4782             [ 6.69 , 4.72 ]
4783             [ 9.02 , 5.39 ]
4784             [ 11.62 , 6.10 ]
4785             [ 14.42 , 6.85 ]
4786             [ 17.45 , 7.62 ]
4787             [ 20.69 , 8.44 ]
4788             [ 24.08 , 9.28 ]
4789             [ 27.68 , 10.17 ]
4790             [ -1 , -1 ] (max t went y pts)
4791             NHYDovf=[" J O- OVF" ]
4792 *%-----|-----|
4793 ADD HYD          NHYDs um=[" SN_DE" ], NHYDs t o add=[" N_DE" +" DESI RE" +" J O- OVF" +" J OCK_P" ]
4794 *%-----|-----|
4795 SAVE HYD         NHYD=[" SN_DE" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
4796 HYD_COMMENT=[" Total Flows at Heart's Desire" ]
4797 *%-----|-----|
4798 *#
4799 *# Hydrograph from Heart's Desire routed to Rideau River
4800 *# Channel X-Section obtained from RVCA Hydraulic Mdel - Station 0
4801 *#
4802 ROUTE CHANNEL    NHYDout =[" NI " ] , NHYDi n=[" SN_DE" ] ,
4803 RDT=[ 1 ] ( mi n ),
4804 CHLGTH=[ 563 ] ( m ), CHSLOPE=[ 0.9668 ] ( % ),
4805                      FPSLOPE=[ 0.9668 ] ( % ),
4806 SECNUM=[ 1.0 ], NSEG=[ 3 ]
4807 ( SEGROUGH, SEGDI ST ( m ) ) =
4808 [ 0.075, -30.20
4809 -0.045, 30.20
4810 0.075, 48.48 ] NSEG t i m e s
4811 ( DI STANCE ( m ), ELEVATI ON ( m ) ) =
4812 [-98.46, 81.50]
4813 [-92.24, 81.00]
4814 [-86.88, 80.50]
4815 [-81.54, 80.00]
4816 [-74.36, 79.50]
4817 [-63.54, 79.00]
4818 [-39.23, 78.50]
4819 [-34.51, 78.00]
4820 [-33.01, 77.50]
4821 [-30.20, 77.00]
4822 [-13.42, 76.18]
4823 [-1.14, 76.09]
4824 [17.06, 76.18]

```

```

4825 [ 30.20, 77.00]
4826 [ 32.95, 77.50]
4827 [ 34.06, 78.00]
4828 [ 35.11, 78.50]
4829 [ 36.32, 79.00]
4830 [ 37.74, 79.50]
4831 [ 48.48, 81.50]
4832 *%-----|-----|
4833 *#*****|
4834 *# Catchment S-2
4835 *# - To Jock River (north and south)
4836 *# - Undeveloped floodplain and river
4837 *#*****|
4838 CONTINUOUS NASHYD NHYD=[ "S-2" ], DT=[ 1 ] mi n, AREA=[ 102.94 ] ( ha ),
4839 DWF=[ 0 ] ( cms ), CNVC=[ 72 ], IA=[ 4.67 ] ( mm ),
4840 N=[ 3 ], TP=[ 0.40 ] hrs,
4841 Continuous simulation parameters:
4842 IaRECper=[ 4 ] ( hrs ),
4843 SMN=[ - 1 ] ( mm ), SMAX=[ - 1 ] ( mm ), SK=[ 0.010 ] / ( mm ),
4844 InterEventTime=[ 12 ] ( hrs )
4845 Baseflow simulation parameters:
4846 BaseFlowOption=[ 1 ] ,
4847 InitGWResVol=[ 50 ] ( mm ), GWResK=[ 0.96 ] ( mm/ day/ mm)
4848 VHydCond=[ 0.055 ] ( mm/ hr ), END=- 1
4849 *%-----|-----|
4850 ADD HYD NHYDs um=[ "SN_NI" ], NHYDs to add=[ "NI"+"S-2" ]
4851 *%-----|-----|
4852 SAVE HYD NHYD=[ "SN_NI" ], # OF PCYCLES=[ - 1 ], ICASEsh=[ 1 ]
4853 HYD_COMMENT=[ "Total Flows at Rideau River" ]
4854 *%-----|-----|
4855 *#####|
4856 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4857 START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 5 ]
4858 *% ["C24SC005.stm"] <--storm filename, one per line for NSTORMtime
4859 *%-----|-----|
4860 *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4861 START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 10 ]
4862 *% ["C24SC010.stm"] <--storm filename, one per line for NSTORMtime
4863 *%-----|-----|
4864 *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4865 START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 25 ]
4866 *% ["C24SC025.stm"] <--storm filename, one per line for NSTORMtime
4867 *%-----|-----|
4868 *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4869 START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 50 ]
4870 *% ["C24SC050.stm"] <--storm filename, one per line for NSTORMtime
4871 *%-----|-----|
4872 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4873 *START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 100 ]
4874 *% ["100YC3H.STM"] <--storm filename, one per line for NSTORMtime
4875 *%-----|-----|
4876 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4877 START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 100 ]
4878 *% ["C24SC100.stm"] <--storm filename, one per line for NSTORMtime
4879 *%-----|-----|
4880 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4881 *START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 100 ]
4882 *% ["C24SC100.stm"] <--storm filename, one per line for NSTORMtime
4883 *START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 101 ]
4884 *% ["A24SC100.stm"] <--storm filename, one per line for NSTORMtime
4885 *START TZERO=[ 0.0 ], METOUT=[ 2 ], NSTORM=[ 1 ], NRUN=[ 102 ]
4886 *% ["A24SC100_60.stm"] <--storm filename, one per line for NSTORMtime
4887 FINISH
4888

```


Table with columns for node ID, description, coordinates (N, E, S, W), and flow/velocity data. It lists various infrastructure elements like channels, roads, and subwatersheds.

Table with columns for node ID, description, coordinates (N, E, S, W), and flow/velocity data. It continues the list of infrastructure elements from the previous table.


```

018173 SAVE HYD 1.0 01:SN:KB 54681.13 48.551 N.Date 39:29 13.09 n/a .000
018174 Remark: JFSAi.n.c. SN:KB 0002
018175 remark: JFSAi.n.c. SN:KB 0002
018176 # Hydrograph from Node Todd
018177 # Channel Section obtained from RCPA Hydraulic Model - Station 3633
018178 # JFSA 2021-01-26 change the channel length (at station 3633) from 605m to 608m and change the slope from 0.0498% to 0.2
018179 #
018180 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11
018181 #
018182 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018183 (L/S to 380 / 050/ 045)
018184 (Vmax = 1.295 Dmax = 1.31)
018185 *****
018186 # Catchment Greenbank
018187 # To Greenbank Drain (south of the Jock)
018188 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018189 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018190 #
018191 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018192 (L/S to 380 / 050/ 045)
018193 (Vmax = 1.295 Dmax = 1.31)
018194 *****
018195 # Catchment Greenbank
018196 # To Greenbank Drain (south of the Jock)
018197 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018198 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018199 #
018200 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018201 (L/S to 380 / 050/ 045)
018202 (Vmax = 1.295 Dmax = 1.31)
018203 *****
018204 # Catchment Greenbank
018205 # To Greenbank Drain (south of the Jock)
018206 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018207 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018208 #
018209 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018210 (L/S to 380 / 050/ 045)
018211 (Vmax = 1.295 Dmax = 1.31)
018212 *****
018213 # Catchment Greenbank
018214 # To Greenbank Drain (south of the Jock)
018215 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018216 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018217 #
018218 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018219 (L/S to 380 / 050/ 045)
018220 (Vmax = 1.295 Dmax = 1.31)
018221 *****
018222 # Catchment Greenbank
018223 # To Greenbank Drain (south of the Jock)
018224 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018225 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018226 #
018227 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018228 (L/S to 380 / 050/ 045)
018229 (Vmax = 1.295 Dmax = 1.31)
018230 *****
018231 # Catchment Greenbank
018232 # To Greenbank Drain (south of the Jock)
018233 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018234 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018235 #
018236 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018237 (L/S to 380 / 050/ 045)
018238 (Vmax = 1.295 Dmax = 1.31)
018239 *****
018240 # Catchment Greenbank
018241 # To Greenbank Drain (south of the Jock)
018242 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018243 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018244 #
018245 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018246 (L/S to 380 / 050/ 045)
018247 (Vmax = 1.295 Dmax = 1.31)
018248 *****
018249 # Catchment Greenbank
018250 # To Greenbank Drain (south of the Jock)
018251 # JFSA 2021-01-18 add a Reachbank point as per JFSA P598(04)-11, June 2016
018252 # JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
018253 #
018254 # ROUTE CHANNEL -> 1.0 01:SN:TO 54681.13 48.522 N.Date 37:57 13.09 n/a .000
018255 (L/S to 380 / 050/ 045)
018256 (Vmax = 1.295 Dmax = 1.31)
018257 *****

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Table with columns for line numbers and descriptive text for various hydrology and water resource management projects. The text includes project names (e.g., 'Catchment DESIRE', 'JOCKVA area'), technical specifications, flow rates, dates, and references to other documents or figures. The data is organized in a structured list format.

Table with multiple columns containing technical data, including part numbers (e.g., 041155, 041168), descriptions (e.g., [Horton parameters: Fow 76.20; Fc 13.20; DCAV=14; Fe=00]), and values (e.g., 254.24, 16.750, N.Date, 28.01, 30.40, n/a, 0.00).

Table with columns for ID, Description, Area, and various parameters. The table lists numerous entries (e.g., 048633, 048634, 048635) with their respective details, including flow rates, dates, and system types. The entries are organized in a structured grid format.

```

052375 SAVE HYD 1.0 01:SN,M 55194.85 70.602 NoDate 37:03 18.87 n/a .000
052388 *****
052389 remark:Total Flows at Jockvale Road
052400 *****
052410 # Hydrograph from Jockvale Road routed to Heart's Desire
052420 # Channel X-Section obtained from RCVA Hydraulic Model - Station 689
052430 *****
052440 R0005:CO0392-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
052445 ROUTE CHANNEL > 1.0 02:SN,IE 55194.85 70.602 NoDate 27:57 18.87 n/a .000
052460 [R/S/a= 1.00] out<- 1.0 01:SN,DE 55194.85 70.452 NoDate 37:20 18.87 n/a .000
052470 [L/S/a= 1.00] in 223.13 1.59 NoDate 28:03 18.87 n/a .000
052480 [V/mx= 1.303;Dmax= 1.891]
052490 *****
052500 # Catchment DESIRE
052510 # To Jock River (north of the Jock)
052520 # Routing file comment: Based on Heart's Desire Community
052530 *****
052540 R0005:CO0393-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
052555 CONTINUES NASHID 1.0 01:R,R,W 3680.00 11.879 NoDate 36:59 20.23 313 .000
052565 [CN= 66.0; No. 3.00; Tp= 5.29]
052575 [IaREC= 4.00; SM= 56.62; SMAX=350.79; SKe= .010]
052585 [InterEventTime= 12.00]
052595 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
052600 # of 1.52
052610 R0005:CO0394-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
052625 ROUTE CHANNEL > 1.0 02:SN,IE 55194.85 70.452 NoDate 28:09 35.73 n/a .000
052640 [R/S/a= 1.00] out<- 1.0 01:SN,DE 55194.85 70.452 NoDate 28:09 35.73 n/a .000
052650 [L/S/a= 1.00] in 223.13 1.59 NoDate 28:09 35.73 n/a .000
052660 [V/mx= 1.303;Dmax= 1.891]
052670 *****
052680 # Residential development & golf course
052690 # JFSA 2021-01-11 update: CORRGas per IEI GROUP July 2008.
052700 # JFSA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX LAND 32.5 ha as
052710 R0005:CO0394-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
052725 CONTINUES NASHID 1.0 01:JOCKVA 225.13 14.675 NoDate 28:09 35.73 626 .000
052735 [X/Mx= 50.0;Tm= 50]
052745 [LRS= 2; CN= 74.0]
052755 [Perious area: IArea= 4.67; SLP=0.00; LQ= 40.; MPA= 250; SCP= .0]
052765 [Imperious area: IArea= 1.57; SLP=1.00; LQ= 41311.; MR= 0.01; SCL= .0]
052775 [IaRECmp= 4.00; IaRECC= 4.00]
052785 [SMN= 36.67; SMAX=24.49; SKe= .010]
052795 R0005:CO0395-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
052805 AID HYD + 1.0 02:JOCKVA 225.13 14.675 NoDate 28:09 35.73 n/a .000
052815 + 1.0 02:RESRE 23.78 1.359 NoDate 28:03 27.22 n/a .000
052825 + 1.0 02:FO OF 0.00 0.00 NoDate 0:00 0.00 n/a .000
052835 + 1.0 02:JOCK P 257.63 4.781 NoDate 28:48 35.73 n/a .000
052845 SLM 1.0 01:SN,DE 55476.26 71.017 NoDate 37:17 18.95 n/a .000
052855 R0005:CO0396-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
052870 SAVE HYD 1.0 01:JOCKVA 257.63 16.950 NoDate 28:09 35.73 n/a .000
052885 *****
052890 remark:Total Flows at NK first pond
052900 *****
052910 # Jockvale SSM Facility
052920 # Rating curve obtained from Jockvale Servicing Study (CCL 1999)
052930 *****
052940 R0005:CO0397-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
052955 ROUTE RESERVOIR > 1.0 02:JOCKVA 257.63 16.950 NoDate 28:09 35.73 n/a .000
052965 [R/S/a= 1.00] out<- 1.0 02:JOCK P 257.63 4.781 NoDate 28:48 35.73 n/a .000
052975 overflow= 1.0 03:FO OF 0.00 0.00 NoDate 0:00 0.00 n/a .000
052985 [MxStoked= 1.0;LSD= 0.0000040] nb N/C=0; 0; TotDrOf= 0 h:rs]
052995 R0005:CO0398-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
053005 AID HYD + 1.0 02:RESRE 23.78 1.359 NoDate 28:03 27.22 n/a .000
053015 + 1.0 02:IE 23.78 1.359 NoDate 28:03 27.22 n/a .000
053025 + 1.0 02:FO OF 0.00 0.00 NoDate 0:00 0.00 n/a .000
053035 + 1.0 02:JOCK P 257.63 4.781 NoDate 28:48 35.73 n/a .000
053045 SLM 1.0 01:SN,DE 55476.26 71.017 NoDate 37:17 18.95 n/a .000
053055 R0005:CO0399-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
053070 SAVE HYD 1.0 01:SN,IE 55476.26 71.017 NoDate 37:17 18.95 n/a .000
053085 *****
053090 remark:Total Flows at Heart's Desire
053100 *****
053110 # Hydrograph from Heart's Desire routed to Rideau River
053120 # Channel X-Section obtained from RCVA Hydraulic Model - Station 0
053130 *****
053140 R0005:CO0400-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
053155 ROUTE CHANNEL > 1.0 02:SN,IE 55476.26 71.017 NoDate 37:17 18.95 n/a .000
053165 [R/S/a= 1.00] out<- 1.0 02:SN,IE 55476.26 71.017 NoDate 37:17 18.95 n/a .000
053175 [L/S/a= 1.00] in 1967.045]
053185 [V/mx= 1.676;Dmax= 0.5]
053195 *****
053200 # Catchment S-2
053210 # To Jock River (north and south)
053220 # Undeveloped floodplain and river
053230 *****
053240 R0005:CO0401-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
053255 CONTINUES NASHID 1.0 01:R,R,W 102.94 2.262 NoDate 28:20 19.00 .333 .000
053265 [CN= 72.0; No. 3.00; Tp= .40]
053275 [IaREC= 4.00; SM= 39.75; SMAX=264.99; SKe= .010]
053285 [InterEventTime= 12.00]
053295 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
053300 # of 1.67
053310 R0005:CO0402-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
053325 AID HYD + 1.0 02:IE 102.94 2.262 NoDate 28:20 19.00 n/a .000
053335 + 1.0 02:SN,IE 102.94 2.262 NoDate 28:20 19.00 n/a .000
053345 SLM 1.0 01:SN,IE 55579.20 71.168 NoDate 37:08 18.95 n/a .000
053355 R0005:CO0403-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
053370 SAVE HYD 1.0 01:SN,NI 55579.20 71.168 NoDate 37:08 18.95 n/a .000
053385 *****
053390 remark:Total Flows at Rideau River
053400 *****
053410 ** END OF RUN : 9
053420 *****
053430 *****
053440 *****
053450 *****
053460 *****
053470 *****
053480 *****
053490 *****
053500 *****
053510 *****
053520 *****
053530 *****
053540 *****
053550 *****
053560 *****
053570 *****
053580 *****
053590 *****
053600 *****
053610 *****
053620 *****
053630 *****
053640 *****
053650 *****
053660 *****
053670 *****
053680 *****
053690 *****
053700 *****
053710 *****
053720 *****
053730 *****
053740 *****
053750 *****
053760 *****
053770 *****
053780 *****
053790 *****
053800 *****
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053820 *****
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053860 *****
053870 *****
053880 *****
053890 *****
053900 *****
053910 *****
053920 *****
053930 *****
053940 *****
053950 *****
053960 *****
053970 *****
053980 *****
053990 *****
054000 *****
054010 *****
054020 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
054030 # of 1.32
054040 R0005:CO0404-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
054055 CONTINUES NASHID 1.0 01:R,R,W 3680.00 11.879 NoDate 36:59 20.23 313 .000
054065 [CN= 66.0; No. 3.00; Tp= 5.29]
054075 [IaREC= 4.00; SM= 56.62; SMAX=350.79; SKe= .010]
054085 [InterEventTime= 12.00]
054095 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
054100 # of 1.32
054110 R0005:CO0405-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
054125 CONTINUES NASHID 1.0 01:SW,13 971.00 4.365 NoDate 32:35 18.83 291 .000
054135 [CN= 61.0; No. 3.00; Tp= 1.31]
054145 [IaREC= 4.00; SM= 64.50; SMAX=430.01; SKe= .010]
054155 [InterEventTime= 12.00]
054165 *****
054170 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
054180 # of 1.65
054190 R0005:CO0406-----Dfai n-ID NND-----AREHA-QPEAKm-TpeakDate_hh:mm---Rvmm R.C---DFWcm
054200 ROUTE CHANNEL > 1.0 02:SN,IE 55476.26 71.017 NoDate 37:17 18.95 n/a .000
054210 [R/S/a= 1.00] out<- 1.0 01:SN,IE 55476.26 71.017 NoDate 37:17 18.95 n/a .000
054220 [L/S/a= 1.00] in 1967.045]
054230 [V/mx= 1.676;Dmax= 0.5]
054240 *****
054250 *****
054260 *****
054270 *****
054280 *****
054290 *****
054300 *****
054310 *****
054320 *****
054330 *****
054340 *****
054350 *****
054360 *****
054370 *****
054380 *****
054390 *****
054400 *****
054410 *****
054420 *****
054430 *****
054440 *****
054450 *****
054460 *****
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054480 *****
054490 *****
054500 *****
054510 *****
054520 *****
054530 *****
054540 *****
054550 *****
054560 *****
054570 *****
054580 *****
054590 *****
054600 *****
054610 *****
054620 *****
054630 *****
054640 *****
054650 *****
054660 *****
054670 *****
054680 *****
054690 *****
054700 *****
054710 *****
054720 *****
054730 *****
054740 *****
054750 *****
054760 *****
054770 *****
054780 *****
054790 *****
054800 *****
054810 *****
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054880 *****
054890 *****
054900 *****
054910 *****
054920 *****
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054970 *****
054980 *****
054990 *****
055000 *****
055010 *****
055020 *****
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055080 *****
055090 *****
055100 *****
055110 *****
055120 *****
055130 *****
055140 *****
055150 *****
055160 *****
055170 *****
055180 *****
055190 *****
055200 *****
055210 *****
055220 *****
055230 *****
055240 *****
055250 *****
055260 *****
055270 *****
055280 *****
055290 *****
055300 *****
055310 *****
055320 *****
055330 *****
055340 *****
055350 *****
055360 *****
055370 *****
055380 *****
055390 *****
055400 *****
055410 *****
055420 *****
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055630 *****
055640 *****
055650 *****
055660 *****
055670 *****
055680 *****
055690 *****
055700 *****
055710 *****
055720 *****
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055750 *****
055760 *****
055770 *****
055780 *****
055790 *****
055800 *****
055810 *****
055820 *****
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055840 *****
055850 *****
055860 *****
055870 *****
055880 *****
055890 *****
055900 *****
055910 *****
055920 *****
055930 *****
055940 *****
055950 *****
055960 *****
055970 *****
055980 *****
055990 *****
056000 *****
056010 *****
056020 *****
056030 *****
056040 *****
056050 *****
056060 *****
056070 *****
056080 *****
056090 *****
056100 *****

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056113 # (Vmax = .546; Dmax = 1.499)
056128 #
056133 # Addition of Subwatershed Jack River at Ashton to Node 12
056143 #
056153 # R010: C00036 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056163 # ADD HYD + 1.0 02: S_N2 7725.00 3,519.00 Date 31:53 18.46 n/a 0.00
056173 # 1.0 02: R_ASH 1781.00 10,819.00 Date 32:42 24.81 n/a 0.00
056183 # SIMM 1.0 01: S_N2 9706.00 12,834.00 Date 32:45 19.65 n/a 0.00
056189 # R010: C00037 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056200 # SAVE HYD 1.0 01: S_N2 9506.00 12,834.00 Date 32:45 19.65 n/a 0.00
056213 # frame_HLSN2
056223 # remark:flow at S_N2 near Ashton
056233 #
056243 # Sum of hydrographs from Node 12 routed to Node 11
056253 # (Approximate cross-section - see cross-section 258)
056263 # like n=0.04 for summer conditions and n=0.025 for spring conditions
056273 #
056283 # Sum of hydrographs from Node 12 routed to Node 11 with Dumpy section 248
056293 #
056303 # R010: C00038 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056313 # ROUTE CHANNEL -> 1.0 02: S_N2 9506.00 55,834.00 Date 32:45 19.65 n/a 0.00
056323 # [RfIn: 1.00] out c 1.0 01: Dm1 9506.00 12,710.00 Date 33:02 19.65 n/a 0.00
056333 # [L/S = 972 / 0.50 / 0.00]
056343 # (Vmax = .680; Dmax = 2.598)
056353 #
056363 # Addition of Subwatershed 11 and No Name Creek to Node 11
056373 #
056383 # R010: C00039 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056393 # ADD HYD + 1.0 02: Dm1 9506.00 12,710.00 Date 33:02 19.65 n/a 0.00
056403 # 1.0 02: SW11 500.00 5,639.00 Date 29:22 21.19 n/a 0.00
056413 # 1.0 02: NC_CK 1917.00 7,897.00 Date 34:28 21.19 n/a 0.00
056423 # SIMM 1.0 01: S_N1 11923.00 21,813.00 Date 33:05 19.96 n/a 0.00
056433 #
056443 # Sum of hydrographs from Node 11 routed to Node 10
056453 # Section 1
056463 #
056473 # R010: C00040 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056483 # ROUTE CHANNEL -> 1.0 02: S_N1 11923.00 21,813.00 Date 33:05 19.96 n/a 0.00
056493 # [RfIn: 1.00] out c 1.0 01: N0 11923.00 14,761.00 Date 39:58 19.96 n/a 0.00
056503 # [L/S = 44028 / 1.57 / 0.00]
056513 # (Vmax = .452; Dmax = 1.212)
056523 #
056533 # Addition of Subwatershed 10 to Node 10
056543 #
056553 # R010: C00041 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056563 # ADD HYD + 1.0 02: N0 11923.00 14,761.00 Date 39:58 19.96 n/a 0.00
056573 # 1.0 02: SW10 5666.00 21,255.00 Date 37:58 24.81 n/a 0.00
056583 # SIMM 1.0 01: S_N0 17589.00 35,808.00 Date 38:35 21.52 n/a 0.00
056589 # R010: C00042 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056599 # SAVE HYD 1.0 01: S_N0 17589.00 35,808.00 Date 38:35 21.52 n/a 0.00
056613 # frame_HLSN0
056623 # remark:flow at S_N0: N0 + SW10
056633 # Addition of Kings Creek to S_N10
056643 #
056653 # R010: C00043 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056663 # ADD HYD + 1.0 02: S_N0 17589.00 35,808.00 Date 38:35 21.52 n/a 0.00
056673 # 1.0 02: NC_CK 8376.00 20,208.00 Date 39:59 21.19 n/a 0.00
056683 # SIMM 1.0 01: S_N10A 25965.00 55,807.00 Date 39:58 21.41 n/a 0.00
056693 #
056703 # Sum of hydrographs from Node 10 routed to Node 9
056713 # Section 2
056723 #
056733 # R010: C00044 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056743 # ROUTE CHANNEL -> 1.0 02: S_N1 11923.00 21,813.00 Date 33:05 19.96 n/a 0.00
056753 # [RfIn: 1.00] out c 1.0 01: N0 25965.00 54,076.00 Date 39:59 21.41 n/a 0.00
056763 # [L/S = 3092 / 0.70 / 0.00]
056773 # (Vmax = .682; Dmax = 1.695)
056783 #
056793 # Addition of Subwatershed 9 and Nichols Creek to Node 9
056803 #
056813 # R010: C00045 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056823 # ADD HYD + 1.0 02: N0 25965.00 54,076.00 Date 39:59 21.41 n/a 0.00
056833 # 1.0 02: SW9 1132.00 8,921.00 Date 30:54 23.73 n/a 0.00
056843 # 1.0 02: NC_CK 4464.00 10,128.00 Date 39:59 19.29 n/a 0.00
056853 # SIMM 1.0 01: S_N0A 31561.00 66,284.00 Date 39:59 21.20 n/a 0.00
056863 #
056873 # Sum of hydrographs from Node 9 routed to Node 8
056883 # Section 3
056893 #
056903 # R010: C00046 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056913 # ROUTE CHANNEL -> 1.0 02: S_N 31561.00 66,284.00 Date 39:59 21.20 n/a 0.00
056923 # [RfIn: 1.00] out c 1.0 01: N8 31561.00 61,483.00 Date 39:57 21.20 n/a 0.00
056933 # [L/S = 2209 / 0.88 / 0.00]
056943 # (Vmax = .363; Dmax = 1.619)
056953 #
056963 # Addition of Subwatershed 8 and Hobbs' Drain to Node 8
056973 #
056983 # R010: C00047 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
056993 # ADD HYD + 1.0 02: N8 31561.00 61,483.00 Date 39:57 21.20 n/a 0.00
057003 # 1.0 02: SW8 131.00 8,921.00 Date 30:54 23.73 n/a 0.00
057013 # 1.0 02: HB_DR 3854.00 11,811.00 Date 38:37 21.19 n/a 0.00
057023 # SIMM 1.0 01: S_N 35546.00 73,344.00 Date 39:57 21.19 n/a 0.00
057033 #
057043 # Sum of hydrographs from Node 8 routed to Node 7
057053 # Section 4
057063 #
057073 # R010: C00048 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057083 # ROUTE CHANNEL -> 1.0 02: S_N 35546.00 73,344.00 Date 39:57 21.19 n/a 0.00
057093 # [RfIn: 1.00] out c 1.0 01: N7 35546.00 61,416.00 Date 45:01 21.19 n/a 0.00
057103 # [L/S = 2740 / 0.59 / 0.00]
057113 # (Vmax = .218; Dmax = 1.987)
057123 #
057133 # Addition of Subwatershed 7 to Node 7
057143 #
057153 # R010: C00049 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057163 # ADD HYD + 1.0 02: N7 35546.00 61,416.00 Date 45:01 21.19 n/a 0.00
057173 # 1.0 02: SW7 3197.00 8,809.00 Date 36:26 17.07 n/a 0.00
057183 # SIMM 1.0 01: S_N7 38743.00 65,819.00 Date 44:06 20.85 n/a 0.00
057189 # R010: C00050 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057200 # SAVE HYD 1.0 01: S_N7 38743.00 65,819.00 Date 44:06 20.85 n/a 0.00
057213 # frame_HLSN7
057223 # remark:flow at S_N7: N7 + SW7
057233 # Insertion of a reservoir to simulate the effects of the Richmond Fen.
057243 # Storage area and volumes were estimated from available topo maps.
057253 # Release rate from Fen was assumed to be controlled by the downstream
057263 # river cross section for summer conditions. It was assumed that for up to
057273 # 0.75 m of water, the main channel of the river provided the storage. Above
057283 # this depth, the wetland stands to significantly store water.
057293 #
057303 # R010: C00051 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057313 # ROUTE CHANNEL -> 1.0 01: RES_RF 38743.00 31,796.00 Date 60:32 20.85 n/a 0.00
057323 # [RfIn: 1.00] out c 1.0 01: RES_RF 38743.00 31,796.00 Date 60:32 20.85 n/a 0.00
057333 # [MkStoDec: 2597E04]
057343 # R010: C00052 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057353 # SAVE HYD 1.0 01: RES_RF 38743.00 31,796.00 Date 60:32 20.85 n/a 0.00
057363 # frame_HLSRF
057373 # remark:outflow of Richmond Fen
057383 #
057393 # Sum of hydrographs from Node 7 routed to Node 6
057403 # Section 5
057413 #
057423 # R010: C00053 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057433 # ROUTE CHANNEL -> 1.0 02: S_N 38743.00 31,796.00 Date 60:32 20.85 n/a 0.00
057443 # [RfIn: 1.00] out c 1.0 01: N6 38743.00 31,737.00 Date 62:00 20.85 n/a 0.00
057453 # [L/S = 3056 / 0.82 / 0.25]
057463 # (Vmax = .477; Dmax = .960)
057473 #
057483 # Addition of Subwatershed 6 and Van Gaal Drain to Node 6
057493 #
057503 # R010: C00054 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057513 # ADD HYD + 1.0 02: N6 38743.00 31,737.00 Date 62:00 20.85 n/a 0.00
057523 # 1.0 02: SW6 165.00 818.00 Date 33:04 21.69 n/a 0.00
057533 # 1.0 02: VG_DR 1332.00 6,069.00 Date 35:17 24.81 n/a 0.00
057543 # SIMM 1.0 01: S_N 40240.01 31,737.00 Date 62:00 20.99 n/a 0.00
057553 #
057563 # Sum of hydrographs from Node 6 routed to Node 5
057573 # Section 6
057583 #
057593 # R010: C00055 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057603 # ROUTE CHANNEL -> 1.0 02: S_N 40240.01 31,737.00 Date 62:00 20.99 n/a 0.00
057613 # [RfIn: 1.00] out c 1.0 01: N5 40240.01 31,713.00 Date 62:48 20.99 n/a 0.00
057623 # [L/S = 182 / 0.50 / 0.15]
057633 # (Vmax = .412; Dmax = 1.069)
057643 #
057653 # Addition of Subwatershed 5 and Flowing Creek to Node 5
057663 #
057673 # R010: C00056 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057683 # ADD HYD + 1.0 02: N5 40240.01 31,713.00 Date 62:48 20.99 n/a 0.00
057693 # 1.0 02: FC 4025.00 5,246.00 Date 28:36 21.27 n/a 0.00
057703 # 1.0 02: FL_CK 1612.00 49,451.00 Date 33:21 25.91 n/a 0.00
057713 # SIMM 1.0 01: S_N 45409.01 51,448.00 Date 34:54 21.56 n/a 0.00
057723 #
057733 # Sum of hydrographs from Node 5 routed to Node 5A
057743 # Section 7
057753 #
057763 # R010: C00057 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057773 # ROUTE CHANNEL -> 1.0 02: S_N 45409.01 51,448.00 Date 34:54 21.56 n/a 0.00
057783 # [RfIn: 1.00] out c 1.0 01: N5A 45409.01 51,312.00 Date 35:12 21.56 n/a 0.00
057793 # [L/S = 556 / 0.90 / 0.40]
057803 # (Vmax = .485; Dmax = 1.131)
057813 #
057823 # Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
057833 #
057843 # R010: C00058 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057853 # ADD HYD + 1.0 02: N5A 45409.01 51,312.00 Date 35:12 21.56 n/a 0.00
057863 # 1.0 02: SW5A 20.00 614.00 Date 29:36 21.27 n/a 0.00
057873 # 1.0 02: SW_SAI 1612.00 5,817.00 Date 37:54 27.06 n/a 0.00
057883 # SIMM 1.0 01: S_N5A 46844.01 56,788.00 Date 35:22 21.73 n/a 0.00
057893 #
057903 # Sum of hydrographs from Node 5A routed to Node 4
057913 # Section 8
057923 #
057933 # R010: C00059 ----- DfIn-ID NND ----- AREQA-QPEAKm-TpeakDte-hh:mm ----- Rvmm-R-C-DFWcm
057943 # ROUTE CHANNEL -> 1.0 02: S_N5A 46844.01 56,788.00 Date 35:22 21.73 n/a 0.00
057953 # [RfIn: 1.00] out c 1.0 01: N4 46844.01 54,543.00 Date 36:36 21.73 n/a 0.00
057963 # [L/S = 4630 / 0.40 / 0.51]
057973 # (Vmax = .793; Dmax = 3.295)

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063599 # [Impervious area: IArea=1.57:SLIP= 50. LGA=1473. :MN= 013:SCI= 0]

063600 # [Area= 4.00: IAREC= 4.00]

063601 # [SMN= 36.67: SMMW=24.49: SSG= 010]

063602 # [LRS= 2. :CN= 77.0]

063603 # Foster Pond

063604 # - Rating curve obtained assuming 4000/h in 24 hours for quality control

063605 # and ratio of the catchment area to the West Clarke pond rating curve

063606 # from the MS for the next coordinates

063607 #

063608 # *****

063609 # [Impervious area: IArea= 4.67:SLIP= 50. LGA= 699. :MN= 013:SCI= 0]

063610 # [Area= 4.00: IAREC= 4.00]

063611 # [SMN= 31.15: SMMW=207.66: SSG= 010]

063612 # [LRS= 2. :CN= 77.0]

063613 # [Impervious area: IArea= 4.67:SLIP= 50. LGA= 699. :MN= 013:SCI= 0]

063614 # [Area= 4.00: IAREC= 4.00]

063615 # [SMN= 31.15: SMMW=207.66: SSG= 010]

063616 # ROUTE RESEVER > - DfIn= 1-D NND -> AREAH= QPEAK= TpeakDt= hh:mm -> Rvmm R.C. -> DWfm

063617 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063618 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063619 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063620 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063621 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063622 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063623 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063624 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063625 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063626 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063627 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063628 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063629 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063630 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063631 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063632 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063633 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063634 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063635 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063636 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063637 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063638 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063639 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063640 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063641 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063642 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063643 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063644 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063645 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063646 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063647 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063648 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063649 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063650 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063651 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063652 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063653 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063654 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063655 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063656 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063657 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063658 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063659 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063660 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063661 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063662 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063663 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063664 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063665 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063666 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063667 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063668 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063669 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063670 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063671 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063672 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063673 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063674 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063675 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063676 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063677 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063678 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063679 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063680 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063681 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063682 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063683 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063684 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063685 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063686 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063687 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063688 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063689 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063690 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063691 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063692 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063693 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063694 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063695 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063696 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063697 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063698 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063699 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

063700 # [MSysStn= 00000E00] m0: NcOf= 0. ToDirOf= 0 hrs

Table with multiple columns containing alphanumeric codes (e.g., 071075, 071088), numerical values, and descriptive text. The text includes technical specifications, station names, and flow descriptions. The table is organized into sections, with some rows highlighted in grey.


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078555 # Rating curve obtained from CCL hydraulic modeling
078560 *****
078570 ROUTE CH00390 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
078580 ROUTE RESERVOIR ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
078590 out = 1.0 01:SN,M 175.99 12.212 No,date 28:07 36.76 n/a 0.00
078600 overlow sc = 1.0 03:ML,CV 0.00 0.00 No,date 0:00 0.00 n/a 0.00
078610 [MS:otted=4.673E+00] m, No,Of= 0, TotDurOf= 0 hrs.] *****
078620 ROUTE CH00390 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
078630 ADD HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
078640 + 1.0 02:ML,CV 0.00 0.00 No,date 0:00 0.00 n/a 0.00
078650 + 1.0 01:SN,M 175.99 12.212 No,date 28:07 36.76 n/a 0.00
078660 SIMM 1.0 01:SN,M 55194.85 85.786 No,date 37:17 23.16 n/a 0.00
078670 ROUTE CH00391 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
078680 SAVE HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
078690 Iname = SN,N 0010
078700 remark:Total Flow at Jockvale Road
078710 *****
078720 # Hydrograph from Jockvale Road routed to Heart's Desire
078730 # Channel X Section obtained from RMA2 Hydraulic Model Station 689
078740 *****
078750 ROUTE CH00392 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
078760 ROUTE CHANNEL -> 1.0 02:SN,M 55194.85 85.786 No,date 37:17 23.16 n/a 0.00
078770 [RFD= 1.00] out = 1.0 01:SN,DE 55194.85 85.715 No,date 37:21 23.16 n/a 0.00
078780 [L/S= 1962. / 2231.045]
078790 [Vmax = 1.390; Dmax = 2.066]
078800 *****
078810 # Catchment DESIRE
078820 # To Jock River (north of the Jock)
078830 # Rural estate subdivision (Heart's Desire Community)
078840 *****
078850 ROUTE CH00393 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
078860 CONTINUOUS STANKRID 1.0 01:RESIRE 23.78 1.694 No,date 28:03 32.93 509 0.00
078870 [XlMn = 25; TlMn = 25]
078880 [LRS = 2; Cn = 77.0]
078890 # Impervious area Iper = 4.67; SLP1 = 0.0; LQ = 4.0; MNP = 250; SCP = 0]
078900 [Impervious area Iper = 1.57; SLP1 = 0.0; LQ = 4.0; MNP = 250; SCP = 0]
078910 [Iper = 4.00; IperC = 0.0]
078920 [SMN = 31.15; SMX = 207.66; SKE = 0.0]
078930 *****
078940 # Catchment JOCKVA
078950 # To Jockvale SWM Facility
078960 # Residential development & golf course
078970 # JFSA 2012-01-11 updated (OCCA after updating CORIG Gas per IRI GROUP, July 2008.
078980 # JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX LAND 32.5 ha as
078990 *****
079000 ROUTE CH00394 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079010 CONTINUOUS STANKRID 1.0 01:JOCKVA 225.13 19.945 No,date 28:08 41.88 647 0.00
079020 [XlMn = 50; TlMn = 50]
079030 [LRS = 2; Cn = 74.0]
079040 # Impervious area Iper = 4.67; SLP1 = 0.0; LQ = 4.0; MNP = 250; SCP = 0]
079050 [Impervious area Iper = 1.57; SLP1 = 0.0; LQ = 4.0; MNP = 250; SCP = 0]
079060 [Iper = 4.00; IperC = 0.0]
079070 [SMN = 36.67; SMX = 244.49; SKE = 0.0]
079080 *****
079090 ROUTE CH00395 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079100 ADD HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079110 + 1.0 02:EX LAND 32.50 2.275 No,date 27:55 41.99 n/a 0.00
079120 + 1.0 02:JOCKVA 225.13 19.945 No,date 28:08 41.88 n/a 0.00
079130 + 1.0 02:RES M 0.00 0.00 No,date 0:00 0.00 n/a 0.00
079140 SIMM 1.0 01:SN,M 257.63 19.945 No,date 28:08 41.88 n/a 0.00
079150 ROUTE CH00396 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079160 SAVE HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079170 Iname = JOCKVA TO 0100
079180 remark:Total Flow at KB first pond
079190 *****
079200 # Jockvale SWM Facility
079210 # Rating curve obtained from Jockvale Servicing Study (CCL 1999)
079220 *****
079230 ROUTE CH00397 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079240 ROUTE RESERVOIR ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079250 out = 1.0 01:JOCK,P 257.63 6.542 No,date 28:42 41.88 n/a 0.00
079260 overlow sc = 1.0 03:ML,CV 0.00 0.00 No,date 0:00 0.00 n/a 0.00
079270 [MS:otted=4.673E+00] m, No,Of= 0, TotDurOf= 0 hrs.] *****
079280 ROUTE CH00398 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079290 ADD HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079300 + 1.0 02:RESIRE 23.78 1.694 No,date 28:03 32.93 n/a 0.00
079310 + 1.0 02:RES M 0.00 0.00 No,date 0:00 0.00 n/a 0.00
079320 + 1.0 01:JOCK,P 257.63 6.542 No,date 28:42 41.88 n/a 0.00
079330 SIMM 1.0 01:SN,DE 55476.26 86.165 No,date 37:21 23.26 n/a 0.00
079340 ROUTE CH00399 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079350 SAVE HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079360 Iname = SN,DE.0010
079370 remark:Total Flow at Heart's Desire
079380 *****
079390 # Hydrograph from Heart's Desire routed to Rideau River
079400 # Channel X Section obtained from RMA2 Hydraulic Model Station 0
079410 *****
079420 ROUTE CH00400 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079430 ROUTE CHANNEL ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079440 [RFD= 1.00] out = 1.0 01:SN,DE 55476.26 86.163 No,date 37:18 23.26 n/a 0.00
079450 [L/S= 1962. / 967.0]
079460 [Vmax = 1.785; Dmax = 1.035]
079470 *****
079480 # Catchment S 2
079490 # To Jock River (north and south)
079500 # Undeveloped floodable river
079510 *****
079520 ROUTE CH00401 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079530 CONTINUOUS NASIHD 1.0 01:S 2 102.94 2.957 No,date 28:20 23.57 364 0.00
079540 [CEN: 72.0; No: 3.00; Tp: 4.0]
079550 [Iper = 4.00; SMN = 39.75; SMX = 264.99; SKE = 0.0]
079560 [InterEvent Time = 12.00]
079570 *****
079580 ROUTE CH00402 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079590 ADD HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079600 + 1.0 02:SN,M 55476.26 86.183 No,date 37:18 23.26 n/a 0.00
079610 + 1.0 02:RES M 102.94 2.957 No,date 28:20 23.57 n/a 0.00
079620 SIMM 1.0 01:SN,N 55579.20 86.348 No,date 37:18 23.26 n/a 0.00
079630 ROUTE CH00403 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
079640 SAVE HYD ***** [CEN: 75.00; No: 3.00; Tp: 3.76] *****
079650 Iname = SN,N 0010
079660 remark:Total Flow at Rideau River
079670 *****
079680 # END OF RUN : 24
079690 *****
079700 *****
079710 *****
079720 *****
079730 RESN-COMMAND *****
079740 ROUTES:CH0001 *****
079750 *****
079760 [TZERO = 00 hrs on 0]
079770 [METX = 2 (imperial, 2=metric output)]
079780 [NIN = 025]
079790 *****
079800 *****
079810 # SWMDO No: 5.072/Jan 2001-BETA7 - INPUT DATA FILE
079820 *****
079830 # Project Name: [Jock River] PROJECT Number: [1474.16]
079840 # Date: 04-Nov-2021
079850 # Modifier: [JMM]
079860 # Company: JFSa Inc.
079870 # License: 2540237
079880 *****
079890 # CALIBRATION OF SIMMER MODEL PARAMETERS
079900 # USING CONTINUOUS SIMULATIONS
079910 # Rainfall data from JFSa rain gauge installed at site + other gauges by the City
079920 # Use data collected from May 1st to July 1st, 2000
079930 # 2020-11-30 change TMS10 in COMPUTE DUALHD (TMS10 = 0.1 instead of 0.0001)
079940 # 2020-12-01 correct pond curve value
079950 # 2020-12-01 change WCLAR, BRAC, XMP to 0.55, SLP1=[0.51] (imperial slope), and LGL up to 70mm
079960 # 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NDDm=[N,TOT], NDDm=[SN,TOT]) from 0.03 % (as per S
079970 # 2021-02-19 Change the slope for ROUTE CHANNEL Station 5002 (NDDm=[N,CN], NDDm=[SN,CN]) from 0.01 % (as per S
079980 *****
079990 ROUTES:CH0002 *****
080000 READ STORM
080010 # Filenam = storm01
080020 # Commat = Pluie SCS de 24 BRES 1.25 ans pour Ottawa CDA
080030 [SDF=10.00;SDR= 24.00;POT= 74.39]
080040 ROUTES:CH0003 *****
080050 # MODIFY STORM
080060 [REACT= 1.00;TSH FT= 960.00 min]
080070 [SDF=10.00;SDR= 40.00;POT= 74.39]
080080 *****
080090 #DEFAULT VALUES
080100 # Filenam = T:\PRJ\1474.16\Design\20201026-QuantityControlAnalysis\SWMDO\SME_Model\ed3\GtrGrp.DEP
080110 [CNSGV = 1 (read and print data)]
080120 # FilTitle= File comment: [Based on various calibration exercises in Ota
080130 *****
080140 # Horton's infiltration equation parameters:
080150 [Tau = 60.00 min]
080160 # Parameters for PERKINS surfaces in STANKRID
080170 [Iper = 4.67 mm [LQ=50.00 mm [SNP = 250]
080180 # Parameters for IMPERV surfaces in STANKRID
080190 [Iper = 1.57 mm [CL = 1.50] [MNP = 0.13]
080200 # Parameters used in routing
080210 [Ia = 4.67 mm [N = 3.00]
080220 # Average monthly Pan Evaporation data in (mm)
080230 # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
080240 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
080250 # Average monthly Potential Evapotranspiration in (mm)
080260 # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
080270 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
080280 ROUTES:CH0005 *****
080290 # COMPUTE API
080300 [API=1.00;AP1= 8500.00;AP1k= 9989]
080310 [API=1.00;AP1= 65.00;AP1k= 44.87]
080320 *****
080330 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
080340 *****
080350 ROUTES:CH0006 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080360 CONTINUOUS NASIHD 1.0 01:JR,RW 3680.00 15.500 No,date 36:57 25.80 347 0.00
080370 [CEN: 72.0; No: 3.00; Tp: 4.3]
080380 [Iper = 4.00; SMN = 57.05; SMX = 380.32; SKE = 0.0]
080390 [InterEvent Time = 12.00]
080400 *****
080410 # The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)

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080420 # of 1.32
080430 ROUTES:CH0007 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080440 CONTINUOUS NASIHD 1.0 01:SW13 971.00 5.778 No,date 32:34 24.02 323 0.00
080450 [CEN: 72.0; No: 3.00; Tp: 3.76]
080460 [Iper = 4.00; SMN = 64.50; SMX = 430.01; SKE = 0.0]
080470 [InterEvent Time = 12.00]
080480 *****
080490 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
080500 *****
080510 ROUTES:CH0008 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080520 CONTINUOUS NASIHD 1.0 01:JR,GM 3074.00 7.521 No,date 39:59 20.65 278 0.00
080530 [CEN: 55.0; No: 3.00; Tp=11.32]
080540 [Iper = 4.00; SMN = 83.24; SMX = 554.96; SKE = 0.0]
080550 [InterEvent Time = 12.00]
080560 ROUTES:CH0009 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080570 CONTINUOUS NASIHD 1.0 01:JR,SW 1781.00 14.160 No,date 32:40 31.50 423 0.00
080580 [CEN: 72.0; No: 3.00; Tp = 3.91]
080590 [Iper = 4.00; SMN = 39.75; SMX = 264.99; SKE = 0.0]
080600 [InterEvent Time = 12.00]
080610 ROUTES:CH0010 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080620 CONTINUOUS NASIHD 1.0 01:SW11 500.00 7.521 No,date 29:22 27.01 363 0.00
080630 [CEN: 66.0; No: 3.00; Tp = 1.24]
080640 [Iper = 4.00; SMN = 52.62; SMX = 350.79; SKE = 0.0]
080650 [InterEvent Time = 12.00]
080660 *****
080670 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
080680 # of 1.50
080690 ROUTES:CH0011 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080700 CONTINUOUS NASIHD 1.0 01:NN,CK 1917.00 10.351 No,date 34:27 27.01 363 0.00
080710 [CEN: 66.0; No: 3.00; Tp = 5.29]
080720 [Iper = 4.00; SMN = 52.62; SMX = 350.79; SKE = 0.0]
080730 [InterEvent Time = 12.00]
080740 *****
080750 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
080760 # of 1.52
080770 ROUTES:CH0012 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080780 CONTINUOUS NASIHD 1.0 01:SW10 5666.00 27.457 No,date 37:54 31.50 423 0.00
080790 [CEN: 72.0; No: 3.00; Tp = 3.0]
080800 [Iper = 4.00; SMN = 39.75; SMX = 264.99; SKE = 0.0]
080810 [InterEvent Time = 12.00]
080820 *****
080830 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
080840 # of 1.75
080850 ROUTES:CH0013 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080860 CONTINUOUS NASIHD 1.0 01:SW11 8776.00 26.276 No,date 39:58 27.01 363 0.00
080870 [CEN: 66.0; No: 3.00; Tp=11.66]
080880 [Iper = 4.00; SMN = 52.62; SMX = 350.79; SKE = 0.0]
080890 [InterEvent Time = 12.00]
080900 *****
080910 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
080920 # of 1.68
080930 ROUTES:CH0014 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
080940 CONTINUOUS NASIHD 1.0 01:SW9 1132.00 11.752 No,date 30:54 30.18 406 0.00
080950 [CEN: 72.0; No: 3.00; Tp = 2.51]
080960 [Iper = 4.00; SMN = 43.07; SMX = 287.10; SKE = 0.0]
080970 [InterEvent Time = 12.00]
080980 *****
080990 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081000 # of 1.52
081010 ROUTES:CH0015 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081020 CONTINUOUS NASIHD 1.0 01:SW10 4484.00 13.075 No,date 39:59 24.61 331 0.00
081030 [CEN: 62.0; No: 3.00; Tp=11.32]
081040 [Iper = 4.00; SMN = 61.90; SMX = 412.66; SKE = 0.0]
081050 [InterEvent Time = 12.00]
081060 *****
081070 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081080 # of 1.50
081090 ROUTES:CH0016 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081100 CONTINUOUS NASIHD 1.0 01:SW8 131.00 2.266 No,date 25:57 25.20 339 0.00
081110 [CEN: 63.0; No: 3.00; Tp = 9.0]
081120 [Iper = 4.00; SMN = 50.42; SMX = 396.11; SKE = 0.0]
081130 [InterEvent Time = 12.00]
081140 *****
081150 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081160 # of 1.6
081170 ROUTES:CH0017 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081180 CONTINUOUS NASIHD 1.0 01:BR,LR 3854.00 15.333 No,date 38:34 27.01 363 0.00
081190 [Iper = 4.00; SMN = 52.62; SMX = 350.79; SKE = 0.0]
081200 [Iper = 4.00; SMN = 52.62; SMX = 350.79; SKE = 0.0]
081210 [InterEvent Time = 12.00]
081220 *****
081230 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081240 # of 1.82
081250 ROUTES:CH0018 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081260 CONTINUOUS NASIHD 1.0 01:SW7 3197.00 11.663 No,date 36:24 21.75 292 0.00
081270 [CEN: 57.0; No: 3.00; Tp = 6.65]
081280 [Iper = 4.00; SMN = 37.32; SMX = 508.81; SKE = 0.0]
081290 [InterEvent Time = 12.00]
081300 *****
081310 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081320 # of 1.75
081330 ROUTES:CH0019 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081340 CONTINUOUS NASIHD 1.0 01:SW6 165.00 1.076 No,date 33:03 27.63 371 0.00
081350 [CEN: 67.0; No: 3.00; Tp = 4.18]
081360 [Iper = 4.00; SMN = 50.42; SMX = 396.11; SKE = 0.0]
081370 [InterEvent Time = 12.00]
081380 *****
081390 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081400 # of 1.67
081410 ROUTES:CH0020 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081420 CONTINUOUS NASIHD 1.0 01:VGLR 1332.00 7.882 No,date 35:14 31.50 423 0.00
081430 [CEN: 72.0; No: 3.00; Tp = 5.95]
081440 [Iper = 4.00; SMN = 39.75; SMX = 264.99; SKE = 0.0]
081450 [InterEvent Time = 12.00]
081460 ROUTES:CH0021 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081470 CONTINUOUS NASIHD 1.0 01:SW5 224.00 6.882 No,date 28:45 35.66 479 0.00
081480 [CEN: 77.0; No: 3.00; Tp = 7.5]
081490 [Iper = 4.00; SMN = 31.15; SMX = 207.66; SKE = 0.0]
081500 [InterEvent Time = 12.00]
081510 *****
081520 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081530 # of 1.20
081540 ROUTES:CH0022 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081550 CONTINUOUS NASIHD 1.0 01:FL,CK 4945.00 37.664 No,date 33:18 32.85 442 0.00
081560 [CEN: 74.0; No: 3.00; Tp = 4.45]
081570 [Iper = 4.00; SMN = 65.67; SMX = 244.49; SKE = 0.0]
081580 [InterEvent Time = 12.00]
081590 ROUTES:CH0023 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081600 CONTINUOUS NASIHD 1.0 01:SW,SA2 20.00 7.988 No,date 28:35 39.36 529 0.00
081610 [CEN: 81.0; No: 3.00; Tp = 6.2]
081620 [Iper = 4.00; SMN = 25.41; SMX = 168.09; SKE = 0.0]
081630 [InterEvent Time = 12.00]
081640 *****
081650 # The Tp was modified according to a Peak Reduction factor (MO Chart B2-4)
081660 # of 1.0
081670 ROUTES:CH0024 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081680 CONTINUOUS NASIHD 1.0 01:SW,SA1 1412.00 7.480 No,date 37:50 34.24 460 0.00
081690 [CEN: 75.0; No: 3.00; Tp = 5.81]
081700 [Iper = 4.00; SMN = 35.81; SMX = 225.43; SKE = 0.0]
081710 [InterEvent Time = 12.00]
081720 ROUTES:CH0025 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081730 CONTINUOUS NASIHD 1.0 01:SW,SA 585.00 10.942 No,date 29:56 39.36 529 0.00
081740 [CEN: 81.0; No: 3.00; Tp = 7.5]
081750 [Iper = 4.00; SMN = 25.21; SMX = 168.09; SKE = 0.0]
081760 [InterEvent Time = 12.00]
081770 ROUTES:CH0026 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081780 CONTINUOUS NASIHD 1.0 01:LM,CK 1021.00 14.476 No,date 30:46 38.60 519 0.00
081790 [CEN: 80.0; No: 3.00; Tp = 2.49]
081800 [Iper = 4.00; SMN = 26.32; SMX = 175.50; SKE = 0.0]
081810 [InterEvent Time = 12.00]
081820 ROUTES:CH0027 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081830 CONTINUOUS NASIHD 1.0 01:SW,SA2 177.00 5.438 No,date 28:45 35.66 479 0.00
081840 [CEN: 77.0; No: 3.00; Tp = 7.5]
081850 [Iper = 4.00; SMN = 31.15; SMX = 207.66; SKE = 0.0]
081860 [InterEvent Time = 12.00]
081870 ROUTES:CH0028 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081880 CONTINUOUS NASIHD 1.0 01:SM,IR 1122.00 37.664 No,date 33:45 39.36 529 0.00
081890 [CEN: 81.0; No: 3.00; Tp = 3.25]
081900 [Iper = 4.00; SMN = 25.41; SMX = 168.09; SKE = 0.0]
081910 [InterEvent Time = 12.00]
081920 ROUTES:CH0029 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
081930 CONTINUOUS NASIHD 1.0 01:ML,CK 1021.00 14.476 No,date 30:46 38.60 519 0.00
081940 [CEN: 76.0; No: 3.00; Tp = 3.0]
081950 [Iper = 4.00; SMN = 52.62; SMX = 350.79; SKE = 0.0]
081960 [InterEvent Time = 12.00]
081970 *****
081980 # Routing hydrographs
081990 # Starting with the addition of Lock River Headwater and Subwatershed 13
082000 *****
082010 ROUTES:CH0030 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
082020 ADD HYD ***** [CEN: 75.00; No: 3.00; Tp = 3.76] *****
082030 + 1.0 02:JR,HW 3680.00 15.500 No,date 36:56 25.80 n/a 0.00
082040 + 1.0 02:SW13 971.00 5.778 No,date 32:34 24.02 n/a 0.00
082050 SIMM 1.0 01:SN,SA 3074.00 7.521 No,date 39:59 20.65 n/a 0.00
082060 # Sum of hydrographs from Node 13 routed to Node 13A
082070 # (Approximated cross-section - see cross-section 258)
082080 [L = 0.04 for summer conditions and 0.025 for spring conditions]
082090 *****
082100 ROUTES:CH0031 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
082110 ROUTE CHANNEL ***** [CEN: 75.00; No: 3.00; Tp = 3.76] *****
082120 [RFD= 1.00] out = 1.0 01:SN,SA 4651.00 15.935 No,date 39:17 25.42 n/a 0.00
082130 [L/S= 1962. / 2231.045]
082140 [Vmax = 548; Dmax = 3.659]
082150 *****
082160 # Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
082170 *****
082180 ROUTES:CH0032 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
082190 ADD HYD ***** [CEN: 75.00; No: 3.00; Tp = 3.76] *****
082200 + 1.0 02:SN,SA 4651.00 15.935 No,date 39:17 25.42 n/a 0.00
082210 + 1.0 01:JR,GM 3074.00 7.521 No,date 39:59 20.65 n/a 0.00
082220 SIMM 1.0 01:SN,SA 7725.00 23.402 No,date 39:59 23.42 n/a 0.00
082230 # Insertion of a reservoir to simulate the effects of the Goodwood Marsh
082240 *****
082250 ROUTES:CH0033 ***** Dfwn-1D NDD ***** AREHA-QPEAKm-TpeakDte,hb,mm-Rvmm R.C.-Dfwm
082260 ROUTE RESERVOIR -> 1.0 02:SN,SA 7725.00 23.402 No,date 39:59 23.42 n/a 0.00
082270 out = 1.0 01:RES,GM 7725.00 3.678 No,date 60:27 23.52 n/a 0.00
082280 *****

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08603# R0025: C00866..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08604# ROUTE RESERVOIR > 1.0 02: ST-2 3.51 091 Ndate 28:27 45.54 n/a 000
08605# overflow < 1.0 03: AT-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08606# [I/SP= 4.00; I/AREC= 4.00]
08607# [MSI.stokd=6660E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08608# R0025: C00867..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08609# CONTI NKS STANDHD 1.0 01: ST-4 5.9 091 Ndate 29:04 55.4 612 000
[X/Mp= 46; TlMp= 57]
08610# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08611# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08612# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 109; MNI= 013; SCI= 0]
08613# [I/AREC= 4.00; I/AREP= 4.00]
08614#
08615# R0025: C00868..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08616# ROUTE RESERVOIR > 1.0 02: ST-2 3.51 091 Ndate 28:27 45.54 n/a 000
08617# overflow < 1.0 03: AT-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08618# [I/SP= 4.00; I/AREC= 4.00]
08619# [MSI.stokd=2899E-02 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08620# R0025: C00869..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08621# CONTI NKS STANDHD 1.0 01: ST-4 5.9 091 Ndate 29:04 55.4 612 000
08622# [Cnw 69.0; N= 3.00; Tpw= 11.8]
08623# [InterEventTime= 12.00]
08624# [I/AREC= 4.00; I/AREP= 4.00]
08625#
08626# ROUTE PIPE > 1.0 02: O-8 60.55 1.147 Ndate 29:04 28.63 n/a 000
08627# [RFD= 1.00] out < 1.0 01: BR/PIPE 60.55 1.141 Ndate 29:08 28.63 n/a 000
08628# [L/S= 527 / 2300 / 043]
08629# [I/SP= 1.78; Dmax= 540]
08630#
08631# R0025: C0091..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08632# ADD HD + 1.0 02: BR/NI 28.31 4.0 Ndate 28:12 37.38 n/a 000
08633# + 1.0 02: BR/NI 28.31 4.0 Ndate 28:12 37.38 n/a 000
08634# + 1.0 02: AI-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08635# overflow < 1.0 03: AT-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08636# + 1.0 02: ST/STR 59 052 Ndate 28:07 45.54 n/a 000
08637# + 1.0 02: ST/STR 59 052 Ndate 28:07 45.54 n/a 000
08638# + 1.0 02: BR/PIPE 60.55 1.141 Ndate 29:08 28.63 n/a 000
08639# + 1.0 02: BR/PIPE 60.55 1.141 Ndate 29:08 28.63 n/a 000
08640# SIML + 1.0 01: ST/2-IN 326.12 5.017 Ndate 28:36 23.53 n/a 000
08641# [X/Mp= 68; TlMp= 85]
08642# CONTI NKS STANDHD 1.0 01: AT 3.51 615 Ndate 28:01 58.43 785 000
08643# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08644# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08645# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 265; MNI= 013; SCI= 0]
08646# [I/AREC= 4.00; I/AREP= 4.00]
08647#
08648# ROUTE RESERVOIR > 1.0 02: AT 3.51 615 Ndate 28:01 58.43 n/a 000
08649# overflow < 1.0 03: AT-OF 0 000 Ndate 0:00 00 n/a 000
08650# [I/SP= 4.00; I/AREC= 4.00]
08651# [MSI.stokd=9358E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08652# R0025: C0094..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08653# CONTI NKS STANDHD 1.0 01: ST-3 7.1 108 Ndate 28:00 45.54 612 000
08654# [X/Mp= 46; TlMp= 57]
08655# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08656# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08657# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 119; MNI= 013; SCI= 0]
08658# [I/AREC= 4.00; I/AREP= 4.00]
08659#
08660# R0025: C0095..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08661# ROUTE RESERVOIR > 1.0 02: ST-3 7.1 108 Ndate 28:00 45.54 n/a 000
08662# overflow < 1.0 03: ST/OF 0 000 Ndate 0:00 00 n/a 000
08663# [I/SP= 4.00; I/AREC= 4.00]
08664# [MSI.stokd=3355E-02 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08665# R0025: C0096..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08666# ADD HD + 1.0 02: ST/2-IN 326.12 5.017 Ndate 28:36 23.53 n/a 000
08667# + 1.0 02: ST/2-IN 326.12 5.017 Ndate 28:36 23.53 n/a 000
08668# + 1.0 02: AI-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08669# + 1.0 02: AI-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08670# + 1.0 02: ST/OF 0 000 Ndate 0:00 00 n/a 000
08671# + 1.0 02: ST/OF 0 000 Ndate 0:00 00 n/a 000
08672# SIML + 1.0 01: PT/ST3 330.34 5.204 Ndate 28:28 23.95 n/a 000
08673# ROUTE CHANNEL > 1.0 02: PT/ST3 330.34 5.204 Ndate 28:28 23.95 n/a 000
08674# [RFD= 1.00] out < 1.0 01: BR/NI 330.34 4.806 Ndate 29:00 23.95 n/a 000
08675# [L/S= 592 / 2300 / 043]
08676# [I/SP= 541; Dmax= 1081]
08677#
08678# R0025: C0098..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08679# CONTI NKS STANDHD 1.0 01: DE 2.28 062 Ndate 29:03 37.38 502 000
08680# [Cnw 46.0; N= 3.00; Tpw= 11.8]
08681# [I/AREC= 4.00; I/AREP= 4.00]
08682#
08683# R0025: C0099..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08684# CONTI NKS STANDHD 1.0 01: AT 12.04 1.933 Ndate 28:03 58.43 785 000
08685# [X/Mp= 68; TlMp= 85]
08686# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08687# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08688# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 491; MNI= 013; SCI= 0]
08689# [I/AREC= 4.00; I/AREP= 4.00]
08690#
08691# R0025: C0100..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08692# ROUTE RESERVOIR > 1.0 02: AT 12.04 1.933 Ndate 28:03 58.43 n/a 000
08693# overflow < 1.0 03: AI-STR 12.04 1.933 Ndate 28:03 58.43 n/a 000
08694# [I/SP= 4.00; I/AREC= 4.00]
08695# [MSI.stokd=1551E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08696# R0025: C0101..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08697# CONTI NKS STANDHD 1.0 01: ST-4 5.9 055 Ndate 28:00 45.54 612 000
08698# [X/Mp= 46; TlMp= 57]
08699# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08700# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08701# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 84; MNI= 013; SCI= 0]
08702# [I/AREC= 4.00; I/AREP= 4.00]
08703#
08704# ROUTE RESERVOIR > 1.0 02: ST-4 5.9 055 Ndate 28:00 45.54 n/a 000
08705# overflow < 1.0 03: ST/OF 0 000 Ndate 0:00 00 n/a 000
08706# [I/SP= 4.00; I/AREC= 4.00]
08707# [MSI.stokd=2092E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08708# R0025: C0103..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08709# CONTI NKS STANDHD 1.0 01: AT 5.30 922 Ndate 28:02 58.43 785 000
08710# [X/Mp= 68; TlMp= 85]
08711# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08712# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08713# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 326; MNI= 013; SCI= 0]
08714# [I/AREC= 4.00; I/AREP= 4.00]
08715#
08716# ROUTE RESERVOIR > 1.0 02: ST-4 5.30 922 Ndate 28:02 58.43 n/a 000
08717# overflow < 1.0 03: AI-STR 5.30 922 Ndate 28:25 58.43 n/a 000
08718# [I/SP= 4.00; I/AREC= 4.00]
08719# [MSI.stokd=1488E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08720# R0025: C0105..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08721# ADD HD + 1.0 02: DE 2.28 062 Ndate 29:03 37.38 502 000
08722# + 1.0 02: DE 2.28 062 Ndate 29:03 37.38 n/a 000
08723# + 1.0 02: ST/STR 12.04 1.933 Ndate 28:03 58.43 n/a 000
08724# + 1.0 02: ST/STR 12.04 1.933 Ndate 28:03 58.43 n/a 000
08725# + 1.0 02: ST/OF 0 000 Ndate 0:00 00 n/a 000
08726# + 1.0 02: ST/OF 0 000 Ndate 0:00 00 n/a 000
08727# SIML + 1.0 01: PT/ST4 350.31 5.557 Ndate 28:56 25.77 n/a 000
08728# ROUTE CHANNEL > 1.0 02: PT/ST4 350.31 5.557 Ndate 28:56 25.77 n/a 000
08729# [RFD= 1.00] out < 1.0 01: BR/NI 350.31 5.293 Ndate 29:13 25.77 n/a 000
08730# [L/S= 528 / 2300 / 043]
08731# [I/SP= 603; Dmax= 1137]
08732#
08733# R0025: C0107..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08734# CONTI NKS STANDHD 1.0 01: AT 2.51 695 Ndate 28:43 40.67 547 000
08735# [Cnw 86.0; N= 3.00; Tpw= 911]
08736# [I/AREC= 4.00; I/AREP= 4.00]
08737#
08738# R0025: C0108..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08739# CONTI NKS STANDHD 1.0 01: AT 3.41 618 Ndate 28:01 58.43 785 000
08740# [X/Mp= 68; TlMp= 85]
08741# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08742# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08743# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 261; MNI= 013; SCI= 0]
08744# [I/AREC= 4.00; I/AREP= 4.00]
08745#
08746# ROUTE RESERVOIR > 1.0 02: CI 3.41 618 Ndate 28:01 58.43 n/a 000
08747# out < 1.0 01: CI-STR 3.41 159 Ndate 28:23 58.43 n/a 000
08748# overflow < 1.0 03: CI-OF 0 000 Ndate 0:00 00 n/a 000
08749# [MSI.stokd=9080E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08750# R0025: C0110..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08751# CONTI NKS STANDHD 1.0 01: ST-5 45 071 Ndate 28:00 45.54 612 000
08752# [X/Mp= 46; TlMp= 57]
08753# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08754# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08755# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 95; MNI= 013; SCI= 0]
08756# [I/AREC= 4.00; I/AREP= 4.00]
08757#
08758# ROUTE RESERVOIR > 1.0 02: ST-5 45 071 Ndate 28:00 45.54 n/a 000
08759# out < 1.0 01: ST/STR 45 071 Ndate 28:07 45.54 n/a 000
08760# overflow < 1.0 03: ST/OF 0 000 Ndate 0:00 00 n/a 000
08761# [MSI.stokd=2455E-02 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08762# R0025: C0112..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08763# ADD HD + 1.0 02: BR/NI 28.31 4.0 Ndate 28:12 37.38 n/a 000
08764# + 1.0 02: BR/NI 28.31 4.0 Ndate 28:12 37.38 n/a 000
08765# + 1.0 02: AI-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08766# + 1.0 02: AI-STR 2.50 117 Ndate 28:22 58.43 n/a 000
08767# + 1.0 02: ST/OF 0 000 Ndate 0:00 00 n/a 000
08768# + 1.0 02: ST/OF 0 000 Ndate 0:00 00 n/a 000
08769# SIML + 1.0 01: ST/2-IN 326.12 5.017 Ndate 28:36 23.53 n/a 000
08770# R0025: C0113..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08771# CONTI NKS STANDHD 1.0 01: STRAND 7.59 918 Ndate 28:10 57.02 767 000
08772# [X/Mp= 64; TlMp= 85]
08773# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08774# [Previous area: I/APER= 4.67; SLPD= 50; LG= 40; MNP= 250; SCP= 0]
08775# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 1230; MNI= 013; SCI= 0]
08776# [I/AREC= 4.00; I/AREP= 4.00]
08777#
08778# ROUTE RESERVOIR > 1.0 02: STRAND 7.59 918 Ndate 28:10 57.02 n/a 000
08779# overflow < 1.0 03: ST-OF 7.59 918 Ndate 28:10 57.02 n/a 000
08780# [MSI.stokd=2455E-02 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08781# R0025: C0115..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08782# ADD HD + 1.0 02: S-POND 356 5.40 Ndate 29:19 57.02 n/a 000
08783# + 1.0 02: S-POND 356 5.40 Ndate 29:19 57.02 n/a 000
08784# + 1.0 02: CI-OF 0 000 Ndate 0:00 00 n/a 000
08785# + 1.0 02: CI-OF 0 000 Ndate 0:00 00 n/a 000
08786# SIML + 1.0 01: S-NK 364.27 5.589 Ndate 29:11 26.85 n/a 000
08787# R0025: C0116..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08788# SAVE IND + 1.0 01: SNAKUT 364.27 5.589 Ndate 29:11 26.85 n/a 000
08789# [Name: SNAKUT 0025]
08790# [remrk: SNAKUT Outflow]
08791#
08792# R0025: C0117..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08793# CONTI NKS STANDHD 1.0 01: Area-A 66.75 7.878 Ndate 28:09 56.06 754 000
08794# [X/Mp= 64; TlMp= 80]
08795# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08796# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08797# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 155; MNI= 013; SCI= 0]
08798# [I/AREC= 4.00; I/AREP= 4.00]
08799#
08800# ROUTE RESERVOIR > 1.0 02: Area-A 66.75 7.878 Ndate 28:09 56.06 n/a 000
08801# overflow < 1.0 03: SNAKUT 66.75 7.878 Ndate 28:09 56.06 n/a 000
08802# [MSI.stokd=2641E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08803# R0025: C00118..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08804# SAVE IND + 1.0 01: SNAKUT 66.75 7.878 Ndate 28:09 56.06 n/a 000
08805# [Name: SNAKUT 0025]
08806# [remrk: SNAKUT Outflow]
08807#
08808# R0025: C00119..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08809# ROUTE RESERVOIR > 1.0 02: Area-A 66.75 7.878 Ndate 28:09 56.06 n/a 000
08810# overflow < 1.0 03: SNAKUT 66.75 7.878 Ndate 28:09 56.06 n/a 000
08811# [MSI.stokd=2641E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08812# R0025: C00120..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08813# SAVE IND + 1.0 01: SNAKUT 66.75 7.878 Ndate 28:09 56.06 n/a 000
08814# [Name: SNAKUT 0025]
08815# [remrk: SNAKUT Outflow]
08816#
08817# R0025: C00121..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08818# ADD HD + 1.0 02: SNAKUT 364.27 5.589 Ndate 29:11 26.85 n/a 000
08819# + 1.0 02: SNAKUT 364.27 5.589 Ndate 29:11 26.85 n/a 000
08820# + 1.0 02: SNAKUT 364.27 5.589 Ndate 29:11 26.85 n/a 000
08821# + 1.0 02: SNAKUT 364.27 5.589 Ndate 29:11 26.85 n/a 000
08822# SIML + 1.0 01: PT/ST5 431.02 6.356 Ndate 29:13 31.37 n/a 000
08823# R0025: C00122..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08824# CONTI NKS STANDHD 1.0 01: C6 1.87 354 Ndate 28:01 58.43 785 000
08825# [X/Mp= 68; TlMp= 85]
08826# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08827# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08828# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 193; MNI= 013; SCI= 0]
08829# [I/AREC= 4.00; I/AREP= 4.00]
08830#
08831# ROUTE RESERVOIR > 1.0 02: C7 1.87 354 Ndate 28:01 58.43 n/a 000
08832# overflow < 1.0 03: ST-6 0 000 Ndate 0:00 00 n/a 000
08833# [I/SP= 4.00; I/AREC= 4.00]
08834# [MSI.stokd=501E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08835# R0025: C00123..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08836# CONTI NKS STANDHD 1.0 01: C7 1.62 309 Ndate 28:01 58.43 785 000
08837# [X/Mp= 68; TlMp= 85]
08838# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08839# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08840# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 191; MNI= 013; SCI= 0]
08841# [I/AREC= 4.00; I/AREP= 4.00]
08842#
08843# ROUTE RESERVOIR > 1.0 02: C7 1.62 309 Ndate 28:01 58.43 n/a 000
08844# overflow < 1.0 03: ST-6 0 000 Ndate 0:00 00 n/a 000
08845# [I/SP= 4.00; I/AREC= 4.00]
08846# [MSI.stokd=433E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08847# R0025: C00124..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08848# CONTI NKS STANDHD 1.0 01: ST-6 4.1 065 Ndate 28:00 45.54 612 000
08849# [X/Mp= 46; TlMp= 57]
08850# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08851# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08852# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 91; MNI= 013; SCI= 0]
08853# [I/AREC= 4.00; I/AREP= 4.00]
08854#
08855# ROUTE RESERVOIR > 1.0 02: C7 1.62 309 Ndate 28:01 58.43 n/a 000
08856# overflow < 1.0 03: ST-6 0 000 Ndate 0:00 00 n/a 000
08857# [I/SP= 4.00; I/AREC= 4.00]
08858# [MSI.stokd=433E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08859# R0025: C00125..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08860# CONTI NKS STANDHD 1.0 01: ST-6 4.1 065 Ndate 28:00 45.54 612 000
08861# [X/Mp= 46; TlMp= 57]
08862# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08863# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08864# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 91; MNI= 013; SCI= 0]
08865# [I/AREC= 4.00; I/AREP= 4.00]
08866#
08867# ROUTE RESERVOIR > 1.0 02: C7 1.62 309 Ndate 28:01 58.43 n/a 000
08868# overflow < 1.0 03: ST-6 0 000 Ndate 0:00 00 n/a 000
08869# [I/SP= 4.00; I/AREC= 4.00]
08870# [MSI.stokd=433E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08871# R0025: C00126..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08872# CONTI NKS STANDHD 1.0 01: ST-6 4.1 065 Ndate 28:00 45.54 612 000
08873# [X/Mp= 46; TlMp= 57]
08874# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08875# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08876# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 91; MNI= 013; SCI= 0]
08877# [I/AREC= 4.00; I/AREP= 4.00]
08878#
08879# ROUTE RESERVOIR > 1.0 02: C7 1.62 309 Ndate 28:01 58.43 n/a 000
08880# overflow < 1.0 03: ST-6 0 000 Ndate 0:00 00 n/a 000
08881# [I/SP= 4.00; I/AREC= 4.00]
08882# [MSI.stokd=433E-01 mb, TotOfVol=0.0000E+00 mb, N.Of=0, TotDurOf=0 hrs.]
08883# R0025: C00127..... Dfna=1 DfN NDD..... AREHA-QPEAKm-TPeakDte,h:mm..... Rvmm R C..... DfWcm
08884# CONTI NKS STANDHD 1.0 01: ST-6 4.1 065 Ndate 28:00 45.54 612 000
08885# [X/Mp= 46; TlMp= 57]
08886# [Horton parameters: Fw= 76.20; Fc= 13.20; ICAVd= 14; Fw= 0]
08887# [Previous area: I/APER= 4.67; SLPD= 50; LG= 50; MNP= 250; SCP= 0]
08888# [Impervious area: I/AMP= 1.57; SLPD= 50; LG= 91; MNI= 013; SCI= 0]
08889# [I/AREC= 4.00; I/AREP= 4.00]
08890#
08891# ROUTE RESERVOIR > 1.0 02: C7 1.62 309 Ndate 28:01 58.43 n/a 000
08892# overflow < 1.0 03: ST-6 0 000 Ndate 0:0
```

089775	*****	091645	CONTINUOUS STANDI	1.0 01: S-1-D	2.03	.369	Ndate	28.00	57.38	771	000
089778	#	091655	[M Pm: 65; TlM: 65]								
089780	#	091665	[LRS= 2; Cn: 75.0]								
089800	#	091675	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
089810	#	091685	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
089820	#	091695	[IRFCM ppm: 4.00; IAREPER= 4.00]								
089830	#	091705	[SM N= 31.15; SMMW=225.45; SKE= 010]								
089840	#	091715	Catchment 4.00								
089850	#	091725	[M Pm: 65; TlM: 65]								
089860	#	091735	[LRS= 2; Cn: 75.0]								
089870	#	091745	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
089880	#	091755	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
089890	#	091765	[IRFCM ppm: 4.00; IAREPER= 4.00]								
089900	#	091775	[SM N= 31.15; SMMW=225.45; SKE= 010]								
089910	#	091785	Catchment 4.00								
089920	#	091795	[M Pm: 65; TlM: 65]								
089930	#	091805	[LRS= 2; Cn: 75.0]								
089940	#	091815	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
089950	#	091825	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
089960	#	091835	[IRFCM ppm: 4.00; IAREPER= 4.00]								
089970	#	091845	[SM N= 31.15; SMMW=225.45; SKE= 010]								
089980	#	091855	5 Year + 12% Curve								
089990	#	091865	[M Pm: 65; TlM: 65]								
090000	#	091875	[LRS= 2; Cn: 75.0]								
090010	#	091885	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090020	#	091895	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090030	#	091905	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090040	#	091915	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090050	#	091925	Catchment 4.00								
090060	#	091935	[M Pm: 65; TlM: 65]								
090070	#	091945	[LRS= 2; Cn: 75.0]								
090080	#	091955	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090090	#	091965	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090100	#	091975	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090110	#	091985	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090120	#	091995	Catchment 4.00								
090130	#	092005	[M Pm: 65; TlM: 65]								
090140	#	092015	[LRS= 2; Cn: 75.0]								
090150	#	092025	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090160	#	092035	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090170	#	092045	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090180	#	092055	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090190	#	092065	Catchment 4.00								
090200	#	092075	[M Pm: 65; TlM: 65]								
090210	#	092085	[LRS= 2; Cn: 75.0]								
090220	#	092095	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090230	#	092105	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090240	#	092115	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090250	#	092125	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090260	#	092135	Catchment 4.00								
090270	#	092145	[M Pm: 65; TlM: 65]								
090280	#	092155	[LRS= 2; Cn: 75.0]								
090290	#	092165	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090300	#	092175	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090310	#	092185	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090320	#	092195	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090330	#	092205	Catchment 4.00								
090340	#	092215	[M Pm: 65; TlM: 65]								
090350	#	092225	[LRS= 2; Cn: 75.0]								
090360	#	092235	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090370	#	092245	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090380	#	092255	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090390	#	092265	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090400	#	092275	Catchment 4.00								
090410	#	092285	[M Pm: 65; TlM: 65]								
090420	#	092295	[LRS= 2; Cn: 75.0]								
090430	#	092305	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090440	#	092315	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090450	#	092325	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090460	#	092335	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090470	#	092345	Catchment 4.00								
090480	#	092355	[M Pm: 65; TlM: 65]								
090490	#	092365	[LRS= 2; Cn: 75.0]								
090500	#	092375	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090510	#	092385	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090520	#	092395	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090530	#	092405	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090540	#	092415	Catchment 4.00								
090550	#	092425	[M Pm: 65; TlM: 65]								
090560	#	092435	[LRS= 2; Cn: 75.0]								
090570	#	092445	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090580	#	092455	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090590	#	092465	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090600	#	092475	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090610	#	092485	Catchment 4.00								
090620	#	092495	[M Pm: 65; TlM: 65]								
090630	#	092505	[LRS= 2; Cn: 75.0]								
090640	#	092515	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090650	#	092525	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090660	#	092535	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090670	#	092545	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090680	#	092555	Catchment 4.00								
090690	#	092565	[M Pm: 65; TlM: 65]								
090700	#	092575	[LRS= 2; Cn: 75.0]								
090710	#	092585	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090720	#	092595	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090730	#	092605	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090740	#	092615	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090750	#	092625	Catchment 4.00								
090760	#	092635	[M Pm: 65; TlM: 65]								
090770	#	092645	[LRS= 2; Cn: 75.0]								
090780	#	092655	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090790	#	092665	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090800	#	092675	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090810	#	092685	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090820	#	092695	Catchment 4.00								
090830	#	092705	[M Pm: 65; TlM: 65]								
090840	#	092715	[LRS= 2; Cn: 75.0]								
090850	#	092725	[Pervious area: 1Aper: 4.67; SLLP=2.00; LCP= 40.0; MNP: 250; SCP= 0]								
090860	#	092735	[Imperious area: 1Apm: 1.57; SLLP= 75.0; LGA= 116.0; MNI= 013; SCI= 0]								
090870	#	092745	[IRFCM ppm: 4.00; IAREPER= 4.00]								
090880	#	092755	[SM N= 31.15; SMMW=225.45; SKE= 010]								
090890	#	092765	Catchment 4.00								
090900	#	092775									


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097255 # [SMN No. 26.32: SMAV=175.50, SK= 010]
097260 R0025:CU0266 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097270 COMPUTE DUAL/DID 1.0 01:FRASER-D 21.61 3,258 N.Date 28:01 56.29 n/a 0.00
097280 Mjor System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00
097290 Mnor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00
097300 [M SysSt= 4729E03, TotOfVol= 0.000E+00, N.Of= 0, TotDurOf= 0 hrs.]
097310 ADD IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097320 + 1.0 02:FRASER-J 0.00 0.00 N.Date 0.00 n/a 0.00
097330 + 1.0 02:FRASER-D 21.61 3,258 N.Date 28:01 56.43 n/a 0.00
097340 SIMM 1.0 01:FRASER-S 21.61 2,281 N.Date 27:55 56.43 n/a 0.00
097350 [L/S= 204.1109, 0.5]
097360 ADD IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097370 + 1.0 02:FRASER-S 25.04 17,344 N.Date 28:23 34.14 n/a 0.00
097380 + 1.0 02:FRASER-D 25.04 17,344 N.Date 28:23 34.14 n/a 0.00
097390 + 1.0 02:FRASER-S 21.61 2,281 N.Date 27:55 56.43 n/a 0.00
097400 + 1.0 02:FRASER-D 21.61 2,281 N.Date 27:55 56.43 n/a 0.00
097410 + 1.0 02:FRASER-DN 54279 103,735 N.Date 28:12 29.13 n/a 0.00
097420 + 1.0 02:FC-01-S 8.03 756 N.Date 27:53 44.53 n/a 0.00
097430 + 1.0 02:FC-02-S 16.05 1,519 N.Date 27:44 69.13 n/a 0.00
097440 + 1.0 02:FC-03-S 7.37 358 N.Date 27:43 53.66 n/a 0.00
097450 SIMM 1.0 01:2441 54658.48 103,701 N.Date 38:36 29.12 n/a 0.00
097460 R0025:CU0269 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097470 SAVE IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097480 # name : 241.0025
097490 remark: Total Flow at Ken Burnett Outlet
097500 # Hydrograph from Node Ken Burnett to station 3633
097510 # Channel X Section obtained from ROCA Hydraulic Model - Station 4241
097520 R0025:CU0270 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097530 ROUTE CHANNEL >> 1.0 02:2442 54658.48 103,701 N.Date 38:36 29.12 n/a 0.00
097540 # [RMS 1.00] out < 1.0 01:2441-out 54658.48 103,716 N.Date 38:34 29.12 n/a 0.00
097550 [L/S= 294.1109, 0.5]
097560 [SMN No. 1.254: Dmx= 2.210]
097570 R0025:CU0271 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097580 ADD IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097590 + 1.0 02:2441-out 54658.48 103,716 N.Date 38:34 29.12 n/a 0.00
097600 + 1.0 02:FR-01-S 8.24 565 N.Date 27:45 53.74 n/a 0.00
097610 + 1.0 02:FR-02-S 1.59 113 N.Date 27:47 53.69 n/a 0.00
097620 SIMM 1.0 01:SN,KB 54681.18 103,731 N.Date 38:34 29.13 n/a 0.00
097630 R0025:CU0272 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097640 SAVE IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097650 # name : SN,KB 0025
097660 remark: IHD ELEMENTS (Time Before Station 3633)
097670 # Hydrograph from Station 3633 to Node Todd
097680 # Channel X Section obtained from ROCA Hydraulic Model - Station 3633
097690 # JFSA 2021-02-26 change channel length (at station 3633) from 650m to 609m and change the slope from 0.0498% to 0.2
097700 #
097710 R0025:CU0273 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097720 ROUTE CHANNEL >> 1.0 02:SN,KB 54681.18 103,735 N.Date 38:34 29.13 n/a 0.00
097730 # [RMS 1.00] out < 1.0 01:SN,KB 54681.18 103,735 N.Date 38:38 29.13 n/a 0.00
097740 [L/S= 294.1109, 0.5]
097750 [Vmax = 6.75: Dmx= 1.657]
097760 #
097770 # Catchment Greenbank
097780 # To Greenbank Drain (south of the Jock)
097790 # - JFSA 2021-01-18 add Greenbank pond as per JFSA P998.06-15, June 2016
097800 # JFSA 2021-01-19 update area from 37.49 ha to 36.6 ha based on GIS measurements
097810 # [RFS 1.00] out < 1.0 02:SN,KB 54681.18 103,735 N.Date 38:34 29.13 n/a 0.00
097820 R0025:CU0274 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097830 SAVE IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097840 [L/S= 4.6: 71.MP= 68]
097850 [L/S= 2 : CN= 77.0]
097860 [Impervious area: Iper= 4.67: SLLP= 0.0: LG= 4.0: MNP= 250: SCP= 0]
097870 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
097880 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
097890 [SMN No. 31.15: SMAV=207.66: SK= 010]
097900 R0025:CU0275 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097910 ROUTE RESERVOIR >> 1.0 02:Greenbank 36.60 5,574 N.Date 28:02 58.33 n/a 0.00
097920 out < 1.0 01:Greenbank 36.60 3,118 N.Date 28:11 58.33 n/a 0.00
097930 [Impervious area: Iper= 0.02: AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
097940 MMS offset= 8739E+03 nb, TotOfVol= 0.000E+00, N.Of= 0, TotDurOf= 0 hrs.]
097950 R0025:CU0276 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097960 ADD IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
097970 + 1.0 02:2: SN,KB 54681.18 103,735 N.Date 38:38 29.13 n/a 0.00
097980 + 1.0 02:Greenbank 36.60 3,118 N.Date 28:11 58.33 n/a 0.00
097990 SIMM 1.0 01:Green 54717.78 103,842 N.Date 38:38 29.15 n/a 0.00
098000 R0025:CU0277 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098010 SAVE IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098020 # name : Green 0025
098030 remark: Total Flow at Greenbank Drain
098040 #
098050 # Catchment TODD
098060 # - To Todd Drain (south of the Jock)
098070 # Subdivided into 10 separate basins based on Burhaven South M/S
098080 # - 2020-11-30 increase imp. packed on P998.04-11
098090 # - 2020-11-30 update Tributary Drainage Area to = 146.015 ha based on P998.04-11
098100 # - 2020-11-30 split TODD Drainage Area to MWLR, MNOR, PADL and ALL
098110 #
098120 # JFSA 2021-01-19 add TODD Area as part of Clark (N.W.C.L.A.R.M.) and remove it from Todd
098130 R0025:CU0278 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098140 CONTINUES STANDBY 1.0 01:TODD.MS 2.10 339 N.Date 28:00 53.59 720 0.00
098150 [X.MP= 53: 71.MP= 57]
098160 [L/S= 2 : CN= 77.0]
098170 [Impervious area: Iper= 4.67: SLLP= 0.0: LG= 4.0: MNP= 250: SCP= 0]
098180 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
098190 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
098200 [SMN No. 31.15: SMAV=207.66: SK= 010]
098210 R0025:CU0279 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098220 CONTINUES STANDBY 1.0 01:TODD.MS 12.02 620 N.Date 28:00 53.57 720 0.00
098230 [X.MP= 53: 71.MP= 57]
098240 [L/S= 2 : CN= 77.0]
098250 [Impervious area: Iper= 4.67: SLLP= 0.0: LG= 4.0: MNP= 250: SCP= 0]
098260 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
098270 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
098280 [IAREC mp= 4.00: IAREC= 4.00]
098290 [SMN No. 31.15: SMAV=207.66: SK= 010]
098300 R0025:CU0280 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098310 CONTINUES STANDBY 1.0 01:TODD.MS 30.23 4,269 N.Date 28:02 55.13 741 0.00
098320 [X.MP= 52: 71.MP= 44]
098330 [L/S= 2 : CN= 77.0]
098340 [Impervious area: Iper= 4.67: SLLP= 0.0: LG= 4.0: MNP= 250: SCP= 0]
098350 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
098360 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
098370 [SMN No. 31.15: SMAV=207.66: SK= 010]
098380 R0025:CU0281 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098390 CONTINUES STANDBY 1.0 01:TODD.ALL 112.91 13,312 N.Date 28:05 53.38 714 0.00
098400 [L/S= 2 : CN= 77.0]
098410 [Impervious area: Iper= 4.67: SLLP= 0.0: LG= 4.0: MNP= 250: SCP= 0]
098420 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
098430 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
098440 [SMN No. 31.15: SMAV=207.66: SK= 010]
098450 R0025:CU0282 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098460 CONTINUES STANDBY 1.0 01:TODD.P 3.06 535 N.Date 28:00 57.01 766 0.00
098470 [X.MP= 52: 71.MP= 63]
098480 [L/S= 2 : CN= 77.0]
098490 [Impervious area: Iper= 4.67: SLLP= 0.0: LG= 4.0: MNP= 250: SCP= 0]
098500 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
098510 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
098520 [SMN No. 31.15: SMAV=207.66: SK= 010]
098530 # 5 Year + 12% Capture
098540 R0025:CU0283 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098550 ROUTE RESERVOIR >> 1.0 02:TODD.M 30.23 4,269 N.Date 28:02 55.13 n/a 0.00
098560 out < 1.0 02:TODD.M 29.48 3,314 N.Date 27:57 55.13 n/a 0.00
098570 [Impervious area: Iper= 0.02: TODD.M 2.06 268 N.Date 27:54 53.60 n/a 0.00]
098580 MMS offset= 1175E+03 nb, TotOfVol= 4.415E-01 nb, N.Of= 1, TotDurOf= 0 hrs.]
098590 # 5 Year + 12% Capture
098600 R0025:CU0284 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098610 ROUTE RESERVOIR >> 1.0 02:TODD.M 30.23 4,269 N.Date 28:02 55.13 n/a 0.00
098620 out < 1.0 02:TODD.M 29.48 3,314 N.Date 27:57 55.13 n/a 0.00
098630 MMS offset= 845E+04 nb, TotOfVol= 2.217E-02 nb, N.Of= 1, TotDurOf= 0 hrs.]
098640 R0025:CU0285 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098650 ROUTE RESERVOIR >> 1.0 02:TODD.M 30.23 4,269 N.Date 28:02 55.13 n/a 0.00
098660 out < 1.0 02:TODD.M 29.48 3,314 N.Date 27:57 55.13 n/a 0.00
098670 MMS offset= 845E+04 nb, TotOfVol= 2.217E-02 nb, N.Of= 1, TotDurOf= 0 hrs.]
098680 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
098690 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
098700 R0025:CU0286 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098710 CONTINUES STANDBY 1.0 01:AC 25.50 2,974 N.Date 28:03 49.44 665 0.00
098720 [X.MP= 42: 71.MP= 52]
098730 [L/S= 2 : CN= 75.0]
098740 [Impervious area: Iper= 4.67: SLLP= 0.0: LG= 4.0: MNP= 250: SCP= 0]
098750 [Major System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00]
098760 [Minor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:55 56.43 n/a 0.00]
098770 [SMN No. 33.81: SMAV=225.43: SK= 010]
098780 R0025:CU0287 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098790 COMPUTE DUAL/DID 1.0 01:AC 25.50 2,974 N.Date 28:03 49.44 n/a 0.00
098800 Mjor System / 1.0 02:AC-M 0.00 0.00 N.Date 0.00 n/a 0.00
098810 Mnor System \ 1.0 03:FRASER-N 21.61 2,281 N.Date 27:54 49.64 n/a 0.00
098820 [M SysSt= 8120E+03, TotOfVol= 0.000E+00, N.Of= 0, TotDurOf= 0 hrs.]
098830 R0025:CU0288 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098840 ADD IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098850 + 1.0 02:TODD.M 2.06 268 N.Date 27:54 53.60 n/a 0.00
098860 + 1.0 02:TODD.M 2.06 268 N.Date 27:54 53.60 n/a 0.00
098870 + 1.0 02:TODD.M 3.06 535 N.Date 28:00 57.01 n/a 0.00
098880 + 1.0 02:TODD.M 12.02 620 N.Date 28:04 53.38 n/a 0.00
098890 SIMM 1.0 01:2:CLAR.M 1.73 213 N.Date 27:54 52.78 n/a 0.00
098900 [L/S= 14: 23.MP= 10]
098910 R0025:CU0289 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098920 SAVE IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
098930 # name : TODD 0025
098940 remark: Total Flow at Todd Drain
098950 #
098960 # Todd Pond 3
098970 # Rainfall data obtained from Burhaven South M/S modeling
098980 # - static 2007, Tributary Drainage Area to MS Pond 3 = 193 ha
098990 #
099000 R0025:CU0290 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
099010 ROUTE RESERVOIR >> 1.0 02:TODD 120.62 14,962 N.Date 28:03 53.48 n/a 0.00
099020 out < 1.0 02:AC.M 120.62 9,717 N.Date 28:14 53.48 n/a 0.00
099030 over flow < 1.0 03:FC.PF 0.00 0.00 N.Date 0.00 n/a 0.00
099040 MMS offset= 2619E+06 nb, TotOfVol= 0.000E+00, N.Of= 0, TotDurOf= 0 hrs.]
099050 R0025:CU0291 > Dfmn ID NDD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
099060 ADD IHD .....AREHA:QPEAKm:PeakDate:hh:mm.....Rvmm R.C.....Dfwm
099070 + 1.0 02:Green 54717.78 103,842 N.Date 38:38 29.15 n/a 0.00
099080 + 1.0 02:FC-PF 0.00 0.00 N.Date 0.00 n/a 0.00
099090 + 1.0 02:TODD.M 120.62 9,717 N.Date 28:14 53.48 n/a 0.00
099100 + 1.0 02:AC.M 0.00 0.00 N.Date 0.00 n/a 0.00
099110 SIMM 1.0 01:SN,TD 54838.44 104,100 N.Date 38:38 29.21 n/a 0.00

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108475 #
108480 # Addition of Subwatershed Lock River at Goodwood Marsh to Node 13A
108489 #
108500 RO505: C00032.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
108511 ADD HYD + 1.0 02: N3A 4651.00 19.136 Ndate 39:06 29.90 n/a 000
108522 ROUTE CHANNEL -> 1.0 02: IR_CDM 3074.00 8.912 Ndate 39:59 24.31 n/a 000
108533 SIMM 1.0 01: SNI3A 7725.00 27.939 Ndate 39:54 27.68 n/a 000
108544 #
108555 # Insertion of a reservoir to simulate the effects of the Goodwood Marsh
108560 #
108570 RO505: C00033.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
108588 ROUTE RESERVOIR -> 1.0 02: SNI3A 7725.00 27.939 Ndate 39:54 27.68 n/a 000
108600 (Msk Outflow: 1481E04)
108610 #
108620 RO505: C00034.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
108638 SAVE HYD 1.0 01: RES_GM 7725.00 3.808 Ndate 61:35 27.67 n/a 000
108645 #
108655 # remark:flow at Res GM
108660 #
108670 # Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
108675 # (Approximated cross-section - see cross-section 258)
108685 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
108690 RO505: C00035.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
108700 ROUTE CHANNEL -> 1.0 02: RES_GM 7725.00 3.808 Ndate 61:35 27.67 n/a 000
108710 (RFD= 1.00) out c 1.0 01: N12 7725.00 3.804 Ndate 64:19 27.68 n/a 000
108720 (L/S n= 522. / 076. 040)
108730 (Vmax = .556; Dmax= 1.541)
108740 #
108755 # Addition of Subwatershed Lock River at Ashton to Node 12
108760 #
108770 RO505: C00036.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
108788 ADD HYD + 1.0 02: N12 7725.00 3.804 Ndate 64:19 27.68 n/a 000
108798 + 1.0 02: IR_ASH 1781.00 16.834 Ndate 32:39 36.85 n/a 000
108800 SIMM 1.0 01: S_N12 9506.00 18.867 Ndate 32:42 29.39 n/a 000
108810 (RFD= 1.00) out c 1.0 01: N12 9506.00 18.867 Ndate 32:42 29.39 n/a 000
108820 ROUTE CHANNEL -> 1.0 02: S_N12 9506.00 18.867 Ndate 32:42 29.39 n/a 000
108830 SAVE HYD 1.0 01: S_N12 9506.00 18.867 Ndate 32:42 29.39 n/a 000
108840 #
108850 # remark:flow at S_N12 near Ashton
108860 #
108870 # Sum of hydrographs from Node 12 routed to Node 11
108875 # (Approximated cross-section - see cross-section 258)
108885 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
108890 #
108900 # Sum of hydrographs from Node 12 routed to Node 11 with Dump Section 248
108910 #
108920 RO505: C00038.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
108938 ROUTE CHANNEL -> 1.0 02: S_N12 9506.00 18.867 Ndate 32:42 29.39 n/a 000
108948 (RFD= 1.00) out c 1.0 01: Dum1 9506.00 18.867 Ndate 32:59 29.39 n/a 000
108958 (L/S n= 972. / 054. 040)
108968 (Vmax = 751; Dmax= 3.029)
108978 #
108985 # Addition of Subwatershed 11 and Noe Creek to Node 11
108990 #
109000 RO505: C00039.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109018 ADD HYD + 1.0 02: Dum1 9506.00 18.867 Ndate 32:59 29.39 n/a 000
109028 + 1.0 02: SW11 500.00 9.061 Ndate 29:21 31.73 n/a 000
109038 ROUTE CHANNEL -> 1.0 02: S_N11 1917.00 12.342 Ndate 34:26 31.73 n/a 000
109048 SIMM 1.0 01: S_N11 11923.00 32.851 Ndate 33:00 29.87 n/a 000
109058 #
109065 # Sum of hydrographs from Node 11 routed to Node 10
109070 # Section 1
109080 #
109090 RO505: C00040.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109108 ROUTE CHANNEL -> 1.0 01: N10 11923.00 20.490 Ndate 40:02 29.87 n/a 000
109118 (RFD= 1.00) out c 1.0 01: N10 11923.00 20.490 Ndate 40:02 29.87 n/a 000
109128 (L/S n= 522. / 157. 040)
109138 (Vmax = 474; Dmax= 1.423)
109148 #
109155 # Addition of Subwatershed 10 to Node 10
109160 #
109170 RO505: C00041.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109188 ADD HYD + 1.0 02: N10 11923.00 20.490 Ndate 40:02 29.87 n/a 000
109198 + 1.0 02: SW10 5666.00 22.402 Ndate 37:52 36.85 n/a 000
109200 SIMM 1.0 01: S_N10 17589.00 52.600 Ndate 38:19 32.12 n/a 000
109210 (RFD= 1.00) out c 1.0 01: S_N10 17589.00 52.600 Ndate 38:19 32.12 n/a 000
109220 ROUTE CHANNEL -> 1.0 02: S_N10 17589.00 52.600 Ndate 38:19 32.12 n/a 000
109230 SAVE HYD 1.0 01: S_N10 17589.00 52.600 Ndate 38:19 32.12 n/a 000
109240 #
109250 # remark:flow at S_N10: N10 + SW10
109260 #
109270 RO505: C00043.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109288 ADD HYD + 1.0 02: S_N10 17589.00 52.600 Ndate 38:19 32.12 n/a 000
109298 + 1.0 02: RG_CK 8376.00 31.024 Ndate 39:59 31.73 n/a 000
109308 SIMM 1.0 01: S_N10A 25965.00 82.764 Ndate 39:45 31.99 n/a 000
109318 #
109325 # Sum of hydrographs from Node 10 routed to Node 9
109330 # Section 2
109340 #
109350 RO505: C00044.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109368 ROUTE CHANNEL -> 1.0 01: N9 31561.00 99.424 Ndate 39:59 31.68 n/a 000
109378 (RFD= 1.00) out c 1.0 01: N9 31561.00 99.424 Ndate 39:59 31.68 n/a 000
109388 (L/S n= 3082. / 074. 050)
109398 (Vmax = 744; Dmax= 2.015)
109408 #
109415 # Addition of Subwatershed 9 and Nchols Creek to Node 9
109420 #
109430 RO505: C00045.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109448 ADD HYD + 1.0 02: N9 25965.00 80.980 Ndate 39:59 31.99 n/a 000
109458 + 1.0 02: SW9 1132.00 14.039 Ndate 30:53 35.35 n/a 000
109468 + 1.0 02: RG_CK 4464.00 15.472 Ndate 39:59 28.95 n/a 000
109478 SIMM 1.0 01: S_N9 31561.00 99.424 Ndate 39:59 31.68 n/a 000
109488 #
109495 # Sum of hydrographs from Node 9 routed to Node 8
109500 # Section 3
109510 #
109520 RO505: C00046.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109538 ROUTE CHANNEL -> 1.0 01: N8 31561.00 99.424 Ndate 39:59 31.68 n/a 000
109548 (RFD= 1.00) out c 1.0 01: N8 31561.00 99.424 Ndate 39:59 31.68 n/a 000
109558 (L/S n= 522. / 083. 050)
109568 (Vmax = 367; Dmax= 1.834)
109578 #
109585 # Addition of Subwatershed 8 and Hobbs' Drain to Node 8
109590 #
109600 RO505: C00047.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109618 ADD HYD + 1.0 02: N8 31561.00 99.424 Ndate 39:59 31.68 n/a 000
109628 + 1.0 02: SW8 131.00 1.439 Ndate 30:53 35.35 n/a 000
109638 + 1.0 02: HB_DR 3554.00 18.180 Ndate 38:32 31.73 n/a 000
109648 SIMM 1.0 01: S_N8 31561.00 99.424 Ndate 39:59 31.68 n/a 000
109658 #
109665 # Sum of hydrographs from Node 8 routed to Node 7
109670 # Section 4
109680 #
109690 RO505: C00048.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109708 ROUTE CHANNEL -> 1.0 02: S_N7 35446.00 111.843 Ndate 39:59 31.68 n/a 000
109718 (RFD= 1.00) out c 1.0 01: N7 35446.00 95.475 Ndate 44:55 31.68 n/a 000
109728 (L/S n= 522. / 051. 050)
109738 (Vmax = 231; Dmax= 2.290)
109748 #
109755 # Addition of Subwatershed 7 to Node 7
109760 #
109770 RO505: C00049.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109788 ADD HYD + 1.0 02: N7 35446.00 95.475 Ndate 44:55 31.68 n/a 000
109798 + 1.0 02: SW7 3197.00 13.937 Ndate 36:23 25.65 n/a 000
109800 SIMM 1.0 01: S_N7 38743.00 102.892 Ndate 43:46 31.18 n/a 000
109810 (RFD= 1.00) out c 1.0 01: S_N7 38743.00 102.892 Ndate 43:46 31.18 n/a 000
109820 SAVE HYD 1.0 01: S_N7 38743.00 102.892 Ndate 43:46 31.18 n/a 000
109830 #
109840 # remark:flow at S_N7: N7 + SW7
109850 #
109860 # Insertion of a reservoir to simulate the effects of the Richmond Fen.
109865 # Storage area and volumes were estimated from available topo maps.
109870 # Release rate from fen was assumed to be controlled by the downstream
109875 # river cross-section for summer conditions. It is assumed that for up to
109880 # 0.75 m of water, the main channel of the river provided the storage. Above
109885 # this depth, the wetland starts to significantly store water.
109890 #
109900 RO505: C00051.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109918 ROUTE RESERVOIR -> 1.0 02: S_N7 38743.00 102.892 Ndate 43:46 31.18 n/a 000
109928 (Msk Outflow: 4394E04)
109938 #
109945 # remark:flow at S_N7: N7 + SW7
109950 #
109960 RO505: C00052.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
109978 SAVE HYD 1.0 01: RES_RF 38743.00 52.029 Ndate 59:07 31.18 n/a 000
109988 #
109995 # remark:outflow of Richmond Fen
110000 #
110010 # Sum of hydrographs from Node 7 routed to Node 6
110015 # Section 5
110020 #
110030 RO505: C00053.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
110048 ROUTE CHANNEL -> 1.0 02: S_N6 38743.00 52.029 Ndate 59:07 31.18 n/a 000
110058 (RFD= 1.00) out c 1.0 01: N6 38743.00 51.784 Ndate 60:27 31.18 n/a 000
110068 (L/S n= 3036. / 062. 050)
110078 (Vmax = 538; Dmax= 1.253)
110088 #
110095 # Addition of Subwatershed 6 and Van Gual Drain to Node 6
110100 #
110110 RO505: C00054.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
110128 ADD HYD + 1.0 02: N6 38743.00 51.784 Ndate 60:27 31.18 n/a 000
110138 + 1.0 02: SW6 185.00 1.285 Ndate 33:02 32.44 n/a 000
110148 + 1.0 02: VG_DR 1332.00 9.332 Ndate 35:12 36.85 n/a 000
110158 SIMM 1.0 01: S_N6 40240.01 51.810 Ndate 60:20 31.47 n/a 000
110168 #
110175 # Sum of hydrographs from Node 6 routed to Node 5
110180 # Section 6
110190 #
110200 RO505: C00055.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
110218 ROUTE CHANNEL -> 1.0 01: N5 40240.01 51.810 Ndate 60:20 31.47 n/a 000
110228 (RFD= 1.00) out c 1.0 01: N5 40240.01 51.810 Ndate 61:06 31.37 n/a 000
110238 (L/S n= 522. / 054. 050)
110248 (Vmax = 469; Dmax= 1.351)
110258 #
110265 # Addition of Subwatershed 5 and Flowing Creek to Node 5
110270 #
110280 RO505: C00056.....Dfai n-ID NND.....AREAH-QPEAKm-TpeakDate-hh:mm--Rvmm R.C---DFwcm
110298 ADD HYD + 1.0 02: N5 40240.01 51.810 Ndate 61:06 31.37 n/a 000
110308 + 1.0 02: FC_CK 4945.00 44.623 Ndate 33:18 38.37 n/a 000
110318 SIMM 1.0 01: S_N5 45499.01 71.514 Ndate 34:20 32.38 n/a 000

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Table with columns: ID, Name, Location, Dimensions, and various performance metrics (e.g., FWHM, RMS, etc.). Includes sub-sections like ROUTE RESERVER and CONTINUES STANDBY.

Table with columns for ID, description, and numerical values. Rows include various engineering notes and data points, such as '11595# R050:CO147...', '11596# SAVE HYD...', '11597# remark:SN:CK 0050...', and '11782# [Previous area: I:Area = 4.67:SLP2=0.0;LGP=...]'.

Table with columns for ID, description, and various numerical/boolean values. Includes rows for R0500-003555, R0500-003556, etc., up to R0500-003577.

Table with multiple columns containing technical specifications, part numbers, and status indicators. The table is organized into sections with headers like 'remark: Total Flow at MH04' and 'remark: Total Flow at MH06'. It lists various components such as 'CONT NUCLES STANHPD', 'COMPUTE DUALHPD', and 'MJOR System', along with their respective IDs and values.

```

157099 + 1.0 02:40:M .00 .000 No.date 0.00 .00 n/a .000
157109 + 1.0 02:41:M .09 218 No.date 28.05 59.73 n/a .000
157113 + 1.0 02:41:M .16 564 No.date 28.06 67.38 n/a .000
157123 + 1.0 02:41:M .23 599 No.date 28.05 62.88 n/a .000
157133 + 1.0 02:43:M .00 .000 No.date 0.00 .00 n/a .000
157143 + 1.0 01:corr 55020.08 145.505 No.date 36:37 39.46 n/a .000
157153 R0100:CO0386.....DfIn n-ID NDD.....AREBA-QPEAgm-TpeakDte,hh:mm.....RvM R.C.....Dfwm
157163 SAVE HYD 1.0 01:corr 55020.08 145.505 No.date 36:37 39.46 n/a .000
157173 *****
157183 *****
157193 *****
157203 *****
157213 *****
157223 *****
157233 *****
157243 *****
157253 *****
157263 *****
157273 *****
157283 R0100:CO0387.....DfIn n-ID NDD.....AREBA-QPEAgm-TpeakDte,hh:mm.....RvM R.C.....Dfwm
157293 ROUTE CHANNEL -> 1.0 02:SN,DE 55020.08 145.490 No.date 36:40 39.46 n/a .000
157303 [R/S n= 1.00] out c 1.0 01:SN,M 55020.08 145.490 No.date 36:40 39.46 n/a .000
157313 [L/S n= 580 / 4457.045] .....
157323 [Vmax = 2.170;Dmax = 2.124] .....
157333 *****
157343 *****
157353 *****
157363 *****
157373 *****
157383 R0100:CO0388.....DfIn n-ID NDD.....AREBA-QPEAgm-TpeakDte,hh:mm.....RvM R.C.....Dfwm
157393 CONTINUOUS STANBYD 1.0 01:MLLS 175.99 20.390 No.date 28:06 56.87 642 .000
157403 [XMP = 38.71M; 38] .....
157413 [L/S n= 2 CN= 74.0] .....
157423 [Previous area: IArea 4.67;SLPP4.00;LQ= 40.;MPP= 250;SCP= .0] .....
157433 [Impervious area: IArea 1.57;SLPP4.00;LQ= 400.;MNI = 013;SC= .0] .....
157443 [IARECmp = 4.00; IARECPer = 4.00] .....
157453 [SM No. 36.67; SMW=244.49; SKe = 010] .....
157463 *****
157473 *****
157483 *****
157493 *****
157503 R0100:CO0389.....DfIn n-ID NDD.....AREBA-QPEAgm-TpeakDte,hh:mm.....RvM R.C.....Dfwm
157513 ROUTE RESERVOIR -> 1.0 02:MLLS 175.99 20.390 No.date 28:06 56.87 n/a .000
157523 out c 1.0 01:MLLP 146.22 4.050 No.date 28:08 56.87 n/a .000
157533 [MMS objectid= 2310E01] nb_TotOfVol= 1693E01] nb_NOfAr = 2; TotDurOfAr = [hrs] .....
157543 *****
157553 *****
157563 *****
157573 *****
157583 *****
157593 *****
157603 R0100:CO0391.....DfIn n-ID NDD.....AREBA-QPEAgm-TpeakDte,hh:mm.....RvM R.C.....Dfwm
157613 SAVE HYD 1.0 01:SN,M 55196.07 146.161 No.date 36:39 39.51 n/a .000
157623 *****
157633 *****
157643 *****
157653 *****
157663 *****
157673 *****
157683 *****
157693 *****
157703 *****
157713 *****
157723 *****
157733 *****
157743 *****
157753 *****
157763 *****
157773 *****
157783 *****
157793 *****
157803 *****
157813 *****
157823 *****
157833 *****
157843 *****
157853 *****
157863 *****
157873 *****
157883 *****
157893 *****
157903 *****
157913 *****
157923 *****
157933 *****
157943 *****
157953 *****
157963 *****
157973 *****
157983 *****
157993 *****
158003 *****
158013 *****
158023 *****
158033 *****
158043 *****
158053 *****
158063 *****
158073 *****
158083 *****
158093 *****
158103 *****
158113 *****
158123 *****
158133 *****
158143 *****
158153 *****
158163 *****
158173 *****
158183 *****
158193 *****
158203 *****
158213 *****
158223 *****
158233 *****
158243 *****
158253 *****
158263 *****
158273 *****
158283 *****
158293 *****
158303 *****
158313 *****
158323 *****
158333 *****
158343 *****
158353 *****
158363 *****
158373 *****
158383 *****
158393 *****
158403 *****
158413 *****
158423 *****
158433 *****
158443 *****
158453 *****
158463 *****
158473 *****
158483 *****
158493 *****
158503 *****
158513 *****
158523 *****
158533 *****
158543 *****
158553 *****
158563 *****
158573 *****
158583 *****
158593 *****
158603 *****
158613 *****
158623 *****
158633 *****
158643 *****
158653 *****
158663 *****
158673 *****
158683 *****
158693 *****
158703 *****
158713 *****
158723 *****
158733 *****
158743 *****
158753 *****
158763 *****
158773 *****
158783 *****
158793 *****
158803 *****
158813 *****
158823 *****
158833 *****
158843 *****
158853 *****
158863 *****
158873 *****
158883 *****
158893 *****
158903 *****
158913 *****
158923 *****
158933 *****
158943 *****
158953 *****

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158963 *****
158973 *****
158983 *****
158993 *****
159003 *****
159013 *****
159023 *****
159033 *****
159043 *****
159053 *****
159063 *****
159073 *****
159083 *****
159093 *****
159103 *****
159113 *****
159123 *****
159133 *****
159143 *****
159153 *****
159163 *****
159173 *****
159183 *****
159193 *****
159203 *****
159213 *****
159223 *****
159233 *****
159243 *****
159253 *****
159263 *****
159273 *****
159283 *****
159293 *****
159303 *****
159313 *****
159323 *****
159333 *****
159343 *****
159353 *****
159363 *****
159373 *****
159383 *****
159393 *****
159403 *****
159413 *****
159423 *****
159433 *****
159443 *****
159453 *****
159463 *****
159473 *****
159483 *****
159493 *****
159503 *****
159513 *****
159523 *****
159533 *****
159543 *****
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159573 *****
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159593 *****
159603 *****
159613 *****
159623 *****
159633 *****
159643 *****
159653 *****
159663 *****
159673 *****
159683 *****
159693 *****
159703 *****
159713 *****
159723 *****
159733 *****
159743 *****
159753 *****
159763 *****
159773 *****
159783 *****
159793 *****
159803 *****
159813 *****
159823 *****
159833 *****
159843 *****
159853 *****
159863 *****
159873 *****
159883 *****
159893 *****
159903 *****
159913 *****
159923 *****
159933 *****
159943 *****
159953 *****
159963 *****
159973 *****
159983 *****
159993 *****
160003 *****
160013 *****
160023 *****
160033 *****
160043 *****
160053 *****
160063 *****
160073 *****

```

Attachment E

Model 4B – Jock River Reach One Future Conditions – With SWM controls

JFSA, 2021

SWMHYMO Input & Summary files

```

1  20      Metric units / ID numbers OFF
2  *****
3  *# SWHYMO Ver: 5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *****
5  *# Project Name: [Jock River]      Project Number: [1474-16]
6  *# Date       : 04-03-2021
7  *# Modeller   : [MM]
8  *# Company    : JFSA Inc.
9  *# License #  : 2549237
10 *****
11 *# CALIBRATION OF SUMMER MODEL PARAMETERS
12 *# USING CONTINUOUS SIMULATIONS
13 *# Rainfall data from JFSA raingauge installed at site + other gauges by the City
14 *# Use data collected from May 1st to July 14, 2003
15 *# 2020-11-30 change TMSTO in COMPUTE DUALHYD (TMSTO = 0.1 instead of 0.0001)
16 *# 2020-12-01 correct pond curve values
17 *# 2020-12-01 change WCLAR_BRAZ_XIMP to 0.55, SLPI=[0.5](%) (impervious slope), and
18 LGI up to 700m
19 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 2462 (NHYDout=["N_TO"]
20 ,NHYDin=["SN_TO"]) from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model
21 will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELS
22 aren't well suited to really flat slopes.
23 *# 2021-02-19 Change the slope for ROUTE CHANNEL Station 5002 (NHYDout=["N_WC"]
24 ,NHYDin=["SN_CE"]) from 0.01 % (as per Stantec Report 2007) to 0.0255 % so the model
25 will be more stable and give reasonable results. It is justifiable as ROUTE CHANNELS
26 aren't well suited to really flat slopes.
27 *
28 * Calibrated parameters for Summer 2003 data: APII=50, APIK=0.85, CN=varies,
29 *                                               SK=0.01, InterEventTime=12,
30 *                                               GWResk=0.96, VHydCond=0.055
31 *
32 *# -----
33 *
34 *START          TZERO=[2003.0501], METOUT=[2], NSTORM=[1], NRUN=[001]
35 *              ["XAVG0315.STM"] average storm data a 15 minute time step
36 *              The above rainf file is an average of the JFSA gauge data
37 *              with the City of Ottawa rainfall data collected during
38 *              the same period.
39 *% 2 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
40 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[2]
41 ["C24SC002.stm"] <-- storm filename, one per line for NSTORM time
42 *%-----|-----|
43 *%-----|-----|
44 READ STORM     STORM_FILENAME=["storm 001"]
45 *%-----|-----|
46 MODIFY STORM   ICASEms=[1], NSHIFT=[96],
47 RedFACT=[1],
48 *%-----|-----|
49 DEFAULT VALUES ICASEdef=[1], read and print values
50 DEFVAL_FILENAME=["CitiGate.DEF"]
51 *%-----|-----|
52 *%-----|-----|
53 COMPUTE API     APII=[50], APIK=[.85]/day
54 *%-----|-----|
55 *%-----|-----|
56 *#
57 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
58 *# of 1.32
59 *%-----|-----|
60 CONTINUOUS NASHYD NHYD=["JR_HW"], DT=[1]min, AREA=[3680](ha),
61 DWF=[0](cms), CNVC=[64], IA=[2.5](mm),
62 N=[3.0], TP=[7.13]hrs,
63 Continuous simulation parameters:
64 IARECper=[4](hrs),
65 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
66 InterEventTime=[12](hrs)
67 Baseflow simulation parameters:

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60 BaseFlowOption=[ 1] ,
61 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
62 VHydCond=[ 0.055](mm/hr), END=- 1
63 *%-----|-----
64 *#
65 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
66 *# of 1.32
67 *%-----|-----
68 CONTINUOUS NASHYD NHYD=[ "SW_13"], DT=[ 1]min, AREA=[ 971](ha),
69 DWF=[ 0](cms), CNVC=[ 61], IA=[ 2.5](mm),
70 N=[ 3.0], TP=[ 3.76]hrs,
71 Continuous simulation parameters:
72 IaRECper=[ 4](hrs),
73 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
74 InterEventTime=[ 12](hrs)
75 Baseflow simulation parameters:
76 BaseFlowOption=[ 1] ,
77 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
78 VHydCond=[ 0.055](mm/hr), END=- 1
79 *%-----|-----
80 *#
81 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
82 *# of 1.80
83 *%-----|-----
84 CONTINUOUS NASHYD NHYD=[ "JR_GWM"], DT=[ 1]min, AREA=[ 3074](ha),
85 DWF=[ 0](cms), CNVC=[ 55], IA=[ 2.5](mm),
86 N=[ 3], TP=[ 11.33]hrs,
87 Continuous simulation parameters:
88 IaRECper=[ 4](hrs),
89 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
90 InterEventTime=[ 12](hrs)
91 Baseflow simulation parameters:
92 BaseFlowOption=[ 1] ,
93 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
94 VHydCond=[ 0.055](mm/hr), END=- 1
95 *%-----|-----
96 CONTINUOUS NASHYD NHYD=[ "JR_ASH"], DT=[ 1]min, AREA=[ 1781](ha),
97 DWF=[ 0](cms), CNVC=[ 72], IA=[ 2.5](mm),
98 N=[ 3.0], TP=[ 3.91]hrs,
99 Continuous simulation parameters:
100 IaRECper=[ 4](hrs),
101 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
102 InterEventTime=[ 12](hrs)
103 Baseflow simulation parameters:
104 BaseFlowOption=[ 1] ,
105 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
106 VHydCond=[ 0.055](mm/hr), END=- 1
107 *%-----|-----
108 CONTINUOUS NASHYD NHYD=[ "SW_11"], DT=[ 1]min, AREA=[ 500](ha),
109 DWF=[ 0](cms), CNVC=[ 66], IA=[ 2.5](mm),
110 N=[ 3.0], TP=[ 1.24]hrs,
111 Continuous simulation parameters:
112 IaRECper=[ 4](hrs),
113 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
114 InterEventTime=[ 12](hrs)
115 Baseflow simulation parameters:
116 BaseFlowOption=[ 1] ,
117 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
118 VHydCond=[ 0.055](mm/hr), END=- 1
119 *%-----|-----
120 *#
121 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
122 *# of 1.80
123 *%-----|-----
124 CONTINUOUS NASHYD NHYD=[ "NN_CK"], DT=[ 1]min, AREA=[ 1917](ha),
125 DWF=[ 0](cms), CNVC=[ 66], IA=[ 2.5](mm),

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126 N=[ 3.0], TP=[ 5.29] hrs,
127 Continuous simulation parameters:
128 IaRECPper=[ 4] (hrs),
129 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
130 InterEventTime=[ 12] (hrs)
131 Baseflow simulation parameters:
132 BaseFlowOption=[ 1] ,
133 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
134 VHydCond=[ 0.055] (mm/ hr), END=- 1
135 *%-----|-----
136 *#
137 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
138 *# of 1.52
139 *%-----|-----
140 CONTINUOUS NASHYD NHYD=[ "SW_10"], DT=[ 1] min, AREA=[ 5666] (ha),
141 DWF=[ 0] (cms), CNVC=[ 72], IA=[ 2.5] (mm),
142 N=[ 3.0], TP=[ 8.00] hrs,
143 Continuous simulation parameters:
144 IaRECPper=[ 4] (hrs),
145 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
146 InterEventTime=[ 12] (hrs)
147 Baseflow simulation parameters:
148 BaseFlowOption=[ 1] ,
149 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
150 VHydCond=[ 0.055] (mm/ hr), END=- 1
151 *%-----|-----
152 *#
153 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
154 *# of 1.75
155 *%-----|-----
156 CONTINUOUS NASHYD NHYD=[ "KG_CK"], DT=[ 1] min, AREA=[ 8376] (ha),
157 DWF=[ 0] (cms), CNVC=[ 66], IA=[ 2.5] (mm),
158 N=[ 3.0], TP=[ 11.66] hrs,
159 Continuous simulation parameters:
160 IaRECPper=[ 4] (hrs),
161 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
162 InterEventTime=[ 12] (hrs)
163 Baseflow simulation parameters:
164 BaseFlowOption=[ 1] ,
165 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
166 VHydCond=[ 0.055] (mm/ hr), END=- 1
167 *%-----|-----
168 *#
169 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
170 *# of 1.68
171 *%-----|-----
172 CONTINUOUS NASHYD NHYD=[ "SW_9"], DT=[ 1] min, AREA=[ 1132] (ha),
173 DWF=[ 0] (cms), CNVC=[ 70], IA=[ 2.5] (mm),
174 N=[ 3.0], TP=[ 2.51] hrs,
175 Continuous simulation parameters:
176 IaRECPper=[ 4] (hrs),
177 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
178 InterEventTime=[ 12] (hrs)
179 Baseflow simulation parameters:
180 BaseFlowOption=[ 1] ,
181 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
182 VHydCond=[ 0.055] (mm/ hr), END=- 1
183 *%-----|-----
184 *#
185 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
186 *# of 1.82
187 *%-----|-----
188 CONTINUOUS NASHYD NHYD=[ "NC_CK"], DT=[ 1] min, AREA=[ 4464] (ha),
189 DWF=[ 0] (cms), CNVC=[ 62], IA=[ 2.5] (mm),
190 N=[ 3.0], TP=[ 11.32] hrs,
191 Continuous simulation parameters:

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192 IaREcper=[ 4] (hr s),
193 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
194 InterEventTime=[ 12] (hr s)
195 Baseflow simulation parameters:
196 BaseFlowOption=[ 1] ,
197 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
198 VHydCond=[ 0. 055] (mm/ hr), END=- 1
199 *%-----|-----
200 *#
201 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
202 *# of 1.80
203 *%-----|-----
204 CONTINUOUS NASHYD NHYD=[ "SW_8" ], DT=[ 1] mi n, AREA=[ 131] (ha),
205 DWF=[ 0] (cms), CN C=[ 63], IA=[ 2. 5] (mm),
206 N=[ 3. 0], TP=[ 0. 90] hr s,
207 Continuous simulation parameters:
208 IaREcper=[ 4] (hr s),
209 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
210 InterEventTime=[ 12] (hr s)
211 Baseflow simulation parameters:
212 BaseFlowOption=[ 1] ,
213 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
214 VHydCond=[ 0. 055] (mm/ hr), END=- 1
215 *%-----|-----
216 *#
217 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
218 *# of 1.65
219 *%-----|-----
220 CONTINUOUS NASHYD NHYD=[ "HB_DR" ], DT=[ 1] mi n, AREA=[ 3854] (ha),
221 DWF=[ 0] (cms), CN C=[ 66], IA=[ 2. 5] (mm),
222 N=[ 3. 0], TP=[ 8. 42] hr s,
223 Continuous simulation parameters:
224 IaREcper=[ 4] (hr s),
225 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
226 InterEventTime=[ 12] (hr s)
227 Baseflow simulation parameters:
228 BaseFlowOption=[ 1] ,
229 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
230 VHydCond=[ 0. 055] (mm/ hr), END=- 1
231 *%-----|-----
232 *#
233 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
234 *# of 1.82
235 *%-----|-----
236 CONTINUOUS NASHYD NHYD=[ "SW_7" ], DT=[ 1] mi n, AREA=[ 3197] (ha),
237 DWF=[ 0] (cms), CN C=[ 57], IA=[ 2. 5] (mm),
238 N=[ 3. 0], TP=[ 6. 65] hr s,
239 Continuous simulation parameters:
240 IaREcper=[ 4] (hr s),
241 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
242 InterEventTime=[ 12] (hr s)
243 Baseflow simulation parameters:
244 BaseFlowOption=[ 1] ,
245 InitGWResVol=[ 50] (mm), GWResK=[ 0. 96] (mm/ day/ mm)
246 VHydCond=[ 0. 055] (mm/ hr), END=- 1
247 *%-----|-----
248 *#
249 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
250 *# of 1.75
251 *%-----|-----
252 CONTINUOUS NASHYD NHYD=[ "SW_6" ], DT=[ 1] mi n, AREA=[ 165] (ha),
253 DWF=[ 0] (cms), CN C=[ 67], IA=[ 2. 5] (mm),
254 N=[ 3. 0], TP=[ 4. 18] hr s,
255 Continuous simulation parameters:
256 IaREcper=[ 4] (hr s),
257 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),

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258 InterEventTime=[ 12] (hrs)
259 Baseflow simulation parameters:
260 BaseFlowOption=[ 1] ,
261 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
262 VHydCond=[ 0.055] (mm/ hr), END=- 1
263 *%-----|-----
264 *#
265 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
266 *# of 1.67
267 *%-----|-----
268 CONTINUOUS NASHYD NHYD=[ "VG_DR" ], DT=[ 1] min, AREA=[ 1332] (ha),
269 DWF=[ 0] (cms), CNVC=[ 72], IA=[ 2.5] (mm),
270 N=[ 3.0], TP=[ 5.95] hrs,
271 Continuous simulation parameters:
272 IaRECper=[ 4] (hrs),
273 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
274 InterEventTime=[ 12] (hrs)
275 Baseflow simulation parameters:
276 BaseFlowOption=[ 1] ,
277 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
278 VHydCond=[ 0.055] (mm/ hr), END=- 1
279 *%-----|-----
280 CONTINUOUS NASHYD NHYD=[ "SW_5" ], DT=[ 1] min, AREA=[ 224] (ha),
281 DWF=[ 0] (cms), CNVC=[ 77], IA=[ 2.5] (mm),
282 N=[ 3.0], TP=[ 0.75] hrs,
283 Continuous simulation parameters:
284 IaRECper=[ 4] (hrs),
285 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
286 InterEventTime=[ 12] (hrs)
287 Baseflow simulation parameters:
288 BaseFlowOption=[ 1] ,
289 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
290 VHydCond=[ 0.055] (mm/ hr), END=- 1
291 *%-----|-----
292 *#
293 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
294 *# of 1.20
295 *%-----|-----
296 CONTINUOUS NASHYD NHYD=[ "FL_CK" ], DT=[ 1] min, AREA=[ 4945] (ha),
297 DWF=[ 0] (cms), CNVC=[ 74], IA=[ 2.5] (mm),
298 N=[ 3.0], TP=[ 4.45] hrs,
299 Continuous simulation parameters:
300 IaRECper=[ 4] (hrs),
301 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
302 InterEventTime=[ 12] (hrs)
303 Baseflow simulation parameters:
304 BaseFlowOption=[ 1] ,
305 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
306 VHydCond=[ 0.055] (mm/ hr), END=- 1
307 *%-----|-----
308 CONTINUOUS NASHYD NHYD=[ "SW_5A2" ], DT=[ 1] min, AREA=[ 20] (ha),
309 DWF=[ 0] (cms), CNVC=[ 81], IA=[ 2.5] (mm),
310 N=[ 3.0], TP=[ 0.62] hrs,
311 Continuous simulation parameters:
312 IaRECper=[ 4] (hrs),
313 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
314 InterEventTime=[ 12] (hrs)
315 Baseflow simulation parameters:
316 BaseFlowOption=[ 1] ,
317 InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/ day/ mm)
318 VHydCond=[ 0.055] (mm/ hr), END=- 1
319 *%-----|-----
320 *#
321 *# The Tp was modified according to a Peak Reduction factor (MFO-Chart B2-4)
322 *# of 1.61
323 *%-----|-----

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324 CONTI NUOUS NASHYD NYHD=[" SW_5A1"], DT=[1] mi n, AREA=[1412] (ha),
325 DWF=[0] (cms), CNV C=[75], IA=[2. 5] (mm),
326 N=[3. 0], TP=[8. 00] hr s,
327 Continuous simulation parameters:
328 IaRECper=[4] (hr s),
329 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
330 Inter Event Time=[12] (hr s)
331 Baseflow simulation parameters:
332 BaseFlowOpt ion=[1] ,
333 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
334 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

336 CONTI NUOUS NASHYD NYHD=[" SW_4"], DT=[1] mi n, AREA=[585] (ha),
337 DWF=[0] (cms), CNV C=[81], IA=[2. 5] (mm),
338 N=[3. 0], TP=[1. 75] hr s,
339 Continuous simulation parameters:
340 IaRECper=[4] (hr s),
341 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
342 Inter Event Time=[12] (hr s)
343 Baseflow simulation parameters:
344 BaseFlowOpt ion=[1] ,
345 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
346 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

348 CONTI NUOUS NASHYD NYHD=[" LM_CK"], DT=[1] mi n, AREA=[1021] (ha),
349 DWF=[0] (cms), CNV C=[80], IA=[2. 5] (mm),
350 N=[3. 0], TP=[2. 46] hr s,
351 Continuous simulation parameters:
352 IaRECper=[4] (hr s),
353 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
354 Inter Event Time=[12] (hr s)
355 Baseflow simulation parameters:
356 BaseFlowOpt ion=[1] ,
357 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
358 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

360 CONTI NUOUS NASHYD NYHD=[" SW_2"], DT=[1] mi n, AREA=[177] (ha),
361 DWF=[0] (cms), CNV C=[77], IA=[2. 5] (mm),
362 N=[3. 0], TP=[0. 75] hr s,
363 Continuous simulation parameters:
364 IaRECper=[4] (hr s),
365 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
366 Inter Event Time=[12] (hr s)
367 Baseflow simulation parameters:
368 BaseFlowOpt ion=[1] ,
369 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
370 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

372 CONTI NUOUS NASHYD NYHD=[" SM_DR"], DT=[1] mi n, AREA=[1122] (ha),
373 DWF=[0] (cms), CNV C=[81], IA=[2. 5] (mm),
374 N=[3. 0], TP=[3. 25] hr s,
375 Continuous simulation parameters:
376 IaRECper=[4] (hr s),
377 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),
378 Inter Event Time=[12] (hr s)
379 Baseflow simulation parameters:
380 BaseFlowOpt ion=[1] ,
381 In it GWRes Vol=[50] (mm), GWRes K=[0. 96] (mm/ day/ mm)
382 VHydCond=[0. 055] (mm/ hr), END=- 1

*%-----|

384 CONTI NUOUS NASHYD NYHD=[" MO_DR"], DT=[1] mi n, AREA=[2737] (ha),
385 DWF=[0] (cms), CNV C=[76], IA=[2. 5] (mm),
386 N=[3. 0], TP=[3. 03] hr s,
387 Continuous simulation parameters:
388 IaRECper=[4] (hr s),
389 SM N=[- 1] (mm), SMAX=[- 1] (mm), SK=[0. 010] / (mm),

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390 InterEventTime=[ 12]( hrs)
391 Baseflow simulation parameters:
392 BaseFlowOption=[ 1] ,
393 InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
394 VHydCond=[ 0.055]( mm/ hr) , END=- 1
395 *%-----|-----|
396 * -JFSA 2020 replaced SW_1 with a detailed model from Stantec Report 2007
397 *CONTINUOUS NASHYD NHYD=["SW_1"], DT=[ 1]mi n, AREA=[ 3176]( ha),
398 * DWF=[ 0]( cms) , CN/C=[ 78] , IA=[ 2.5]( mm),
399 * N=[ 3.0] , TP=[ 3.56]hrs,
400 * Continuous simulation parameters:
401 * IaRECper=[ 4]( hrs),
402 * SMN=[ -1]( mm) , SMAX=[ -1]( mm) , SK=[ 0.010]/( mm),
403 * InterEventTime=[ 12]( hrs)
404 * Baseflow simulation parameters:
405 * BaseFlowOption=[ 1] ,
406 * InitGWResVol=[ 50]( mm) , GWResK=[ 0.96]( mm/ day/ mm)
407 * VHydCond=[ 0.055]( mm/ hr) , END=- 1
408 *%-----|-----|
409 *#
410 *# Routing hydrographs
411 *#
412 *# Starting with the addition of Jock River Headwater and Subwatershed 13
413 *#
414 ADD HYD NHYDsum=["S_N13"], NHYDs to add=["JR_HW"+"SW_13"]
415 *%-----|-----|
416 *#
417 *# Sum of hydrographs from Node 13 routed to Node 13A
418 *# (Approximated cross-section - see cross-section 258)
419 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
420 *#
421 ROUTE CHANNEL NHYDout=["N13A"] , NHYDin=["S_N13"],
422 RDT=[ 1]( mi n),
423 CHLGTH=[ 9074]( m) , CHSLOPE=[ 0.0220]( %),
424 FPSLOPE=[ 0.0220]( %),
425 SECNUM=[ 1.0] , NSEG=[ 1]
426 ( SEGROUGH, SEGDI ST ( m))=[ 0.04, 15.5] NSEG times
427 ( DI STANCE ( m) , ELEVATI ON ( m))=
428 [- 40, 132.5]
429 [- 30, 132]
430 [- 25, 131.5]
431 [- 13, 130]
432 [- 8, 127.00]
433 [- 7, 126.50]
434 [- 6, 126]
435 [- 5.5, 125.50]
436 [0, 123.75]
437 [4.5, 125.50]
438 [6, 126]
439 [7.5, 126.5]
440 [9, 127]
441 [10, 127.5]
442 [11.5, 128.0]
443 [15.5, 129.5]
444 *%-----|-----|
445 *#
446 *# Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
447 *#
448 ADD HYD NHYDsum=["SN13A"], NHYDs to add=["N13A"+"JR_GWM"]
449 *%-----|-----|
450 *#
451 *# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
452 *#
453 ROUTE RESERVOIR NHYDout=["RES_GM"] , NHYDin=["SN13A"],
454 RDT=[ 1]( mi n),
455 TABLE of ( OUTFLOW STORAGE ) values

```

```

456 (cms) - (ha-m)
457 [ 0.0 , 0.0 ]
458 [ 1.991, 2.144 ]
459 [ 2.693, 39.826 ]
460 [ 3.509, 81.697 ]
461 [ 4.578, 318.774 ]
462 [ 5.647, 594.947 ]
463 [ 7.109, 910.219 ]
464 [ 8.616, 1264.589 ]
465 [ 10.371, 1658.057 ]
466 [ 12.402, 2090.622 ]
467 [ 22.056, 3462.487 ]
468 [ -1 , -1 ] (max twenty pts)
469
470 NHYDovf=[ " " ] ,
471 *%-----|-----|
472 *#
473 SAVE HYD NHYD=[ "RES_GM" ], # OF PCYCLES=[ -1 ], I CASEs h=[ -1 ]
474 HYD_FILE_NAME=[ "H_RESGM" ]
475 HYD_COMMENT=[ "Out flow from Res GM" ]
476 *%-----|-----|
477 *# Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
478 *# (Approximated cross-section - see cross-section 258)
479 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
480 ROUTE CHANNEL NHYDout=[ "N12" ] , NHYDin=[ "RES_GM" ] ,
481 RDT=[ 1 ] (min) ,
482 CHLGTH=[ 5926 ] (m) , CHSLOPE=[ 0.0759 ] ( % ) ,
483 FPSLOPE=[ 0.0759 ] ( % ) ,
484 SECNUM=[ 1.0 ] , NSEG=[ 1 ]
485 ( SEGROUGH, SEGDIST (m) )=[ 0.04, 15.5 ] NSEG times
486 ( DISTANCE (m) , ELEVATION (m) )=
487 [- 40, 132.5]
488 [- 30, 132]
489 [- 25, 131.5]
490 [- 13, 130]
491 [- 8, 127.00]
492 [- 7, 126.50]
493 [- 6, 126]
494 [- 5.5, 125.50]
495 [ 0, 123.75]
496 [ 4.5, 125.50]
497 [ 6, 126]
498 [ 7.5, 126.5]
499 [ 9, 127]
500 [ 10, 127.5]
501 [ 11.5, 128.00]
502 [ 15.5, 129.5]
503 *%-----|-----|
504 *#
505 *# Addition of Subwatershed Jock River at Ashton to Node 12
506 *#
507 ADD HYD NHYDsum=[ "S_N12" ] , NHYDsto add=[ "N12"+"JR_ASH" ]
508 SAVE HYD NHYD=[ "S_N12" ] , # OF PCYCLES=[ -1 ], I CASEs h=[ -1 ]
509 HYD_FILE_NAME=[ "H_SN12" ]
510 HYD_COMMENT=[ "flow at S_N12 near Ashton" ]
511 *%-----|-----|
512 *#
513 *# Sum of hydrographs from Node 12 routed to Node 11
514 *# (Approximated cross-section - see cross-section 258)
515 *# Use n=0.04 for summer conditions and n=0.025 for spring conditions
516 *ROUTE CHANNEL NHYDout=[ "N11" ] , NHYDin=[ "S_N12" ] ,
517 * RDT=[ 1 ] (min) ,
518 * CHLGTH=[ 972 ] (m) , CHSLOPE=[ 0.0514 ] ( % ) ,
519 * FPSLOPE=[ 0.0514 ] ( % ) ,
520 * SECNUM=[ 1.0 ] , NSEG=[ 1 ]
521 * ( SEGROUGH, SEGDIST (m) )=[ 0.04, 15.5 ] NSEG times
522 * ( DISTANCE (m) , ELEVATION (m) )=

```

```

522 * [- 40, 132.5]
523 * [- 30, 132]
524 * [- 25, 131.5]
525 * [- 13, 130]
526 * [- 8, 127.00]
527 * [- 7, 126.50]
528 * [- 6, 126]
529 * [- 5.5, 125.50]
530 * [0, 123.75]
531 * [4.5, 125.50]
532 * [6, 126]
533 * [7.5, 126.5]
534 * [9, 127]
535 * [10, 127.5]
536 * [11.5, 128.00]
537 * [15.5, 129.5]
538 *%-----|-----
539 *#
540 *# Sum of hydrographs from Node 12 routed to Node 11 with Dummy section 248
541 *#
542 ROUTE CHANNEL NHYDout=["Duml1"], NHYDin=["S_N12"],
543 RDT=[1](min),
544 CHLGTH=[972](m), CHSLOPE=[0.054](%),
545 FPSLOPE=[0.054](%),
546 SECNUM=[1.0], NSEG=[1]
547 ( SEGROUGH, SEGDIST (m))=[0.04, 15.5] NSEG times
548 ( DISTANCE (m), ELEVATION (m))=
549 [- 40, 132.5]
550 [- 30, 132]
551 [- 25, 131.5]
552 [- 13, 130]
553 [- 8, 127.00]
554 [- 7, 126.50]
555 [- 6, 126]
556 [- 5.5, 125.50]
557 [0, 123.75]
558 [4.5, 125.50]
559 [6, 126]
560 [7.5, 126.5]
561 [9, 127]
562 [10, 127.5]
563 [11.5, 128.00]
564 [15.5, 129.5]
565 *%-----|-----
566 *#
567 *# Addition of Subwatershed 11 and No Name Creek to Node 11
568 *#
569 ADD HYD NHYDsum=["S_N11"], NHYDstoadd=["Duml1"+"SW_11"+"NN_CK"]
570 *%-----|-----
571 *#
572 *# Sum of hydrographs from Node 11 routed to Node 10
573 *# Section 1
574 *#
575 ROUTE CHANNEL NHYDout=["N10"], NHYDin=["S_N11"],
576 RDT=[1](min),
577 CHLGTH=[14028](m), CHSLOPE=[0.1568](%),
578 FPSLOPE=[0.1568](%),
579 SECNUM=[1.0], NSEG=[5]
580 ( SEGROUGH, SEGDIST (m))=
581 [0.04, -52.82
582 0.1, -6.47
583 -0.05, 6.47
584 0.1, 45.36
585 0.04, 423.88] NSEG times
586 ( DISTANCE (m), ELEVATION (m))=
587 [- 226.24 , 112.50]

```

```

588             [- 167. 50 , 111. 50]
589             [- 106. 81 , 111. 00]
590             [- 92. 37 , 110. 00]
591             [- 52. 82 , 109. 00]
592             [- 24. 90, 109. 00]
593             [- 17. 02, 108. 50]
594             [- 6. 47, 108. 00]
595             [ 6. 47, 108. 00]
596             [ 15. 67, 108. 50]
597             [ 18. 95, 109. 00]
598             [ 45. 36, 109. 50]
599             [ 120. 79, 110. 00]
600             [ 145. 72, 111. 00]
601             [ 181. 56, 111. 50]
602             [ 423. 88, 112. 50]
603 *%-----|-----
604 *#
605 *# Addition of Subwatershed 10 to Node 10
606 *#
607 ADD HYD          NHYDs um=["S_N10"], NHYDs to add=["N10"+"SW_10"]
608 *%-----|-----
609 SAVE HYD         NHYD=["S_N10"], # OF PCYCLES=[- 1], I CASEs h=[- 1]
610                 HYD_ FI LENAME=["H_SN10"]
611                 HYD_ COMMENT=["flow at S_N10: N10 + SW_10"]
612 *%-----|-----
613 *# Addition of Kings Creek to S_N10
614 *#
615 ADD HYD          NHYDs um=["S_N10A"], NHYDs to add=["S_N10"+"KG_CK"]
616 *%-----|-----
617 *#
618 *# Sum of hydrographs from Node 10 routed to Node 9
619 *# Section 2
620 *#
621 ROUTE CHANNEL    NHYDout=["N9"] , NHYDin=["S_N10A"] ,
622                 RDT=[ 1] ( mi n) ,
623                 CHLGTH=[ 3982] ( m) ,   CHSLOPE=[ 0. 0753] ( % ) ,
624                                     FPSLOPE=[ 0. 0753] ( % ) ,
625                 SECNUM=[ 1. 0] ,       NSEG=[ 4]
626                 ( SEGROUGH, SEGDI ST ( m) ) =
627                 [ 0. 04, - 30. 27
628                 0. 05, - 18. 42
629                 - 0. 05, 18. 42
630                 0. 04, 131. 58] NSEG t i m e s
631                 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
632                 [- 446. 74, 106. 00]
633                 [- 415. 68, 105. 50]
634                 [- 285. 40, 105. 00]
635                 [- 173. 77, 104. 50]
636                 [- 144. 95, 104. 00]
637                 [- 111. 18, 103. 50]
638                 [- 94. 06, 103. 00]
639                 [- 71. 02, 102. 50]
640                 [- 30. 27, 102. 00]
641                 [- 19. 33, 100. 00]
642                 [- 18. 42, 99. 50]
643                 [ 18. 42, 99. 50]
644                 [ 20. 77, 100. 00]
645                 [ 27. 93, 101. 00]
646                 [ 52. 29, 101. 00]
647                 [ 68. 80, 101. 50]
648                 [ 79. 66, 103. 00]
649                 [ 91. 50, 103. 50]
650                 [ 131. 58, 104. 00]
651 *%-----|-----
652 *#
653 *# Addition of Subwatershed 9 and Nichols Creek to Node 9

```

```

654  *#
655  ADD HYD          NHYDs um=[ "S_N9" ], NHYDs  to  add=[ "N9"+"SW_9"+"NC_CK" ]
656  *%-----|-----|
657  *#
658  *# Sum of hydrographs from Node 9 routed to Node 8
659  *# Section 3
660  *#
661  ROUTE CHANNEL    NHYDout =[ "N8" ] , NHYDin =[ "S_N9" ] ,
662                  RDT=[ 1 ] ( mi n ) ,
663                  CHLGTH=[ 2269 ] ( m ) ,   CHSLOPE=[ 0.0882 ] ( % ) ,
664                                          FPSLOPE=[ 0.0882 ] ( % ) ,
665                  SECNUM=[ 1.0 ] ,         NSEG=[ 3 ]
666                  ( SEGROUGH, SEGDI ST ( m ) ) =
667                    [ 0.1, -17.99
668                    -0.045, 17.31
669                    0.1, 456.58 ] NSEG t i m e s
670                  ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
671                    [ -201.19, 100.50 ]
672                    [ -135.21, 100.00 ]
673                    [ -94.83, 99.50 ]
674                    [ -67.05, 99.00 ]
675                    [ -17.99, 98.50 ]
676                    [ -16.02, 98.00 ]
677                    [ -13.95, 97.50 ]
678                    [ 13.95, 97.50 ]
679                    [ 15.64, 98.00 ]
680                    [ 17.31, 98.50 ]
681                    [ 162.02, 98.50 ]
682                    [ 172.89 , 99.00 ]
683                    [ 314.38, 99.00 ]
684                    [ 343.78, 99.50 ]
685                    [ 365.67, 100.00 ]
686                    [ 376.68, 100.00 ]
687                    [ 393.11, 99.50 ]
688                    [ 404.97, 99.50 ]
689                    [ 431.70, 100.00 ]
690                    [ 456.58, 100.50 ]
691  *%-----|-----|
692  *#
693  *# Addition of Subwatershed 8 and Hobb's Drain to Node 8
694  *#
695  ADD HYD          NHYDs um=[ "S_N8" ], NHYDs  to  add=[ "N8"+"SW_8"+"HB_DR" ]
696  *%-----|-----|
697  *#
698  *# Sum of hydrographs from Node 8 routed to Node 7
699  *# Section 4
700  *#
701  ROUTE CHANNEL    NHYDout =[ "N7" ] , NHYDin =[ "S_N8" ] ,
702                  RDT=[ 1 ] ( mi n ) ,
703                  CHLGTH=[ 3750 ] ( m ) ,   CHSLOPE=[ 0.0533 ] ( % ) ,
704                                          FPSLOPE=[ 0.0533 ] ( % ) ,
705                  SECNUM=[ 1.0 ] ,         NSEG=[ 3 ]
706                  ( SEGROUGH, SEGDI ST ( m ) ) =
707                    [ 0.12, -18.11
708                    -0.07, 17.22
709                    0.12, 590.05 ] NSEG t i m e s
710                  ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
711                    [ -433.21, 102.00 ]
712                    [ -425.34, 101.50 ]
713                    [ -377.56, 101.50 ]
714                    [ -366.23, 101.00 ]
715                    [ -202.60, 100.50 ]
716                    [ -96.25, 99.50 ]
717                    [ -68.36 99.00 ]
718                    [ -18.11, 98.50 ]
719                    [ -13.81, 97.50 ]

```

```

720 [ 13. 81, 97. 50]
721 [ 17. 22, 98. 50]
722 [ 161. 95, 98. 50]
723 [ 173. 11, 99. 00]
724 [ 314. 05, 99. 00]
725 [ 365. 52, 100. 00]
726 [ 404. 70, 99. 50]
727 [ 476. 74, 100. 50]
728 [ 502. 31, 101. 00]
729 [ 584. 69, 101. 00]
730 [ 585. 79, 101. 00]
731 [ 590. 05, 102. 00]
732 *%-----|-----
733 *#
734 *# Addition of Subwatershed 7 to Node 7
735 *#
736 ADD HYD          NHYDs um=[ "S_N7" ], NHYDs to add=[ "N7"+"SW_7" ]
737 *%-----|-----
738 SAVE HYD         NHYD=[ "S_N7" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ - 1 ]
739                 HYD_FI LENAME=[ "H_SN7" ]
740                 HYD_COMMENT=[ "flow at S_N7: N7 + SW_7" ]
741 *%-----|-----
742 *# Insertion of a reservoir to simulate the effects of the Richmond Fen.
743 *# Storage area and volumes were estimated from available topo maps.
744 *# Release rate from fen was assumed to be controlled by the downstream
745 *# river cross-section for summer conditions. It is was assumed that for up to
746 *# 0.75 m of water, the main channel of the river provided the storage. Above
747 *# this depth, the wetland starts to signigicantly store water.
748 *#
749 ROUTE RESERVOIR NHYDout =[ "RES_RF" ] , NHYDi n=[ "S_N7" ] ,
750                 RDT=[ 1 ] ( mi n ) ,
751                 TABLE of ( OUTFLOW STORAGE ) values
752                         ( cms ) - ( ha- m )
753                 TABLE of ( OUTFLOW STORAGE ) values
754                         ( cms ) - ( ha- m )
755                         [ 0. 0 , 0. 0 ]
756                         [ 0. 9051, 2. 40]
757                         [ 2. 907, 4. 13]
758                         [ 9. 744, 9. 18]
759                         [ 20. 304, 14. 96]
760                         [ 34. 167, 310. 21]
761                         [ 74. 993, 605. 46]
762                         [ 104. 876, 900. 71]
763                         [ 140. 56, 2892. 00]
764                         [ 225. 00, 3615. 63]
765                         [ - 1 , - 1 ] (max t went y pts)
766                 NHYDovf=[ " " ] ,
767 *%-----|-----
768 SAVE HYD         NHYD=[ "RES_RF" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ - 1 ]
769                 HYD_FI LENAME=[ "H_Res RF" ]
770                 HYD_COMMENT=[ "out flow of Ri chmnd Fen" ]
771 *%-----|-----
772 *#
773 *# Sum of hydrographs from Node 7 routed to Node 6
774 *# Section 5
775 *#
776 ROUTE CHANNEL    NHYDout =[ "N6" ] , NHYDi n=[ "RES_RF" ] ,
777                 RDT=[ 1 ] ( mi n ) ,
778                 CHLGTH=[ 3056 ] ( m ) , CHSLOPE=[ 0. 0818 ] ( % ) ,
779                 FPSLOPE=[ 0. 0818 ] ( % ) ,
780                 SECNUM=[ 1. 0 ] , NSEG=[ 5 ]
781                 ( SEGROUGH, SEGDI ST ( m ) ) =
782                 [ 0. 025, - 70. 8
783                 0. 1, - 23. 9
784                 - 0. 05, 23. 9
785                 0. 06, 39. 8

```

```

786           0. 05, 96. 3] NSEG times
787           ( DI STANCE ( m), ELEVATI ON ( m))=
788             [- 100. 8, 97. 00]
789             [- 70. 8, 96. 50]
790             [- 52. 0, 96. 00]
791             [- 35. 1, 95. 50]
792             [- 30. 6, 95. 00]
793             [- 23. 9, 94. 54]
794             [ 23. 9, 94. 54]
795             [ 39. 8, 95. 00]
796             [ 50. 4, 95. 50]
797             [ 93. 5, 96. 00]
798             [ 94. 9, 96. 50]
799             [ 96. 3, 97. 00]
800 *%-----|-----
801 *#
802 *# Addition of Subwatershed 6 and Van Gaal Drain to Node 6
803 *#
804 ADD HYD           NHYDs um=[ "S_N6" ], NHYDs to add=[ "N6"+"SW_6"+"VG_DR" ]
805 *%-----|-----
806 *#
807 *# Sum of hydrographs from Node 6 routed to Node 5
808 *# Section 6
809 *#
810 ROUTE CHANNEL     NHYDout =[ "N5" ] , NHYDin =[ "S_N6" ] ,
811                   RDT=[ 1] ( mi n),
812                   CHLGTH=[ 1852] ( m),   CHSLOPE=[ 0. 0540] ( %),
813                                           FPSLOPE=[ 0. 0540] ( %),
814                   SECNUM=[ 1. 0],       NSEG=[ 3]
815                   ( SEGROUGH, SEGDI ST ( m))=
816                     [ 0. 035, - 131. 59
817                     - 0. 045, 48. 96
818                     0. 1, 239. 04] NSEG times
819                   ( DI STANCE ( m), ELEVATI ON ( m))=
820                     [- 686. 30, 94. 50]
821                     [- 675. 70, 94. 00]
822                     [- 492. 52, 93. 00]
823                     [- 467. 28, 94. 00]
824                     [- 131. 59, 94. 00]
825                     [- 92. 79, 92. 50]
826                     [- 18. 06, 91. 00]
827                     [ 18. 06, 91. 00]
828                     [ 43. 47, 92. 50]
829                     [ 48. 96, 94. 00]
830                     [ 177. 43, 94. 00]
831                     [ 239. 04, 94. 50]
832 *%-----|-----
833 *#
834 *# Addition of Subwatershed 5 and Flowing Creek to Node 5
835 *#
836 ADD HYD           NHYDs um=[ "S_N5" ], NHYDs to add=[ "N5"+"SW_5"+"FL_CK" ]
837 *%-----|-----
838 *#
839 *# Sum of hydrographs from Node 5 routed to Node 5A
840 *# Section 7
841 *#
842 ROUTE CHANNEL     NHYDout =[ "N5A" ] , NHYDin =[ "S_N5" ] ,
843                   RDT=[ 1] ( mi n),
844                   CHLGTH=[ 556] ( m),   CHSLOPE=[ 0. 0900] ( %),
845                                           FPSLOPE=[ 0. 0900] ( %),
846                   SECNUM=[ 1. 0],       NSEG=[ 4]
847                   ( SEGROUGH, SEGDI ST ( m))=
848                     [ 0. 04, - 41. 5
849                     0. 1, - 14. 0
850                     - 0. 045, 14. 0
851                     0. 1, 41. 1] NSEG times

```



```

852          ( DI STANCE ( m) , ELEVATI ON ( m) ) =
853          [- 275. 8, 93. 00]
854          [- 248. 6, 92. 50]
855          [- 237. 0, 92. 00]
856          [- 219. 3, 91. 50]
857          [- 202. 1, 91. 50]
858          [- 186. 0, 92. 00]
859          [- 129. 2, 92. 00]
860          [- 117. 6, 91. 50]
861          [- 100. 6, 91. 00]
862          [- 41. 5, 91. 00]
863          [- 20. 0, 91. 00]
864          [- 14. 0, 90. 54]
865          [ 14. 0, 90. 54]
866          [ 15. 3, 91. 00]
867          [ 17. 3, 91. 50]
868          [ 38. 4, 92. 00]
869          [ 39. 8, 92. 50]
870          [ 41. 1, 93. 00]
871 *%-----|-----
872 *#
873 *# Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
874 *#
875 ADD HYD          NHYDs um=[ "S_N5A" ] , NHYDs t o add=[ "N5A"+"SW_5A2"+"SW_5A1" ]
876 *%-----|-----
877 *#
878 *# Sum of hydrographs from Node 5A routed to Node 4
879 *# Section 8
880 *#
881 ROUTE CHANNEL    NHYDout =[ "N4" ] , NHYDi n=[ "S_N5A" ] ,
882                  RDT=[ 1 ] ( mi n) ,
883                  CHLGTH=[ 4630 ] ( m) ,   CHSLOPE=[ 0. 0432 ] ( % ) ,
884                                          FPSLOPE=[ 0. 0432 ] ( % ) ,
885                  SECNUM=[ 1. 0 ] ,       NSEG=[ 3 ]
886          ( SEGROUGH, SEGDI ST ( m) ) =
887          [ 0. 05, -28. 2
888          -0. 035, 28. 2
889          0. 05, 173. 1 ] NSEG t i mes
890          ( DI STANCE ( m) , ELEVATI ON ( m) ) =
891          [- 38. 9, 92. 00]
892          [- 35. 8, 91. 50]
893          [- 33. 3, 91. 00]
894          [- 28. 2, 90. 50]
895          [- 15. 0, 87. 48]
896          [- 5. 0, 88. 34]
897          [ 5. 0, 86. 20]
898          [ 15. 0, 88. 55]
899          [ 28. 2, 90. 50]
900          [ 29. 7, 91. 00]
901          [ 46. 5, 91. 00]
902          [ 127. 8, 91. 00]
903          [ 148. 7, 91. 50]
904          [ 173. 1, 92. 00]
905 *%-----|-----
906 *#
907 *# Addition of Subwatershed 4 and Leamy Creek to Node 4
908 *#
909 ADD HYD          NHYDs um=[ "S_N4" ] , NHYDs t o add=[ "N4"+"SW_4"+"LM_CK" ]
910 SAVE HYD        NHYD=[ "S_N4" ] , # OF PCYCLES=[ - 1 ] , I CASEs h=[ 1 ]
911                HYD_COMMENT=[ "f l ow at S_N4" ]
912 *%-----|-----
913 *#
914 *# Sum of hydrographs from Node 4 routed to Node 2
915 *# Section 9
916 *#
917 ROUTE CHANNEL    NHYDout =[ "N2" ] , NHYDi n=[ "S_N4" ] ,

```

```

918 RDT=[ 1] ( mi n),
919 CHLGTH=[ 1667] ( m), CHSLOPE=[ 0. 0600] ( %),
920 FPSLOPE=[ 0. 0600] ( %),
921 SECNUM=[ 1. 0], NSEG=[ 4]
922 ( SEGROUGH, SEGDI ST ( m))=
923 [ 0. 1, - 28. 0
924 - 0. 04, 28. 4
925 0. 06, 31. 7
926 0. 04, 80. 2] NSEG t i m e s
927 ( DI STANCE ( m), ELEVATI ON ( m))=
928 [- 36. 3, 92. 00]
929 [- 32. 6, 91. 50]
930 [- 30. 2, 91. 00]
931 [- 28. 0, 90. 45]
932 [- 15. 0, 87. 48]
933 [- 5. 0, 88. 34]
934 [ 5. 0, 86. 20]
935 [ 15. 0, 88. 55]
936 [ 28. 0, 90. 45]
937 [ 28. 4, 90. 50]
938 [ 30. 4, 91. 00]
939 [ 31. 7, 91. 50]
940 [ 80. 2, 92. 00]
941 *%-----|-----|
942 *#
943 *# Addition of Subwatershed 2 with Monohan Drain and Smith Drain to Node 2
944 *#
945 ADD HYD NHYDs um=[ "S_N2"], NHYDs t o add=[ "N2"+"SW_2"+"SM_DR"+"MO_DR"]
946 *%-----|-----|
947 SAVE HYD NHYD=[ "S_N2"], # OF PCYCLES=[ - 1], I CAS E s h=[ - 1]
948 HYD_FI L E N A M E=[ "H_S_N2"]
949 HYD_C O M M E N T=[ "f l o w a t S_N2 J o c k R i v e r G a u g e a t M o d i e D r. "]
950 *%-----|-----|
951 *#
952 *# Sum of hydrographs from Node 2 routed to Node 1
953 *# Section 10
954 *#
955 *#*****
956 *%READ HYD NHYD=[ "S_N2"],
957 *% HYD_FI L E N A M E=[ "H_S_N2"]
958 *%-----|-----|
959 *#
960 *# Hydrograph from Node 2 routed to Node 416
961 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 9025
962 *#
963 ROUTE CHANNEL NHYDout =["N_416"] , NHYDin=["S_N2"] ,
964 RDT=[ 1] ( mi n),
965 CHLGTH=[ 2327] ( m), CHSLOPE=[ 0. 0498] ( %),
966 FPSLOPE=[ 0. 0498] ( %),
967 SECNUM=[ 1. 0], NSEG=[ 3]
968 ( SEGROUGH, SEGDI ST ( m))=
969 [ 0. 075, - 23. 96
970 - 0. 055, 23. 96
971 0. 075, 157. 38] NSEG t i m e s
972 ( DI STANCE ( m), ELEVATI ON ( m))=
973 [- 336. 97, 93. 5]
974 [- 318. 85, 93]
975 [- 259, 92. 5]
976 [- 133. 18, 92]
977 [- 33. 17, 92]
978 [- 27. 21, 92]
979 [- 26. 14, 91. 5]
980 [- 24. 99, 91]
981 [- 23. 96, 90. 5]
982 [- 14. 33, 88. 26]
983 [- 0. 68, 88. 12]

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984 [ 14. 33, 88. 26]
985 [ 23. 96, 90. 5]
986 [ 32. 12, 91]
987 [ 43. 74, 91. 5]
988 [ 57. 09, 92]
989 [ 73. 53, 92. 5]
990 [ 108. 27, 93]
991 [ 125. 88, 93. 5]
992 [ 144. 81, 94]
993 [ 157. 38, 94. 5]
994 *%-----|-----|
995 *#*****|*****|
996 *# Catchment SW1a
997 *# - Portion of RVCA catchment SW1 outside of Reach 1 subwatershed
998 *# - Undeveloped agricultural land
999 *#*****|*****|
1000 CONTINUOUS NASHYD NHYD=[ "SW_1a" ], DT=[ 1] mi n, AREA=[ 536. 42] ( ha ),
1001 DWF=[ 0] ( cms ), CN C=[ 72], I A=[ 4. 67] ( mm ),
1002 N=[ 3], TP=[ 2. 79] hr s,
1003 Continuous simulation parameters:
1004 I aRECper=[ 4] ( hr s ),
1005 SM N=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
1006 I nterEventTi me=[ 12] ( hr s)
1007 Baseflow simulation parameters:
1008 BaseFlowOpt ion=[ 1] ,
1009 I n i t GWR es Vol =[ 50] ( mm ), GWR es K=[ 0. 96] ( mm / day / mm)
1010 VHydCond=[ 0. 055] ( mm / hr ), END=- 1
1011 *%-----|-----|
1012 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
1013 CONTINUOUS STANDHYD NHYD=[ "S-1-Okeefe" ], DT=[ 1] ( mi n), AREA=[ 44. 93] ( ha ), XI MP=[ 0. 65],
TI MP=[ 0. 65], DWF=[ 0] ( cms ),
1014 LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
I Aper=[ 4. 67] ( mm ), SLPP=[ 2. 0] ( % ),
1015 LGP=[ 40] ( m ), MNP=[ 0. 25], SCP=[ 0] ( mi n), Impervious surfaces:
I Ai mp=[ 1. 57] ( mm ), SLPI=[ 0. 75] ( % ),
1016 LGI =[ 547. 296] ( m ), MNI =[ 0. 013], SCI =[ 0] ( mi n),
1017 Continuous simulation parameters:
1018 I aRECper=[ 4] ( hr s), I aRECI mp=[ 4] ( hr s),
1019 SM N=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
1020 I nterEventTi me=[ 12] ( hr s), END=- 1
1021 *%-----|-----|
1022 COMPUTE DUALHYD NHYDin=[ "S-1-Okeefe" ], CI NLET=[ 4. 591] ( cms ), NI NLET=[ 1],
1023 M i j NHYD=[ "S-1-OkM" ]
1024 M nNHYD=[ "S-1-OkMN" ]
1025 TM I STO=[ 9999999] ( cu - m)
1026 *%-----|-----|
1027 ADD HYD NHYDsum=[ "S-1-OkS" ], NHYDs to add=[ "S-1-OkM" +"S-1-OkMN" ]
1028 *%-----|-----|
1029 ROUTE RESERVOIR NHYDout=[ "S-1-OkSR" ], NHYDin=[ "S-1-OkS" ],
1030 RDT=[ 1] ( mi n),
1031 TABLE of ( OUTFLOW STORAGE ) values
1032 ( cms ) - ( ha - m)
1033 [ 0. 0 , 0. 0 ]
1034 [ 0. 5370, 1. 7917 ]
1035 [ - 1 , - 1 ] (max twenty pts)
1036 NHYDovf=[ "S-1-OkSovf" ]
1037 *%-----|-----|
1038 ADD HYD NHYDsum=[ "SN_416" ], NHYDs to
add=[ "N_416" +"SW_1a" +"S-1-OkSR" +"S-1-OkSovf" ]
1039 *%-----|-----|
1040 SAVE HYD NHYD=[ "SN_416" ], # OF PCYCLES=[ - 1], I CASEsh=[ 1]
1041 HYD_COMMENT=[ "Total Flows at Highway 416 before Station 7245" ]
1042 *%-----|-----|
1043 *#
1044 *# Hydrograph from Node 416 routed to Node at Okeefe drain

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1045 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 7245
1046 *#
1047 ROUTE CHANNEL      NHYDout=["N_OK"] , NHYDin=["SN_416"] ,
1048                    RDT=[1](min),
1049                    CHLGTH=[497](m),   CHSLOPE=[0.3018](%),
1050                    FPSLOPE=[0.3018](%),
1051                    SECNUM=[1.0],      NSEG=[3]
1052                    ( SEGROUGH, SEGDI ST (m))=
1053                    [0.075, -19.40
1054                    -0.055, 19.40
1055                    0.075, 377.02] NSEG times
1056                    ( DI STANCE (m), ELEVATI ON (m))=
1057                    [-1061.41, 92.50]
1058                    [-945.91, 92.00]
1059                    [-783.64, 91.50]
1060                    [-136.74, 91.00]
1061                    [-86.04, 91.00]
1062                    [-20.86, 91.00]
1063                    [-20.18, 90.50]
1064                    [-19.40, 90.00]
1065                    [-11.68, 86.89]
1066                    [0.00, 86.10]
1067                    [12.09, 86.81]
1068                    [19.40, 90.00]
1069                    [34.68, 90.50]
1070                    [60.56, 91.00]
1071                    [170.14, 91.00]
1072                    [175.05, 90.50]
1073                    [180.29, 90.00]
1074                    [193.41, 90.00]
1075                    [195.98, 90.50]
1076                    [377.02, 92.50]
1077 *%-----|-----|
1078 *#*****|*****|
1079 *#      Catchment OKEEFE
1080 *#      - To O'Keefe drain (north of the Jock)
1081 *#      - Developed with assumed 43% imp.
1082 *#      - 2020-12-01 add Okeefe model (Area 513.02 HA) instead of current Okeefe (Area
1083 *#      - 2020-11-20 Okeefe detailed model was added as per the NOVATECH SWWHYMD model
1084 *#      - (Citi-Gate 2014).
1085 *%-----|-----|
1086 *#*****|*****|
1087 *#*****|*****|
1088 CONTINUOUS NASHYD  NHYD=["O-1R"], DT=[1]min, AREA=[63.72](ha),
1089                    DWF=[0](cms), CNVC=[61], IA=[6.2](mm), N=[3], TP=[.9]hrs,
1090                    Continuous simulation parameters:
1091                    IaRECPER=[4](hrs),
1092                    SMIN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1093                    InterEventTime=[12](hrs)
1094                    Baseflow simulation parameters:
1095                    BaseFlowOption=[1],
1096                    InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
1097                    VHydCond=[0.055](mm/hr), END=-1
1098 *%-----|-----|
1099 *#*****|*****|
1100 *#*****|*****|
1101 ROUTE CHANNEL      NHYDout=["O-1R"], NHYDin=["O-1"], RDT=[1](min),
1102                    CHLGTH=[960](m), CHSLOPE=[0.63](%), FPSLOPE=[0.63](%),
1103                    SECNUM=[1], NSEG=[3]
1104                    ( SEGROUGH, SEGDI ST (m))=[0.06, 4 -.043, 6 0.06, 10] NSEG times
1105                    ( DI STANCE (m), ELEVATI ON (m))=[0.00, 2.0]
1106                    [0.0, 2.0]
1107                    [4.0, 0.0]
1108                    [6.0, 0.0]

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1108 [ 10.0, 2.0]
1109 *%-----|-----|
1110 CONTINUOUS NASHYD NHYD=["O-2"], DT=[1] min, AREA=[28.61] (ha),
1111 DWF=[0] (cms), CN C=[57], IA=[5.2] (mm), N=[3], TP=[1.1] hrs,
1112 Continuous simulation parameters:
1113 IaRECPper=[4] (hrs),
1114 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1115 InterEventTime=[12] (hrs)
1116 Baseflow simulation parameters:
1117 BaseFlowOption=[1],
1118 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
1119 VHydCond=[0.055] (mm/hr), END=-1
1120 *%-----|-----|
1121 CONTINUOUS NASHYD NHYD=["O-4"], DT=[1] min, AREA=[46.94] (ha),
1122 DWF=[0] (cms), CN C=[49], IA=[9.2] (mm), N=[3], TP=[0.9] hrs,
1123 Continuous simulation parameters:
1124 IaRECPper=[4] (hrs),
1125 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1126 InterEventTime=[12] (hrs)
1127 Baseflow simulation parameters:
1128 BaseFlowOption=[1],
1129 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
1130 VHydCond=[0.055] (mm/hr), END=-1
1131 *%-----|-----|
1132 *TOTAL EXTERNAL FLOW NORTH OF O'KEEFE CT. CROSSING
1133 ADD HYD NHYDsum=["OKF-N"], NHYDstoadd=["O-1R"+"O-2"+"O-4"]
1134 *%-----|-----|
1135 *ROUTE FLOW THROUGH AREA O-6
1136 ROUTE CHANNEL ROUTE CHANNEL NHYDout=["OKF-NR"], NHYDin=["OKF-N"], RDT=[1] (min),
1137 CHLGTH=[210] (m), CHSLOPE=[.81] (%), FPSLOPE=[.81] (%),
1138 SECNUM=[1], NSEG=[3]
1139 (SEGROUGH, SEGDIST (m))=[0.043, 22.43 - 0.043, 25.07
1140 0.043, 45.54] NSEG times
1141 (DISTANCE (m), ELEVATION (m))=[0.00, 3.73]
1142 (14.62, 1.56)
1143 (18.41, 1.44)
1144 (22.43, 0.00)
1145 (25.07, 0.70)
1146 (29.10, 1.79)
1147 (33.73, 2.71)
1148 (45.54, 3.58)
1149 *%-----|-----|
1150 CONTINUOUS NASHYD NHYD=["O-6"], DT=[1] min, AREA=[16.46] (ha),
1151 DWF=[0] (cms), CN C=[43], IA=[9.2] (mm), N=[3], TP=[0.7] hrs,
1152 Continuous simulation parameters:
1153 IaRECPper=[4] (hrs),
1154 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1155 InterEventTime=[12] (hrs)
1156 Baseflow simulation parameters:
1157 BaseFlowOption=[1],
1158 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
1159 VHydCond=[0.055] (mm/hr), END=-1
1160 *%-----|-----|
1161 CONTINUOUS STANDHYD NHYD=["O-3"], DT=[1] (min), AREA=[39.67] (ha), XI MP=[0.15],
1162 TI MP=[0.30], DWF=[0] (cms),
1163 LOSS=[2], SCS curve number CN=[50], Pervious surfaces:
1164 IAper=[4.67] (mm), SLPP=[0.32] (%),
1165 LGP=[440] (m), MNP=[0.035], SCP=[0] (min), Impervious surfaces:
1166 IAimp=[1.57] (mm), SLPI=[0.32] (%),
1167 LGI=[1880] (m), MNI=[0.013], SCI=[0] (min),
1168 Continuous simulation parameters:
1169 IaRECPper=[4] (hrs), IaRECImp=[4] (hrs),
1170 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1171 InterEventTime=[12] (hrs), END=-1
1172 *%-----|-----|
1173 CONTINUOUS STANDHYD NHYD=["O-5"], DT=[1] (min), AREA=[60.63] (ha), XI MP=[0.13],

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1171  T I M P=[ 0. 26] ,  D W F=[ 0] ( c m s ) ,
1172      L O S S=[ 2] ,  S C S  c u r v e  n u m b e r  C N=[ 61] ,
1173      P e r v i o u s  s u r f a c e s :  I A p e r=[ 4. 67] ( m m ) ,  S L P P=[ 1. 38] ( % ) ,
      L G P=[ 550] ( m ) ,  M N P=[ 0. 035] ,  S C P=[ 0] ( m i n ) ,  I m p e r v i o u s  s u r f a c e s :
1174      I A i m p=[ 1. 57] ( m m ) ,  S L P I=[ 1. 38] ( % ) ,
1175      L G I=[ 1450] ( m ) ,  M N I=[ 0. 013] ,  S C I=[ 0] ( m i n ) ,
1176      C o n t i n u o u s  s i m u l a t i o n  p a r a m e t e r s :
1177      I a R E C p e r=[ 4] ( h r s ) ,  I a R E C i m p=[ 4] ( h r s ) ,
1178      S M N=[ - 1] ( m m ) ,  S M A X=[ - 1] ( m m ) ,  S K=[ 0. 010] / ( m m ) ,
1179      I n t e r E v e n t T i m e=[ 12] ( h r s ) ,  E N D=- 1
1180  *%-----|-----|
1181  *TOTAL EXTERNAL FLOWS WEST OF THE SITE AND NORTH OF O'KEEFE CRT
1182  *%-----|-----|
1183  ADD HYD      N H Y D s u m=[ " P T 1 " ] ,  N H Y D s  t o  a d d=[ " O K F - N R " + " O - 3 " + " O - 5 " + " O - 6 " ]
1184  CONTINUOUS NASHYD  N H Y D=[ " O - 7 " ] ,  D T=[ 1] m i n ,  A R E A=[ 5. 28] ( h a ) ,
1185      D W F=[ 0] ( c m s ) ,  C N V C=[ 54] ,  I A=[ 7. 5] ( m m ) ,  N=[ 3] ,  T P=[ 0. 6] h r s ,
1186      C o n t i n u o u s  s i m u l a t i o n  p a r a m e t e r s :
1187      I a R E C p e r=[ 4] ( h r s ) ,
1188      S M N=[ - 1] ( m m ) ,  S M A X=[ - 1] ( m m ) ,  S K=[ 0. 010] / ( m m ) ,
1189      I n t e r E v e n t T i m e=[ 12] ( h r s )
1190      B a s e f l o w  s i m u l a t i o n  p a r a m e t e r s :
1191      B a s e F l o w O p t i o n=[ 1] ,
1192      I n i t G W R e s V o l=[ 50] ( m m ) ,  G W R e s K=[ 0. 96] ( m m / d a y / m m )
1193      V H y d C o n d=[ 0. 055] ( m m / h r ) ,  E N D=- 1
1194  *%-----|-----|
1195  *ANALYSIS POINT 1 - TOTAL FLOW NORTH OF FALLOWFIELD DR. AND O'KEEFE CRT.
1196  ADD HYD      N H Y D s u m=[ " F F " ] ,  N H Y D s  t o  a d d=[ " P T 1 " + " O - 7 " ]
1197  *%-----|-----|
1198  *ROUTE FLOW through O'Keefe Drain 1
1199  ROUTE CHANNEL  N H Y D o u t=[ " D R A I N 1 " ] ,  N H Y D i n=[ " F F " ] ,  R D T=[ 1] ( m i n ) ,
1200      C H L G T H=[ 302] { m } ,  C H S L O P E=[ 1. 00] ( % ) ,  F P S L O P E=[ 1. 00] ( % ) ,
1201      S E C N U M=[ 1] ,  N S E G=[ 3]
1202      ( S E G R O U G H ,  S E G D I S T ( m ) )=[ 0. 07 , 13. 45 - 0. 043 , 16. 55 0. 07 , 30. 00] N S E G
      t i m e s
1203      ( D I S T A N C E ( m ) ,  E L E V A T I O N ( m ) )=[ 0. 00 , 1. 70]
1204      ( 3. 45 , 0. 60)
1205      ( 13. 45 , 0. 50)
1206      ( 14. 45 , 0. 00)
1207      ( 15. 55 , 0. 00)
1208      ( 16. 55 , 0. 50)
1209      ( 26. 55 , 0. 60)
1210      ( 30. 00 , 1. 70)
1211  *%-----|-----|
1212  CONTINUOUS NASHYD  N H Y D=[ " D I " ] ,  D T=[ 1] m i n ,  A R E A=[ 1. 17] ( h a ) ,
1213      D W F=[ 0] ( c m s ) ,  C N V C=[ 84] ,  I A=[ 9. 0] ( m m ) ,  N=[ 3] ,  T P=[ 0. 28] h r s ,
1214      C o n t i n u o u s  s i m u l a t i o n  p a r a m e t e r s :
1215      I a R E C p e r=[ 4] ( h r s ) ,
1216      S M N=[ - 1] ( m m ) ,  S M A X=[ - 1] ( m m ) ,  S K=[ 0. 010] / ( m m ) ,
1217      I n t e r E v e n t T i m e=[ 12] ( h r s )
1218      B a s e f l o w  s i m u l a t i o n  p a r a m e t e r s :
1219      B a s e F l o w O p t i o n=[ 1] ,
1220      I n i t G W R e s V o l=[ 50] ( m m ) ,  G W R e s K=[ 0. 96] ( m m / d a y / m m )
1221      V H y d C o n d=[ 0. 055] ( m m / h r ) ,  E N D=- 1
1222  *%-----|-----|
1223  CONTINUOUS STANDHYD  N H Y D=[ " A I " ] ,  D T=[ 1] m i n ,  A R E A=[ 2. 50] ( h a ) ,  X I M P=[ 0. 68] ,  T I M P=[ 0. 85] ,
      D W F=[ 0] ( c m s ) ,  L O S S=[ 1] :
1224      H o r t o n :  F o=[ 76. 20] ( m m / h r ) ,  F c=[ 13. 20] ( m m / h r ) ,  D C A Y=[ 4. 14] ( / h r ) ,
      F=[ 0. 00] ( m m ) ,
1225      P e r v i o u s  a r e a s :  I A p e r=[ 4. 67] ( m m ) ,  S L P P=[ 0. 5] ( % ) ,  L G P=[ 50] ( m ) ,
      M N P=[ 0. 250] ,  S C P=[ 0] ( m i n ) ,
1226      I m p e r v i o u s  a r e a s :  I A i m p=[ 1. 57] ( m m ) ,  S L P I=[ 0. 5] ( % ) ,
      L G I=[ 223. 607] ( m ) ,  M N I=[ 0. 013] ,  S C I=[ 0] ( m i n ) ,
1227      C o n t i n u o u s  s i m u l a t i o n  p a r a m e t e r s :
1228      I a R E C p e r=[ 4] ( h r s ) ,  I a R E C i m p=[ 4] ( h r s ) ,  I n t e r E v e n t T i m e=[ 12] ( h r s ) ,
      E N D=- 1

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1229 *%-----|-----|
1230 ROUTE RESERVOIR NHYDout=[ "A1-STR"], NHYDin=[ "A1"], RDT=[ 1](mi n),
1231          TABLE of ( OUTFLOW STORAGE ) values
1232          ( cms ) - ( ha- m)
1233          [ 0.000 , 0.000 ]
1234          [ 0.035 , 0.038 ]
1235          [ 0.072 , 0.051 ]
1236          [ 0.100 , 0.059 ]
1237          [ 0.125 , 0.070 ]
1238          [ 0.160 , 0.074 ]
1239          [ 0.185 , 0.081 ]
1240          [ -1 , -1 ] (max t went y pt s)
1241          NHYDovf=[ "A1-OVF" ]
1242 *%-----|-----|
1243 CONTINUOUS STANDHYD NHYD=[ "ST-2"], DT=[ 1]mi n, AREA=[ 0.59](ha), XI MP=[ 0.46],
1244 TIMP=[ 0.57], DWF=[ 0](cms), LOSS=[ 1]:
1245 Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1246          F=[ 0.00](mm),
1247          Pervious areas: I A per=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1248          MNP=[ 0.250], SCP=[ 0](mi n),
1249          Imper vious areas: I A i mp=[ 1.57](mm), SLPI=[ 0.5](%),
1250          LGI=[ 108.628](m), MNI=[ 0.013], SCI=[ 0](mi n),
1251          Continuous simulation parameters:
1252          I a REC per=[ 4](hr s), I a REC i mp=[ 4](hr s), I nter Event Ti me=[ 12](hr s),
1253          END=- 1
1254 *%-----|-----|
1255 ROUTE RESERVOIR NHYDout=[ "ST2STR"], NHYDin=[ "ST-2"], RDT=[ 1](mi n),
1256          TABLE of ( OUTFLOW STORAGE ) values
1257          ( cms ) - ( ha- m)
1258          [ 0.000 , 0.0000 ]
1259          [ 0.052 , 0.0010 ]
1260          [ 0.053 , 0.0080 ]
1261          [ -1 , -1 ] (max t went y pt s)
1262          NHYDovf=[ "ST2OVF" ]
1263 *%-----|-----|
1264 *%-----|-----|
1265 *TOTAL FLOW NORTH OF STRANDHERD DR. (EAST BRANCH) CROSSING
1266 *%-----|-----|
1267 CONTINUOUS NASHYD NHYD=[ "O-8"], DT=[ 1]mi n, AREA=[ 60.55](ha),
1268 DWF=[ 0](cms), CNV C=[ 69], I A=[ 4.0](mm), N=[ 3], TP=[ 1.0]hr s,
1269 Continuous simulation parameters:
1270 I a REC per=[ 4](hr s),
1271 SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1272 Inter Event Ti me=[ 12](hr s)
1273 Baseflow simulation parameters:
1274 BaseFl owOpt ion=[ 1] ,
1275 I n i t GWRes Vol=[ 50](mm), GWRes K=[ 0.96](mm/ day/ mm)
1276 VHydCond=[ 0.055](mm/ hr), END=- 1
1277 *%-----|-----|
1278 ROUTE PIPE PTYPE=[ 2]rect, NHYDout=[ "O8PIPE"], RNUMBER=[ 1], PWDTH=[ 1800](mm),
1279 PHEIGHT=[ 1200](mm), PLNGTH=[ 335.1](m),
1280 PROUGH=[ 0.013], PSLOPE=[ 0.001](m/ m), NHYDin=[ "O-8"], RDT=[ 1](mi n)
1281 *%-----|-----|
1282 *%-----|-----|
1283 ADD HYD NHYDs um=[ "ST2-IN"], NHYDs to
1284 add=[ "DRAIN1"+"D1"+"A1-STR"+"A1-OVF"+"ST2STR"+"ST2OVF"+"O8PIPE" ]
1285 *%-----|-----|
1286 CONTINUOUS STANDHYD NHYD=[ "A7"], DT=[ 1]mi n, AREA=[ 3.51](ha), XI MP=[ 0.68], TIMP=[ 0.85],
1287 DWF=[ 0](cms), LOSS=[ 1]:
1288 Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1289          F=[ 0.00](mm),
1290          Pervious areas: I A per=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1291          MNP=[ 0.250], SCP=[ 0](mi n),
1292          Imper vious areas: I A i mp=[ 1.57](mm), SLPI=[ 0.5](%),
1293          LGI=[ 264.953](m), MNI=[ 0.013], SCI=[ 0](mi n),
1294          Continuous simulation parameters:

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1284 IaRECPper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
END=- 1
1285 *%-----|-----|
1286 ROUTE RESERVOIR NHYDout=["A7-STR"], NHYDin=["A7"], RDT=[ 1](min),
1287 TABLE of ( OUTFLOW STORAGE ) values
1288 (cms) - (ha-m)
1289 [ 0.000 , 0.000 ]
1290 [ 0.049 , 0.054 ]
1291 [ 0.102 , 0.072 ]
1292 [ 0.140 , 0.082 ]
1293 [ 0.175 , 0.099 ]
1294 [ 0.225 , 0.105 ]
1295 [ 0.260 , 0.114 ]
1296 [ -1 , -1 ] (max twenty pts)
1297 NHYDovf=["A7-OVF"]
1298 *%-----|-----|
1299 CONTINUOUS STANDHYD NHYD=["ST-3"], DT=[ 1]min, AREA=[ 0.71](ha), XI MP=[ 0.46],
TIMP=[ 0.57], DWF=[ 0](cms), LOSS=[ 1]:
1300 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
1301 Pervious areas: IAper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
MNP=[ 0.250], SCP=[ 0](min),
1302 Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 0.5](%),
LGI=[ 119.164](m), MNI=[ 0.013], SCI=[ 0](min),
1303 Continuous simulation parameters:
1304 IaRECPper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
END=- 1
1305 *%-----|-----|
1306 ROUTE RESERVOIR NHYDout=["ST3STR"], NHYDin=["ST-3"], RDT=[ 1](min),
1307 TABLE of ( OUTFLOW STORAGE ) values
1308 (cms) - (ha-m)
1309 [ 0.000 , 0.0000 ]
1310 [ 0.063 , 0.0010 ]
1311 [ 0.064 , 0.0094 ]
1312 [ -1 , -1 ] (max twenty pts)
1313 NHYDovf=["ST3OVF"]
1314 *%-----|-----|
1315 *ANALYSIS POINT 2 - TOTAL FLOW AT OUTLET OF STREET 2/3 INTERSECTION
1316 *%-----|-----|
1317 ADD HYD NHYDsum=["PT2ST3"], NHYDsto
add=["ST2-IN"+"A7-STR"+"A7-OVF"+"ST3STR"+"ST3OVF"]
1318 *%-----|-----|
1319 *ROUTE FLOW through O Keefe Drain 2
1320 ROUTE CHANNEL NHYDout=["DRAIN2"], NHYDin=["PT2ST3"], RDT=[ 1](min),
1321 CHLGTH=[ 592](m), CHSLOPE=[.23](%), FPSLOPE=[.23](%),
1322 SECNUM=[ 1], NSEG=[ 3]
1323 ( SEGROUGH, SEGDIST (m))=[ 0.07, 12.60 -0.043, 17.40 0.07, 30.00] NSEG
times
1324 ( DISTANCE (m), ELEVATION (m))=[ 0.00, 1.70]
1325 ( 2.60, 0.95)
1326 ( 12.60, 0.75)
1327 ( 14.10, 0.00)
1328 ( 15.90, 0.00)
1329 ( 17.40, 0.75)
1330 ( 27.40, 0.95)
1331 ( 30.00, 1.70)
1332 *%-----|-----|
1333 CONTINUOUS NASHYD NHYD=["D2"], DT=[ 1]min, AREA=[ 2.28](ha), DWF=[ 0](cms), CNVC=[ 84],
IA=[ 9.0](mm),
1334 N=[ 3], TP=[ 0.99]hrs,
1335 Continuous simulation parameters:
1336 IaRECPper=[ 4](hrs),
1337 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1338 InterEventTime=[ 12](hrs)
1339 Baseflow simulation parameters:
1340 BaseFlowOption=[ 1],

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1341      In i t GWRes Vol =[ 50] ( mm) , GWRes K=[ 0. 96] ( mm/ day/ mm)
1342      VHydCond=[ 0. 055] ( mm/ hr) ,      END=- 1
1343 *%-----|-----|
1344 CONTI NUOUS STANDHYD NHYD=[ " A17" ] , DT=[ 1] mi n , AREA=[ 12. 04] ( ha) , XI MP=[ 0. 68] ,
TI MP=[ 0. 85] , DWF=[ 0] ( cms) , LOSS=[ 1] :
1345      Hort on: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] (/ hr) ,
F=[ 0. 00] ( mm) ,
1346      Per vious areas: I Aper =[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1347      Impervious areas: I Ai mp=[ 1. 57] ( mm) , SLPI =[ 0. 5] ( % ) ,
LGI =[ 490. 714] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1348      Conti nuous si mul ati on paramet ers:
1349      IaRECper=[ 4] ( hrs) , IaRECI mp=[ 4] ( hrs) , Int er Event Ti me=[ 12] ( hrs) ,
END=- 1
1350 *%-----|-----|
1351 ROUTE RESERVOI R      NHYDout =[ " A17STR" ] , NHYDi n=[ " A17" ] , RDT=[ 1] ( mi n) ,
1352      TABLE of ( OUTFLOW STORAGE ) values
1353      ( cms) - ( ha- m)
1354      [ 0. 000 , 0. 000 ]
1355      [ 0. 169 , 0. 185 ]
1356      [ 0. 349 , 0. 248 ]
1357      [ 0. 482 , 0. 283 ]
1358      [ 0. 602 , 0. 338 ]
1359      [ 0. 771 , 0. 359 ]
1360      [ 0. 891 , 0. 391 ]
1361      [ - 1 , - 1 ] ( max t went y pt s)
1362      NHYDovf=[ " A17OVF" ]
1363 *%-----|-----|
1364 CONTI NUOUS STANDHYD NHYD=[ " ST- 4" ] , DT=[ 1] mi n , AREA=[ 0. 35] ( ha) , XI MP=[ 0. 46] ,
TI MP=[ 0. 57] , DWF=[ 0] ( cms) , LOSS=[ 1] :
1365      Hort on: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] (/ hr) ,
F=[ 0. 00] ( mm) ,
1366      Per vious areas: I Aper =[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1367      Impervious areas: I Ai mp=[ 1. 57] ( mm) , SLPI =[ 0. 5] ( % ) , LGI =[ 83. 666] ( m) ,
MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1368      Conti nuous si mul ati on paramet ers:
1369      IaRECper=[ 4] ( hrs) , IaRECI mp=[ 4] ( hrs) , Int er Event Ti me=[ 12] ( hrs) ,
END=- 1
1370 *%-----|-----|
1371 ROUTE RESERVOI R      NHYDout =[ " ST4STR" ] , NHYDi n=[ " ST- 4" ] , RDT=[ 1] ( mi n) ,
1372      TABLE of ( OUTFLOW STORAGE ) values
1373      ( cms) - ( ha- m)
1374      [ 0. 000 , 0. 0000 ]
1375      [ 0. 031 , 0. 0010 ]
1376      [ 0. 032 , 0. 0050 ]
1377      [ - 1 , - 1 ] ( max t went y pt s)
1378      NHYDovf=[ " ST4OVF" ]
1379 *%-----|-----|
1380 CONTI NUOUS STANDHYD NHYD=[ " A18" ] , DT=[ 1] mi n , AREA=[ 5. 30] ( ha) , XI MP=[ 0. 68] , TI MP=[ 0. 85] ,
DWF=[ 0] ( cms) , LOSS=[ 1] :
1381      Hort on: Fo=[ 76. 20] ( mm/ hr) , Fc=[ 13. 20] ( mm/ hr) , DCAY=[ 4. 14] (/ hr) ,
F=[ 0. 00] ( mm) ,
1382      Per vious areas: I Aper =[ 4. 67] ( mm) , SLPP=[ 0. 5] ( % ) , LGP=[ 50] ( m) ,
MNP=[ 0. 250] , SCP=[ 0] ( mi n) ,
1383      Impervious areas: I Ai mp=[ 1. 57] ( mm) , SLPI =[ 0. 5] ( % ) ,
LGI =[ 325. 576] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
1384      Conti nuous si mul ati on paramet ers:
1385      IaRECper=[ 4] ( hrs) , IaRECI mp=[ 4] ( hrs) , Int er Event Ti me=[ 12] ( hrs) ,
END=- 1
1386 *%-----|-----|
1387 ROUTE RESERVOI R      NHYDout =[ " A18STR" ] , NHYDi n=[ " A18" ] , RDT=[ 1] ( mi n) ,
1388      TABLE of ( OUTFLOW STORAGE ) values
1389      ( cms) - ( ha- m)
1390      [ 0. 000 , 0. 000 ]
1391      [ 0. 074 , 0. 082 ]

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1392         [ 0.154 , 0.109 ]
1393         [ 0.212 , 0.125 ]
1394         [ 0.265 , 0.149 ]
1395         [ 0.339 , 0.158 ]
1396         [ 0.392 , 0.172 ]
1397         [ -1 , -1 ] (max twenty pts)
1398         NHYDovf=[ "A18OVF" ]
1399 *%-----|-----|
1400 *ANALYSIS POINT 3 - TOTAL FLOW AT OUTLET OF STREET 4
1401 *%-----|-----|
1402 ADD HYD          NHYDs um=[ "PT3ST4" ], NHYDs to
add=[ "DRAIN2"+"D2"+"A17STR"+"A17OVF"+"ST4STR"+"ST4OVF"+"A18STR"+"A18OVF" ]
1403 *%-----|-----|
1404 *ROUTE FLOW through O Keefe Drain 3
1405 ROUTE CHANNEL   NHYDout=[ "DRAIN3" ], NHYDin=[ "PT3ST4" ], RDT=[ 1 ] (min),
1406                CHLGTH=[ 525 ] (m), CHSLOPE=[ .23 ] (%), FPSLOPE=[ .23 ] (%),
1407                SECNUM=[ 1 ], NSEG=[ 3 ]
1408                ( SEGROUGH, SEGDIST (m) )=[ 0.07, 12.50 -0.043, 17.50 0.07, 30.00 ] NSEG
times
1409                ( DISTANCE (m), ELEVATION (m) )=[ 0.00, 1.70 ]
1410                ( 2.50, 1.00 )
1411                ( 12.50, 0.80 )
1412                ( 14.10, 0.00 )
1413                ( 15.90, 0.00 )
1414                ( 17.50, 0.80 )
1415                ( 27.50, 1.00 )
1416                ( 30.00, 1.70 )
1417 *%-----|-----|
1418 CONTINUOUS NASHYD NHYD=[ "D3" ], DT=[ 1 ] min, AREA=[ 2.51 ] (ha),
1419                DWF=[ 0 ] (cms), CNVC=[ 86 ], IA=[ 8.7 ] (mm), N=[ 3 ], TP=[ 0.73 ] hrs,
1420                Continuous simulation parameters:
1421                IARECper=[ 4 ] (hrs),
1422                SMN=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
1423                InterEventTime=[ 12 ] (hrs)
1424                Baseflow simulation parameters:
1425                BaseFlowOption=[ 1 ],
1426                InitGWResVol=[ 50 ] (mm), GWResK=[ 0.96 ] (mm/day/mm)
1427                VHydCond=[ 0.055 ] (mm/hr), END=- 1
1428 *%-----|-----|
1429 CONTINUOUS STANDHYD NHYD=[ "C1" ], DT=[ 1 ] min, AREA=[ 3.41 ] (ha), XI MP=[ 0.68 ], TI MP=[ 0.85 ],
DWF=[ 0 ] (cms), LOSS=[ 1 ]:
1430                Horton: Fo=[ 76.20 ] (mm/hr), Fc=[ 13.20 ] (mm/hr), DCAY=[ 4.14 ] (/hr),
1431                F=[ 0.00 ] (mm),
1432                Pervious areas: IAPER=[ 4.67 ] (mm), SLPP=[ 0.5 ] (%), LGP=[ 50 ] (m),
MNP=[ 0.250 ], SCP=[ 0 ] (min),
1433                Impervious areas: IAI MP=[ 1.57 ] (mm), SLPI=[ 0.5 ] (%),
LGI=[ 261.151 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (min),
1434                Continuous simulation parameters:
IARECper=[ 4 ] (hrs), IARECI MP=[ 4 ] (hrs), InterEventTime=[ 12 ] (hrs),
END=- 1
1435 *%-----|-----|
1436 ROUTE RESERVOIR NHYDout=[ "C1-STR" ], NHYDin=[ "C1" ], RDT=[ 1 ] (min),
1437                TABLE of ( OUTFLOW STORAGE ) values
1438                ( cms ) - ( ha-m )
1439                [ 0.000 , 0.000 ]
1440                [ 0.048 , 0.052 ]
1441                [ 0.099 , 0.070 ]
1442                [ 0.136 , 0.080 ]
1443                [ 0.170 , 0.096 ]
1444                [ 0.218 , 0.102 ]
1445                [ 0.252 , 0.111 ]
1446                [ -1 , -1 ] (max twenty pts)
1447                NHYDovf=[ "C1-OVF" ]
1448 *%-----|-----|
1449 CONTINUOUS STANDHYD NHYD=[ "ST-5" ], DT=[ 1 ] min, AREA=[ 0.45 ] (ha), XI MP=[ 0.46 ],
TI MP=[ 0.57 ], DWF=[ 0 ] (cms), LOSS=[ 1 ]:

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1450      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1451      F=[ 0.00](mm),
1452      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1453      MNP=[ 0.250], SCP=[ 0](min),
1454      Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 0.5](%), LGI=[ 94.868](m),
1455      MNI=[ 0.013], SCI=[ 0](min),
1456      Continuous simulation parameters:
1457      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1458      END=- 1
1459
1460 *%-----|-----|
1461 ROUTE RESERVOIR      NHYDout=[ "ST5STR"], NHYDin=[ "ST-5"], RDT=[ 1](min),
1462      TABLE of ( OUTFLOW STORAGE ) values
1463      (cms) - (ha-m)
1464      [ 0.000 , 0.0000 ]
1465      [ 0.040 , 0.0010 ]
1466      [ 0.041 , 0.0062 ]
1467      [ -1 , -1 ] (max twenty pts)
1468      NHYDovf=[ "ST5OVF"]
1469
1470 *%-----|-----|
1471 ADD HYD              NHYDsum=[ "ST5-E"], NHYDs to
1472      add=[ "DRAIN3"+"D3"+"C1-STR"+"C1-OVF"+"ST5STR"+"ST5OVF"]
1473
1474 *%-----|-----|
1475 CONTINUOUS STANDHYD NHYD=[ "STRAND"], DT=[ 1](min), AREA=[ 7.59](ha),
1476      XI MP=[ 0.64], TI MP=[ 0.85], DWF=[ 0](cms), LOSS=[ 1]:
1477      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1478      F=[ 0.00](mm),
1479      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 40](m),
1480      MNP=[ 0.250], SCP=[ 0](min),
1481      Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 0.5](%), LGI=[ 1230](m),
1482      MNI=[ 0.013], SCI=[ 0](min),
1483      Continuous simulation parameters:
1484      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1485      END=- 1
1486
1487 *%-----|-----|
1488 ROUTE RESERVOIR      NHYDout=[ "S-POND"], NHYDin=[ "STRAND"], RDT=[ 1](min),
1489      TABLE of ( OUTFLOW STORAGE ) values
1490      (cms) - (ha-m)
1491      [ 0.000 , 0.000 ]
1492      [ 0.033 , 0.188 ]
1493      [ 0.057 , 0.253 ]
1494      [ 0.104 , 0.287 ]
1495      [ 0.160 , 0.336 ]
1496      [ 0.340 , 0.346 ]
1497      [ 0.471 , 0.360 ]
1498      [ 0.824 , 0.390 ]
1499      [ -1 , -1 ] (max twenty pts)
1500      NHYDovf=[ "S-OVF"]
1501
1502 *%-----|-----|
1503 ADD HYD              NHYDsum=[ "SSAOUT"], NHYDs to add=[ "ST5-E"+"S-POND"+"S-OVF"]
1504
1505 *%-----|-----|
1506 SAVE HYD             NHYD=[ "SSAOUT"], # OF PCYCLES=[ 5], ICASEsh=[ 1]
1507      HYD_COMMENT=[ "SSAOUT"]
1508
1509 *%-----|-----|
1510 CONTINUOUS STANDHYD NHYD=[ "Area-A"], DT=[ 1]min, AREA=[ 66.75](ha), XI MP=[ 0.64],
1511      TI MP=[ 0.80], DWF=[ 0](cms), LOSS=[ 1]:
1512      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
1513      F=[ 0.00](mm),
1514      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 0.5](%), LGP=[ 50](m),
1515      MNP=[ 0.250], SCP=[ 0](min),
1516      Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 0.5](%),
1517      LGI=[ 1155.422](m), MNI=[ 0.013], SCI=[ 0](min),
1518      Continuous simulation parameters:
1519      IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
1520      END=- 1
1521
1522 *%-----|-----|
1523 SAVE HYD             NHYD=[ "Area-A"], # OF PCYCLES=[ 1], ICASEsh=[ 1]

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1502 HYD_COMMENT=[ " SMMF- A Inflow" ]
1503 *%-----|-----|
1504 ROUTE RESERVOIR NHYDout=[ " SMMF- A" ], NHYDin=[ " Area- A" ], RDT=[ 1 ] (mi n),
1505 TABLE of ( OUTFLOW STORAGE ) values
1506 ( cms ) - ( ha- m)
1507 [ 0.000 , 0.000 ]
1508 [ 0.103 , 1.077 ]
1509 [ 0.128 , 1.749 ]
1510 [ 0.382 , 2.282 ]
1511 [ 0.703 , 2.582 ]
1512 [ 1.256 , 2.978 ]
1513 [ 1.567 , 3.202 ]
1514 [ 1.955 , 3.493 ]
1515 [ 2.100 , 3.600 ]
1516 [ -1 , -1 ] (max twenty pts)
1517 NHYDovf=[ " SWWAOV" ]
1518 *%-----|-----|
1519 SAVE HYD NHYD=[ " SMMF- A" ], # OF PCYCLES=[ 1 ], ICASEsh=[ 1 ]
1520 HYD_COMMENT=[ " SMMF- A Outflow" ]
1521 *%-----|-----|
1522 *ANALYSIS POINT 4 - TOTAL FLOW AT OUTLET OF STREET 5
1523 *%-----|-----|
1524 ADD HYD NHYDs um=[ " PT4ST5" ], NHYDs to add=[ " SSAOUT" + " SMMF- A" + " SWWAOV" ]
1525 *%-----|-----|
1526 CONTINUOUS STANDHYD NHYD=[ " C6" ], DT=[ 1 ] mi n, AREA=[ 1.87 ] (ha), XI MP=[ 0.68 ], TI MP=[ 0.85 ],
DWF=[ 0 ] (cms), LOSS=[ 1 ]:
1527 Horton: Fo=[ 76.20 ] (mm/ hr), Fc=[ 13.20 ] (mm/ hr), DCAY=[ 4.14 ] (/ hr),
F=[ 0.00 ] (mm),
1528 Pervious areas: IAper=[ 4.67 ] (mm), SLPP=[ 0.5 ] (%), LGP=[ 50 ] (m),
MNP=[ 0.250 ], SCP=[ 0 ] (mi n),
1529 Impervious areas: IAimp=[ 1.57 ] (mm), SLPI=[ 0.5 ] (%),
LGI=[ 193.391 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),
1530 Continuous simulation parameters:
1531 IARECper=[ 4 ] (hrs), IARECimp=[ 4 ] (hrs), InterEventTime=[ 12 ] (hrs),
END=- 1
1532 *%-----|-----|
1533 ROUTE RESERVOIR NHYDout=[ " C6- STR" ], NHYDin=[ " C6" ], RDT=[ 1 ] (mi n),
1534 TABLE of ( OUTFLOW STORAGE ) values
1535 ( cms ) - ( ha- m)
1536 [ 0.000 , 0.000 ]
1537 [ 0.026 , 0.029 ]
1538 [ 0.054 , 0.038 ]
1539 [ 0.075 , 0.044 ]
1540 [ 0.093 , 0.052 ]
1541 [ 0.120 , 0.056 ]
1542 [ 0.138 , 0.061 ]
1543 [ -1 , -1 ] (max twenty pts)
1544 NHYDovf=[ " C6- OVF" ]
1545 *%-----|-----|
1546 CONTINUOUS STANDHYD NHYD=[ " C7" ], DT=[ 1 ] mi n, AREA=[ 1.62 ] (ha), XI MP=[ 0.68 ], TI MP=[ 0.85 ],
DWF=[ 0 ] (cms), LOSS=[ 1 ]:
1547 Horton: Fo=[ 76.20 ] (mm/ hr), Fc=[ 13.20 ] (mm/ hr), DCAY=[ 4.14 ] (/ hr),
F=[ 0.00 ] (mm),
1548 Pervious areas: IAper=[ 4.67 ] (mm), SLPP=[ 0.5 ] (%), LGP=[ 50 ] (m),
MNP=[ 0.250 ], SCP=[ 0 ] (mi n),
1549 Impervious areas: IAimp=[ 1.57 ] (mm), SLPI=[ 0.5 ] (%),
LGI=[ 180.000 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),
1550 Continuous simulation parameters:
1551 IARECper=[ 4 ] (hrs), IARECimp=[ 4 ] (hrs), InterEventTime=[ 12 ] (hrs),
END=- 1
1552 *%-----|-----|
1553 ROUTE RESERVOIR NHYDout=[ " C7- STR" ], NHYDin=[ " C7" ], RDT=[ 1 ] (mi n),
1554 TABLE of ( OUTFLOW STORAGE ) values
1555 ( cms ) - ( ha- m)
1556 [ 0.000 , 0.000 ]
1557 [ 0.023 , 0.025 ]

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1558         [ 0.047 , 0.033 ]
1559         [ 0.065 , 0.038 ]
1560         [ 0.081 , 0.045 ]
1561         [ 0.104 , 0.048 ]
1562         [ 0.120 , 0.053 ]
1563         [ -1 , -1 ] (max twenty pts)
1564         NHYDovf=["C7-OVF"]
1565 *%-----|-----|
1566 CONTINUOUS STANDHYD NHYD=["ST-6"], DT=[1] min, AREA=[0.41](ha), XI MP=[0.46], TI MP=[0.57],
DWF=[0](cms), LOSS=[1]:
1567 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1568 Pervious areas: I A per=[4.67](mm), SLPP=[0.5](%), LGP=[50](m),
MNP=[0.250], SCP=[0](min),
1569 Impervious areas: I A i mp=[1.57](mm), SLPI=[0.5](%), LGI=[90.554](m),
MNI=[0.013], SCI=[0](min),
1570 Continuous simulation parameters:
1571 I a REC per=[4](hrs), I a REC i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
1572 *%-----|-----|
1573 ROUTE RESERVOIR NHYDout=["ST6STR"], NHYDin=["ST-6"], RDT=[1](min),
1574 TABLE of ( OUTFLOW STORAGE ) values
1575 (cms) - (ha-m)
1576 [ 0.000 , 0.0000 ]
1577 [ 0.036 , 0.0010 ]
1578 [ 0.037 , 0.0058 ]
1579 [ -1 , -1 ] (max twenty pts)
1580 NHYDovf=["ST6OVF"]
1581 *%-----|-----|
1582 *ANALYSIS POINT 5 - TOTAL FLOW AT OUTLET OF STREET 6
1583 *%-----|-----|
1584 ADD HYD NHYDsum=["PT5ST6"], NHYDs to
add=["PT4ST5"+"C6-STR"+"C6-OVF"+"C7-STR"+"C7-OVF"+"ST6STR"+"ST6OVF"]
1585 *%-----|-----|
1586 *ROUTE FLOW through O Keefe Drain 4
1587 ROUTE CHANNEL NHYDout=["DRAI N4"], NHYDin=["PT5ST6"], RDT=[1](min),
1588 CHLGTH=[324](m), CHSLOPE=[.10](%), FPSLOPE=[.10](%),
1589 SECNUM=[1], NSEG=[3]
1590 ( SEGROUGH, SEGDI ST (m))=[0.07, 12.00 -0.043, 18.00 0.07, 30.00] NSEG
t i m e s
1591 ( DI STANCE (m), ELEVATI ON (m))=[0.00, 2.00]
1592 (2.00, 1.20)
1593 (12.00, 1.00)
1594 (14.00, 0.00)
1595 (16.00, 0.00)
1596 (18.00, 1.00)
1597 (28.00, 1.20)
1598 (30.00, 2.00)
1599 *%-----|-----|
1600 CONTINUOUS NASHYD NHYD=["D4"], DT=[1] min, AREA=[1.73](ha), DWF=[0](cms), CNV C=[88],
I A=[8.4](mm),
1601 N=[3], TP=[0.60] hrs,
1602 Continuous simulation parameters:
1603 I a REC per=[4](hrs),
1604 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1605 I n t e r E v e n t T i m e=[12](hrs)
1606 Baseflow simulation parameters:
1607 BaseFlowOpt i on=[1] ,
1608 I n i t G W R e s V o l=[50](mm), G W R e s K=[0.96](mm/day/mm)
1609 VHydCond=[0.055](mm/hr), END=-1
1610 *%-----|-----|
1611 CONTINUOUS STANDHYD NHYD=["Area-B"], DT=[1] min, AREA=[24.04](ha), XI MP=[0.62],
TI MP=[0.77], DWF=[0](cms), LOSS=[1]:
1612 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
1613 Pervious areas: I A per=[4.67](mm), SLPP=[1.4](%), LGP=[50](m),

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1614 MNP=[ 0.250], SCP=[ 0](mi n),
Impervious areas: IAImp=[ 1.57](mm), SLPI=[ 1.4](%),
LGI=[ 693.397](m), MNI=[ 0.013], SCI=[ 0](mi n),
1615 Continuous simulation parameters:
1616 IARECper=[ 4](hrs), IARECimp=[ 4](hrs), InterEventTime=[ 12](hrs),
END=- 1

1617 *%-----|-----|
1618 ROUTE RESERVOIR NHYDout=["SWMF- B"], NHYDin=["Area- B"], RDT=[ 1](mi n),
1619 TABLE of ( OUTFLOW STORAGE ) values
1620 (cms) - (ha- m)
1621 [ 0.000 , 0.000 ]
1622 [ 0.025 , 0.090 ]
1623 [ 0.175 , 0.510 ]
1624 [ 0.350 , 0.710 ]
1625 [ 0.495 , 0.820 ]
1626 [ 0.648 , 0.980 ]
1627 [ 0.965 , 1.045 ]
1628 [ 1.072 , 1.140 ]
1629 [ -1 , -1 ] (max twenty pts)
1630 NHYDovf=["SWMBOVF"]

1631 *%-----|-----|
1632 ADD HYD NHYDs um=["D4- EX"], NHYDs to add=["DRAIN4"+"D4"+"SWMF- B"+"SWMBOVF"]
1633 *%-----|-----|
1634 *ROUTE FLOW THROUGH O Keefe Drain 5
1635 * JFSA: Nov. 2020, added en points to close X-Section
1636 ROUTE CHANNEL NHYDout=["DRAIN5"], NHYDin=["D4- EX"], RDT=[ 1](mi n),
1637 CHLGTH=[ 413.0](m), CHSLOPE=[ 0.16](%), FPSLOPE=[ 0.16](%),
1638 SECNUM=[ 1], NSEG=[ 3]
1639 ( SEGROUGH, SEGDIST (m))=[ 0.043, 12.29 - 0.033, 17.97
1640 0.043, 32.84] NSEG times
1641 ( DISTANCE (m), ELEVATION (m))=(- 0.01, 2.50)
1642 [ 0.00, 1.41]
1643 [ 6.13, 0.97]
1644 [ 12.29, 0.89]
1645 [ 15.71, 0.00]
1646 [ 17.97, 0.39]
1647 [ 23.04, 0.35]
1648 [ 32.83, 0.96]
1649 ( 32.84, 2.50)

1650 *%-----|-----|
1651 CONTINUOUS NASHYD NHYD=["D5"], DT=[ 1]mi n, AREA=[ 1.90](ha),
1652 DWF=[ 0](cms), CN/C=[ 86], IA=[ 8.7](mm), N=[ 3], TP=[ 0.69]hrs,
1653 Continuous simulation parameters:
1654 IARECper=[ 4](hrs),
1655 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1656 InterEventTime=[ 12](hrs)
1657 Baseflow simulation parameters:
1658 BaseFlowOption=[ 1],
1659 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
1660 VHydCond=[ 0.055](mm/hr), END=- 1

1661 *%-----|-----|
1662 *EXTERNAL FLOWS SOUTHEAST OF THE SITE NORTH OF McKenna Casey Dr.
1663 CONTINUOUS NASHYD NHYD=["O- 13SDF"], DT=[ 1]mi n, AREA=[ 9.74](ha),
1664 DWF=[ 0](cms), CN/C=[ 81], IA=[ 4.0](mm), N=[ 3], TP=[ .43]hrs,
1665 Continuous simulation parameters:
1666 IARECper=[ 4](hrs),
1667 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
1668 InterEventTime=[ 12](hrs)
1669 Baseflow simulation parameters:
1670 BaseFlowOption=[ 1],
1671 InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
1672 VHydCond=[ 0.055](mm/hr), END=- 1

1673 *%-----|-----|
1674 *SNOW DISPOSAL FACILITY
1675 *PARAMETERS BASED ON ROBINSON 2006 MODEL
1676 ROUTE RESERVOIR NHYDout=["SDF"], NHYDin=["O- 13SDF"], RDT=[ 1](mi n),

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```

1677          TABLE of ( OUTFLOW STORAGE ) values
1678                ( cms ) - ( ha-m)
1679                [ 0.000, 0.000]
1680                [ 0.150, 0.600]
1681                ( 0.200, 1.500)
1682                [ -1 , -1 ] (max twenty pts)
1683                NHYDovf=[ "OVFSDF" ]
1684 *%-----|-----|
1685 *ANALYSIS POINT 6 - McKenna Casey Dr.
1686 *%-----|-----|
1687 ADD HYD          NHYDs um=[ "PT6MC" ], NHYDs to add=[ "DRAIN5"+"D5"+"SDF" ]
1688 *%-----|-----|
1689 CONTINUOUS NASHYD NHYD=[ "O-15" ], DT=[ 1] min, AREA=[ 10.67] (ha),
1690                DWF=[ 0] (cms), CNVC=[ 82], IA=[ 7.5] (mm), N=[ 3], TP=[ 0.30] hrs,
1691                Continuous simulation parameters:
1692                IARECper=[ 4] (hrs),
1693                SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1694                InterEventTime=[ 12] (hrs)
1695                Baseflow simulation parameters:
1696                BaseFlowOption=[ 1] ,
1697                InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/day/mm)
1698                VHydCond=[ 0.055] (mm/hr), END=- 1
1699 *%-----|-----|
1700 *TOTAL FLOW NORTH OF MCKENNA CASEY DR.
1701 ADD HYD          NHYDs um=[ "MC" ], NHYDs to add=[ "PT6MC"+"O-15" ]
1702 *%-----|-----|
1703 *ROUTE FLOW THROUGH AREA O-14
1704 * JFSA: Nov. 2020, added end points to close X-section
1705 ROUTE CHANNEL   NHYDout=[ "O-14Ch" ], NHYDin=[ "MC" ], RDT=[ 1] (min),
1706                CHLGH=[ 845.3] (m), CHSLOPE=[ 0.10] ( % ), FPSLOPE=[ 0.10] ( % ),
1707                SECNUM=[ 1], NSEG=[ 3]
1708                ( SEGROUGH, SEGDIST (m) )=[ 0.06, 15.00 -0.033, 18.04 0.06, 31.85] NSEG
1709                times
1710                ( DISTANCE (m), ELEVATION (m) )=[ -0.01, 2.5
1711                ( 0.00, 1.53]
1712                ( 5.56, 1.47)
1713                ( 9.21, 1.45)
1714                ( 12.45, 1.53)
1715                ( 13.70, 1.50)
1716                ( 15.00, 0.69)
1717                ( 15.34, 0.00)
1718                ( 16.51, 0.05)
1719                ( 17.30, 0.17)
1720                ( 18.04, 0.74)
1721                ( 19.29, 1.32)
1722                ( 22.73, 1.47)
1723                ( 31.84, 1.41)
1724                ( 31.85, 2.50)
1725 *%-----|-----|
1726 *% -Change O-14 from NASHYD to STANDHYD, name it "S-1-Okeefe" and add it to S-1
1727 subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
1728 *% -JFSA 2021-02-16, add detailed subcatchment drainage area for each subcatchment
1729 in Corrigan sub-catchment. After adding part of O-14 to S_1 sub-catchment so O-14
1730 becomes 5 ha instead of 30.02 ha and TP becomes 0.133 (5*0.8/30.02) instead of 0.8
1731 CONTINUOUS NASHYD NHYD=[ "O-14" ], DT=[ 1] min, AREA=[ 5] (ha),
1732                DWF=[ 0] (cms), CNVC=[ 82], IA=[ 7.5] (mm), N=[ 3], TP=[ 0.133] hrs,
1733                Continuous simulation parameters:
1734                IARECper=[ 4] (hrs),
1735                SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
1736                InterEventTime=[ 12] (hrs)
1737                Baseflow simulation parameters:
1738                BaseFlowOption=[ 1] ,
1739                InitGWResVol=[ 50] (mm), GWResK=[ 0.96] (mm/day/mm)
1740                VHydCond=[ 0.055] (mm/hr), END=- 1
1741 *
1742 *%-----|-----|

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1739 *ANALYSIS POINT 7 - JOCK RIVER
1740 * 2020-12-01 To Foster Drain
1741 * 2020-12-01 replace ("PT7JR") by ("OKEEFE")
1742 *%-----|-----
1743 ADD HYD          NHYDs um=["OKEEFE"], NHYDs to add=["O-14Ch"+"O-14"]
1744 *%-----|-----
1745 *CONTINUOUS STANDHYD NHYD=["OKEEFE"], DT=[1](min), AREA=[448](ha),
1746 *                XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
1747 *                SCS curve number CN=[77],
1748 *                Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
1749 *                LGP=[40](m), MNP=[0.25], SCP=[0](min),
1750 *                Impervious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
1751 *                LGI=[1728](m), MNI=[0.013], SCI=[0](min),
1752 *                Continuous simulation parameters:
1753 *                I a REC per=[4](hrs), I a REC i mp=[4](hrs),
1754 *                SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1755 *                Inter Event Ti me=[18](hrs), END=-1
1756 *#*****
1757 *#      Okeefe Pond
1758 *#      - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1759 *#      and a ratio of the catchment area to the West Clarke pond rating curve
1760 *#      from the M55 for the next coordinates
1761 *#*****
1762 *ROUTE RESERVOIR  NHYDout=["P_OKE"], NHYDin=["OKEEFE"],
1763 *                RDT=[1](min),
1764 *                TABLE of ( OUTFLOW STORAGE ) values
1765 *                ( cms ) - ( ha-m )
1766 *                [ 0.0 , 0.0 ]
1767 *                [ 14.13 , 13.0 ]
1768 *                [ -1 , -1 ] (maximum one hundred pairs of points)
1769 *                NHYDovf=["ok-OVF"],
1770 *%-----|-----
1771 * -JFSA 2021-02-25 "S-1-D2" and "S-1-D3" are part of S-1 sub-catchment. They are
1772 * moved to drain before station 6215 on Jock River
1773 *CONTINUOUS STANDHYD NHYD=["S-1-D2"], DT=[1](min), AREA=[18.67](ha), XI MP=[0.65],
1774 *                TI MP=[0.65], DWF=[0](cms),
1775 *                LOSS=[2], SCS curve number CN=[75], Pervious surfaces:
1776 *                I A per=[4.67](mm), SLPP=[2.0](%),
1777 *                LGP=[40](m), MNP=[0.25], SCP=[0](min), Impervious surfaces:
1778 *                I A i mp=[1.57](mm), SLPI=[0.75](%),
1779 *                LGI=[352.798](m), MNI=[0.013], SCI=[0](min),
1780 *                Continuous simulation parameters:
1781 *                I a REC per=[4](hrs), I a REC i mp=[4](hrs),
1782 *                SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1783 *                Inter Event Ti me=[12](hrs), END=-1
1784 *%-----|-----
1785 *CONTINUOUS NASHYD  NHYD=["S-1-D2"], DT=[1]min, AREA=[18.67](ha),
1786 *                DWF=[0](cms), CN C=[77], I A=[4.67](mm),
1787 *                N=[3], TP=[1.120]hrs,
1788 *                Continuous simulation parameters:
1789 *                I a REC per=[4](hrs),
1790 *                SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
1791 *                Inter Event Ti me=[12](hrs)
1792 *                Baseflow simulation parameters:
1793 *                BaseFl owOpt ion=[1],
1794 *                I ni t GWRes Vol =[50](mm), GWRes K=[0.96](mm/day/mm)
1795 *                VHydCond=[0.055](mm/hr), END=-1
1796 *%-----|-----
1797 *COMPUTE DUALHYD  NHYDin=["S-1-D2"], CI NLET=[2.097](cms), NI NLET=[1],
1798 *                M a j NHYD=["S-1-D2J"]
1799 *                M n NHYD=["S-1-D2N"]
1800 *                TMJ STO=[9999999](cu-m)
1801 *%-----|-----
1802 *ADD HYD          NHYDs um=["S-1-D2S"], NHYDs to add=["S-1-D2J"+"S-1-D2N"]
1803 *%-----|-----

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1800 ROUTE RESERVOIR      NHYDout =[" S- 1- D2R" ] , NHYDin=[" S- 1- D2S" ] ,
1801                      RDT=[ 1 ] (mi n) ,
1802                      TABLE of ( OUTFLOW STORAGE ) values
1803                          ( cms ) - ( ha- m)
1804                          [ 0. 0      , 0. 0 ]
1805                          [ 0. 2231, 0. 7445 ]
1806                          [   -1   ,  -1   ] (max twenty pts)
1807                      NHYDovf=[" S- 1- D2Rovf" ]
1808 *%-----|-----|
1809 CONTINUOUS STANDHYD  NHYD=[" S- 1- D3" ] , DT=[ 1 ] (mi n) , AREA=[ 6. 79 ] (ha) , XI MP=[ 0. 65 ] ,
TI MP=[ 0. 65 ] , DWF=[ 0 ] (cms) ,
1810                      LOSS=[ 2 ] , SCS curve number CN=[ 75 ] , Pervious surfaces:
1811                      I Aper=[ 4. 67 ] (mm) , SLPP=[ 2. 0 ] ( % ) ,
1812                      LGP=[ 40 ] (m) , MNP=[ 0. 25 ] , SCP=[ 0 ] (mi n) , Impervious surfaces:
1813                      I Ai mp=[ 1. 57 ] (mm) , SLPI=[ 0. 75 ] ( % ) ,
1814                      LGI=[ 212. 760 ] (m) , MNI=[ 0. 013 ] , SCI=[ 0 ] (mi n) ,
1815                      Continuous simulation parameters:
1816                      I aREcper=[ 4 ] (hrs) , I aRECI mp=[ 4 ] (hrs) ,
1817                      SM N=[ - 1 ] (mm) , SMAX=[ - 1 ] (mm) , SK=[ 0. 010 ] / (mm) ,
1818                      InterEvent Time=[ 12 ] (hrs) , END=- 1
1819 *%-----|-----|
1820 *CONTINUOUS NASHYD   NHYD=[" S- 1- D3" ] , DT=[ 1 ] mi n , AREA=[ 6. 79 ] (ha) ,
1821 *                      DWF=[ 0 ] (cms) , CN C=[ 77 ] , I A=[ 4. 67 ] (mm) ,
1822 *                      N=[ 3 ] , TP=[ 1. 281 ] hrs ,
1823 *                      Continuous simulation parameters:
1824 *                      I aREcper=[ 4 ] (hrs) ,
1825 *                      SM N=[ - 1 ] (mm) , SMAX=[ - 1 ] (mm) , SK=[ 0. 010 ] / (mm) ,
1826 *                      InterEvent Time=[ 12 ] (hrs)
1827 *                      Baseflow simulation parameters:
1828 *                      BaseFl owOpt ion=[ 1 ] ,
1829 *                      I ni tGWRes Vol =[ 50 ] (mm) , GWRes K=[ 0. 96 ] (mm/ day/ mm)
1830 *                      VHydCond=[ 0. 055 ] (mm/ hr) , END=- 1
1831 *%-----|-----|
1832 COMPUTE DUALHYD      NHYDin=[" S- 1- D3" ] , CI NLET=[ 0. 831 ] (cms) , NI NLET=[ 1 ] ,
1833                      M a j NHYD=[" S- 1- D3J " ]
1834                      M nNHYD=[" S- 1- D3N" ]
1835                      TM S TO=[ 9999999 ] (cu- m)
1836 *%-----|-----|
1837 ADD HYD              NHYDs um=[" S- 1- D3S" ] , NHYDs to add=[" S- 1- D3J "+" S- 1- D3N" ]
1838 *%-----|-----|
1839 ROUTE RESERVOIR      NHYDout =[" S- 1- D3R" ] , NHYDin=[" S- 1- D3S" ] ,
1840                      RDT=[ 1 ] (mi n) ,
1841                      TABLE of ( OUTFLOW STORAGE ) values
1842                          ( cms ) - ( ha- m)
1843                          [ 0. 0      , 0. 0 ]
1844                          [ 0. 0811, 0. 2708 ]
1845                          [   -1   ,  -1   ] (max twenty pts)
1846                      NHYDovf=[" S- 1- D3Rovf" ]
1847 *%-----|-----|
1848 ADD HYD              NHYDs um=[" SN_ OK" ] , NHYDs to
add=[" "_N_ OK" +" OKEEFE" +" S- 1- D2R" +" S- 1- D3R" +" S- 1- D2Rovf" +" S- 1- D3Rovf" ]
1849 *%-----|-----|
1850 SAVE HYD            NHYD=[" SN_ OK" ] , # OF PCYCLES=[ - 1 ] , I CASEs h=[ 1 ]
1851                      HYD_ COMMENT=[" Total Fl ows at Okeefe Dr ai n" ]
1852 *%-----|-----|
1853 *# Hydrograph from Node Okeefe routed to Node at Foster Drain
1854 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6215
1855 *#
1856 ROUTE CHANNEL        NHYDout =[" N_ FO" ] , NHYDin=[" SN_ OK" ] ,
1857                      RDT=[ 1 ] (mi n) ,
1858                      CHLGTH=[ 1183 ] (m) , CHSLOPE=[ 0. 0761 ] ( % ) ,
1859                      FPSLOPE=[ 0. 0761 ] ( % ) ,
1860                      SECNUM=[ 1. 0 ] , NSEG=[ 3 ]
1861                      ( SEGROUGH, SEGDI ST (m) ) =
[ 0. 050, - 33. 89

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1862         - 0.035, 31.59
1863         0.050, 34.41] NSEG times
1864         ( DISTANCE (m), ELEVATION (m)) =
1865         [- 794.18, 91.00]
1866         [- 775.41, 91.50]
1867         [- 702.63, 91.50]
1868         [- 546.19, 91.50]
1869         [- 529.54, 91.50]
1870         [- 323.44, 91.00]
1871         [- 320.71, 91.00]
1872         [- 183.59, 91.00]
1873         [- 182.54, 90.50]
1874         [- 181.36, 90.00]
1875         [- 177.37, 90.00]
1876         [- 87.70, 90.00]
1877         [- 33.89, 90.00]
1878         [- 18.52, 86.88]
1879         [0.00, 85.20]
1880         [16.20, 86.83]
1881         [31.59, 90.00]
1882         [33.03, 90.50]
1883         [34.41, 91.00]
1884 *%-----|-----|
1885 *#*****|
1886 *#   Catchment FOSTER
1887 *#   - To Foster ditch (north of the Jock)
1888 *#   - Partially developed (medium density); remaining agricultural
1889 *#   - 2020-12-01 JFSA Foster area is 332 as per Foster SWWF Environmental Study
1890 *#   - 2020-12-01 decrease Foster drainage area from (373 HA) to (307.98 HA) after
1891 *#   - 2021-02-12 update Foster area to 325.44 ha as measured from QGIS
1892 *#*****|
1893 CONTINUOUS STANDHYD NHYD=["FOSTER"], DT=[1] min, AREA=[325.44] (ha),
1894 XI MP=[0.55], TI MP=[0.55], DWF=[0] (cms), LOSS=[2],
1895 SCS curve number CN=[74],
1896 Pervious surfaces: I A per=[4.67] (mm), SLPP=[0.5] (%),
1897                   LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
1898 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[0.5] (%),
1899                   LGI=[1472.956] (m), MNI=[0.013], SCI=[0] (min),
1900 Continuous simulation parameters:
1901 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
1902 SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1903 I n t e r E v e n t T i m e=[18] (hrs), E N D=-1
1904 *#*****|
1905 *#   Foster Pond
1906 *#   - Rating curve obtained assuming 40m3/ha in 24 hours for quality control
1907 *#   and a ratio of the catchment area to the West Clarke pond rating curve
1908 *#   from the MS for the next coordinates
1909 *#*****|
1910 ROUTE RESERVOIR NHYDout=["P_FOS"], NHYDin=["FOSTER"],
1911 RDT=[1] (min),
1912 TABLE of ( OUTFLOW STORAGE ) values
1913         ( cms ) - ( ha-m)
1914         [ 0.0 , 0.0 ]
1915         [ 10.34 , 10]
1916         [ -1 , -1 ] (max twenty pts)
1917 NHYDovf=["FO-OVF"]
1918 *%-----|-----|
1919 ADD HYD NHYDsum=["FOSTER-OUT"], NHYDsto add=["P_FOS"+"FO-OVF"]
1920 *%-----|-----|
1921 *#*****|
1922 * -Brazeau area from P 1800-19 =[71.751], change to 63.59 ha based on GIS measurements
1923 * -JFSA, 2021-01-19 update "W_CLAR_BRAZ" to 73.29 ha based on GIS measurements
1924 * -JFSA, 2021-01-22 Brazeau ("MS_P10"+"P10-OVF")brazeau pond discharges directly

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to the jock river through a road side ditch on the west side of Borrisokane road (station 6016)

1925 CONTINUOUS STANDHYD NHYD=["W_CLAR_BRAZ"], DT=[1] min, AREA=[73.29] (ha),
1926 XI MP=[0.6], TI MP=[0.65], DWF=[0] (cms), LOSS=[2],
1927 SCS curve number CN=[77],
1928 Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
1929 LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
1930 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[0.5] (%),
1931 LGI=[699.00] (m), MNI=[0.013], SCI=[0] (min),
1932 Continuous simulation parameters:
1933 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
1934 SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1935 Inter Event Time=[18] (hrs), END=-1

*%-----|-----|

* 2020-12-01 correct pond curve values

1938 ROUTE RESERVOIR NHYDout=["MS_P10"], NHYDin=["W_CLAR_BRAZ"],
1939 RDT=[1] (min),

1940 TABLE of (OUTFLOW STORAGE) values
1941 (cms) - (ha - m)
1942 [0.0 , 0.0]
1943 [0.068 , 0.001]
1944 [0.271 , 0.022]
1945 [0.379 , 0.051]
1946 [0.48 , 0.091]
1947 [0.853 , 0.341]
1948 [1.005 , 0.61]
1949 [1.128 , 1.231]
1950 [1.155 , 1.592]
1951 [1.194 , 1.876]
1952 [1.2 , 1.921]
1953 [1.259 , 2.369]
1954 [1.3 , 2.665]
1955 [1.349 , 2.813]
1956 [-1 , -1] (max twenty pts)
1957 NHYDovf=["P10-OVF"]

*%-----|-----|

* -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain before station 980 on Foster Drain

1960 CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1] min, AREA=[4.94] (ha),
1961 XI MP=[0.55], TI MP=[0.55], DWF=[0] (cms), LOSS=[2],
1962 SCS curve number CN=[74],
1963 Pervious surfaces: I A per=[4.67] (mm), SLPP=[0.5] (%),
1964 LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
1965 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[0.5] (%),
1966 LGI=[181.475] (m), MNI=[0.013], SCI=[0] (min),
1967 Continuous simulation parameters:
1968 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
1969 SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1970 Inter Event Time=[18] (hrs), END=-1

*%-----|-----|

1972 *CONTINUOUS NASHYD NHYD=["S-1-FO-D2"], DT=[1] min, AREA=[4.94] (ha),
1973 * DWF=[0] (cms), CN/C=[77], I A=[4.67] (mm),
1974 * N=[3], TP=[1.10] hrs,
1975 * Continuous simulation parameters:
1976 * I a REC per=[4] (hrs),
1977 * SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
1978 * Inter Event Time=[12] (hrs)
1979 * Baseflow simulation parameters:
1980 * BaseFlowOption=[1],
1981 * I n i t GWR es Vol =[50] (mm), GWR es K=[0.96] (mm/day/mm)
1982 * VHydCond=[0.055] (mm/hr), END=-1

*%-----|-----|

1984 COMPUTE DUALHYD NHYDin=["S-1-FO-D2"], CI NLET=[0.508] (cms), NI NLET=[1],
1985 M i j NHYD=["S-1-FO-D2J"]
1986 M nNHYD=["S-1-FO-D2N"]
1987 T M I STO=[9999999] (cu-m)

```

1988 *%-----|-----|
1989 ADD HYD NHYDsum=["S-1-FO-D2S"], NHYDs to add=["S-1-FO-D2J"+"S-1-FO-D2N"]
1990 *%-----|-----|
1991 ROUTE RESERVOIR NHYDout=["S-1-FO-D2R"], NHYDin=["S-1-FO-D2S"],
1992 RDT=[1](min),
1993 TABLE of ( OUTFLOW STORAGE ) values
1994 (cms) - (ha-m)
1995 [ 0.0 , 0.0 ]
1996 [ 0.0590, 0.1970 ]
1997 [ -1 , -1 ] (max twenty pts)
1998 NHYDovf=["S-1FOD2ovf"]
1999 *%-----|-----|
2000 ADD HYD NHYDsum=["980"], NHYDs to
add=["FOSTER-OUT"+"S-1-FO-D2R"+"S-1FOD2ovf"]
2001 *%-----|-----|
2002 SAVE HYD NHYD=["980"], # OF PCYCLES=[-1], I CASEsh=[1]
2003 HYD_COMMENT=["Total Flows at Station 980 on Foster Drain"]
2004 *%-----|-----|
2005 *#
2006 *# Hydrograph from Node Foster SWM (Station 980) to Node at station 520
2007 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 980
2008 *#
2009 ROUTE CHANNEL NHYDout=["980-out"], NHYDin=["980"],
2010 RDT=[1](min),
2011 CHLGTH=[460](m), CHSLOPE=[0.04348](%),
2012 FPSLOPE=[0.04348](%),
2013 SECNUM=[1.0], NSEG=[3]
2014 ( SEGROUGH, SEGDI ST (m))=
2015 [0.050, 45.90
2016 -0.035, 53.30
2017 0.050, 100] NSEG times
2018 ( DISTANCE (m), ELEVATION (m))=
2019 [0, 91.75 ]
2020 [42.4, 92.18 ]
2021 [43.5, 92.16 ]
2022 [44.1, 92.1 ]
2023 [44.6, 92 ]
2024 [44.8, 91.86 ]
2025 [45.9, 91.04 ]
2026 [46.4, 90.65 ]
2027 [46.8, 90.36 ]
2028 [47.9, 90.32 ]
2029 [48.7, 90.35 ]
2030 [50.7, 90.33 ]
2031 [52.2, 90.38 ]
2032 [52.5, 90.59 ]
2033 [53.3, 91.28 ]
2034 [54, 91.83 ]
2035 [54.3, 92 ]
2036 [54.8, 92.08 ]
2037 [55.4, 92.12 ]
2038 [100, 91.84 ]
2039 *%-----|-----|
2040 * -JFSA 2021-02-26 "S-1-FO-DI" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2041 CONTINUOUS STANDHYD NHYD=["S-1-FO-DI"], DT=[1]min, AREA=[5.11](ha),
2042 XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
2043 SCS curve number CN=[74],
2044 Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
2045 LGP=[40](m), MNP=[0.25], SCP=[0](min),
2046 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
2047 LGI=[184.572](m), MNI=[0.013], SCI=[0](min),
2048 Continuous simulation parameters:
2049 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
2050 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2051 Inter Event Time=[18](hrs), END=-1

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2052 *%-----|-----|
2053 COMPUTE DUALHYD NHYDin=["S-1-FO-DI"], CINLET=[0.605](cms), NINLET=[1],
2054 MajNHYD=["S-1-FO-DIJ"]
2055 MnNHYD=["S-1-FO-DIN"]
2056 TMSTO=[9999999](cu-m)
2057 *%-----|-----|
2058 ADD HYD NHYDsum=["S-1-FO-DIS"], NHYDs to add=["S-1-FO-DIN"+"S-1-FO-DIJ"]
2059 *%-----|-----|
2060 ROUTE RESERVOIR NHYDout=["S-1-FO-DIR"], NHYDin=["S-1-FO-DIS"],
2061 RDT=[1](min),
2062 TABLE of ( OUTFLOW STORAGE ) values
2063 (cms) - (ha-m)
2064 [ 0.0 , 0.0 ]
2065 [ 0.0611, 0.2038 ]
2066 [ -1 , -1 ] (max twenty pts)
2067 NHYDovf=["S-1FODlovf"]
2068 *%-----|-----|
2069 ADD HYD NHYDsum=["520"], NHYDs to add=["980-out"+"S-1-FO-DIR"+"S-1FODlovf"]
2070 *%-----|-----|
2071 SAVE HYD NHYD=["520"], # OF PCYCLES=[-1], ICASEsh=[1]
2072 HYD_COMMENT=["Total Flows at Sation 520 on Foster Drain"]
2073 *%-----|-----|
2074 *# Hydrograph from Node at Station 520 (Foster Drain) to Node at station 6016 (Jock
River)
2075 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 520
2076 *#
2077 ROUTE CHANNEL NHYDout=["520-out"], NHYDin=["520"],
2078 RDT=[1](min),
2079 CHLGTH=[860](m), CHSLOPE=[0.5872](%),
2080 FPSLOPE=[0.5872](%),
2081 SECNUM=[1.0], NSEG=[3]
2082 ( SEGROUGH, SEGDI ST (m))=
2083 [0.050, 45.90
2084 -0.035, 54.3
2085 0.050, 100.1097] NSEG times
2086 ( DI STANCE (m), ELEVATI ON (m))=
2087 [0, 91.26 ]
2088 [44.9, 91.46 ]
2089 [45.1, 91.37 ]
2090 [45.9, 90.84 ]
2091 [47, 90.32 ]
2092 [47.5, 90.22 ]
2093 [48, 90.17 ]
2094 [50.7, 90.19 ]
2095 [51.5, 90.17 ]
2096 [52.2, 90.13 ]
2097 [52.7, 90.12 ]
2098 [53.3, 90.14 ]
2099 [53.5, 90.31 ]
2100 [53.9, 90.59 ]
2101 [54.3, 90.87 ]
2102 [54.7, 91.04 ]
2103 [55.3, 91.24 ]
2104 [55.5, 91.26 ]
2105 [63.7, 91.37 ]
2106 [100.1097, 91.43 ]
2107 *%-----|-----|
2108 * -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2109 CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1]min, AREA=[14.96](ha),
2110 XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
2111 SCS curve number CN=[74],
2112 Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
2113 LGP=[40](m), MNP=[0.25], SCP=[0](min),
2114 Imper vious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
2115 LGI=[315.806](m), MNI=[0.013], SCI=[0](min),

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2116 Continuous simulation parameters:
2117 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs),
2118 SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2119 InterEventTime=[ 18](hrs), END=- 1
2120 *%-----|-----|
2121 *CONTINUOUS NASHYD NHYD=["S-1-FO-F-D"], DT=[ 1]min, AREA=[ 14.96](ha),
2122 * DWF=[ 0](cms), CNVC=[ 77], IA=[ 4.67](mm),
2123 * N=[ 3], TP=[ 1.007]hrs,
2124 * Continuous simulation parameters:
2125 * IaRECper=[ 4](hrs),
2126 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2127 * InterEventTime=[ 12](hrs)
2128 * Baseflow simulation parameters:
2129 * BaseFlowOption=[ 1],
2130 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2131 * VHydCond=[ 0.055](mm/hr), END=- 1
2132 *%-----|-----|
2133 COMPUTE DUALHYD NHYDin=["S-1-FO-F-D"], CI NLET=[ 1.615](cms), NI NLET=[ 1],
2134 Maj NHYD=["S-1FO-F-DJ"]
2135 MnNHYD=["S-1FO-F-DN"]
2136 TMI STO=[ 9999999](cu-m)
2137 *%-----|-----|
2138 ADD HYD NHYDsum=["S-1FO-F-DS"], NHYDs to add=["S-1FO-F-DJ"+"S-1FO-F-DN"]
2139 *%-----|-----|
2140 ROUTE RESERVOIR NHYDout=["S-1FO-F-DR"], NHYDin=["S-1FO-F-DS"],
2141 RDT=[ 1](min),
2142 TABLE of ( OUTFLOW STORAGE ) values
2143 (cms) - (ha-m)
2144 [ 0.0 , 0.0 ]
2145 [ 0.1788, 0.5966 ]
2146 [ -1 , -1 ] (max twenty pts)
2147 NHYDovf=["S-1FoFDovf"]
2148 *%-----|-----|
2149 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2150 CONTINUOUS STANDHYD NHYD=["S-1-D8"], DT=[ 1]min, AREA=[ 5.27](ha), XI MP=[ 0.325],
TI MP=[ 0.65], DWF=[ 0](cms), LOSS=[ 1]:
2151 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
2152 Previous areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](min),
2153 Impervious areas: IAimp=[ 0.785](mm), SLPI=[ 0.75](%),
LGI=[ 187.439](m), MNI=[ 0.013], SCI=[ 0](min),
2154 Continuous simulation parameters:
2155 IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
END=- 1
2156 *%-----|-----|
2157 *CONTINUOUS NASHYD NHYD=["S-1-D8"], DT=[ 1]min, AREA=[ 5.27](ha),
2158 * DWF=[ 0](cms), CNVC=[ 77], IA=[ 4.67](mm),
2159 * N=[ 3], TP=[ 1.10]hrs,
2160 * Continuous simulation parameters:
2161 * IaRECper=[ 4](hrs),
2162 * SMN=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2163 * InterEventTime=[ 12](hrs)
2164 * Baseflow simulation parameters:
2165 * BaseFlowOption=[ 1],
2166 * InitGWResVol=[ 50](mm), GWResK=[ 0.96](mm/day/mm)
2167 * VHydCond=[ 0.055](mm/hr), END=- 1
2168 *%-----|-----|
2169 COMPUTE DUALHYD NHYDin=["S-1-D8"], CI NLET=[ 0.672](cms), NI NLET=[ 1],
2170 Maj NHYD=["S-1-D8J"]
2171 MnNHYD=["S-1-D8N"]
2172 TMI STO=[ 9999999](cu-m)
2173 *%-----|-----|
2174 ADD HYD NHYDsum=["S-1-D8S"], NHYDs to add=["S-1-D8J"+"S-1-D8N"]
2175 *%-----|-----|

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2176 *ADD HYD NHYDs um=[ "S-1-D" ], NHYDs to add=[ "S-1-Okeefe"+"S-1"+"S-1-Fost" ]
2177 *%-----|-----|
2178 *COMPUTE DUALHYD NHYDi n=[ "S-1-D" ], CI NLET=[ 11.616 ] (cms), NI NLET=[ 1 ],
2179 * Maj NHYD=[ "S-1-D-M" ]
2180 * Mn NHYD=[ "S-1-D-MN" ]
2181 * TMSTO=[ 5974 ] (cu-m)
2182 *%-----|-----|
2183 *ADD HYD NHYDs um=[ "S-1-DEV" ], NHYDs to add=[ "S-1-D-M"+"S-1-D-MN" ]
2184 *%-----|-----|
2185 ROUTE RESERVOIR NHYDout =["S-1-D8R" ] , NHYDi n=["S-1-D8S" ] ,
2186 RDT=[ 1 ] (mi n),
2187 TABLE of ( OUTFLOW STORAGE ) values
2188 ( cms ) - ( ha-m)
2189 [ 0.0 , 0.0 ]
2190 [ 0.0630, 0.2102 ]
2191 [ -1 , -1 ] (max twenty pts)
2192 NHYDovf=["S-1-D8Rovf" ]
2193 *%-----|-----|
2194 * -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2195 CONTINUOUS NASHYD NHYD=["S-1-A" ], DT=[ 1 ] mi n, AREA=[ 75.88 ] (ha),
2196 DWF=[ 0 ] (cms), CNC=[ 77 ], I A=[ 4.67 ] (mm),
2197 N=[ 3 ], TP=[ 0.619 ] hrs,
2198 Continuous simulation parameters:
2199 I a RECper=[ 4 ] (hrs),
2200 SMN=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2201 InterEvent Time=[ 12 ] (hrs)
2202 Baseflow simulation parameters:
2203 BaseFlowOption=[ 1 ] ,
2204 Ini t GWRes Vol = [ 50 ] (mm), GWRes K=[ 0.96 ] (mm3/day/mm)
2205 VHydCond=[ 0.055 ] (mm3/hr), END=- 1
2206 *%-----|-----|
2207 * -JFSA, 2021-01-22 "WCLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the jock river through a road side ditch on the west side of Borrisokane
road (station 6016)
2208 CONTINUOUS NASHYD NHYD=["WCLAR_UNDE" ], DT=[ 1 ] mi n, AREA=[ 35.65 ] (ha),
2209 DWF=[ 0 ] (cms), CNC=[ 77 ], I A=[ 4.67 ] (mm),
2210 N=[ 3 ], TP=[ 1.10 ] hrs,
2211 Continuous simulation parameters:
2212 I a RECper=[ 4 ] (hrs),
2213 SMN=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2214 InterEvent Time=[ 12 ] (hrs)
2215 Baseflow simulation parameters:
2216 BaseFlowOption=[ 1 ] ,
2217 Ini t GWRes Vol = [ 50 ] (mm), GWRes K=[ 0.96 ] (mm3/day/mm)
2218 VHydCond=[ 0.055 ] (mm3/hr), END=- 1
2219 *%-----|-----|
2220 ADD HYD NHYDs um=[ "SN_FO" ], NHYDs to
add=[ "N_FO"+"520-out"+"MS_P10"+"P10-OVF"+"WCLAR_UNDE"+"S-1FoFDovf"+"S-1FO-F-DR"+"S-1-D8R
ovf"+"S-1-D8R"+"S-1-A" ]
2221 *%-----|-----|
2222 SAVE HYD NHYD=["SN_FO" ], # OF PCYCLES=[ -1 ], I CASEs h=[ 1 ]
2223 HYD_COMMENT=["Total Flows at Foster Drain" ]
2224 *%-----|-----|
2225 *# Hydrograph from Node Foster routed to Node at Cedarview Road
2226 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 6016
2227 *#
2228 ROUTE CHANNEL NHYDout =["NCE" ] , NHYDi n=["SN_FO" ] ,
2229 RDT=[ 1 ] (mi n),
2230 CHLGTH=[ 159 ] (m), CHSLOPE=[ 0.0818 ] ( % ),
2231 FPSLOPE=[ 0.0818 ] ( % ),
2232 SECNUM=[ 1.0 ], NSEG=[ 3 ]
2233 ( SEGROUGH, SEGDI ST (m) ) =
2234 [ 0.050, -15.46
2235 -0.035, 26.55
2236 0.050, 116.76 ] NSEG times

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2237 ( DI STANCE ( m), ELEVATI ON ( m)) =
2238 [- 645. 23, 91. 50]
2239 [- 391. 20, 91. 50]
2240 [- 91. 00, 91. 50]
2241 [- 85. 52, 91. 50]
2242 [- 15. 46, 89. 40]
2243 [- 9. 79, 89. 31]
2244 [- 3. 22, 86. 24]
2245 [3. 22, 85. 07]
2246 [10. 96, 85. 79]
2247 [16. 44, 86. 49]
2248 [26. 55, 89. 45]
2249 [29. 03, 90. 27]
2250 [35. 76, 90. 67]
2251 [36. 67, 91. 00]
2252 [108. 08, 91. 00]
2253 [109. 82, 90. 50]
2254 [112. 04, 90. 50]
2255 [114. 62, 91. 00]
2256 [116. 76, 91. 50]
2257 *%-----|-----|
2258 *#*****|
2259 *# Catchment S-1
2260 *# - To Jock River (north and south of Jock)
2261 *# - Primarily agricultural fields; portion of sand quarry
2262 *%-----|-----|
2263 *% -2020-12-17 "S-1-Undev" and "S-1-Fost" was a part of Foster drain, they are below
the foster pond. Now they are added to S-1 subcatchment based on Project 1474-BCDC,
JFSA, Nov. 2020
2264 *% -2020-12-17 Change O-14 (it was part of Okeefe drain) to "S-1-Okeefe" and add it
to S-1 subcatchment based on Project 1474-BCDC, JFSA, Nov. 2020
2265 *% -2020-12-17 Add "S-1-BCDC" as NASHYD
2266 *% -2020-12-17 all other S-1 subcatchment as STANDHYD with DUALHYD and ROUTE RESERVOIR
2267 *%-----|-----|
2268 *#*****|
2269 * -JFSA 2021-02-26 "S-1-A" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2270 *CONTINUOUS NASHYD NHYD=["S-1-A"], DT=[1] min, AREA=[75.88](ha),
2271 * DWF=[0](cms), CNVC=[77], IA=[4.67](mm),
2272 * N=[3], TP=[0.619]hrs,
2273 * Continuous simulation parameters:
2274 * IaRECper=[4](hrs),
2275 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2276 * InterEventTime=[12](hrs)
2277 * Baseflow simulation parameters:
2278 * BaseFlowOption=[1],
2279 * InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2280 * VHydCond=[0.055](mm/hr), END=-1
2281 *%-----|-----|
2282 CONTINUOUS NASHYD NHYD=["S-1-B"], DT=[1] min, AREA=[55.36](ha),
2283 DWF=[0](cms), CNVC=[77], IA=[4.67](mm),
2284 N=[3], TP=[0.451]hrs,
2285 Continuous simulation parameters:
2286 IaRECper=[4](hrs),
2287 SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
2288 InterEventTime=[12](hrs)
2289 Baseflow simulation parameters:
2290 BaseFlowOption=[1],
2291 InitGWResVol=[50](mm), GWResK=[0.96](mm/day/mm)
2292 VHydCond=[0.055](mm/hr), END=-1
2293 *%-----|-----|
2294 *# - JFSA 2021-02-24 change the name from S-1-BCDC to S-1-A and S-1-B. Change their
TP values based on the new areas compared to the old ones.
2295 *CONTINUOUS NASHYD NHYD=["S-1-BCDC"], DT=[1] min, AREA=[134.9](ha),
2296 * DWF=[0](cms), CNVC=[77], IA=[4.67](mm),
2297 * N=[3], TP=[1.10]hrs,

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2298 * Continuous simulation parameters:
2299 * IaRECper=[ 4]( hrs),
2300 * SMN=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2301 * InterEventTime=[ 12]( hrs)
2302 * Baseflow simulation parameters:
2303 * BaseFlowOption=[ 1],
2304 * InitGWResVol=[ 50]( mm), GWResK=[ 0. 96]( mm/ day/ mm)
2305 * VHydCond=[ 0. 055]( mm/ hr), END=- 1
2306 *%-----|-----
2307 *# - JFSA 2021-02-24 "S-1-BCDC-1" and "S-1-BCDC-2" are not existing anymore.
"S-1-BCDC-1" is part of "S-1-FO-D2" and "S-1-BCDC-2" is part of "S-1-D2" and "S-1-D3"
2308 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-1"], DT=[ 1]mi n, AREA=[ 0. 3]( ha),
2309 * DWF=[ 0]( cms), CN C=[ 77], IA=[ 4. 67]( mm),
2310 * N=[ 3], TP=[ 1. 10]hr s,
2311 * Continuous simulation parameters:
2312 * IaRECper=[ 4]( hrs),
2313 * SMN=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2314 * InterEventTime=[ 12]( hrs)
2315 * Baseflow simulation parameters:
2316 * BaseFlowOption=[ 1],
2317 * InitGWResVol=[ 50]( mm), GWResK=[ 0. 96]( mm/ day/ mm)
2318 * VHydCond=[ 0. 055]( mm/ hr), END=- 1
2319 *%-----|-----
2320 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-2"], DT=[ 1]mi n, AREA=[ 1. 3]( ha),
2321 * DWF=[ 0]( cms), CN C=[ 77], IA=[ 4. 67]( mm),
2322 * N=[ 3], TP=[ 1. 10]hr s,
2323 * Continuous simulation parameters:
2324 * IaRECper=[ 4]( hrs),
2325 * SMN=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2326 * InterEventTime=[ 12]( hrs)
2327 * Baseflow simulation parameters:
2328 * BaseFlowOption=[ 1],
2329 * InitGWResVol=[ 50]( mm), GWResK=[ 0. 96]( mm/ day/ mm)
2330 * VHydCond=[ 0. 055]( mm/ hr), END=- 1
2331 *%-----|-----
2332 *# - JFSA 2021-01-19, after adding Greenbank pond, "S-1-BCDC-3" is not existing
anymore
2333 *CONTINUOUS NASHYD NHYD=["S-1-BCDC-3"], DT=[ 1]mi n, AREA=[ 3. 9]( ha),
2334 * DWF=[ 0]( cms), CN C=[ 77], IA=[ 4. 67]( mm),
2335 * N=[ 3], TP=[ 1. 10]hr s,
2336 * Continuous simulation parameters:
2337 * IaRECper=[ 4]( hrs),
2338 * SMN=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2339 * InterEventTime=[ 12]( hrs)
2340 * Baseflow simulation parameters:
2341 * BaseFlowOption=[ 1],
2342 * InitGWResVol=[ 50]( mm), GWResK=[ 0. 96]( mm/ day/ mm)
2343 * VHydCond=[ 0. 055]( mm/ hr), END=- 1
2344 *%-----|-----
2345 * -JFSA 2021-02-25 "S-1-Okeefe" is a part of S-1 sub-catchment. It is moved to drain
before station 7245 on Jock River
2346 *CONTINUOUS STANDHYD NHYD=["S-1-Okeefe"], DT=[ 1]( mi n), AREA=[ 44. 93]( ha), XI MP=[ 0. 65],
TI MP=[ 0. 65], DWF=[ 0]( cms),
2347 * LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
I Aper=[ 4. 67]( mm), SLPP=[ 2. 0]( %),
2348 * LGP=[ 40]( m), MNP=[ 0. 25], SCP=[ 0]( mi n), Impervious surfaces:
I Ai mp=[ 1. 57]( mm), SLPI=[ 0. 75]( %),
2349 * LGI=[ 547. 296]( m), MNI=[ 0. 013], SCI=[ 0]( mi n),
2350 * Continuous simulation parameters:
2351 * IaRECper=[ 4]( hrs), IaRECI mp=[ 4]( hrs),
2352 * SMN=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2353 * InterEventTime=[ 12]( hrs), END=- 1
2354 *%-----|-----
2355 *COMPUTE DUALHYD NHYDin=["S-1-Okeefe"], CI NLET=[ 4. 796]( cms), NI NLET=[ 1],
2356 * Mi j NHYD=["S-1-OkM"]
2357 * M nNHYD=["S-1-OkMN"]

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2358 *          TMI STO=[ 9999999 ] ( cu - m)
2359 *%-----|-----
2360 *ADD HYD          NHYDs um=[ " S- 1- OkS" ], NHYDs to add=[ " S- 1- OkM" +" S- 1- OkMN" ]
2361 *%-----|-----
2362 *ROUTE RESERVOIR NHYDout=[ " S- 1- OkSR" ] , NHYDin=[ " S- 1- OkS" ] ,
2363 *          RDT=[ 1 ] ( mi n),
2364 *          TABLE of ( OUTFLOW STORAGE ) values
2365 *                  ( cms ) - ( ha - m)
2366 *                  [ 0.0      , 0.0 ]
2367 *                  [ 0.5370, 1.7917 ]
2368 *                  [ -1      , -1      ] (max twenty pts)
2369 *          NHYDovf=[ " S- 1- OkSovf" ]
2370 *%-----|-----
2371 *CONTINUOUS NASHYD NHYD=[ " S- 1- Okeefe" ], DT=[ 1 ] mi n, AREA=[ 44.93 ] ( ha),
2372 *          DWF=[ 0 ] ( cms), CN C=[ 77 ], IA=[ 4.67 ] ( mm),
2373 *          N=[ 3 ], TP=[ 1.049 ] hrs,
2374 *          Continuous simulation parameters:
2375 *          IaRECper=[ 4 ] ( hrs),
2376 *          SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0.010 ] / ( mm),
2377 *          InterEventTime=[ 12 ] ( hrs)
2378 *          Baseflow simulation parameters:
2379 *          BaseFlowOption=[ 1 ] ,
2380 *          InitGWResVol=[ 50 ] ( mm), GWResK=[ 0.96 ] ( mm/ day/ mm)
2381 *          VHydCond=[ 0.055 ] ( mm/ hr), END=- 1
2382 *%-----|-----
2383 * -JFSA 2021-02-26 "S-1-FO-DI" is a part of S-1 sub-catchment. It is moved to drain
before station 520 on Foster Drain
2384 *CONTINUOUS STANDHYD NHYD=[ " S- 1- FO- DI" ], DT=[ 1 ] mi n, AREA=[ 5.11 ] ( ha),
2385 *          XI MP=[ 0.65 ], TI MP=[ 0.65 ], DWF=[ 0 ] ( cms), LOSS=[ 2 ],
2386 *          SCS curve number CN=[ 74 ],
2387 *          Pervious surfaces: IAper=[ 4.67 ] ( mm), SLPP=[ 0.5 ] ( %),
2388 *          LGP=[ 40 ] ( m), MNP=[ 0.25 ], SCP=[ 0 ] ( mi n),
2389 *          Impervious surfaces: IAimp=[ 1.57 ] ( mm), SLPI=[ 0.5 ] ( %),
2390 *          LGI=[ 184.572 ] ( m), MNI=[ 0.013 ], SCI=[ 0 ] ( mi n),
2391 *          Continuous simulation parameters:
2392 *          IaRECper=[ 4 ] ( hrs), IaRECimp=[ 4 ] ( hrs),
2393 *          SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0.010 ] / ( mm),
2394 *          InterEventTime=[ 18 ] ( hrs), END=- 1
2395 *%-----|-----
2396 *COMPUTE DUALHYD NHYDin=[ " S- 1- FO- DI" ], CI NLET=[ 0.605 ] ( cms), NI NLET=[ 1 ],
2397 *          Mi j NHYD=[ " S- 1- FO- DIJ" ]
2398 *          M nNHYD=[ " S- 1- FO- DIN" ]
2399 *          TMI STO=[ 9999999 ] ( cu - m)
2400 *%-----|-----
2401 *ADD HYD          NHYDs um=[ " S- 1- FO- DIS" ], NHYDs to add=[ " S- 1- FO- DIN" +" S- 1- FO- DIJ" ]
2402 *%-----|-----
2403 *ROUTE RESERVOIR NHYDout=[ " S- 1- FO- DIR" ] , NHYDin=[ " S- 1- FO- DIS" ] ,
2404 *          RDT=[ 1 ] ( mi n),
2405 *          TABLE of ( OUTFLOW STORAGE ) values
2406 *                  ( cms ) - ( ha - m)
2407 *                  [ 0.0      , 0.0 ]
2408 *                  [ 0.0611, 0.2038 ]
2409 *                  [ -1      , -1      ] (max twenty pts)
2410 *          NHYDovf=[ " S- 1FODIovf" ]
2411 *%-----|-----
2412 *CONTINUOUS NASHYD NHYD=[ " S- 1- FO- DI" ], DT=[ 1 ] mi n, AREA=[ 5.11 ] ( ha),
2413 *          DWF=[ 0 ] ( cms), CN C=[ 77 ], IA=[ 4.67 ] ( mm),
2414 *          N=[ 3 ], TP=[ 1.10 ] hrs,
2415 *          Continuous simulation parameters:
2416 *          IaRECper=[ 4 ] ( hrs),
2417 *          SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0.010 ] / ( mm),
2418 *          InterEventTime=[ 12 ] ( hrs)
2419 *          Baseflow simulation parameters:
2420 *          BaseFlowOption=[ 1 ] ,
2421 *          InitGWResVol=[ 50 ] ( mm), GWResK=[ 0.96 ] ( mm/ day/ mm)
2422 *          VHydCond=[ 0.055 ] ( mm/ hr), END=- 1

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2423 *%-----|-----|
2424 *   -JFSA 2021-02-26 "S-1-FO-D2" is a part of S-1 sub-catchment. It is moved to drain
before station 980 on Foster Drain
2425 *CONTINUOUS STANDHYD NHYD=["S-1-FO-D2"], DT=[1] min, AREA=[4.94](ha),
2426 *   XI MP=[0.55], TI MP=[0.55], DWF=[0](cms), LOSS=[2],
2427 *   SCS curve number CN=[74],
2428 *   Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
2429 *   LGP=[40](m), MNP=[0.25], SCP=[0](min),
2430 *   Impervious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
2431 *   LGI=[181.475](m), MNI=[0.013], SCI=[0](min),
2432 *   Continuous simulation parameters:
2433 *   I a REC per=[4](hrs), I a REC i mp=[4](hrs),
2434 *   S M N=[-1](mm), S M A X=[-1](mm), S K=[0.010]/(mm),
2435 *   I n t e r E v e n t T i m e=[18](hrs),   E N D=- 1
2436 *%-----|-----|
2437 *CONTINUOUS NASHYD   NHYD=["S-1-FO-D2"], DT=[1] min, AREA=[4.94](ha),
2438 *   DWF=[0](cms), CN C=[77], I A=[4.67](mm),
2439 *   N=[3], TP=[1.10] hrs,
2440 *   Continuous simulation parameters:
2441 *   I a REC per=[4](hrs),
2442 *   S M N=[-1](mm), S M A X=[-1](mm), S K=[0.010]/(mm),
2443 *   I n t e r E v e n t T i m e=[12](hrs)
2444 *   Baseflow simulation parameters:
2445 *   B a s e F l o w O p t i o n=[1] ,
2446 *   I n i t G W R e s V o l=[50](mm), G W R e s K=[0.96](mm/day/mm)
2447 *   V H y d C o n d=[0.055](mm/hr),   E N D=- 1
2448 *%-----|-----|
2449 *COMPUTE DUALHYD    NHYD i n=["S-1-FO-D2"], C I N L E T=[0.508](cms), N I N L E T=[1],
2450 *   M i j N H Y D=["S-1-FO-D2J"]
2451 *   M n N H Y D=["S-1-FO-D2N"]
2452 *   T M I S T O=[9999999](cu-m)
2453 *%-----|-----|
2454 *ADD HYD             NHYD s u m=["S-1-FO-D2S"], NHYD s t o a d d=["S-1-FO-D2J"+"S-1-FO-D2N"]
2455 *%-----|-----|
2456 *ROUTE RESERVOIR   NHYD o u t=["S-1-FO-D2R"], NHYD i n=["S-1-FO-D2S"],
2457 *   R D T=[1](min),
2458 *   TABLE of ( O U T F L O W S T O R A G E ) values
2459 *   (cms) - (ha-m)
2460 *   [ 0.0 , 0.0 ]
2461 *   [ 0.0590, 0.1970 ]
2462 *   [ -1 , -1 ] (max twenty pts)
2463 *   N H Y D o v f=["S-1FOD2ovf"]
2464 *%-----|-----|
2465 *   -JFSA 2021-02-26 "S-1-FO-F-D" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2466 *CONTINUOUS STANDHYD NHYD=["S-1-FO-F-D"], DT=[1] min, AREA=[14.96](ha),
2467 *   XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
2468 *   SCS curve number CN=[74],
2469 *   Pervious surfaces: I A per=[4.67](mm), SLPP=[0.5](%),
2470 *   LGP=[40](m), MNP=[0.25], SCP=[0](min),
2471 *   Impervious surfaces: I A i mp=[1.57](mm), SLPI=[0.5](%),
2472 *   LGI=[315.806](m), MNI=[0.013], SCI=[0](min),
2473 *   Continuous simulation parameters:
2474 *   I a REC per=[4](hrs), I a REC i mp=[4](hrs),
2475 *   S M N=[-1](mm), S M A X=[-1](mm), S K=[0.010]/(mm),
2476 *   I n t e r E v e n t T i m e=[18](hrs),   E N D=- 1
2477 *%-----|-----|
2478 *CONTINUOUS NASHYD   NHYD=["S-1-FO-F-D"], DT=[1] min, AREA=[14.96](ha),
2479 *   DWF=[0](cms), CN C=[77], I A=[4.67](mm),
2480 *   N=[3], TP=[1.007] hrs,
2481 *   Continuous simulation parameters:
2482 *   I a REC per=[4](hrs),
2483 *   S M N=[-1](mm), S M A X=[-1](mm), S K=[0.010]/(mm),
2484 *   I n t e r E v e n t T i m e=[12](hrs)
2485 *   Baseflow simulation parameters:
2486 *   B a s e F l o w O p t i o n=[1] ,

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2487 *          I n i t GWRes Vol =[ 50 ] ( mm ) ,   GWRes K=[ 0. 96 ] ( mm/ day/ mm)
2488 *          VHydCond=[ 0. 055 ] ( mm/ hr ) ,     END=- 1
2489 *%-----|-----|
2490 *COMPUTE DUALHYD  NHYD i n=[ " S- 1- FO- F- D' ] ,   CI NLET=[ 1. 749 ] ( cms ) ,   NI NLET=[ 1 ] ,
2491 *          M a j NHYD=[ " S- 1FO- F- DJ " ]
2492 *          M nNHYD=[ " S- 1FO- F- DN" ]
2493 *          TMI STO=[ 9999999 ] ( cu- m)
2494 *%-----|-----|
2495 *ADD HYD          NHYDs um=[ " S- 1FO- F- DS" ] ,   NHYDs t o add=[ " S- 1FO- F- DJ "+" S- 1FO- F- DN" ]
2496 *%-----|-----|
2497 *ROUTE RESERVOI R  NHYDout =[ " S- 1FO- F- DR" ] ,   NHYD i n=[ " S- 1FO- F- DS" ] ,
2498 *          RDT=[ 1 ] ( mi n ) ,
2499 *          TABLE of ( OUTFLOW STORAGE ) values
2500 *                  ( cms ) - ( ha- m)
2501 *                  [ 0. 0      , 0. 0 ]
2502 *                  [ 0. 1788 , 0. 5966 ]
2503 *                  [      -1 , -1      ] ( max t wenty pts)
2504 *          NHYDovf=[ " S- 1FoFDovf " ]
2505 *%-----|-----|
2506 CONTINUOUS STANDHYD NHYD=[ " S- 1- D1 " ] ,   DT=[ 1 ] ( mi n ) ,   AREA=[ 21. 67 ] ( ha ) ,   XI MP=[ 0. 65 ] ,
TI MP=[ 0. 65 ] ,   DWF=[ 0 ] ( cms ) ,
2507 LOSS=[ 2 ] ,   SCS curve number CN=[ 75 ] ,   Pervious surfaces:
I Aper=[ 4. 67 ] ( mm ) ,   SLPP=[ 2. 0 ] ( % ) ,
2508 LGP=[ 40 ] ( m ) ,   MNP=[ 0. 25 ] ,   SCP=[ 0 ] ( mi n ) ,   Impervious surfaces:
I Ai mp=[ 1. 57 ] ( mm ) ,   SLPI=[ 0. 75 ] ( % ) ,
2509 LGI=[ 380. 088 ] ( m ) ,   MNI=[ 0. 013 ] ,   SCI=[ 0 ] ( mi n ) ,
2510 Continuous simulation parameters:
I aRECper=[ 4 ] ( hrs ) ,   I aRECI mp=[ 4 ] ( hrs ) ,
2511 SM N=[ - 1 ] ( mm ) ,   SMAX=[ - 1 ] ( mm ) ,   SK=[ 0. 010 ] / ( mm ) ,
2512 Inter Event Ti me=[ 12 ] ( hrs ) ,   END=- 1
2513
2514 *%-----|-----|
2515 *CONTINUOUS NASHYD  NHYD=[ " S- 1- D1 " ] ,   DT=[ 1 ] mi n ,   AREA=[ 21. 67 ] ( ha ) ,
2516 *          DWF=[ 0 ] ( cms ) ,   CN C=[ 77 ] ,   I A=[ 4. 67 ] ( mm ) ,
2517 *          N=[ 3 ] ,   TP=[ 1. 066 ] hrs ,
2518 *          Continuous simulation parameters:
2519 *          I aRECper=[ 4 ] ( hrs ) ,
2520 *          SM N=[ - 1 ] ( mm ) ,   SMAX=[ - 1 ] ( mm ) ,   SK=[ 0. 010 ] / ( mm ) ,
2521 *          Inter Event Ti me=[ 12 ] ( hrs )
2522 *          Baseflow simulation parameters:
2523 *          BaseFl owOpt ion=[ 1 ] ,
2524 *          I n i t GWRes Vol =[ 50 ] ( mm ) ,   GWRes K=[ 0. 96 ] ( mm/ day/ mm)
2525 *          VHydCond=[ 0. 055 ] ( mm/ hr ) ,     END=- 1
2526 *%-----|-----|
2527 COMPUTE DUALHYD  NHYD i n=[ " S- 1- D1 " ] ,   CI NLET=[ 2. 409 ] ( cms ) ,   NI NLET=[ 1 ] ,
2528 *          M a j NHYD=[ " S- 1- D1 J " ]
2529 *          M nNHYD=[ " S- 1- D1 N" ]
2530 *          TMI STO=[ 9999999 ] ( cu- m)
2531 *%-----|-----|
2532 ADD HYD          NHYDs um=[ " S- 1- D1 S" ] ,   NHYDs t o add=[ " S- 1- D1 J "+" S- 1- D1 N" ]
2533 *%-----|-----|
2534 ROUTE RESERVOI R  NHYDout =[ " S- 1- D1 R" ] ,   NHYD i n=[ " S- 1- D1 S" ] ,
2535 *          RDT=[ 1 ] ( mi n ) ,
2536 *          TABLE of ( OUTFLOW STORAGE ) values
2537 *                  ( cms ) - ( ha- m)
2538 *                  [ 0. 0      , 0. 0 ]
2539 *                  [ 0. 2590 , 0. 8642 ]
2540 *                  [      -1 , -1      ] ( max t wenty pts)
2541 *          NHYDovf=[ " S- 1- D1 Rovf " ]
2542 *%-----|-----|
2543 * -JFSA 2021-02-25 " S- 1- D2" and " S- 1- D3" are part of S- 1 sub-catchment. They are
moved to drain before station 6215 on Jock River
2544 *CONTINUOUS STANDHYD NHYD=[ " S- 1- D2 " ] ,   DT=[ 1 ] ( mi n ) ,   AREA=[ 18. 67 ] ( ha ) ,   XI MP=[ 0. 65 ] ,
TI MP=[ 0. 65 ] ,   DWF=[ 0 ] ( cms ) ,
2545 *          LOSS=[ 2 ] ,   SCS curve number CN=[ 75 ] ,   Pervious surfaces:
I Aper=[ 4. 67 ] ( mm ) ,   SLPP=[ 2. 0 ] ( % ) ,
2546 *          LGP=[ 40 ] ( m ) ,   MNP=[ 0. 25 ] ,   SCP=[ 0 ] ( mi n ) ,   Impervious surfaces:

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2547 * I Ai mp=[ 1. 57] ( mm) , SLPI =[ 0. 75] ( %),
2548 * LGI =[ 352. 798] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
2549 * Continuous simulation parameters:
2550 * IaRECper =[ 4] ( hr s) , IaRECI mp =[ 4] ( hr s) ,
2551 * SM N=[ - 1] ( mm) , SMAX=[ - 1] ( mm) , SK=[ 0. 010] / ( mm) ,
2552 * InterEventTime =[ 12] ( hr s) , END=- 1
2553 *%-----|-----
2553 *CONTINUOUS NASHYD NHYD=[ " S- 1- D2" ] , DT=[ 1] mi n , AREA=[ 18. 67] ( ha) ,
2554 * DWF=[ 0] ( cms) , CN C=[ 77] , IA=[ 4. 67] ( mm) ,
2555 * N=[ 3] , TP=[ 1. 120] hr s ,
2556 * Continuous simulation parameters:
2557 * IaRECper =[ 4] ( hr s) ,
2558 * SM N=[ - 1] ( mm) , SMAX=[ - 1] ( mm) , SK=[ 0. 010] / ( mm) ,
2559 * InterEventTime =[ 12] ( hr s)
2560 * Baseflow simulation parameters:
2561 * BaseFlowOption =[ 1] ,
2562 * In it GWRes Vol =[ 50] ( mm) , GWRes K=[ 0. 96] ( mm/ day/ mm)
2563 * VHydCond =[ 0. 055] ( mm/ hr) , END=- 1
2564 *%-----|-----
2565 *COMPUTE DUALHYD NHYDin =[ " S- 1- D2" ] , CINLET =[ 2. 062] ( cms) , NI NLET =[ 1] ,
2566 * M aj NHYD =[ " S- 1- D2J" ]
2567 * M nNHYD =[ " S- 1- D2N" ]
2568 * TM I STO =[ 9999999] ( cu- m)
2569 *%-----|-----
2570 *ADD HYD NHYDs um =[ " S- 1- D2S" ] , NHYDs to add =[ " S- 1- D2J" + " S- 1- D2N" ]
2571 *%-----|-----
2572 *ROUTE RESERVOIR NHYDout =[ " S- 1- D2R" ] , NHYDin =[ " S- 1- D2S" ] ,
2573 * RDT =[ 1] ( mi n) ,
2574 * TABLE of ( OUTFLOW STORAGE ) values
2575 * ( cms) - ( ha- m)
2576 * [ 0. 0 , 0. 0 ]
2577 * [ 0. 2231, 0. 7445 ]
2578 * [ - 1 , - 1 ] ( max twenty pts)
2579 * NHYDovf =[ " S- 1- D2Rovf" ]
2580 *%-----|-----
2581 *CONTINUOUS STANDHYD NHYD=[ " S- 1- D3" ] , DT=[ 1] ( mi n) , AREA=[ 6. 79] ( ha) , XI MP=[ 0. 65] ,
2582 * TI MP=[ 0. 65] , DWF=[ 0] ( cms) ,
2583 * LOSS=[ 2] , SCS curve number CN=[ 75] , Pervious surfaces:
I Aper =[ 4. 67] ( mm) , SLPP=[ 2. 0] ( %),
2584 * LGP=[ 40] ( m) , MNP=[ 0. 25] , SCP=[ 0] ( mi n) , Impervious surfaces:
I Ai mp =[ 1. 57] ( mm) , SLPI =[ 0. 75] ( %),
2585 * LGI =[ 212. 760] ( m) , MNI =[ 0. 013] , SCI =[ 0] ( mi n) ,
2586 * Continuous simulation parameters:
2587 * IaRECper =[ 4] ( hr s) , IaRECI mp =[ 4] ( hr s) ,
2588 * SM N=[ - 1] ( mm) , SMAX=[ - 1] ( mm) , SK=[ 0. 010] / ( mm) ,
2589 * InterEventTime =[ 12] ( hr s) , END=- 1
2590 *%-----|-----
2590 *CONTINUOUS NASHYD NHYD=[ " S- 1- D3" ] , DT=[ 1] mi n , AREA=[ 6. 79] ( ha) ,
2591 * DWF=[ 0] ( cms) , CN C=[ 77] , IA=[ 4. 67] ( mm) ,
2592 * N=[ 3] , TP=[ 1. 281] hr s ,
2593 * Continuous simulation parameters:
2594 * IaRECper =[ 4] ( hr s) ,
2595 * SM N=[ - 1] ( mm) , SMAX=[ - 1] ( mm) , SK=[ 0. 010] / ( mm) ,
2596 * InterEventTime =[ 12] ( hr s)
2597 * Baseflow simulation parameters:
2598 * BaseFlowOption =[ 1] ,
2599 * In it GWRes Vol =[ 50] ( mm) , GWRes K=[ 0. 96] ( mm/ day/ mm)
2600 * VHydCond =[ 0. 055] ( mm/ hr) , END=- 1
2601 *%-----|-----
2602 *COMPUTE DUALHYD NHYDin =[ " S- 1- D3" ] , CINLET =[ 0. 719] ( cms) , NI NLET =[ 1] ,
2603 * M aj NHYD =[ " S- 1- D3J" ]
2604 * M nNHYD =[ " S- 1- D3N" ]
2605 * TM I STO =[ 9999999] ( cu- m)
2606 *%-----|-----
2607 *ADD HYD NHYDs um =[ " S- 1- D3S" ] , NHYDs to add =[ " S- 1- D3J" + " S- 1- D3N" ]
2608 *%-----|-----

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2609 *ROUTE RESERVOIR      NHYDout=[ "S- 1- D3R" ] , NHYDin=[ "S- 1- D3S" ] ,
2610 *                      RDT=[ 1 ] (mi n),
2611 *                      TABLE of ( OUTFLOW STORAGE ) values
2612 *                      ( cms ) - ( ha- m)
2613 *                      [ 0.0      , 0.0 ]
2614 *                      [ 0.0811, 0.2708 ]
2615 *                      [ -1      , -1      ] (max twenty pts)
2616 *                      NHYDovf=[ "S- 1- D3Rovf" ]
2617 *%-----|-----
2618 CONTINUOUS STANDHYD NHYD=[ "S- 1- D4" ], DT=[ 1 ] (mi n), AREA=[ 3.28 ] (ha), XI MP=[ 0.65 ],
TI MP=[ 0.65 ], DWF=[ 0 ] (cms),
2619 LOSS=[ 2 ], SCS curve number CN=[ 75 ], Pervious surfaces:
I Aper=[ 4.67 ] (mm), SLPP=[ 2.0 ] ( % ),
2620 LGP=[ 40 ] (m), MNP=[ 0.25 ], SCP=[ 0 ] (mi n), Impervious surfaces:
I Ai mp=[ 1.57 ] (mm), SLPI=[ 0.75 ] ( % ),
2621 LGI=[ 147.874 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),
2622 Continuous simulation parameters:
2623 IaRECper=[ 4 ] (hrs), IaRECI mp=[ 4 ] (hrs),
2624 SM N=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2625 InterEvent Time=[ 12 ] (hrs), END=- 1
2626 *%-----|-----
2627 *CONTINUOUS NASHYD NHYD=[ "S- 1- D4" ], DT=[ 1 ] mi n, AREA=[ 3.28 ] (ha),
2628 * DWF=[ 0 ] (cms), CN C=[ 77 ], IA=[ 4.67 ] (mm),
2629 * N=[ 3 ], TP=[ 1.10 ] hrs,
2630 * Continuous simulation parameters:
2631 * IaRECper=[ 4 ] (hrs),
2632 * SM N=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2633 * InterEvent Time=[ 12 ] (hrs)
2634 * Baseflow simulation parameters:
2635 * BaseFl owOpt ion=[ 1 ] ,
2636 * Ini t GWRes Vol =[ 50 ] (mm), GWRes K=[ 0.96 ] (mm/ day/ mm)
2637 * VHydCond=[ 0.055 ] (mm/ hr), END=- 1
2638 *%-----|-----
2639 COMPUTE DUALHYD NHYDin=[ "S- 1- D4" ], CI NLET=[ 0.421 ] (cms), NI NLET=[ 1 ],
2640 * Mj NHYD=[ "S- 1- D4J " ]
2641 * MnNHYD=[ "S- 1- D4N" ]
2642 * TM STO=[ 9999999 ] (cu- m)
2643 *%-----|-----
2644 ADD HYD NHYDsum=[ "S- 1- D4S" ], NHYDs to add=[ "S- 1- D4J "+"S- 1- D4N" ]
2645 *%-----|-----
2646 ROUTE RESERVOIR NHYDout=[ "S- 1- D4R" ] , NHYDin=[ "S- 1- D4S" ] ,
2647 * RDT=[ 1 ] (mi n),
2648 * TABLE of ( OUTFLOW STORAGE ) values
2649 * ( cms ) - ( ha- m)
2650 * [ 0.0      , 0.0 ]
2651 * [ 0.0392, 0.1308 ]
2652 * [ -1      , -1      ] (max twenty pts)
2653 * NHYDovf=[ "S- 1- D4Rovf" ]
2654 *%-----|-----
2655 CONTINUOUS STANDHYD NHYD=[ "S- 1- D5" ], DT=[ 1 ] (mi n), AREA=[ 12.84 ] (ha), XI MP=[ 0.65 ],
TI MP=[ 0.65 ], DWF=[ 0 ] (cms),
2656 LOSS=[ 2 ], SCS curve number CN=[ 75 ], Pervious surfaces:
I Aper=[ 4.67 ] (mm), SLPP=[ 2.0 ] ( % ),
2657 LGP=[ 40 ] (m), MNP=[ 0.25 ], SCP=[ 0 ] (mi n), Impervious surfaces:
I Ai mp=[ 1.57 ] (mm), SLPI=[ 0.75 ] ( % ),
2658 LGI=[ 292.57 ] (m), MNI=[ 0.013 ], SCI=[ 0 ] (mi n),
2659 Continuous simulation parameters:
2660 IaRECper=[ 4 ] (hrs), IaRECI mp=[ 4 ] (hrs),
2661 SM N=[ -1 ] (mm), SMAX=[ -1 ] (mm), SK=[ 0.010 ] / (mm),
2662 InterEvent Time=[ 12 ] (hrs), END=- 1
2663 *%-----|-----
2664 *CONTINUOUS NASHYD NHYD=[ "S- 1- D5" ], DT=[ 1 ] mi n, AREA=[ 12.84 ] (ha),
2665 * DWF=[ 0 ] (cms), CN C=[ 77 ], IA=[ 4.67 ] (mm),
2666 * N=[ 3 ], TP=[ 1.10 ] hrs,
2667 * Continuous simulation parameters:
2668 * IaRECper=[ 4 ] (hrs),

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2669 * SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
2670 * InterEventTime=[ 12 ] ( hr s )
2671 * Baseflow simulation parameters:
2672 * BaseFlowOption=[ 1 ] ,
2673 * InitGWResVol=[ 50 ] ( mm ) , GWResK=[ 0. 96 ] ( mm / day / mm )
2674 * VHydCond=[ 0. 055 ] ( mm / hr ) , END=- 1
2675 *%-----|
2676 COMPUTE DUALHYD NHYDin=[ " S- 1- D5 " ] , CI NLET=[ 1. 5 ] ( cms ) , NI NLET=[ 1 ] ,
2677 Mij NHYD=[ " S- 1- D5J " ]
2678 MnNHYD=[ " S- 1- D5N " ]
2679 TMSTO=[ 9999999 ] ( cu- m )
2680 *%-----|
2681 ADD HYD NHYDsum=[ " S- 1- D5S " ] , NHYDsto add=[ " S- 1- D5J " + " S- 1- D5N " ]
2682 *%-----|
2683 ROUTE RESERVOIR NHYDout=[ " S- 1- D5R " ] , NHYDin=[ " S- 1- D5S " ] ,
2684 RDT=[ 1 ] ( mi n ) ,
2685 TABLE of ( OUTFLOW STORAGE ) values
2686 ( cms ) - ( ha- m )
2687 [ 0. 0 , 0. 0 ]
2688 [ 0. 1535 , 0. 5120 ]
2689 [ - 1 , - 1 ] ( max twenty pts )
2690 NHYDovf=[ " S- 1- D5Rovf " ]
2691 *%-----|
2692 CONTINUOUS STANDHYD NHYD=[ " S- 1- D6 " ] , DT=[ 1 ] ( mi n ) , AREA=[ 1. 75 ] ( ha ) , XI MP=[ 0. 65 ] ,
TI MP=[ 0. 65 ] , DWF=[ 0 ] ( cms ) ,
2693 LOSS=[ 2 ] , SCS curve number CN=[ 75 ] , Pervious surfaces:
I Aper=[ 4. 67 ] ( mm ) , SLPP=[ 2. 0 ] ( % ) ,
2694 LGP=[ 40 ] ( m ) , MNP=[ 0. 25 ] , SCP=[ 0 ] ( mi n ) , Impervious surfaces:
IAi mp=[ 1. 57 ] ( mm ) , SLPI=[ 0. 75 ] ( % ) ,
2695 LGI=[ 108. 01 ] ( m ) , MNI=[ 0. 013 ] , SCI=[ 0 ] ( mi n ) ,
2696 Continuous simulation parameters:
IaRECper=[ 4 ] ( hr s ) , IaRECI mp=[ 4 ] ( hr s ) ,
2697 SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
2698 InterEventTime=[ 12 ] ( hr s ) , END=- 1
2699 *%-----|
2700 *CONTINUOUS NASHYD NHYD=[ " S- 1- D6 " ] , DT=[ 1 ] mi n , AREA=[ 1. 75 ] ( ha ) ,
2701 DWF=[ 0 ] ( cms ) , CN C=[ 77 ] , IA=[ 4. 67 ] ( mm ) ,
2702 N=[ 3 ] , TP=[ 1. 10 ] hr s ,
2703 Continuous simulation parameters:
2704 IaRECper=[ 4 ] ( hr s ) ,
2705 SMN=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
2706 InterEventTime=[ 12 ] ( hr s )
2707 Baseflow simulation parameters:
2708 BaseFlowOption=[ 1 ] ,
2709 InitGWResVol=[ 50 ] ( mm ) , GWResK=[ 0. 96 ] ( mm / day / mm )
2710 VHydCond=[ 0. 055 ] ( mm / hr ) , END=- 1
2711 *%-----|
2712 COMPUTE DUALHYD NHYDin=[ " S- 1- D6 " ] , CI NLET=[ 0. 232 ] ( cms ) , NI NLET=[ 1 ] ,
2713 Mij NHYD=[ " S- 1- D6J " ]
2714 MnNHYD=[ " S- 1- D6N " ]
2715 TMSTO=[ 9999999 ] ( cu- m )
2716 *%-----|
2717 ADD HYD NHYDsum=[ " S- 1- D6S " ] , NHYDsto add=[ " S- 1- D6J " + " S- 1- D6N " ]
2718 *%-----|
2719 ROUTE RESERVOIR NHYDout=[ " S- 1- D6R " ] , NHYDin=[ " S- 1- D6S " ] ,
2720 RDT=[ 1 ] ( mi n ) ,
2721 TABLE of ( OUTFLOW STORAGE ) values
2722 ( cms ) - ( ha- m )
2723 [ 0. 0 , 0. 0 ]
2724 [ 0. 0209 , 0. 0698 ]
2725 [ - 1 , - 1 ] ( max twenty pts )
2726 NHYDovf=[ " S- 1- D6Rovf " ]
2727 *%-----|
2728 CONTINUOUS STANDHYD NHYD=[ " S- 1- D7 " ] , DT=[ 1 ] ( mi n ) , AREA=[ 2. 03 ] ( ha ) , XI MP=[ 0. 65 ] ,
2729 TI MP=[ 0. 65 ] , DWF=[ 0 ] ( cms ) ,
2730 LOSS=[ 2 ] , SCS curve number CN=[ 75 ] , Pervious surfaces:

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2731 I Ape r=[ 4. 67]( mm), SLPP=[ 2. 0]( %),
LGP=[ 40]( m), MNP=[ 0. 25], SCP=[ 0]( mi n), I mperv ious surfaces:
2732 I A i mp=[ 1. 57]( mm), SLPI=[ 0. 75]( %),
2733 LGI=[ 116. 33]( m), MNI=[ 0. 013], SCI=[ 0]( mi n),
2734 C ont i nuous s i mulation parameters:
2735 I aRECper=[ 4]( hrs), I aRECI mp=[ 4]( hrs),
2736 SM N=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2737 I nterEventTi me=[ 12]( hrs), END=- 1
2737 *%-----|-----|
2738 *CONTINUOUS NASHYD NHYD=["S- 1- D7"], DT=[ 1] mi n, AREA=[ 2. 03]( ha),
2739 * DWF=[ 0]( cms), CN C=[ 77], I A=[ 4. 67]( mm),
2740 * N=[ 3], TP=[ 1. 10] hrs,
2741 * C ont i nuous s i mulation parameters:
2742 * I aRECper=[ 4]( hrs),
2743 * SM N=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2744 * I nterEventTi me=[ 12]( hrs)
2745 * B as eflow s i mulation parameters:
2746 * B as eFlowOpt ion=[ 1],
2747 * I n i t GWR es Vol=[ 50]( mm), GWR es K=[ 0. 96]( mm/ day/ mm)
2748 * VHydCond=[ 0. 055]( mm/ hr), END=- 1
2749 *%-----|-----|
2750 COMPUTE DUALHYD NHYD i n=["S- 1- D7"], CI NLET=[ 0. 265]( cms), NI NLET=[ 1],
2751 M a j NHYD=["S- 1- D7J"]
2752 M nNHYD=["S- 1- D7N"]
2753 TM STO=[ 9999999]( cu- m)
2754 *%-----|-----|
2755 ADD HYD NHYDs um=["S- 1- D7S"], NHYDs t o add=["S- 1- D7J"+"S- 1- D7N"]
2756 *%-----|-----|
2757 ROUTE RESERVOIR NHYDout=["S- 1- D7R"], NHYD i n=["S- 1- D7S"],
2758 RDT=[ 1]( mi n),
2759 T A B L E of ( O U T F L O W S T O R A G E ) v a l u e s
2760 ( cms ) - ( ha- m)
2761 [ 0. 0 , 0. 0 ]
2762 [ 0. 0243, 0. 0810 ]
2763 [ - 1 , - 1 ] (max twenty pts)
2764 NHYDovf=["S- 1- D7Rovf"]
2765 *%-----|-----|
2766 * -JFSA 2021-02-26 "S-1-D8" is a part of S-1 sub-catchment. It is moved to drain
before station 6016 on Jock River
2767 *CONTINUOUS STANDHYD NHYD=["S- 1- D8"], DT=[ 1]( mi n), AREA=[ 5. 27]( ha), XI MP=[ 0. 65],
TI MP=[ 0. 65], DWF=[ 0]( cms),
2768 * LOSS=[ 2], SCS curve number CN=[ 75], Pervious surfaces:
I Ape r=[ 4. 67]( mm), SLPP=[ 2. 0]( %),
2769 * LGP=[ 40]( m), MNP=[ 0. 25], SCP=[ 0]( mi n), I mperv ious surfaces:
I A i mp=[ 1. 57]( mm), SLPI=[ 0. 75]( %),
2770 * LGI=[ 187. 439]( m), MNI=[ 0. 013], SCI=[ 0]( mi n),
2771 * C ont i nuous s i mulation parameters:
2772 * I aRECper=[ 4]( hrs), I aRECI mp=[ 4]( hrs),
2773 * SM N=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2774 * I nterEventTi me=[ 12]( hrs), END=- 1
2775 *%-----|-----|
2776 *CONTINUOUS NASHYD NHYD=["S- 1- D8"], DT=[ 1] mi n, AREA=[ 5. 27]( ha),
2777 * DWF=[ 0]( cms), CN C=[ 77], I A=[ 4. 67]( mm),
2778 * N=[ 3], TP=[ 1. 10] hrs,
2779 * C ont i nuous s i mulation parameters:
2780 * I aRECper=[ 4]( hrs),
2781 * SM N=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
2782 * I nterEventTi me=[ 12]( hrs)
2783 * B as eflow s i mulation parameters:
2784 * B as eFlowOpt ion=[ 1],
2785 * I n i t GWR es Vol=[ 50]( mm), GWR es K=[ 0. 96]( mm/ day/ mm)
2786 * VHydCond=[ 0. 055]( mm/ hr), END=- 1
2787 *%-----|-----|
2788 *COMPUTE DUALHYD NHYD i n=["S- 1- D8"], CI NLET=[ 2. 279]( cms), NI NLET=[ 1],
2789 * M a j NHYD=["S- 1- D8J"]
2790 * M nNHYD=["S- 1- D8N"]

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2791 *          TMI STO=[ 9999999 ] ( cu- m)
2792 *%-----|-----|
2793 *ADD HYD          NHYDs um=[ " S- 1- D8S" ], NHYDs to add=[ " S- 1- D8J" +" S- 1- D8N" ]
2794 *%-----|-----|
2795 *ADD HYD          NHYDs um=[ " S- 1- D" ], NHYDs to add=[ " S- 1- Okeefe" +" S- 1" +" S- 1- Fost" ]
2796 *%-----|-----|
2797 *COMPUTE DUALHYD  NHYDi n=[ " S- 1- D" ], CI NLET=[ 11. 616 ] ( cms ), NI NLET=[ 1 ],
2798 *                Mj NHYD=[ " S- 1- D- M" ]
2799 *                MnNHYD=[ " S- 1- D- MN" ]
2800 *          TMI STO=[ 5974 ] ( cu- m)
2801 *%-----|-----|
2802 *ADD HYD          NHYDs um=[ " S- 1- DEV" ], NHYDs to add=[ " S- 1- D- M" +" S- 1- D- MN" ]
2803 *%-----|-----|
2804 *ROUTE RESERVOIR NHYDout=[ " S- 1- D8R" ] , NHYDi n=[ " S- 1- D8S" ] ,
2805 *                RDT=[ 1 ] ( mi n),
2806 *                TABLE of ( OUTFLOW STORAGE ) values
2807 *                    ( cms ) - ( ha- m)
2808 *                    [ 0. 0      , 0. 0 ]
2809 *                    [ 0. 0630, 0. 2102 ]
2810 *                    [ - 1      , - 1      ] ( max twenty pts)
2811 *                NHYDovf=[ " S- 1- D8Rovf" ]
2812 *%-----|-----|
2813 *%-----|-----|
2814 *          - JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
2815 *#      Catchment W_CLAR
2816 *#      - To West Clarke Drain (south of the Jock)
2817 *#      - Subdivision with 43% imp. as per Barrhaven South MS
2818 *#      - 2020-11-30 update CLARKE Tributary Drainage Area to = 121 ha based on
P598(04)-11
2819 *#      - 2020-11-30 split CLARKE Drainage Area to MAJOR and ALL
2820 *#*****
2821 CONTINUOUS STANDHYD NHYD=[ " W_CLAR_M" ], DT=[ 1 ] mi n, AREA=[ 1. 772 ] ( ha ),
2822 XI MP=[ 0. 46 ], TI MP=[ 0. 59 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
2823 SCS curve number CN=[ 77 ],
2824 Pervious surfaces: I Aper=[ 4. 67 ] ( mm ), SLPP=[ 1 ] ( % ),
2825 LGP=[ 40 ] ( m ), MNP=[ 0. 25 ], SCP=[ 0 ] ( mi n),
2826 Impervious surfaces: I Ai mp=[ 1. 57 ] ( mm ), SLPI=[ 1 ] ( % ),
2827 LGI=[ 109 ] ( m ), MNI=[ 0. 013 ], SCI=[ 0 ] ( mi n),
2828 Continuous simulation parameters:
2829 Ia RECper=[ 4 ] ( hr s), Ia RECI mp=[ 4 ] ( hr s),
2830 SM N=[ - 1 ] ( mm ), SMAX=[ - 1 ] ( mm ), SK=[ 0. 010 ] / ( mm ),
2831 Inter Event Time=[ 18 ] ( hr s), END=- 1
2832 *%-----|-----|
2833 *COMPUTE DUALHYD  NHYDi n=[ " W_CLAR_M" ], CI NLET=[ 0. 213 ] ( cms ), NI NLET=[ 1 ],
2834 *                Mj NHYD=[ " W_CLAR_Mj" ]
2835 *                MnNHYD=[ " W_CLAR_Mn" ]
2836 *                TMI STO=[ 0. 1 ] ( cu- m)
2837 *%-----|-----|
2838 *# 5- Year + 12% Capture
2839 ROUTE RESERVOIR NHYDout=[ " W_CLAR_Mn" ] , NHYDi n=[ " W_CLAR_M" ] ,
2840 RDT=[ 1 ] ( mi n),
2841 TABLE of ( OUTFLOW STORAGE ) values
2842 ( cms ) - ( ha- m)
2843 [ 0. 0      , 0. 0 ]
2844 [ 0. 213 , 0. 0001 ]
2845 [ - 1      , - 1      ] ( max twenty pts)
2846 NHYDovf=[ " W_CLAR_Mj" ] ,
2847 *%-----|-----|
2848 *          - Clarke_All area from P 598(04)-11 = 120.207 ha, change to 127.298 ha based on
GIS measurements,
2849 *          - JFSA, 2021-01-19 update W_CLAR_ALL to (121.17-1.772=119.398) ha based on GIS
measurements W_CLAR is 121.17 ha and W_CLAR_M is 1.772 ha
2850 CONTINUOUS STANDHYD NHYD=[ " W_CLAR_ALL" ], DT=[ 1 ] mi n, AREA=[ 119. 398 ] ( ha ),
2851 XI MP=[ 0. 60 ], TI MP=[ 0. 65 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
2852 SCS curve number CN=[ 77 ],

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2853          Pervious surfaces: I A per=[ 4.67](mm), SLPP=[ 1](%),
2854                    LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
2855          Impervious surfaces: I A imp=[ 1.57](mm), SLPI=[ 1](%),
2856                    LGI=[ 892.18](m), MNI=[ 0.013], SCI=[ 0](min),
2857          Continuous simulation parameters:
2858          I a REC per=[ 4](hrs), I a REC imp=[ 4](hrs),
2859          SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2860          Inter Event Time=[ 18](hrs), END=- 1
2861  *%-----|-----|
2862  ADD HYD          NHYDsum=[ "W_CLAR" ], NHYDs to add=[ "W_CLAR_ALL"+"W_CLAR_Mj" ]
2863  *%-----|-----|
2864  SAVE HYD        NHYD=[ "W_CLAR" ], # OF PCYCLES=[ -1 ], I CASEs h=[ 1 ]
2865                  HYD_COMMENT=[ "Total Flows to West Clarke" ]
2866  *#*****|
2867  *# West Clarke Pond 2
2868  *# - Rating curve obtained from Barrhaven South M&S modeling
2869  *# - Tributary Drainage Area to M&S Pond 2 = 241 ha
2870  *#*****|
2871  ROUTE RESERVOIR NHYDout=[ "MS_P2" ], NHYDin=[ "W_CLAR" ],
2872                  RDT=[ 1](min),
2873                  TABLE of ( OUTFLOW STORAGE ) values
2874                  (cms) - (ha-m)
2875                  [ 0.0 , 0.0 ]
2876                  [ 0.128 , 0.161 ]
2877                  [ 0.138 , 0.409 ]
2878                  [ 0.148 , 0.68 ]
2879                  [ 0.227 , 0.931 ]
2880                  [ 0.354 , 1.223 ]
2881                  [ 0.505 , 1.52 ]
2882                  [ 0.666 , 1.821 ]
2883                  [ 0.831 , 2.123 ]
2884                  [ 0.995 , 2.434 ]
2885                  [ 1.069 , 2.583 ]
2886                  [ 1.51 , 2.647 ]
2887                  [ 4.904 , 2.861 ]
2888                  [ 13.048 , 3.188 ]
2889                  [ 23.745 , 3.523 ]
2890                  [ 36.474 , 3.871 ]
2891                  [ 45.938 , 4.127 ]
2892                  [ 61.652 , 4.539 ]
2893                  [ -1 , -1 ] (max twenty pts)
2894                  NHYDovf=[ "P2-OVF" ]
2895  *%-----|-----|
2896  *#*****|
2897  * - JFSA, 2021-01-22 "W_CLAR_UNDE" (west of Clarke sub-catchment) discharges
directly to the Jock River through a road side ditch on the west side of Borrisokane
road (station 6016)
2898  *CONTINUOUS NASHYD NHYD=[ "W_CLAR_UNDE" ], DT=[ 1]min, AREA=[ 35.65](ha),
2899  * DWF=[ 0](cms), CNVC=[ 77], IA=[ 4.67](mm),
2900  * N=[ 3], TP=[ 1.10]hrs,
2901  * Continuous simulation parameters:
2902  * I a REC per=[ 4](hrs),
2903  * SM N=[ -1](mm), SMAX=[ -1](mm), SK=[ 0.010]/(mm),
2904  * Inter Event Time=[ 12](hrs)
2905  * Baseflow simulation parameters:
2906  * BaseFlowOption=[ 1 ],
2907  * I n i t GWR es Vol=[ 50](mm), GWR es K=[ 0.96](mm/day/mm)
2908  * VHydCond=[ 0.055](mm/hr), END=- 1
2909  *%-----|-----|
2910  ADD HYD          NHYDsum=[ "SN_CE" ], NHYDs to
add=[ "N_CE"+"S-1-D4R"+"S-1-D5R"+"S-1-D4Rovf"+"S-1-D5Rovf"+"MS_P2"+"P2-OVF" ]
2911  *%-----|-----|
2912  SAVE HYD        NHYD=[ "SN_CE" ], # OF PCYCLES=[ -1 ], I CASEs h=[ 1 ]
2913                  HYD_COMMENT=[ "Total Flows before Station 5737 on Jock River" ]
2914  *%-----|-----|
2915  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5737

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2916  *# 2021-02-25 add station 5737 before station 5002. Station 5737 was extracted from the
      HEC-RAS model
      T:\PROJ\1474-16\Design\20201026-QuantityControlAnalysis\HEC-RAS\JockLi dar 2005
2917  *# JFSA 2021-03-02 change the slope to 0.0175% instead of 0.02593 to stabilize the model
2918  ROUTE CHANNEL      NHYDout=["5737"] , NHYDin=["SN_CE"] ,
2919                      RDT=[1](min),
2920                      CHLGTH=[270](m),   CHSLOPE=[0.0175](%),
2921                      FPSLOPE=[0.0175](%),
2922                      SECNUM=[1.0],      NSEG=[3]
2923                      ( SEGROUGH, SEGDIST (m))=
2924                      [0.050, -24.04
2925                      -0.035, 23.92
2926                      0.050, 1130.8] NSEG times
2927                      ( DISTANCE (m), ELEVATION (m))=
2928                      [-1060.52, 94 ]
2929                      [-268.6, 91.5 ]
2930                      [-259.43, 91.5 ]
2931                      [-179.48, 91.5 ]
2932                      [-67.9, 91.5 ]
2933                      [-59.21, 91.5 ]
2934                      [-33.19, 91 ]
2935                      [-26.08, 90.5 ]
2936                      [-24.04, 90 ]
2937                      [-13.14, 86.77 ]
2938                      [0, 85 ]
2939                      [14.68, 86.74 ]
2940                      [23.92, 90 ]
2941                      [25.78, 90.5 ]
2942                      [31.91, 91 ]
2943                      [91.95, 91.5 ]
2944                      [772.15, 92 ]
2945                      [961.49, 92.5 ]
2946                      [1044.69, 93 ]
2947                      [1130.8, 95 ]
2948  *%-----|-----|
2949  ADD HYD      NHYDsum=["5002"], NHYDsto
      add=["5737"+"S-1-D1R"+"S-1-D6R"+"S-1-D7R"+"S-1-D1Rovf"+"S-1-D6Rovf"+"S-1-D7Rovf"]
2950  *%-----|-----|
2951  SAVE HYD      NHYD=["5002"], # OF PCYCLES=[-1], ICASEsh=[1]
2952                      HYD_COMMENT=["Total Flows before Station 5002 on Jock River"]
2953  *%-----|-----|
2954  *# Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain
2955  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 5002
2956  *# JFSA 2021-02-19 Change the slope from 0.01 % (as per Stantec Report 2007) to 0.0255
      % so the model will be more stable and give reasonable results. It is justifiable as
      ROUTE CHANNELs aren't well suited to really flat slopes.
2957  *# JFSA 2021-02-19 Change to three ROUTE CHANNEL with length 275 m each instead of one
      with 825 m length so the model will be more stable
2958  *# JFSA 2021-02-26 change the length of 5002 route channel from 825 m to 736 m That is
      because of adding station 5737 between station 6016 and station 5002. Then the length
      from station 5737 to station 5002 is 736 m Change the slope from 0.0255 % to 0.09511 %
2959  *
2960  ROUTE CHANNEL      NHYDout=["N_WCa"] , NHYDin=["5002"] ,
2961                      RDT=[1](min),
2962                      CHLGTH=[245.33333](m),   CHSLOPE=[0.09511](%),
2963                      FPSLOPE=[0.09511](%),
2964                      SECNUM=[1.0],      NSEG=[3]
2965                      ( SEGROUGH, SEGDIST (m))=
2966                      [0.050, -37.5
2967                      -0.035, 37.50
2968                      0.050, 157.05] NSEG times
2969                      ( DISTANCE (m), ELEVATION (m))=
2970                      [-601.81, 91.5 ]
2971                      [-37.50, 90.00]
2972                      [-19.61, 87.04]
2973                      [0.00, 85.70]

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2974 [ 14. 87, 86. 93]
2975 [ 37. 50, 90. 00]
2976 [ 38. 54, 90. 50]
2977 [ 42. 23, 91]
2978 [ 157. 05, 91. 50]
2979 * [ 161. 44, 91. 50]
2980 * [ 236. 48, 93. 00]
2981 * [ 385. 47, 92. 50]
2982 * [ 390. 78, 92. 50]
2983 *%-----|
2984 ROUTE CHANNEL NHYDout =["N_WCb" ] , NHYDin=["N_WCa" ] ,
2985 RDT=[ 1]( mi n) ,
2986 CHLGTH=[ 245. 33333]( m) , CHSLOPE=[ 0. 09511]( % ,
2987 FPSLOPE=[ 0. 09511]( % ,
2988 SECNUM=[ 1. 0] , NSEG=[ 3]
2989 ( SEGROUGH, SEGDI ST ( m) ) =
2990 [ 0. 050, - 37. 5
2991 - 0. 035, 37. 50
2992 0. 050, 157. 05] NSEG t i m e s
2993 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
2994 [- 601. 81, 91. 5]
2995 [- 37. 50, 90. 00]
2996 [- 19. 61, 87. 04]
2997 [ 0. 00, 85. 70]
2998 [ 14. 87, 86. 93]
2999 [ 37. 50, 90. 00]
3000 [ 38. 54, 90. 50]
3001 [ 42. 23, 91]
3002 [ 157. 05, 91. 50]
3003 *%-----|
3004 ROUTE CHANNEL NHYDout =["N_WC" ] , NHYDin=["N_WCb" ] ,
3005 RDT=[ 1]( mi n) ,
3006 CHLGTH=[ 245. 33333]( m) , CHSLOPE=[ 0. 09511]( % ,
3007 FPSLOPE=[ 0. 09511]( % ,
3008 SECNUM=[ 1. 0] , NSEG=[ 3]
3009 ( SEGROUGH, SEGDI ST ( m) ) =
3010 [ 0. 050, - 37. 5
3011 - 0. 035, 37. 50
3012 0. 050, 157. 05] NSEG t i m e s
3013 ( DI STANCE ( m) , ELEVATI ON ( m) ) =
3014 [- 601. 81, 91. 5]
3015 [- 37. 50, 90. 00]
3016 [- 19. 61, 87. 04]
3017 [ 0. 00, 85. 70]
3018 [ 14. 87, 86. 93]
3019 [ 37. 50, 90. 00]
3020 [ 38. 54, 90. 50]
3021 [ 42. 23, 91]
3022 [ 157. 05, 91. 50]
3023 *#*****
3024 * - JFSA 2021-02-08 Clarke (MS_P2 and P2-OVF) and Clarke Undeveloped area
(W_CLAR_UNDE) drain to Jock River at Station 5002 instead of Station 4534
3025 *ADD HYD NHYDs um=["SN_WC" ] , NHYDs to
add=["MS_P2"+"P2-OVF"+"N_WC"+"W_CLAR_UNDE" ]
3026 *%-----|
3027 *SAVE HYD NHYD=["SN_WC" ] , # OF PCYCLES=[ - 1] , ICASEsh=[ 1]
3028 * HYD_COMMENT=["Total Flows at West Clarke Pond Outlet" ]
3029 *%-----|
3030 *# Hydrograph from Node West Clarke routed to Node at Kennedy - Burnett Drain
3031 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4534
3032 *#
3033 ROUTE CHANNEL NHYDout =["N_KB" ] , NHYDin=["N_WC" ] ,
3034 RDT=[ 1]( mi n) ,
3035 CHLGTH=[ 1020]( m) , CHSLOPE=[ 0. 0498]( % ,
3036 FPSLOPE=[ 0. 0498]( % ,
3037 SECNUM=[ 1. 0] , NSEG=[ 3]

```

```

3038 ( SEGROUGH, SEGDI ST ( m ) ) =
3039 [ 0. 050, - 23. 63
3040 - 0. 035, 23. 63
3041 0. 050, 728. 3 ] NSEG t i m e s
3042 ( DI STANCE ( m ) , ELEVATI ON ( m ) ) =
3043 [ - 1082. 01, 94 ]
3044 [ - 1028. 17, 92. 5 ]
3045 [ - 992. 3, 93. 5 ]
3046 [ - 279. 34, 90 ]
3047 [ - 23. 63, 90 ]
3048 [ - 13. 45, 87. 13 ]
3049 [ - 0. 07, 86. 24 ]
3050 [ 10. 54, 87. 15 ]
3051 [ 23. 63, 90 ]
3052 [ 24. 86, 90. 5 ]
3053 [ 26. 72, 91 ]
3054 [ 45. 07, 91. 5 ]
3055 [ 128. 17, 91. 5 ]
3056 [ 270. 7, 92. 5 ]
3057 [ 728. 3, 95 ]
3058 *%-----|-----|
3059 *#*****|
3060 *# Catchment KEN_BU
3061 *# - To Kennedy-Burnett SWM Facility
3062 *# - Outlets to Fraser-Clarke drain (north of the Jock)
3063 *# - Medium density residential subdivision
3064 * - Add Kennedy Burnett model (Convert PCSWMM from NOVATECH June, 2020 to SWWHYMO)
3065 *#*****|
3066 *CONTINUOUS STANDHYD NHYD=["KEN_BU"], DT=[ 1 ] mi n, AREA=[ 281 ] ( ha ),
3067 * XI MP=[ 0. 55 ], TI MP=[ 0. 55 ], DWF=[ 0 ] ( cms ), LOSS=[ 2 ],
3068 * SCS curve number CN=[ 71 ],
3069 * Per vious surfaces: I A per =[ 4. 67 ] ( mm ), SLPP=[ 1 ] ( % ),
3070 * LGP=[ 40 ] ( m ), MNP=[ 0. 25 ], SCP=[ 0 ] ( mi n ),
3071 * Impervious surfaces: I A i mp =[ 1. 57 ] ( mm ), SLPI =[ 1 ] ( % ),
3072 * LGI =[ 1369 ] ( m ), MNI =[ 0. 013 ], SCI =[ 0 ] ( mi n ),
3073 * Continuous simulation parameters:
3074 * I a REC per =[ 4 ] ( hrs ), I a REC i mp =[ 4 ] ( hrs ),
3075 * SM N=[ - 1 ] ( mm ), SMAX=[ - 1 ] ( mm ), SK=[ 0. 010 ] / ( mm ),
3076 * Inter Event Ti m e =[ 18 ] ( hrs ), END=- 1
3077 *%-----|-----|
3078 *#*****|
3079 *# Existing Kennedy-Burnett SWM Facility
3080 *# - Rating curve obtained from URTKBP
3081 *# - Tributary Drainage Area to Pond = 160 ha
3082 *#*****|
3083 *ROUTE RESERVOIR NHYDout=["KEN_P"], NHYDin=["KEN_BU"],
3084 * RDT=[ 1 ] ( mi n ),
3085 * TABLE of ( OUTFLOW STORAGE ) values
3086 * ( cms ) - ( ha-m )
3087 * [ 0. 0 , 0. 0 ]
3088 * [ 0. 13 , 0. 26 ]
3089 * [ 0. 43 , 0. 56 ]
3090 * [ 0. 67 , 0. 90 ]
3091 * [ 0. 86 , 1. 32 ]
3092 * [ 1. 01 , 1. 79 ]
3093 * [ 1. 15 , 2. 33 ]
3094 * [ - 1 , - 1 ] ( max twenty pts )
3095 * NHYDovf=["KEN_OV"]
3096 *%-----|-----|
3097 * -JFSA, 2021-01-19 update all KEN_BU areas based on GIS measurements
3098 CONTINUOUS STANDHYD NHYD=["KB-01A"], DT=[ 1 ] mi n, AREA=[ 40. 82 ] ( ha ), XI MP=[ 0. 097 ],
3099 TI MP=[ 0. 4 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3099 Hort on: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
3100 F=[ 0. 00 ] ( mm ),
3100 Per vious areas: I A per =[ 4. 67 ] ( mm ), SLPP=[ 0. 5 ] ( % ), LGP=[ 40 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),

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3101      Impervious areas: I Aimp=[ 0.785](mm), SLPI=[ 0.5](%),
3102      LGI=[ 521.664](m), MNI=[ 0.013], SCI=[ 0](min),
3103      Continuous simulation parameters:
3103      I aRECPer=[ 4](hrs), I aRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3103      END=- 1
3104      *%-----|-----|
3104      COMPUTE DUALHYD      NHYDin=[ "KB-01A"], CINLET=[ 3.6](cms), NINLET=[ 1],
3105      MijNHYD=[ "KB-01A-M"]
3106      MnNHYD=[ "KB-01A-MN"]
3107      TMSTO=[ 4995](cu-m)
3108
3109      *%-----|-----|
3109      ADD HYD      NHYDsum=[ "KB-01A-S"], NHYDs to add=[ "KB-01A-M"+"KB-01A-MN"]
3110
3111      *%-----|-----|
3111      CONTINUOUS STANDHYD      NHYD=[ "KB-01B"], DT=[ 1]min, AREA=[ 31.1](ha), XI MP=[ 0.1875],
3112      TI MP=[ 0.375], DWF=[ 0](cms), LOSS=[ 1]:
3113      Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3113      F=[ 0.00](mm),
3114      Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 0.42](%), LGP=[ 40](m),
3114      MNP=[ 0.250], SCP=[ 0](min),
3115      Impervious areas: I Aimp=[ 0.785](mm), SLPI=[ 0.42](%),
3115      LGI=[ 455.339](m), MNI=[ 0.013], SCI=[ 0](min),
3116      Continuous simulation parameters:
3117      I aRECPer=[ 4](hrs), I aRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3117      END=- 1
3118      *%-----|-----|
3118      COMPUTE DUALHYD      NHYDin=[ "KB-01B"], CINLET=[ 1.585](cms), NINLET=[ 1],
3119      MijNHYD=[ "KB-01B-M"]
3120      MnNHYD=[ "KB-01B-MN"]
3121      TMSTO=[ 6075](cu-m)
3122
3123      *%-----|-----|
3123      ADD HYD      NHYDsum=[ "KB-01B-S"], NHYDs to add=[ "KB-01B-M"+"KB-01B-MN"]
3124
3125      *%-----|-----|
3125      CONTINUOUS STANDHYD      NHYD=[ "KB-01C"], DT=[ 1]min, AREA=[ 13.78](ha), XI MP=[ 0.2045],
3126      TI MP=[ 0.409], DWF=[ 0](cms), LOSS=[ 1]:
3127      Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3127      F=[ 0.00](mm),
3128      Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3128      MNP=[ 0.250], SCP=[ 0](min),
3129      Impervious areas: I Aimp=[ 0.785](mm), SLPI=[ 0.5](%),
3129      LGI=[ 303.095](m), MNI=[ 0.013], SCI=[ 0](min),
3130      Continuous simulation parameters:
3131      I aRECPer=[ 4](hrs), I aRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3131      END=- 1
3132      *%-----|-----|
3132      COMPUTE DUALHYD      NHYDin=[ "KB-01C"], CINLET=[ 1.35](cms), NINLET=[ 1],
3133      MijNHYD=[ "KB-01C-M"]
3134      MnNHYD=[ "KB-01C-MN"]
3135      TMSTO=[ 1880](cu-m)
3136
3137      *%-----|-----|
3137      ADD HYD      NHYDsum=[ "KB-01C-S"], NHYDs to add=[ "KB-01C-M"+"KB-01C-MN"]
3138
3139      *%-----|-----|
3139      CONTINUOUS STANDHYD      NHYD=[ "KB-03"], DT=[ 1]min, AREA=[ 84.78](ha), XI MP=[ 0.197],
3140      TI MP=[ 0.394], DWF=[ 0](cms), LOSS=[ 1]:
3141      Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
3141      F=[ 0.00](mm),
3142      Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3142      MNP=[ 0.250], SCP=[ 0](min),
3143      Impervious areas: I Aimp=[ 0.785](mm), SLPI=[ 0.63](%),
3143      LGI=[ 751.798](m), MNI=[ 0.013], SCI=[ 0](min),
3144      Continuous simulation parameters:
3145      I aRECPer=[ 4](hrs), I aRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3145      END=- 1
3146      *%-----|-----|
3146      COMPUTE DUALHYD      NHYDin=[ "KB-03"], CINLET=[ 5.27](cms), NINLET=[ 1],
3147      MijNHYD=[ "KB-03-M"]
3148      MnNHYD=[ "KB-03-MN"]
3149

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3150          TMI STO=[ 15500] (cu-m)
3151  *%-----|-----|
3152  ADD HYD      NHYDs um=[ " KB- 03- S" ], NHYDs to add=[ " KB- 03- M " + " KB- 03- MN" ]
3153  *%-----|-----|
3154  CONTI NUOUS STANDHYD NHYD=[ " KB- 04" ], DT=[ 1] mi n, AREA=[ 6.95] (ha), XI MP=[ 0.85],
TI MP=[ 0.85], DWF=[ 0] (cms), LOSS=[ 1]:
3155          Hort on: Fo=[ 76.20] (mm/ hr), Fc=[ 13.20] (mm/ hr), DCAY=[ 4.14] (/ hr),
F=[ 0.00] (mm),
3156          Perv ious areas: I Aper=[ 4.67] (mm), SLPP=[ 2.0] (%), LGP=[ 40] (m),
MNP=[ 0.250], SCP=[ 0] (mi n),
3157          Imperv ious areas: I Ai mp=[ 0.942] (mm), SLPI =[ 0.5] (%),
LGI =[ 215.252] (m), MNI =[ 0.013], SCI =[ 0] (mi n),
3158          Continuous simulation parameters:
3159          IaRECper=[ 4] (hr s), IaRECI mp=[ 4] (hr s), Int er Event Ti me=[ 12] (hr s),
END=- 1
3160  *%-----|-----|
3161  COMPUTE DUALHYD NHYDi n=[ " KB- 04" ], CI NLET=[ 0.503] (cms), NI NLET=[ 1],
3162          Maj NHYD=[ " KB- 04- M " ]
3163          M nNHYD=[ " KB- 04- MN" ]
3164          TMI STO=[ 1972] (cu-m)
3165  *%-----|-----|
3166  ADD HYD      NHYDs um=[ " KB- 04- S" ], NHYDs to add=[ " KB- 04- M " + " KB- 04- MN" ]
3167  *%-----|-----|
3168  CONTI NUOUS STANDHYD NHYD=[ " KB- 05" ], DT=[ 1] mi n, AREA=[ 5.19] (ha), XI MP=[ 0.93],
TI MP=[ 0.93], DWF=[ 0] (cms), LOSS=[ 1]:
3169          Hort on: Fo=[ 76.20] (mm/ hr), Fc=[ 13.20] (mm/ hr), DCAY=[ 4.14] (/ hr),
F=[ 0.00] (mm),
3170          Perv ious areas: I Aper=[ 4.67] (mm), SLPP=[ 2.0] (%), LGP=[ 40] (m),
MNP=[ 0.250], SCP=[ 0] (mi n),
3171          Imperv ious areas: I Ai mp=[ 1.57] (mm), SLPI =[ 0.5] (%),
LGI =[ 186.011] (m), MNI =[ 0.013], SCI =[ 0] (mi n),
3172          Continuous simulation parameters:
3173          IaRECper=[ 4] (hr s), IaRECI mp=[ 4] (hr s), Int er Event Ti me=[ 12] (hr s),
END=- 1
3174  *%-----|-----|
3175  *%-----|-----|
3176  CONTI NUOUS STANDHYD NHYD=[ " KB- 06" ], DT=[ 1] mi n, AREA=[ 12.93] (ha), XI MP=[ 0.873],
TI MP=[ 0.873], DWF=[ 0] (cms), LOSS=[ 1]:
3177          Hort on: Fo=[ 76.20] (mm/ hr), Fc=[ 13.20] (mm/ hr), DCAY=[ 4.14] (/ hr),
F=[ 0.00] (mm),
3178          Perv ious areas: I Aper=[ 4.67] (mm), SLPP=[ 2.0] (%), LGP=[ 40] (m),
MNP=[ 0.250], SCP=[ 0] (mi n),
3179          Imperv ious areas: I Ai mp=[ 0.942] (mm), SLPI =[ 4.75] (%),
LGI =[ 293.598] (m), MNI =[ 0.013], SCI =[ 0] (mi n),
3180          Continuous simulation parameters:
3181          IaRECper=[ 4] (hr s), IaRECI mp=[ 4] (hr s), Int er Event Ti me=[ 12] (hr s),
END=- 1
3182  *%-----|-----|
3183  COMPUTE DUALHYD NHYDi n=[ " KB- 06" ], CI NLET=[ 2.262] (cms), NI NLET=[ 1],
3184          Maj NHYD=[ " KB- 06- M " ]
3185          M nNHYD=[ " KB- 06- MN" ]
3186          TMI STO=[ 1950] (cu-m)
3187  *%-----|-----|
3188  ADD HYD      NHYDs um=[ " KB- 06- S" ], NHYDs to add=[ " KB- 06- M " + " KB- 06- MN" ]
3189  *%-----|-----|
3190  CONTI NUOUS STANDHYD NHYD=[ " KB- 11" ], DT=[ 1] mi n, AREA=[ 4.03] (ha), XI MP=[ 0.675],
TI MP=[ 0.675], DWF=[ 0] (cms), LOSS=[ 1]:
3191          Hort on: Fo=[ 76.20] (mm/ hr), Fc=[ 13.20] (mm/ hr), DCAY=[ 4.14] (/ hr),
F=[ 0.00] (mm),
3192          Perv ious areas: I Aper=[ 4.67] (mm), SLPP=[ 2.0] (%), LGP=[ 40] (m),
MNP=[ 0.250], SCP=[ 0] (mi n),
3193          Imperv ious areas: I Ai mp=[ 0.785] (mm), SLPI =[ 2.0] (%),
LGI =[ 163.911] (m), MNI =[ 0.013], SCI =[ 0] (mi n),
3194          Continuous simulation parameters:
3195          IaRECper=[ 4] (hr s), IaRECI mp=[ 4] (hr s), Int er Event Ti me=[ 12] (hr s),
END=- 1

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3196 *%-----|-----|
3197 COMPUTE DUALHYD NHYDin=["KB-11"], CINLET=[0.5773](cms), NINLET=[1],
3198 MajNHYD=["KB-11-M"]
3199 MnNHYD=["KB-11-MN"]
3200 TMSTO=[597](cu-m)
3201 *%-----|-----|
3202 ADD HYD NHYDsum=["KB-11-S"], NHYDs to add=["KB-11-M"+"KB-11-MN"]
3203 *%-----|-----|
3204 CONTINUOUS STANDHYD NHYD=["S1"], DT=[1]min, AREA=[4.99](ha), XI MP=[0.93], TI MP=[0.93],
DWF=[0](cms), LOSS=[1]:
3205 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3206 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3207 Impervious areas: I A i mp=[1.57](mm), SLPI=[2.0](%),
LGI=[182.392](m), MNI=[0.013], SCI=[0](min),
3208 Continuous simulation parameters:
3209 I a RE C per=[4](hrs), I a RE C i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3210 *%-----|-----|
3211 CONTINUOUS STANDHYD NHYD=["KB-15"], DT=[1]min, AREA=[2.15](ha), XI MP=[0.79],
TI MP=[0.79], DWF=[0](cms), LOSS=[1]:
3212 Horton: Fo=[76.20](mm/hr), Fc=[13.20](mm/hr), DCAY=[4.14](/hr),
F=[0.00](mm),
3213 Pervious areas: I A per=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.250], SCP=[0](min),
3214 Impervious areas: I A i mp=[0.157](mm), SLPI=[0.3](%),
LGI=[119.722](m), MNI=[0.013], SCI=[0](min),
3215 Continuous simulation parameters:
3216 I a RE C per=[4](hrs), I a RE C i mp=[4](hrs), I n t e r E v e n t T i m e=[12](hrs),
END=-1
3217 *%-----|-----|
3218 *%-----|-----|
3219 ADD HYD NHYDsum=["KB-P1"], NHYDs to
add=["KB-01A-S"+"KB-01B-S"+"KB-01C-S"+"KB-03-S"+"KB-04-S"+"KB-05"+"KB-06-S"+"KB-11-S"+"KB
-15"+"S1"]
3220 *%-----|-----|
3221 ROUTE RESERVOIR NHYDout=["KB-P1R"], NHYDin=["KB-P1"],
3222 RDT=[1](min),
3223 TABLE of ( OUTFLOW STORAGE ) values
3224 (cms) - (ha-m)
3225 [ 0.0 , 0.0 ]
3226 [0.076,0.003]
3227 [0.088,0.006]
3228 [0.136,0.011]
3229 [0.301,0.017]
3230 [0.454,0.027]
3231 [0.631,0.041]
3232 [1.173,0.068]
3233 [1.91,0.111]
3234 [4.847,0.231]
3235 [9.813,0.436]
3236 [12.134,0.617]
3237 [12.438,0.732]
3238 [12.424,0.811]
3239 [12.425,0.894]
3240 [ -1 , -1 ] (max twenty pts)
3241 NHYDovf=["KB-P1ovf"]
3242 *%-----|-----|
3243 ADD HYD NHYDsum=["KB-Pond1"], NHYDs to add=["KB-P1R"+"KB-P1ovf"]
3244 *%-----|-----|
3245 SAVE HYD NHYD=["KB-Pond1"], # OF PCYCLES=[-1], I CASEs h=[1]
3246 HYD_COMMENT=["Total Flows at KB first pond"]
3247 *%-----|-----|
3248 CONTINUOUS STANDHYD NHYD=["KB-07"], DT=[1]min, AREA=[10.86](ha), XI MP=[0.86],
TI MP=[0.86], DWF=[0](cms), LOSS=[1]:

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3249      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
3250      F=[ 0.00](mm),
3251      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3252      MNP=[ 0.250], SCP=[ 0](min),
3253      Impervious areas: IAimp=[ 0.785](mm), SLPI=[ 2.0](%),
3254      LGI=[ 269.072](m), MNI=[ 0.013], SCI=[ 0](min),
3255      Continuous simulation parameters:
3256      IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3257      END=- 1
3258
3259      *%-----|-----|
3260      COMPUTE DUALHYD NHYDin=[ "KB-07"], CINLET=[ 2.094](cms), NINLET=[ 1],
3261      MajNHYD=[ "KB-07-M"]
3262      MinNHYD=[ "KB-07-MN"]
3263      TMSSTO=[ 1378](cu-m)
3264
3265      *%-----|-----|
3266      ADD HYD NHYDsum=[ "KB-07-S"], NHYDsto add=[ "KB-07-M"+"KB-07-MN"]
3267      *%-----|-----|
3268      CONTINUOUS STANDHYD NHYD=[ "KB-08"], DT=[ 1]min, AREA=[ 6.61](ha), XI MP=[ 0.64],
3269      TITMP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3270      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
3271      F=[ 0.00](mm),
3272      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3273      MNP=[ 0.250], SCP=[ 0](min),
3274      Impervious areas: IAimp=[ 0.785](mm), SLPI=[ 2.0](%),
3275      LGI=[ 209.921](m), MNI=[ 0.013], SCI=[ 0](min),
3276      Continuous simulation parameters:
3277      IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3278      END=- 1
3279
3280      *%-----|-----|
3281      COMPUTE DUALHYD NHYDin=[ "KB-08"], CINLET=[ 1.058](cms), NINLET=[ 1],
3282      MajNHYD=[ "KB-08-M"]
3283      MinNHYD=[ "KB-08-MN"]
3284      TMSSTO=[ 787](cu-m)
3285
3286      *%-----|-----|
3287      ADD HYD NHYDsum=[ "KB-08-S"], NHYDsto add=[ "KB-08-M"+"KB-08-MN"]
3288      *%-----|-----|
3289      CONTINUOUS STANDHYD NHYD=[ "KB-09"], DT=[ 1]min, AREA=[ 2.6](ha), XI MP=[ 0.86],
3290      TITMP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3291      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
3292      F=[ 0.00](mm),
3293      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3294      MNP=[ 0.250], SCP=[ 0](min),
3295      Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 2.0](%),
3296      LGI=[ 131.656](m), MNI=[ 0.013], SCI=[ 0](min),
3297      Continuous simulation parameters:
3298      IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3299      END=- 1
3300
3301      *%-----|-----|
3302      *%-----|-----|
3303      CONTINUOUS STANDHYD NHYD=[ "KB-10_1"], DT=[ 1]min, AREA=[ 2.37](ha), XI MP=[ 0.86],
3304      TITMP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3305      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
3306      F=[ 0.00](mm),
3307      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
3308      MNP=[ 0.250], SCP=[ 0](min),
3309      Impervious areas: IAimp=[ 1.57](mm), SLPI=[ 2.0](%),
3310      LGI=[ 125.698](m), MNI=[ 0.013], SCI=[ 0](min),
3311      Continuous simulation parameters:
3312      IaRECper=[ 4](hrs), IaRECImp=[ 4](hrs), InterEventTime=[ 12](hrs),
3313      END=- 1
3314
3315      *%-----|-----|
3316      CONTINUOUS STANDHYD NHYD=[ "KB-10_2"], DT=[ 1]min, AREA=[ 1.14](ha), XI MP=[ 0.86],
3317      TITMP=[ 0.86], DWF=[ 0](cms), LOSS=[ 1]:
3318      Horton: Fo=[ 76.20](mm/ hr), Fc=[ 13.20](mm/ hr), DCAY=[ 4.14](/ hr),
3319      F=[ 0.00](mm),
3320      Pervious areas: IAper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),

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3294 MNP=[ 0.250], SCP=[ 0](mi n),
Impervious areas: I Ai mp=[ 1.57](mm), SLPI=[ 2.0](%), LGI=[ 87.178](m),
MNI=[ 0.013], SCI=[ 0](mi n),
3295 Continuous simulation parameters:
3296 IaRECPer=[ 4](hrs), IaRECI mp=[ 4](hrs), Inter Event Ti me=[ 12](hrs),
END=- 1
3297 *%-----|-----|
3298 *%-----|-----|
3299 CONTINUOUS STANDHYD NHYD=["KB- 12"], DT=[ 1]mi n, AREA=[ 4.86](ha), XI MP=[ 0.79],
TI MP=[ 0.79], DWF=[ 0](cms), LOSS=[ 1]:
3300 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
3301 Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](mi n),
3302 Impervious areas: I Ai mp=[ 1.099](mm), SLPI=[ 2.0](%),
LGI=[ 180.000](m), MNI=[ 0.013], SCI=[ 0](mi n),
3303 Continuous simulation parameters:
3304 IaRECPer=[ 4](hrs), IaRECI mp=[ 4](hrs), Inter Event Ti me=[ 12](hrs),
END=- 1
3305 *%-----|-----|
3306 COMPUTE DUALHYD NHYDi n=["KB- 12"], CI NLET=[ 0.8665](cms), NI NLET=[ 1],
3307 Ma j NHYD=["KB- 12- M"]
3308 M nNHYD=["KB- 12- MN"]
3309 TM STO=[ 632](cu- m)
3310 *%-----|-----|
3311 ADD HYD NHYDs um=["KB- 12- S"], NHYDs to add=["KB- 12- M"+"KB- 12- MN"]
3312 *%-----|-----|
3313 CONTINUOUS STANDHYD NHYD=["KB- 13"], DT=[ 1]mi n, AREA=[ 10.19](ha), XI MP=[ 0.64],
TI MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3314 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
3315 Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](mi n),
3316 Impervious areas: I Ai mp=[ 0.785](mm), SLPI=[ 2.0](%),
LGI=[ 260.640](m), MNI=[ 0.013], SCI=[ 0](mi n),
3317 Continuous simulation parameters:
3318 IaRECPer=[ 4](hrs), IaRECI mp=[ 4](hrs), Inter Event Ti me=[ 12](hrs),
END=- 1
3319 *%-----|-----|
3320 COMPUTE DUALHYD NHYDi n=["KB- 13"], CI NLET=[ 1.722](cms), NI NLET=[ 1],
3321 Ma j NHYD=["KB- 13- M"]
3322 M nNHYD=["KB- 13- MN"]
3323 TM STO=[ 1077](cu- m)
3324 *%-----|-----|
3325 ADD HYD NHYDs um=["KB- 13- S"], NHYDs to add=["KB- 13- M"+"KB- 13- MN"]
3326 *%-----|-----|
3327 CONTINUOUS STANDHYD NHYD=["KB- 14"], DT=[ 1]mi n, AREA=[ 5.47](ha), XI MP=[ 0.64],
TI MP=[ 0.64], DWF=[ 0](cms), LOSS=[ 1]:
3328 Horton: Fo=[ 76.20](mm/hr), Fc=[ 13.20](mm/hr), DCAY=[ 4.14](/hr),
F=[ 0.00](mm),
3329 Pervious areas: I Aper=[ 4.67](mm), SLPP=[ 2.0](%), LGP=[ 40](m),
MNP=[ 0.250], SCP=[ 0](mi n),
3330 Impervious areas: I Ai mp=[ 0.785](mm), SLPI=[ 2.0](%),
LGI=[ 190.962](m), MNI=[ 0.013], SCI=[ 0](mi n),
3331 Continuous simulation parameters:
3332 IaRECPer=[ 4](hrs), IaRECI mp=[ 4](hrs), Inter Event Ti me=[ 12](hrs),
END=- 1
3333 *%-----|-----|
3334 COMPUTE DUALHYD NHYDi n=["KB- 14"], CI NLET=[ 0.8734](cms), NI NLET=[ 1],
3335 Ma j NHYD=["KB- 14- M"]
3336 M nNHYD=["KB- 14- MN"]
3337 TM STO=[ 631](cu- m)
3338 *%-----|-----|
3339 ADD HYD NHYDs um=["KB- 14- S"], NHYDs to add=["KB- 14- M"+"KB- 14- MN"]
3340 *%-----|-----|
3341 *%-----|-----|

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3342 CONTINUOUS STANDHYD NHYD=["KB-16_2"], DT=[1] min, AREA=[3.42] (ha), XI MP=[0.71],
TI MP=[0.71], DWF=[0] (cms), LOSS=[1]:
3343 Horton: Fo=[76.20] (mm/hr), Fc=[13.20] (mm/hr), DCAY=[4.14] (/hr),
F=[0.00] (mm),
3344 Pervious areas: I A per=[4.67] (mm), SLPP=[2.0] (%), LGP=[40] (m),
MNP=[0.250], SCP=[0] (min),
3345 Impervious areas: I A i mp=[0.157] (mm), SLPI=[0.3] (%),
LGI=[150.997] (m), MNI=[0.013], SCI=[0] (min),
3346 Continuous simulation parameters:
3347 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs), I n t e r E v e n t T i m e=[12] (hrs),
END=- 1
3348 *%-----|
3349 ADD HYD NHYDsum=["KB-P2"], NHYDs to
add=["KB-Pond1"+"KB-07-S"+"KB-08-S"+"KB-09"+"KB-10_1"+"KB-10_2"+"KB-12-S"+"KB-13-S"+"KB-1
4-S"+"KB-16_2"]
3350 *%-----|
3351 ROUTE RESERVOIR NHYDout=["KB-P2R"], NHYDin=["KB-P2"],
3352 RDT=[1] (min),
3353 TABLE of ( OUTFLOW STORAGE ) values
3354 (cms) - (ha-m)
3355 [ 0.0 , 0.0 ]
3356 [0.053,0.005]
3357 [0.132,0.009]
3358 [0.269,0.014]
3359 [0.455,0.023]
3360 [0.699,0.037]
3361 [0.947,0.056]
3362 [1.853,0.09]
3363 [2.712,0.146]
3364 [6.626,0.287]
3365 [11.228,0.515]
3366 [14.885,0.738]
3367 [16.473,0.893]
3368 [17.311,0.998]
3369 [17.633,1.063]
3370 [17.634,1.112]
3371 [ -1 , -1 ] (max twenty pts)
3372 NHYDovf=["KB-P2ovf"]
3373 *%-----|
3374 ADD HYD NHYDsum=["KB-Pond2"], NHYDs to add=["KB-P2R"+"KB-P2ovf"]
3375 *%-----|
3376 SAVE HYD NHYD=["KB-Pond2"], # OF PCYCLES=[-1], I CASEs h=[1]
3377 HYD_COMMENT=["Total Flows at KB second pond"]
3378 *%-----|
3379 CONTINUOUS STANDHYD NHYD=["KB-16_1"], DT=[1] min, AREA=[2.8] (ha), XI MP=[0.75],
TI MP=[0.75], DWF=[0] (cms), LOSS=[1]:
3380 Horton: Fo=[76.20] (mm/hr), Fc=[13.20] (mm/hr), DCAY=[4.14] (/hr),
F=[0.00] (mm),
3381 Pervious areas: I A per=[4.67] (mm), SLPP=[2.0] (%), LGP=[40] (m),
MNP=[0.250], SCP=[0] (min),
3382 Impervious areas: I A i mp=[0.157] (mm), SLPI=[0.3] (%),
LGI=[136.626] (m), MNI=[0.013], SCI=[0] (min),
3383 Continuous simulation parameters:
3384 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs), I n t e r E v e n t T i m e=[12] (hrs),
END=- 1
3385 *%-----|
3386 ADD HYD NHYDsum=["KB-P3"], NHYDs to add=["KB-Pond2"+"KB-16_1"]
3387 *%-----|
3388 *%-----|
3389 * One inflow node from pond 3 is added to the model (ROUTE RESERVOIR)
3390 * Another inflow node from right side of pond 3 is not added to the model
3391 ROUTE RESERVOIR NHYDout=["KB-P3R"], NHYDin=["KB-P3"],
3392 RDT=[1] (min),
3393 TABLE of ( OUTFLOW STORAGE ) values
3394 (cms) - (ha-m)
3395 [ 0.0 , 0.0 ]

```

```

3396 [ 0.051, 0.002]
3397 [ 0.048, 0.003]
3398 [ 0.057, 0.029]
3399 [ 0.089, 0.045]
3400 [ 0.133, 0.069]
3401 [ 0.199, 0.106]
3402 [ 0.321, 0.172]
3403 [ 1.029, 0.306]
3404 [ 4.036, 0.527]
3405 [ 8.332, 0.761]
3406 [ 11.727, 0.941]
3407 [ 14.125, 1.067]
3408 [ 15.675, 1.149]
3409 [ 16.555, 1.196]
3410 [ 16.911, 1.214]
3411 [ -1 , -1 ] (max twenty pts)
3412 NHYDovf=["KB- P3ovf"]
3413 *%-----|-----|
3414 ADD HYD NHYDs um=["KB- Pond3"], NHYDs to add=["KB- P3R"+"KB- P3ovf"]
3415 *%-----|-----|
3416 SAVE HYD NHYD=["KB- Pond3"], # OF PCYCLES=[- 1], ICASEsh=[ 1]
3417 HYD_COMMENT=["Total Flows at KB third pond"]
3418 *%-----|-----|
3419 *#*****|
3420 *# EXISTING / PROPOSED Subcatchments (Kennedy-Burnett SWM Facility (118080), SWM
3421 Modeling Approach, NOVATECH Report June, 2020)
3422 *# - TO FRASER- CLARKE DRAIN
3423 *#*****|
3424 CONTINUOUS STANDHYD NHYD=["FC- 01"], DT=[ 1] min, AREA=[ 8.03] (ha), XI MP=[ 0.47],
3425 TI MP=[ 0.47], DWF=[ 0] (cms), LOSS=[ 1]:
3426 Horton: Fo=[ 76.20] (mm/ hr), Fc=[ 13.20] (mm/ hr), DCAY=[ 4.14] (/ hr),
3427 F=[ 0.00] (mm),
3428 Pervious areas: IAper=[ 4.67] (mm), SLPP=[ 2.0] (%), LGP=[ 40] (m),
3429 MNP=[ 0.250], SCP=[ 0] (min),
3430 Impervious areas: IAimp=[ 1.57] (mm), SLPI=[ 1.0] (%),
3431 LGI=[ 231.373] (m), MNI=[ 0.013], SCI=[ 0] (min),
3432 Continuous simulation parameters:
3433 IARECper=[ 4] (hrs), IARECimp=[ 4] (hrs), InterEventTime=[ 12] (hrs),
3434 END=- 1
3435 *%-----|-----|
3436 COMPUTE DUALHYD NHYDin=["FC- 01"], CILET=[ 0.756] (cms), NILET=[ 1],
3437 MajNHYD=["FC- 01- M"]
3438 MnNHYD=["FC- 01- MN"]
3439 TMS TO=[ 714] (cu- m)
3440 *%-----|-----|
3441 ADD HYD NHYDs um=["FC- 01- S"], NHYDs to add=["FC- 01- M"+"FC- 01- MN"]
3442 *%-----|-----|
3443 CONTINUOUS STANDHYD NHYD=["FC- 02"], DT=[ 1] min, AREA=[ 16.05] (ha), XI MP=[ 0.93],
3444 TI MP=[ 0.93], DWF=[ 0] (cms), LOSS=[ 1]:
3445 Horton: Fo=[ 76.20] (mm/ hr), Fc=[ 13.20] (mm/ hr), DCAY=[ 4.14] (/ hr),
3446 F=[ 0.00] (mm),
3447 Pervious areas: IAper=[ 4.67] (mm), SLPP=[ 2.0] (%), LGP=[ 40] (m),
3448 MNP=[ 0.250], SCP=[ 0] (min),
3449 Impervious areas: IAimp=[ 1.57] (mm), SLPI=[ 1.0] (%),
3450 LGI=[ 327.109] (m), MNI=[ 0.013], SCI=[ 0] (min),
3451 Continuous simulation parameters:
3452 IARECper=[ 4] (hrs), IARECimp=[ 4] (hrs), InterEventTime=[ 12] (hrs),
3453 END=- 1
3454 *%-----|-----|
3455 COMPUTE DUALHYD NHYDin=["FC- 02"], CILET=[ 1.159] (cms), NILET=[ 1],
3456 MajNHYD=["FC- 02- M"]
3457 MnNHYD=["FC- 02- MN"]
3458 TMS TO=[ 2385] (cu- m)
3459 *%-----|-----|
3460 ADD HYD NHYDs um=["FC- 02- S"], NHYDs to add=["FC- 02- M"+"FC- 02- MN"]
3461 *%-----|-----|

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3451 CONTINUOUS STANDHYD NHYD=[ "FC- 03" ], DT=[ 1 ] mi n, AREA=[ 7. 37 ] ( ha ), XI MP=[ 0. 64 ],
TI MP=[ 0. 64 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3452 Horton: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
3453 Pervious areas: IAper=[ 4. 67 ] ( mm ), SLPP=[ 2. 0 ] ( % ), LGP=[ 40 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
3454 Impervious areas: IAi mp=[ 1. 57 ] ( mm ), SLPI =[ 1. 0 ] ( % ),
LGI =[ 221. 660 ] ( m ), MNI =[ 0. 013 ], SCI =[ 0 ] ( mi n ),
3455 Continuous simulation parameters:
3456 IaREcper=[ 4 ] ( hr s ), IaRECi mp=[ 4 ] ( hr s ), Inter Event Time=[ 12 ] ( hr s ),
END=- 1
3457 *%-----|
3458 COMPUTE DUALHYD NHYDi n=[ "FC- 03" ], CI NLET=[ 0. 358 ] ( cms ), NI NLET=[ 1 ],
3459 MAj NHYD=[ "FC- 03- M" ]
3460 M nNHYD=[ "FC- 03- MN" ]
3461 TM STO=[ 1131 ] ( cu- m )
3462 *%-----|
3463 ADD HYD NHYDsum=[ "FC- 03- S" ], NHYDs to add=[ "FC- 03- M" +"FC- 03- MN" ]
3464 *%-----|
3465 CONTINUOUS STANDHYD NHYD=[ "FC- 04" ], DT=[ 1 ] mi n, AREA=[ 12. 87 ] ( ha ), XI MP=[ 0. 64 ],
TI MP=[ 0. 64 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3466 Horton: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
3467 Pervious areas: IAper=[ 4. 67 ] ( mm ), SLPP=[ 2. 0 ] ( % ), LGP=[ 40 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
3468 Impervious areas: IAi mp=[ 1. 57 ] ( mm ), SLPI =[ 1. 0 ] ( % ),
LGI =[ 292. 916 ] ( m ), MNI =[ 0. 013 ], SCI =[ 0 ] ( mi n ),
3469 Continuous simulation parameters:
3470 IaREcper=[ 4 ] ( hr s ), IaRECi mp=[ 4 ] ( hr s ), Inter Event Time=[ 12 ] ( hr s ),
END=- 1
3471 *%-----|
3472 COMPUTE DUALHYD NHYDi n=[ "FC- 04" ], CI NLET=[ 0. 741 ] ( cms ), NI NLET=[ 1 ],
3473 MAj NHYD=[ "FC- 04- M" ]
3474 M nNHYD=[ "FC- 04- MN" ]
3475 TM STO=[ 1794 ] ( cu- m )
3476 *%-----|
3477 ADD HYD NHYDsum=[ "FC- 04- S" ], NHYDs to add=[ "FC- 04- M" +"FC- 04- MN" ]
3478 *%-----|
3479 *#*****
3480 *# PROPOSED Subcatchments ( Kennedy- Burnett SWM Facility ( 118080 ), SWM Mdeling
Approach, NOVATECH Report June, 2020)
3481 *# - TO JOCK RI VER
3482 *#*****
3483 CONTINUOUS STANDHYD NHYD=[ "JR- 01" ], DT=[ 1 ] mi n, AREA=[ 8. 24 ] ( ha ), XI MP=[ 0. 64 ],
TI MP=[ 0. 64 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3484 Horton: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),
3485 Pervious areas: IAper=[ 4. 67 ] ( mm ), SLPP=[ 2. 0 ] ( % ), LGP=[ 40 ] ( m ),
MNP=[ 0. 250 ], SCP=[ 0 ] ( mi n ),
3486 Impervious areas: IAi mp=[ 1. 57 ] ( mm ), SLPI =[ 1. 0 ] ( % ),
LGI =[ 234. 379 ] ( m ), MNI =[ 0. 013 ], SCI =[ 0 ] ( mi n ),
3487 Continuous simulation parameters:
3488 IaREcper=[ 4 ] ( hr s ), IaRECi mp=[ 4 ] ( hr s ), Inter Event Time=[ 12 ] ( hr s ),
END=- 1
3489 *%-----|
3490 COMPUTE DUALHYD NHYDi n=[ "JR- 01" ], CI NLET=[ 0. 563 ] ( cms ), NI NLET=[ 1 ],
3491 MAj NHYD=[ "JR- 01- M" ]
3492 M nNHYD=[ "JR- 01- MN" ]
3493 TM STO=[ 1040 ] ( cu- m )
3494 *%-----|
3495 ADD HYD NHYDsum=[ "JR- 01- S" ], NHYDs to add=[ "JR- 01- M" +"JR- 01- MN" ]
3496 *%-----|
3497 CONTINUOUS STANDHYD NHYD=[ "JR- 02" ], DT=[ 1 ] mi n, AREA=[ 1. 59 ] ( ha ), XI MP=[ 0. 64 ],
TI MP=[ 0. 64 ], DWF=[ 0 ] ( cms ), LOSS=[ 1 ] :
3498 Horton: Fo=[ 76. 20 ] ( mm/ hr ), Fc=[ 13. 20 ] ( mm/ hr ), DCAY=[ 4. 14 ] ( / hr ),
F=[ 0. 00 ] ( mm ),

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3499          Pervious areas: I A per=[ 4. 67]( mm), SLPP=[ 2. 0]( %), LGP=[ 40]( m),
3500          MNP=[ 0. 250], SCP=[ 0]( mi n),
3501          Impervious areas: I A imp=[ 1. 57]( mm), SLPI=[ 1. 0]( %),
3502          LGI=[ 102. 956]( m), MNI=[ 0. 013], SCI=[ 0]( mi n),
3503          Continuous simulation parameters:
3504          I a RE C per=[ 4]( hrs), I a RE C imp=[ 4]( hrs), I nter Event Ti me=[ 12]( hrs),
3505          END=- 1
3506
3507 *%-----|-----|
3508 COMPUTE DUALHYD NHYD i n=[ "J R- 02"], CI NLET=[ 0. 153]( cms), NI NLET=[ 1],
3509          M i j NHYD=[ "J R- 02- M"],
3510          M nNHYD=[ "J R- 02- M N"],
3511          T M I STO=[ 153]( cu- m)
3512
3513 *%-----|-----|
3514 ADD HYD          NHYD s um=[ "J R- 02- S"], NHYD s t o a dd=[ "J R- 02- M"+"J R- 02- M N"]
3515
3516 *%-----|-----|
3517 *#*****|*****|
3518 *# Catchment FRASER
3519 *# - To Fraser-Clarke drain (north of the Jock)
3520 *# - Developed land with assumed 43% imp.
3521 *# - 2020-12-17 Change Fraser area to be 35.1 as measured from QGIS
3522 *# - 2020-12-17 All Fraser is undeveloped (Nashyd)
3523 *#*****|*****|
3524 CONTINUOUS NASHYD NHYD=[ "FRASER- DRN"], DT=[ 1] mi n, AREA=[ 13. 65]( ha),
3525          DWF=[ 0]( cms), CN C=[ 77], I A=[ 4. 67]( mm),
3526          N=[ 3], TP=[ 0. 4258] hrs,
3527          Continuous simulation parameters:
3528          I a RE C per=[ 4]( hrs),
3529          SM N=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
3530          I nter Event Ti me=[ 12]( hrs)
3531          Baseflow simulation parameters:
3532          BaseFl owOpt ion=[ 1],
3533          I n i t GW Res Vol=[ 50]( mm), GW Res K=[ 0. 96]( mm/ day/ mm)
3534          VHyd Cond=[ 0. 055]( mm/ hr), END=- 1
3535
3536 *
3537 CONTINUOUS STANDHYD NHYD=[ "FRASER- D"], DT=[ 1] mi n, AREA=[ 21. 61]( ha),
3538          XI MP=[ 0. 585], TI MP=[ 0. 585], DWF=[ 0]( cms), LOSS=[ 2],
3539          SCS curve number CN=[ 80],
3540          Pervious surfaces: I A per=[ 4. 67]( mm), SLPP=[ 1]( %),
3541          LGP=[ 40]( m), MNP=[ 0. 25], SCP=[ 0]( mi n),
3542          Impervious surfaces: I A imp=[ 1. 57]( mm), SLPI=[ 1]( %),
3543          LGI=[ 379. 561]( m), MNI=[ 0. 013], SCI=[ 0]( mi n),
3544          Continuous simulation parameters:
3545          I a RE C per=[ 4]( hrs), I a RE C imp=[ 4]( hrs),
3546          SM N=[ - 1]( mm), SMAX=[ - 1]( mm), SK=[ 0. 010]/( mm),
3547          I nter Event Ti me=[ 18]( hrs), END=- 1
3548
3549 *%-----|-----|
3550 COMPUTE DUALHYD NHYD i n=[ "FRASER- D"], CI NLET=[ 2. 281]( cms), NI NLET=[ 1],
3551          M i j NHYD=[ "FRASER- J"],
3552          M nNHYD=[ "FRASER- N"],
3553          T M I STO=[ 9999999]( cu- m)
3554
3555 *%-----|-----|
3556 ADD HYD          NHYD s um=[ "FRASER- S"], NHYD s t o a dd=[ "FRASER- J"+"FRASER- N"]
3557
3558 *%-----|-----|
3559 *ROUTE RESERVOIR NHYD out=[ "MS_P20"], NHYD i n=[ "FRASER"],
3560          *
3561          RDT=[ 1]( mi n),
3562          *
3563          TABLE of ( OUTFLOW STORAGE ) values
3564          *
3565          ( cms ) - ( ha- m)
3566          *
3567          [ 0. 0 , 0. 0 ]
3568          *
3569          [ 0. 04 , 0. 36]
3570          *
3571          [ - 1 , - 1 ] (max twenty pts)
3572          *
3573          NHYD ovf=[ "P20- OVF"]
3574
3575 *%-----|-----|
3576 ADD HYD          NHYD s um=[ "4241"], NHYD s t o
3577          add=[ "KB- Pond3"+"S- 1- B"+"FRASER- DRN"+"FRASER- S"+"N_ KB"+"FC- 01- S"+"FC- 02- S"+"FC- 03- S"]
3578
3579 *%-----|-----|
3580 SAVE HYD          NHYD=[ "4241"], # OF PCYCLES=[ - 1], I CASE s h=[ 1]

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3561 HYD_COMMENT=[ "Total Flows at Ken-Burnett Outlet" ]
3562 *%-----|-----|
3563 *# Hydrograph from Node Ken-Burnett to station 3633
3564 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 4241
3565 *#
3566 ROUTE CHANNEL NHYDout=[ "4241-out" ], NHYDin=[ "4241" ], RDT=[ 1 ] (min),
3567 CHLGTH=[ 294 ] (m), CHSLOPE=[ 0.1088 ] ( % ), FPSLOPE=[ 0.1088 ] ( % ),
3568 SECNUM=[ 1.0 ], NSEG=[ 3 ]
3569 ( SEGROUGH, SEGDI ST (m) )=[ 0.05, -20.12
3570 -0.035, 45.26
3571 0.05, 403.84 ] NSEG times
3572 ( DISTANCE (m), ELEVATION (m) )=[ ]
3573 [-909.72, 95 ]
3574 [-907.09, 94.5 ]
3575 [-904.65, 94 ]
3576 [-902.26, 93.5 ]
3577 [-44.51, 91.5 ]
3578 [-25.1, 91.5 ]
3579 [-20.98, 91 ]
3580 [-20.61, 90.5 ]
3581 [-20.12, 90 ]
3582 [-6.13, 87.26 ]
3583 [17.51, 86.56 ]
3584 [31.37, 87.2 ]
3585 [45.26, 90 ]
3586 [50.41, 90.5 ]
3587 [63.06, 91 ]
3588 [134.5, 91.5 ]
3589 [190.63, 92 ]
3590 [251.98, 92.5 ]
3591 [321.32, 93.5 ]
3592 [403.84, 95 ]
3593 *%-----|-----|
3594 ADD HYD NHYDsum=[ "SN_KB" ], NHYDs to
add=[ "4241-out"+"FC-04-S"+"JR-01-S"+"JR-02-S" ]
3595 *%-----|-----|
3596 SAVE HYD NHYD=[ "SN_KB" ], # OF PCYCLES=[ -1 ], ICASEsh=[ 1 ]
3597 HYD_COMMENT=[ "Total Flows before Station 3633" ]
3598 *%-----|-----|
3599 *# Hydrograph from Station 3633 to Node Todd
3600 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 3633
3601 *# JFSA 2021-02-26 change the channel length (at station 3633) from 650m to 608m and
change the slope from 0.0498% to 0.24671% That is because of adding station 4241
between station 4534 and station 3633
3602 *#
3603 ROUTE CHANNEL NHYDout=[ "N_TO" ], NHYDin=[ "SN_KB" ], RDT=[ 1 ] (min),
3604 CHLGTH=[ 608 ] (m), CHSLOPE=[ 0.24671 ] ( % ), FPSLOPE=[ 0.24671 ] ( % ),
3605 SECNUM=[ 1.0 ], NSEG=[ 3 ]
3606 ( SEGROUGH, SEGDI ST (m) )=[ 0.05, -23.74
3607 -0.035, 23.74
3608 0.05, 26.50 ] NSEG times
3609 ( DISTANCE (m), ELEVATION (m) )=[ ]
3610 [-29.24, 91.0 ]
3611 [-27.41, 90.5 ]
3612 [-25.64, 90 ]
3613 [-23.74, 89.5 ]
3614 [-22, 89.26 ]
3615 [-20, 88.51 ]
3616 [-19, 88.32 ]
3617 [-15, 88.1 ]
3618 [-10, 88.11 ]
3619 [-5, 88.17 ]
3620 0, 88.27 ]
3621 5, 88.19 ]
3622 10, 88.06 ]
3623 15, 88.48 ]

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3624      16, 88.7
3625      23.74, 89.5
3626      24.68, 90
3627      25.57, 90.5
3628      26.50, 91.0
3629      *          [-29.24, 91]
3630      *          [-27.41, 90.5]
3631      *          [-25.64, 90]
3632      *          [-23.74, 89.5]
3633      *          [-22, 89.26]
3634      *          [-20, 88.51]
3635      *          [-19, 88.32]
3636      *          [-15, 88.1]
3637      *          [-10, 88.11]
3638      *          [-5, 88.17]
3639      *          [0, 88.27]
3640      *          [5, 88.19]
3641      *          [10, 88.06]
3642      *          [15, 88.48]
3643      *          [16, 88.7]
3644      *          [23.74, 89.5]
3645      *          [24.68, 90]
3646      *          [25.57, 90.5]
3647      *%-----|-----|
3648      *#*****|*****|
3649      *#      Catchment Greenbank
3650      *#      - To Greenbank Drain (south of the Jock)
3651      *#      - JFSA 2021-01-18 add Greenbank pond as per JFSA, P598(06)-15, June 2016
3652      *#      - JFSA 2021-01-19 update area from 37.479 ha to 36.6 ha based on GIS measurements
3653      *#*****|*****|
3654      CONTINUOUS STANDHYD NHYD=["Greenbank"], DT=[1] min, AREA=[36.6](ha),
3655      XI MP=[0.639], TI MP=[0.682], DWF=[0](cms), LOSS=[2],
3656      SCS curve number CN=[77],
3657      Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
3658      LGP=[40](m), MNP=[0.25], SCP=[0](min),
3659      Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
3660      LGI=[493.96](m), MNI=[0.013], SCI=[0](min),
3661      Continuous simulation parameters:
3662      I a REC per=[4](hrs), I a REC i mp=[4](hrs),
3663      SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3664      I n t e r E v e n t T i m e=[18](hrs), E N D=-1
3665      *%-----|-----|
3666      ROUTE RESERVOIR NHYDout=["GreenB_MN"], NHYDin=["Greenbank"],
3667      RDT=[1](min),
3668      TABLE of ( OUTFLOW STORAGE ) values
3669      ( cms ) - ( ha - m )
3670      [ 0.0 , 0.0 ]
3671      [ 0.033 , 0.084 ]
3672      [ 0.039 , 0.201 ]
3673      [ 0.113 , 0.292 ]
3674      [ 0.237 , 0.386 ]
3675      [ 0.382 , 0.484 ]
3676      [ 0.539 , 0.585 ]
3677      [ 0.7 , 0.692 ]
3678      [ 0.86 , 0.804 ]
3679      [ 4.684 , 0.922 ]
3680      [ 11.539 , 1.052 ]
3681      [ 20.867 , 1.168 ]
3682      [ 103.616 , 1.974 ]
3683      [ -1 , -1 ] (max twenty pts)
3684      NHYDovf=["GreenB_M"],
3685      *%-----|-----|
3686      *%-----|-----|
3687      ADD HYD NHYDsum=["GreenB"], NHYDs to add=["N_TO"+"GreenB_M"+"GreenB_MN"]
3688      *%-----|-----|
3689      SAVE HYD NHYD=["GreenB"], # OF PCYCLES=[-1], I CASES h=[1]

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3690 HYD_COMMENT=["Total Flows at Greenbank Drain"]
3691 *%-----|-----|
3692 *#*****|
3693 *# Catchment TODD
3694 *# - To Todd Drain (south of the Jock)
3695 *# - Subdivision with 43% imp. as per Barrhaven South M&S
3696 *# - 2020-11-30 increase imp. based on P598(04)-11
3697 *# - 2020-11-30 update TODD Tributary Drainage Area to = 146.015 ha based on
P598(04)-11
3698 *# - 2020-11-30 split TODD Drainage Area to MAJOR, MINOR, POND and ALL
3699 *#*****|
3700 *# - JFSA 2021-01-19 add "TODD_MN1" as part of Clarke("W_CLAR_M") and remove it
from Todd
3701 *CONTINUOUS STANDHYD NHYD=["TODD_MN1"], DT=[1] min, AREA=[1.772](ha),
3702 * XI MP=[0.53], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3703 * SCS curve number CN=[77],
3704 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3705 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3706 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3707 * LGI=[108.689](m), MNI=[0.013], SCI=[0](min),
3708 * Continuous simulation parameters:
3709 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3710 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3711 * InterEventTime=[18](hrs), END=-1
3712 *%-----|-----|
3713 CONTINUOUS STANDHYD NHYD=["TODD_MN2"], DT=[1] min, AREA=[2.1](ha),
3714 * XI MP=[0.53], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3715 * SCS curve number CN=[77],
3716 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3717 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3718 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3719 * LGI=[118.322](m), MNI=[0.013], SCI=[0](min),
3720 * Continuous simulation parameters:
3721 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3722 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3723 * InterEventTime=[18](hrs), END=-1
3724 *%-----|-----|
3725 CONTINUOUS STANDHYD NHYD=["TODD_MN3"], DT=[1] min, AREA=[0.117](ha),
3726 * XI MP=[0.53], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3727 * SCS curve number CN=[77],
3728 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3729 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3730 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3731 * LGI=[27.928](m), MNI=[0.013], SCI=[0](min),
3732 * Continuous simulation parameters:
3733 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3734 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3735 * InterEventTime=[18](hrs), END=-1
3736 *%-----|-----|
3737 CONTINUOUS STANDHYD NHYD=["TODD_M"], DT=[1] min, AREA=[30.230](ha),
3738 * XI MP=[0.52], TI MP=[0.64], DWF=[0](cms), LOSS=[2],
3739 * SCS curve number CN=[77],
3740 * Pervious surfaces: IAper=[4.67](mm), SLPP=[1](%),
3741 * LGP=[40](m), MNP=[0.25], SCP=[0](min),
3742 * Impervious surfaces: IAimp=[1.57](mm), SLPI=[1](%),
3743 * LGI=[448.925](m), MNI=[0.013], SCI=[0](min),
3744 * Continuous simulation parameters:
3745 * IaRECper=[4](hrs), IaRECImp=[4](hrs),
3746 * SMN=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
3747 * InterEventTime=[18](hrs), END=-1
3748 *%-----|-----|
3749 * -JFSA, 2021-01-19 update "TODD_ALL" area from 108.741 ha to 112.908 ha based on
GIS measurements (148.41-30.23-0.117-2.1-3.055=112.908 ha)
3750 CONTINUOUS STANDHYD NHYD=["TODD_ALL"], DT=[1] min, AREA=[112.908](ha),
3751 * XI MP=[0.52], TI MP=[0.57], DWF=[0](cms), LOSS=[2],
3752 * SCS curve number CN=[77],

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3753          Pervious surfaces: I A per=[ 4.67] (mm), SLPP=[ 1] (%),
3754                  LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (mi n),
3755          Impervious surfaces: I A i mp=[ 1.57] (mm), SLPI=[ 1] (%),
3756                  LGI=[ 867.594] (m), MNI=[ 0.013], SCI=[ 0] (mi n),
3757          Continuous simulation parameters:
3758          I a REC per=[ 4] (hrs), I a REC i mp=[ 4] (hrs),
3759          SM N=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
3760          Inter Event Ti me=[ 18] (hrs), END=- 1
3761  *%-----|-----|
3762  CONTINUOUS STANDHYD NHYD=["TODD_P"], DT=[ 1] mi n, AREA=[ 3.055] (ha),
3763          XI MP=[ 0.63], TI MP=[ 0.63], DWF=[ 0] (cms), LOSS=[ 2],
3764          SCS curve number CN=[ 77],
3765          Pervious surfaces: I A per=[ 4.67] (mm), SLPP=[ 1] (%),
3766                  LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (mi n),
3767          Impervious surfaces: I A i mp=[ 1.57] (mm), SLPI=[ 1] (%),
3768                  LGI=[ 142.712] (m), MNI=[ 0.013], SCI=[ 0] (mi n),
3769          Continuous simulation parameters:
3770          I a REC per=[ 4] (hrs), I a REC i mp=[ 4] (hrs),
3771          SM N=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
3772          Inter Event Ti me=[ 18] (hrs), END=- 1
3773  *%-----|-----|
3774  *%-----|-----|
3775  * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3776  *CONTINUOUS STANDHYD NHYD=["TODD_DEVL"], DT=[ 1] mi n, AREA=[ 15.87] (ha),
3777  *          XI MP=[ 0.63], TI MP=[ 0.63], DWF=[ 0] (cms), LOSS=[ 2],
3778  *          SCS curve number CN=[ 77],
3779  *          Pervious surfaces: I A per=[ 4.67] (mm), SLPP=[ 1] (%),
3780                  LGP=[ 40] (m), MNP=[ 0.25], SCP=[ 0] (mi n),
3781  *          Impervious surfaces: I A i mp=[ 1.57] (mm), SLPI=[ 1] (%),
3782                  LGI=[ 325.27] (m), MNI=[ 0.013], SCI=[ 0] (mi n),
3783  *          Continuous simulation parameters:
3784          I a REC per=[ 4] (hrs), I a REC i mp=[ 4] (hrs),
3785          SM N=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
3786          Inter Event Ti me=[ 18] (hrs), END=- 1
3787  *%-----|-----|
3788  * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3789  *CONTINUOUS NASHYD NHYD=["TODD_UnD"], DT=[ 1] mi n, AREA=[ 12.47] (ha),
3790  *          DWF=[ 0] (cms), CN C=[ 77], I A=[ 4.67] (mm),
3791  *          N=[ 3], TP=[ 1.10] hrs,
3792  *          Continuous simulation parameters:
3793          I a REC per=[ 4] (hrs),
3794          SM N=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010] / (mm),
3795          Inter Event Ti me=[ 12] (hrs)
3796          Baseflow simulation parameters:
3797          BaseFlowOption=[ 1],
3798          I n i t GWRes Vol =[ 50] (mm), GWRes K=[ 0.96] (mm/ day/ mm)
3799          VHydCond=[ 0.055] (mm/ hr), END=- 1
3800  *%-----|-----|
3801  *# 5- Year + 12% Capture
3802  *COMPUTE DUALHYD NHYD i n=["TODD_M"], CI NLET=[ 3.314] (cms), NI NLET=[ 1],
3803  *          M i j NHYD=["TODD_M j "]
3804  *          M nNHYD=["TODD_M n"]
3805  *          TM I STO=[ 0.1] (cu-m)
3806  ROUTE RESERVOIR NHYDout=["TODD_M n"], NHYD i n=["TODD_M"],
3807          RDT=[ 1] (mi n),
3808          TABLE of ( OUTFLOW STORAGE ) values
3809                  ( cms ) - ( ha- m)
3810                  [ 0.0 , 0.0 ]
3811                  [ 3.314 , 0.0001 ]
3812                  [ -1 , -1 ] (max twenty pts)
3813          NHYDovf=["TODD_M j "],
3814  *%-----|-----|
3815  *# 5- Year + 12% Capture

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3816 *COMPUTE DUALHYD      NHYDi n=[ " TODD_MN1" ], CI NLET=[ 0. 227 ]( c ms ), NI NLET=[ 1 ],
3817 *                      Mij NHYD=[ " TODD_MN1j " ]
3818 *                      MnnNHYD=[ " TODD_MN1n " ]
3819 *                      TMI STO=[ 0. 1 ]( c u- m )
3820 *ROUTE RESERVOIR      NHYDout =[ " TODD_MN1n" ] , NHYDi n=[ " TODD_MN1" ] ,
3821 *                      RDT=[ 1 ]( mi n ),
3822 *                      TABLE of ( OUTFLOW STORAGE ) values
3823 *                          ( c ms ) - ( ha- m )
3824 *                          [ 0. 0 , 0. 0 ]
3825 *                          [ 0. 227 , 0. 0001 ]
3826 *                          [ -1 , -1 ] (max twenty pts)
3827 *                      NHYDovf=[ " TODD_MN1j " ] ,
3828 *%-----|-----|
3829 *COMPUTE DUALHYD      NHYDi n=[ " TODD_MN2" ], CI NLET=[ 0. 268 ]( c ms ), NI NLET=[ 1 ],
3830 *                      Mij NHYD=[ " TODD_MN2j " ]
3831 *                      MnnNHYD=[ " TODD_MN2n " ]
3832 *                      TMI STO=[ 0. 1 ]( c u- m )
3833 ROUTE RESERVOIR      NHYDout =[ " TODD_MN2n" ] , NHYDi n=[ " TODD_MN2" ] ,
3834 *                      RDT=[ 1 ]( mi n ),
3835 *                      TABLE of ( OUTFLOW STORAGE ) values
3836 *                          ( c ms ) - ( ha- m )
3837 *                          [ 0. 0 , 0. 0 ]
3838 *                          [ 0. 268 , 0. 0001 ]
3839 *                          [ -1 , -1 ] (max twenty pts)
3840 *                      NHYDovf=[ " TODD_MN2j " ] ,
3841 *%-----|-----|
3842 *COMPUTE DUALHYD      NHYDi n=[ " TODD_MN3" ], CI NLET=[ 0. 016 ]( c ms ), NI NLET=[ 1 ],
3843 *                      Mij NHYD=[ " TODD_MN3j " ]
3844 *                      MnnNHYD=[ " TODD_MN3n " ]
3845 *                      TMI STO=[ 0. 1 ]( c u- m )
3846 ROUTE RESERVOIR      NHYDout =[ " TODD_MN3n" ] , NHYDi n=[ " TODD_MN3" ] ,
3847 *                      RDT=[ 1 ]( mi n ),
3848 *                      TABLE of ( OUTFLOW STORAGE ) values
3849 *                          ( c ms ) - ( ha- m )
3850 *                          [ 0. 0 , 0. 0 ]
3851 *                          [ 0. 016 , 0. 0001 ]
3852 *                          [ -1 , -1 ] (max twenty pts)
3853 *                      NHYDovf=[ " TODD_MN3j " ] ,
3854 *%-----|-----|
3855 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
3856 CONTINUOUS STANDHYD NHYD=[ "A2" ], DT=[ 1 ] mi n, AREA=[ 25. 5 ]( ha ),
3857 *                      XI MP=[ 0. 42 ], TI MP=[ 0. 52 ], DWF=[ 0 ]( c ms ), LOSS=[ 2 ],
3858 *                      SCS curve number CN=[ 75 ],
3859 *                      Pervious surfaces: I Aper=[ 4. 67 ]( mm ), SLPP=[ 1 ]( % ),
3860 *                          LGP=[ 40 ]( m ), MNP=[ 0. 25 ], SCP=[ 0 ]( mi n ),
3861 *                      Impervious surfaces: I Aimp=[ 1. 57 ]( mm ), SLPI=[ 1 ]( % ),
3862 *                          LGL=[ 566 ]( m ), MNI=[ 0. 013 ], SCI=[ 0 ]( mi n ),
3863 *                      Continuous simulation parameters:
3864 *                      I aRECper=[ 4 ]( hr s ), I aRECimp=[ 4 ]( hr s ),
3865 *                      SM N=[ -1 ]( mm ), SMAX=[ -1 ]( mm ), SK=[ 0. 010 ]/( mm ),
3866 *                      Inter Event Ti me=[ 18 ]( hr s ), END=-1
3867 *%-----|-----|
3868 COMPUTE DUALHYD      NHYDi n=[ "A2" ], CI NLET=[ 1. 818 ]( c ms ), NI NLET=[ 1 ],
3869 *                      Mij NHYD=[ "A2- Mi" ]
3870 *                      MnnNHYD=[ "A2- MN" ]
3871 *                      TMI STO=[ 924 ]( c u- m )
3872 *%-----|-----|
3873 ADD HYD              NHYDs um=[ " TODD" ], NHYDs to
add=[ " TODD_MN2n" + " TODD_MN3n" + " TODD_Mij " + " TODD_P" + " TODD_ALL" + " WCLAR_Min" ]
3874 *%-----|-----|
3875 SAVE HYD              NHYD=[ " TODD" ], # OF PCYCLES=[ -1 ], I CASEs h=[ 1 ]
3876 *                      HYD_COMMENT=[ " Total Flows at Todd Drain" ]
3877 *%-----|-----|
3878 *#*****
3879 *# Todd Pond 3

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3880  *# - Rating curve obtained from Barrhaven South M&S modeling
3881  *# - stantec 2007, Tributary Drainage Area to M&S Pond 3 = 193 ha
3882  *#*****
3883  ROUTE RESERVOIR      NHYDout=["MS_P3"],  NHYDIn=["TODD"],
3884                      RDT=[1](min),
3885                      TABLE of ( OUTFLOW STORAGE ) values
3886                      (cms) - (ha-m)
3887                      [ 0.0 , 0.0 ]
3888                      [ 0.014 , 0.155 ]
3889                      [ 0.048 , 0.394 ]
3890                      [ 0.061 , 0.56 ]
3891                      [ 0.08 , 0.909 ]
3892                      [ 0.088 , 1.089 ]
3893                      [ 0.109 , 1.652 ]
3894                      [ 0.118 , 1.952 ]
3895                      [ 0.122 , 2.099 ]
3896                      [ 1.972 , 2.269 ]
3897                      [ 9.135 , 2.598 ]
3898                      [ 15.608 , 2.826 ]
3899                      [ 19.256 , 2.942 ]
3900                      [ 27.282 , 3.181 ]
3901                      [ 40.957 , 3.55 ]
3902                      [ 56.372 , 3.929 ]
3903                      [ 73.349 , 4.317 ]
3904                      [ 85.469 , 4.579 ]
3905                      [ 104.771 , 4.977 ]
3906                      [ -1 , -1 ] (max twenty pts)
3907                      NHYDovf=["P3-OVF"]
3908  *%-----|-----|
3909  ADD HYD              NHYDsum=["SN_TO"], NHYDs to
add=["GreenB"+"MS_P3"+"P3-OVF"+"TODD_MN2j"+"A2-M"]
3910  *%-----|-----|
3911  SAVE HYD            NHYD=["SN_TO"], # OF PCYCLES=[-1], ICASEsh=[1]
3912                      HYD_COMMENT=["Total Flows at Todd Drain"]
3913  *%-----|-----|
3914  *#
3915  *# Hydrograph from Todd Drain routed to Corrigan Drain
3916  *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
3917  *# 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so
the model will be more stable and give reasonable results. It is justifiable as ROUTE
CHANNELS aren't well suited to really flat slopes.
3918  *
3919  ROUTE CHANNEL      NHYDout=["N_TO"], NHYDIn=["SN_TO"],
3920                      RDT=[1](min),
3921                      CHLGTH=[280](m),  CHSLOPE=[0.05](%),
3922                      FPSLOPE=[0.05](%),
3923                      SECNUM=[1.0],  NSEG=[3]
3924                      ( SEGROUGH, SEGDIST (m) )=
3925                      [ 0.075, -17.72
3926                      -0.045, 17.72
3927                      0.075, 80.62] NSEG times
3928                      ( DISTANCE (m), ELEVATION (m) )=
3929                      [-83.32, 90.00]
3930                      [-81.36, 89.50]
3931                      [-79.12, 89.00]
3932                      [-76.13, 88.50]
3933                      [-20.46, 88.00]
3934                      [-19.36, 87.50]
3935                      [-18.51, 87.00]
3936                      [-17.72, 86.50]
3937                      [-11.95, 85.24]
3938                      [-0.11, 85.12]
3939                      [11.49, 85.20]
3940                      [17.72, 86.50]
3941                      [19.74, 87.00]
3942                      [21.22, 87.50]

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3943 [ 22. 68, 88. 00]
3944 [ 24. 28, 88. 50]
3945 [ 26. 79, 89. 00]
3946 [ 71. 98, 90. 00]
3947 [ 80. 62, 90. 50]
3948 *%-----|-----|
3949 SAVE HYD NHYD=["N_TO"], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
3950 HYD_COMMENT=["Total inflows at Station 2462"]
3951 *%-----|-----|
3952 *#*****|
3953 *# Catchment CORRIG
3954 *# - To Corrigan Drain (south of the Jock)
3955 *# - Primarily Developed (medium density)
3956 *# - JFSA JAN 2021, add Corrigan subcatchments as per IBI, July 2008
3957 *#*****|
3958 *ROUTE RESERVOIR NHYDout=["MS_P1"], NHYDin=["CORRIG"],
3959 * RDT=[ 1](min),
3960 * TABLE of ( OUTFLOW STORAGE ) values
3961 * (cms) - (ha-m)
3962 * [ 0.0 , 0.0 ]
3963 * [ 0.06 , 0.58]
3964 * [ -1 , -1 ] (max twenty pts)
3965 * NHYDovf=["P1-OVF"]
3966 *%-----|-----|
3967 *ADD HYD NHYDs um=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
3968 *%-----|-----|
3969 *SAVE HYD NHYD=["SN_CO"], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
3970 * HYD_COMMENT=["Total Flows at Corrigan Drain"]
3971 *%-----|-----|
3972 * -JFSA 2021-02-23 "TODD_DEVL" is part of the Corrigan sub-catchment because it
drains to Corrigan SWM as per geoOttawa.ca Feb. 2021. "TODD_DEVL" now is called "corr1"
and its parameters remain the same.
3973 CONTINUOUS STANDHYD NHYD=["corr1"], DT=[ 1]min, AREA=[ 15. 87](ha),
3974 XI MP=[ 0. 63], TI MP=[ 0. 63], DWF=[ 0](cms), LOSS=[ 2],
3975 SCS curve number CN=[ 77],
3976 Pervious surfaces: I Aper=[ 4. 67](mm), SLPP=[ 1](%),
3977 LGP=[ 40](m), MNP=[ 0. 25], SCP=[ 0](min),
3978 Impervious surfaces: I Ai mp=[ 1. 57](mm), SLPI=[ 1](%),
3979 LGI=[ 325. 27](m), MNI=[ 0. 013], SCI=[ 0](min),
3980 Continuous simulation parameters:
3981 Ia RECper=[ 4](hrs), Ia RECI mp=[ 4](hrs),
3982 SM N=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0. 010]/(mm),
3983 Inter Event Ti me=[ 18](hrs), END=- 1
3984 *%-----|-----|
3985 * -JFSA 2021-02-23 add DUALHYD for "corr1". "corr1" DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to "corr1".
3986 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
3987 COMPUTE DUALHYD NHYDin=["corr1"], CI NLET=[ 1. 818](cms), NI NLET=[ 1],
3988 M aj NHYD=["corr1-M"]
3989 M nNHYD=["corr1-MN"]
3990 TM STO=[ 924](cu-m)
3991 *%-----|-----|
3992 * -JFSA 2021-02-23 "TODD_UnD" is part of the Corrigan sub-catchment. "TODD_UnD" now
is called "corr2" and its parameters remain the same.
3993 CONTINUOUS NASHYD NHYD=["corr2"], DT=[ 1]min, AREA=[ 12. 47](ha),
3994 DWF=[ 0](cms), CN C=[ 77], IA=[ 4. 67](mm),
3995 N=[ 3], TP=[ 1. 10]hrs,
3996 Continuous simulation parameters:
3997 Ia RECper=[ 4](hrs),
3998 SM N=[ - 1](mm), SMAX=[ - 1](mm), SK=[ 0. 010]/(mm),
3999 Inter Event Ti me=[ 12](hrs)
4000 Baseflow simulation parameters:
4001 BaseFl owOpt ion=[ 1],
4002 Ini t GWR es Vol=[ 50](mm), GWR es K=[ 0. 96](mm/day/mm)
4003 VHydCond=[ 0. 055](mm/hr), END=- 1

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4004 *%-----|-----|
4005 * -JFSA 2021-01-19 change Al-Corrig to be developed as per geottawa website and
apply the parameters of A2, the nearest sub-catchment to Al-Corrig, LGI is calculated
based on Al-Corrig area
4006 * -JFSA 2021-01-19 update all Corrigan areas based on GIS measurements, and keep
LGI as it is from Corrigan Report, IBI Group, 2008 because LGI calculated is less than
LGI from the Corrigan Report
4007 CONTINUOUS STANDHYD NHYD=["Al-Corrig"], DT=[1] min, AREA=[15.75](ha),
4008 XI MP=[0.42], TI MP=[0.52], DWF=[0](cms), LOSS=[2],
4009 SCS curve number CN=[75],
4010 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4011 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4012 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4013 LGI=[324.037](m), MNI=[0.013], SCI=[0](min),
4014 Continuous simulation parameters:
4015 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
4016 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4017 Inter Event Ti me=[18](hrs), END=-1
4018 *
4019 * -JFSA 2021-01-25 add DUALHYD for Al-Corrig. Al-Corrig DUALHYD Parameters are the
same as A2 DUALHYD Parameters because A2 is the nearest sub-catchment to Al-Corrig.
4020 * At the same time, Corrigan Report, IBI group 2008 has no DUALHYD Parameters for
Al-Corrig
4021 COMPUTE DUALHYD NHYD i n=["Al-Corrig"], CI NLET=[1.818](cms), NI NLET=[1],
4022 M a j NHYD=["Al-M"]
4023 M n NHYD=["Al-MN"]
4024 TM STO=[924](cu-m)
4025 *%-----|-----|
4026 *CONTINUOUS NASHYD NHYD=["Al-Corrig"], DT=[1] min, AREA=[15.75](ha),
4027 * DWF=[0](cms), CN C=[66], I A=[2.5](mm),
4028 * N=[3.0], TP=[0.36] hrs,
4029 * Continuous simulation parameters:
4030 * I a REC per=[4](hrs),
4031 * SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4032 * Inter Event Ti me=[12](hrs)
4033 * Baseflow simulation parameters:
4034 * BaseFl owOpt ion=[1],
4035 * I ni t GWRes Vol =[50](mm), GWRes K=[0.96](mm/day/mm)
4036 * VHydCond=[0.055](mm/hr), END=-1
4037 *%-----|-----|
4038 CONTINUOUS NASHYD NHYD=["B1"], DT=[1] min, AREA=[2.77](ha),
4039 DWF=[0](cms), CN C=[56], I A=[2.5](mm),
4040 N=[3.0], TP=[0.23] hrs,
4041 Continuous simulation parameters:
4042 I a REC per=[4](hrs),
4043 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4044 Inter Event Ti me=[12](hrs)
4045 Baseflow simulation parameters:
4046 BaseFl owOpt ion=[1],
4047 I ni t GWRes Vol =[50](mm), GWRes K=[0.96](mm/day/mm)
4048 VHydCond=[0.055](mm/hr), END=-1
4049 *%-----|-----|
4050 CONTINUOUS STANDHYD NHYD=["A4"], DT=[1] min, AREA=[1.27](ha),
4051 XI MP=[0.65], TI MP=[0.65], DWF=[0](cms), LOSS=[2],
4052 SCS curve number CN=[75],
4053 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4054 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4055 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4056 LGI=[253](m), MNI=[0.013], SCI=[0](min),
4057 Continuous simulation parameters:
4058 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
4059 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4060 Inter Event Ti me=[18](hrs), END=-1
4061 *%-----|-----|
4062 COMPUTE DUALHYD NHYD i n=["A4"], CI NLET=[0.405](cms), NI NLET=[1],
4063 M a j NHYD=["A4-M"]

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4064 MnNHYP=[ " A4- MN" ]
4065 TMI STO=[ 68] ( cu- m)
4066 *%-----|-----|
4067 ADD HYD NHYPsum=[ " MHI 01" ], NHPDs to
add=[ " A1- M" + " A1- MN" + " corr 1- M" + " corr 1- MN" + " corr 2" + " B1" + " A4- MN" ]
4068 *%-----|-----|
4069 SAVE HYD NHPD=[ " MHI 01" ], # OF PCYCLES=[ - 1 ], ICASEsh=[ 1 ]
4070 HYD_COMMENT=[ " Tot al Fl ows at MHI 01" ]
4071 *%-----|-----|
4072 ROUTE PIPE PTYPE=[ 1] circ, NHPDout=[ " 101- 102" ], RNUMBER=[ 1. 0 ], PDIA M=[ 1050] ( mm ),
4073 PLNGTH=[ 368] ( m ), PROUGH=[ 0. 013 ], PSLOPE=[ 0. 0054] ( m / m ),
NHPDin=[ " MHI 01" ], RDT=[ 1 ]
4074 *%-----|-----|
4075 * -JFSA 2021-01-19 move A2 from Corrigan sub-catchment to Todd sub-catchment so the
major system from A2 can be added to Todd
4076 *CONTINUOUS STANDHYD NHPD=[ " A2" ], DT=[ 1] mi n, AREA=[ 25. 5] ( ha ),
4077 * XI MP=[ 0. 42 ], TI MP=[ 0. 52 ], DWF=[ 0] ( cms ), LOSS=[ 2 ],
4078 * SCS curve number CN=[ 75 ],
4079 * Pervious surfaces: IAper=[ 4. 67] ( mm ), SLPP=[ 1] ( % ),
4080 * LGP=[ 40] ( m ), MNP=[ 0. 25 ], SCP=[ 0] ( mi n ),
4081 * ImperVIOUS surfaces: IAi mp=[ 1. 57] ( mm ), SLPI =[ 1] ( % ),
4082 * LGI =[ 566] ( m ), MNI =[ 0. 013 ], SCI =[ 0] ( mi n ),
4083 * Continuous simulation parameters:
4084 * IAReCper=[ 4] ( hrs ), IAReCi mp=[ 4] ( hrs ),
4085 * SMN=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
4086 * InterEvent Time=[ 18] ( hrs ), END=- 1
4087 *%-----|-----|
4088 *COMPUTE DUALHYD NHPDin=[ " A2" ], CINLET=[ 1. 818] ( cms ), NI NLET=[ 1 ],
4089 * Mi j NHPD=[ " A2- M" ]
4090 * MnNHYP=[ " A2- MN" ]
4091 * TMI STO=[ 924] ( cu- m)
4092 *%-----|-----|
4093 ADD HYD NHPDsum=[ " MHI 02" ], NHPDs to add=[ " A2- MN" + " 101- 102" ]
4094 *%-----|-----|
4095 SAVE HYD NHPD=[ " MHI 02" ], # OF PCYCLES=[ - 1 ], ICASEsh=[ 1 ]
4096 HYD_COMMENT=[ " Tot al Fl ows at MHI 02" ]
4097 *%-----|-----|
4098 CONTINUOUS STANDHYD NHPD=[ " A5" ], DT=[ 1] mi n, AREA=[ 1. 6] ( ha ),
4099 * XI MP=[ 0. 71 ], TI MP=[ 0. 71 ], DWF=[ 0] ( cms ), LOSS=[ 2 ],
4100 * SCS curve number CN=[ 75 ],
4101 * Pervious surfaces: IAper=[ 4. 67] ( mm ), SLPP=[ 1] ( % ),
4102 * LGP=[ 40] ( m ), MNP=[ 0. 25 ], SCP=[ 0] ( mi n ),
4103 * ImperVIOUS surfaces: IAi mp=[ 1. 57] ( mm ), SLPI =[ 1] ( % ),
4104 * LGI =[ 300] ( m ), MNI =[ 0. 013 ], SCI =[ 0] ( mi n ),
4105 * Continuous simulation parameters:
4106 * IAReCper=[ 4] ( hrs ), IAReCi mp=[ 4] ( hrs ),
4107 * SMN=[ - 1] ( mm ), SMAX=[ - 1] ( mm ), SK=[ 0. 010] / ( mm ),
4108 * InterEvent Time=[ 18] ( hrs ), END=- 1
4109 *%-----|-----|
4110 ADD HYD NHPDsum=[ " A5T" ], NHPDs to add=[ " A4- M" + " A5" ]
4111 *%-----|-----|
4112 COMPUTE DUALHYD NHPDin=[ " A5T" ], CINLET=[ 0. 357] ( cms ), NI NLET=[ 1 ],
4113 * Mi j NHPD=[ " A5- M" ]
4114 * MnNHYP=[ " A5- MN" ]
4115 * TMI STO=[ 60] ( cu- m)
4116 *%-----|-----|
4117 * -JFSA Jan. 2021, A3 is a part of Todd so it is removed
4118 * -JFSA Jan. 2021, " A2- M" added to " Todd"
4119 *CONTINUOUS STANDHYD NHPD=[ " A3" ], DT=[ 1] mi n, AREA=[ 18. 4] ( ha ),
4120 * XI MP=[ 0. 58 ], TI MP=[ 0. 65 ], DWF=[ 0] ( cms ), LOSS=[ 2 ],
4121 * SCS curve number CN=[ 75 ],
4122 * Pervious surfaces: IAper=[ 4. 67] ( mm ), SLPP=[ 1] ( % ),
4123 * LGP=[ 40] ( m ), MNP=[ 0. 25 ], SCP=[ 0] ( mi n ),
4124 * ImperVIOUS surfaces: IAi mp=[ 1. 57] ( mm ), SLPI =[ 1] ( % ),
4125 * LGI =[ 450] ( m ), MNI =[ 0. 013 ], SCI =[ 0] ( mi n ),
4126 * Continuous simulation parameters:

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4127 *      IaRECper=[ 4](hrs),   IaRECImp=[ 4](hrs),
4128 *      SMN=[- 1](mm),   SMAX=[- 1](mm),   SK=[ 0.010]/(mm),
4129 *      InterEventTime=[ 18](hrs),   END=- 1
4130 *%-----|-----|
4131 *ADD HYD      NHYDsum=[ "A3-A2M"], NHYDs to add=[ "A2-M"+"A3"]
4132 *%-----|-----|
4133 *COMPUTE DUALHYD  NHYDin=[ "A3-A2M"], CINLET=[ 2.208](cms), NINLET=[ 1],
4134 *      MjNHYD=[ "A3R-M"]
4135 *      MnNHYD=[ "A3R-MN"]
4136 *      TMSSTO=[ 908](cu-m)
4137 *%-----|-----|
4138 ROUTE PIPE    PTYPE=[ 1]circ, NHYDout=[ "102-103"], RNUMBER=[ 1.0], PDIAM=[ 1500](mm),
4139 *      PLNGTH=[ 504](m), PROUGH=[ 0.013], PSLOPE=[ 0.0028](m/m),
*      NHYDin=[ "MHI02"], RDT=[ 1]
4140 *%-----|-----|
4141 ADD HYD      NHYDsum=[ "MHI03"], NHYDs to add=[ "102-103"+"A5-MN"]
4142 *%-----|-----|
4143 SAVE HYD     NHYD=[ "MHI03"], # OF PCYCLES=[ - 1], ICASEsh=[ 1]
4144 *      HYD_COMMENT=[ "Total Flows at MHI03"]
4145 *%-----|-----|
4146 ROUTE PIPE    PTYPE=[ 1]circ, NHYDout=[ "103-104"], RNUMBER=[ 1.0], PDIAM=[ 1650](mm),
4147 *      PLNGTH=[ 438](m), PROUGH=[ 0.013], PSLOPE=[ 0.0046](m/m),
*      NHYDin=[ "MHI03"], RDT=[ 1]
4148 *%-----|-----|
4149 CONTINUOUS STANDHYD NHYD=[ "A6"], DT=[ 1]min, AREA=[ 1.56](ha),
4150 *      XI MP=[ 0.71], TI MP=[ 0.71], DWF=[ 0](cms), LOSS=[ 2],
4151 *      SCS curve number CN=[ 75],
4152 *      Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4153 *      LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4154 *      Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),
4155 *      LGI=[ 280](m), MNI=[ 0.013], SCI=[ 0](min),
4156 *      Continuous simulation parameters:
4157 *      IaRECper=[ 4](hrs),   IaRECImp=[ 4](hrs),
4158 *      SMN=[- 1](mm),   SMAX=[- 1](mm),   SK=[ 0.010]/(mm),
4159 *      InterEventTime=[ 18](hrs),   END=- 1
4160 *%-----|-----|
4161 ADD HYD      NHYDsum=[ "A6T"], NHYDs to add=[ "A5-M"+"A6"]
4162 *%-----|-----|
4163 COMPUTE DUALHYD  NHYDin=[ "A6T"], CINLET=[ 0.357](cms), NINLET=[ 1],
4164 *      MjNHYD=[ "A6-M"]
4165 *      MnNHYD=[ "A6-MN"]
4166 *      TMSSTO=[ 60](cu-m)
4167 *%-----|-----|
4168 *      -JFSA Jan. 2021, A7-corrig is a part of Todd so it is removed
4169 *CONTINUOUS STANDHYD NHYD=[ "A7-corrig"], DT=[ 1]min, AREA=[ 11.8](ha),
4170 *      XI MP=[ 0.41], TI MP=[ 0.54], DWF=[ 0](cms), LOSS=[ 2],
4171 *      SCS curve number CN=[ 75],
4172 *      Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4173 *      LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4174 *      Impervious surfaces: IAi mp=[ 1.57](mm), SLPI=[ 1](%),
4175 *      LGI=[ 438](m), MNI=[ 0.013], SCI=[ 0](min),
4176 *      Continuous simulation parameters:
4177 *      IaRECper=[ 4](hrs),   IaRECImp=[ 4](hrs),
4178 *      SMN=[- 1](mm),   SMAX=[- 1](mm),   SK=[ 0.010]/(mm),
4179 *      InterEventTime=[ 18](hrs),   END=- 1
4180 *%-----|-----|
4181 *ADD HYD      NHYDsum=[ "A7-A3RM"], NHYDs to add=[ "A3R-M"+"A7-corrig"]
4182 *%-----|-----|
4183 *COMPUTE DUALHYD  NHYDin=[ "A7-A3RM"], CINLET=[ 1.003](cms), NINLET=[ 1],
4184 *      MjNHYD=[ "A7R-M"]
4185 *      MnNHYD=[ "A7R-MN"]
4186 *      TMSSTO=[ 496](cu-m)
4187 *%-----|-----|
4188 ADD HYD      NHYDsum=[ "MHI04"], NHYDs to add=[ "A6-MN"+"103-104"+"TODD_Min"]
4189 *%-----|-----|
4190 SAVE HYD     NHYD=[ "MHI04"], # OF PCYCLES=[ - 1], ICASEsh=[ 1]

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4191 HYD_COMMENT=["Tot al Fl ows at MH104"]
4192 *%-----|-----|
4193 CONTINUOUS STANDHYD NHYD=[" B2"], DT=[ 1] mi n, AREA=[ 12. 31] (ha),
4194 XI MP=[ 0. 41], TI MP=[ 0. 54], DWF=[ 0] (cms), LOSS=[ 2],
4195 SCS curve number CN=[ 75],
4196 Pervious surfaces: I A per=[ 4. 67] (mm), SLPP=[ 1] (%),
4197 LGP=[ 40] (m), MNP=[ 0. 25], SCP=[ 0] (mi n),
4198 Impervious surfaces: I A i mp=[ 1. 57] (mm), SLPI =[ 1] (%),
4199 LGI =[ 417] (m), MNI =[ 0. 013], SCI =[ 0] (mi n),
4200 Continuous simulation parameters:
4201 I a REC per=[ 4] (hr s), I a REC i mp=[ 4] (hr s),
4202 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
4203 I nter Event Ti me=[ 18] (hr s), END=- 1
4204 *%-----|-----|
4205 COMPUTE DUALHYD NHYD i n=[" B2"], CI NLET=[ 1. 029] (cms), NI NLET=[ 1],
4206 M a j NHYD=[" B2- M "]
4207 M n NHYD=[" B2- MN"]
4208 TM I STO=[ 508] (cu- m)
4209 *%-----|-----|
4210 ROUTE PI PE PTYPE=[ 1] circ, NHYDout=[" 315- 333"], RNUMBER=[ 1. 0], PDI AM=[ 1200] (mm),
4211 PLNGTH=[ 254] (m), PROUGH=[ 0. 013], PSLOPE=[ 0. 001] (m/ m),
NHYD i n=[" B2- MN"], RDT=[ 1]
4212 *%-----|-----|
4213 CONTINUOUS STANDHYD NHYD=[" B3"], DT=[ 1] mi n, AREA=[ 5. 59] (ha),
4214 XI MP=[ 0. 41], TI MP=[ 0. 54], DWF=[ 0] (cms), LOSS=[ 2],
4215 SCS curve number CN=[ 75],
4216 Pervious surfaces: I A per=[ 4. 67] (mm), SLPP=[ 1] (%),
4217 LGP=[ 40] (m), MNP=[ 0. 25], SCP=[ 0] (mi n),
4218 Impervious surfaces: I A i mp=[ 1. 57] (mm), SLPI =[ 1] (%),
4219 LGI =[ 345] (m), MNI =[ 0. 013], SCI =[ 0] (mi n),
4220 Continuous simulation parameters:
4221 I a REC per=[ 4] (hr s), I a REC i mp=[ 4] (hr s),
4222 SM N=[ - 1] (mm), SMAX=[ - 1] (mm), SK=[ 0. 010] / (mm),
4223 I nter Event Ti me=[ 18] (hr s), END=- 1
4224 *%-----|-----|
4225 COMPUTE DUALHYD NHYD i n=[" B3"], CI NLET=[ 0. 459] (cms), NI NLET=[ 1],
4226 M a j NHYD=[" B3- M "]
4227 M n NHYD=[" B3- MN"]
4228 TM I STO=[ 227] (cu- m)
4229 *%-----|-----|
4230 ADD HYD NHYDs um=[" MH333"], NHYDs to add=[" B3- MN" + " 315- 333"]
4231 *%-----|-----|
4232 SAVE HYD NHYD=[" MH333"], # OF PCYCLES=[ - 1], I CASEs h=[ 1]
4233 HYD_COMMENT=["Tot al Fl ows at MH333"]
4234 *%-----|-----|
4235 ROUTE PI PE PTYPE=[ 1] circ, NHYDout=[" 333- 335"], RNUMBER=[ 1. 0], PDI AM=[ 1200] (mm),
4236 PLNGTH=[ 251] (m), PROUGH=[ 0. 013], PSLOPE=[ 0. 001] (m/ m),
NHYD i n=[" MH333"], RDT=[ 1]
4237 *%-----|-----|
4238 ROUTE PI PE PTYPE=[ 1] circ, NHYDout=[" 335- 338"], RNUMBER=[ 1. 0], PDI AM=[ 1200] (mm),
4239 PLNGTH=[ 185] (m), PROUGH=[ 0. 013], PSLOPE=[ 0. 001] (m/ m),
NHYD i n=[" 333- 335"], RDT=[ 1]
4240 *%-----|-----|
4241 ROUTE PI PE PTYPE=[ 1] circ, NHYDout=[" 338- 340"], RNUMBER=[ 1. 0], PDI AM=[ 1350] (mm),
4242 PLNGTH=[ 233] (m), PROUGH=[ 0. 013], PSLOPE=[ 0. 001] (m/ m),
NHYD i n=[" 335- 338"], RDT=[ 1]
4243 *%-----|-----|
4244 CONTINUOUS STANDHYD NHYD=[" B4"], DT=[ 1] mi n, AREA=[ 7. 6] (ha),
4245 XI MP=[ 0. 41], TI MP=[ 0. 54], DWF=[ 0] (cms), LOSS=[ 2],
4246 SCS curve number CN=[ 75],
4247 Pervious surfaces: I A per=[ 4. 67] (mm), SLPP=[ 1] (%),
4248 LGP=[ 40] (m), MNP=[ 0. 25], SCP=[ 0] (mi n),
4249 Impervious surfaces: I A i mp=[ 1. 57] (mm), SLPI =[ 1] (%),
4250 LGI =[ 388] (m), MNI =[ 0. 013], SCI =[ 0] (mi n),
4251 Continuous simulation parameters:
4252 I a REC per=[ 4] (hr s), I a REC i mp=[ 4] (hr s),

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4253 SM N=[ - 1 ] ( mm ) , SMAX=[ - 1 ] ( mm ) , SK=[ 0. 010 ] / ( mm ) ,
4254 I n t e r E v e n t T i m e =[ 18 ] ( h r s ) , E N D = - 1
4255 *%-----|-----|
4256 COMPUTE DUALHYD NHYD i n =[ " B4 " ] , C I N L E T =[ 0. 655 ] ( c m s ) , N I N L E T =[ 1 ] ,
4257 M a j N H Y D =[ " B4 - M " ]
4258 M n N H Y D =[ " B4 - M N " ]
4259 T M S T O =[ 323 ] ( c u - m )
4260 *%-----|-----|
4261 ADD HYD NHYD s u m =[ " M H 340 " ] , N H Y D s t o a d d =[ " 338 - 340 " + " B4 - M N " ]
4262 *%-----|-----|
4263 SAVE HYD NHYD =[ " M H 340 " ] , # O F P C Y C L E S =[ - 1 ] , I C A S E s h =[ 1 ]
4264 H Y D _ C O M M E N T =[ " T o t a l F l o w s a t M H 340 " ]
4265 *%-----|-----|
4266 ROUTE P I P E P T Y P E =[ 1 ] c i r c , N H Y D o u t =[ " 340 - 104 " ] , R N U M B E R =[ 1. 0 ] , P D I A M =[ 1650 ] ( mm ) ,
4267 P L N G T H =[ 240 ] ( m ) , P R O U G H =[ 0. 013 ] , P S L O P E =[ 0. 0015 ] ( m / m ) ,
N H Y D i n =[ " M H 340 " ] , R D T =[ 1 ]
4268 *%-----|-----|
4269 ADD HYD NHYD s u m =[ " M H 104T " ] , N H Y D s t o a d d =[ " 340 - 104 " + " M H 104 " ]
4270 *%-----|-----|
4271 ROUTE P I P E P T Y P E =[ 2 ] r e c t , N H Y D o u t =[ " 104 - 105 " ] , R N U M B E R =[ 1. 0 ] ,
P W D T H =[ 2400 ] ( mm ) b y P H E I G H T =[ 2100 ] ( mm ) ,
4272 P L N G T H =[ 380 ] ( m ) , P R O U G H =[ 0. 013 ] , P S L O P E =[ 0. 001 ] ( m / m ) ,
N H Y D i n =[ " M H 104T " ] , R D T =[ 1 ]
4273 *%-----|-----|
4274 CONTINUOUS STANDHYD NHYD =[ " B5 " ] , D T =[ 1 ] m i n , A R E A =[ 2. 2 ] ( ha ) ,
4275 X I M P =[ 0. 57 ] , T I M P =[ 0. 57 ] , D W F =[ 0 ] ( c m s ) , L O S S =[ 2 ] ,
4276 S C S c u r v e n u m b e r C N =[ 75 ] ,
4277 P e r v i o u s s u r f a c e s : I A p e r =[ 4. 67 ] ( mm ) , S L P P =[ 1 ] ( % ) ,
4278 L G P =[ 40 ] ( m ) , M N P =[ 0. 25 ] , S C P =[ 0 ] ( m i n ) ,
4279 I m p e r v i o u s s u r f a c e s : I A i m p =[ 1. 57 ] ( mm ) , S L P I =[ 1 ] ( % ) ,
4280 L G I =[ 187 ] ( m ) , M N I =[ 0. 013 ] , S C I =[ 0 ] ( m i n ) ,
4281 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
4282 I a R E C p e r =[ 4 ] ( h r s ) , I a R E C i m p =[ 4 ] ( h r s ) ,
4283 S M N =[ - 1 ] ( mm ) , S M A X =[ - 1 ] ( mm ) , S K =[ 0. 010 ] / ( mm ) ,
4284 I n t e r E v e n t T i m e =[ 18 ] ( h r s ) , E N D = - 1
4285 *%-----|-----|
4286 COMPUTE DUALHYD NHYD i n =[ " B5 " ] , C I N L E T =[ 0. 260 ] ( c m s ) , N I N L E T =[ 1 ] ,
4287 M a j N H Y D =[ " B5 - M " ]
4288 M n N H Y D =[ " B5 - M N " ]
4289 T M S T O =[ 250 ] ( c u - m )
4290 *%-----|-----|
4291 CONTINUOUS STANDHYD NHYD =[ " A8 " ] , D T =[ 1 ] m i n , A R E A =[ 0. 96 ] ( ha ) ,
4292 X I M P =[ 0. 71 ] , T I M P =[ 0. 71 ] , D W F =[ 0 ] ( c m s ) , L O S S =[ 2 ] ,
4293 S C S c u r v e n u m b e r C N =[ 75 ] ,
4294 P e r v i o u s s u r f a c e s : I A p e r =[ 4. 67 ] ( mm ) , S L P P =[ 1 ] ( % ) ,
4295 L G P =[ 40 ] ( m ) , M N P =[ 0. 25 ] , S C P =[ 0 ] ( m i n ) ,
4296 I m p e r v i o u s s u r f a c e s : I A i m p =[ 1. 57 ] ( mm ) , S L P I =[ 1 ] ( % ) ,
4297 L G I =[ 186 ] ( m ) , M N I =[ 0. 013 ] , S C I =[ 0 ] ( m i n ) ,
4298 C o n t i n u o u s s i m u l a t i o n p a r a m e t e r s :
4299 I a R E C p e r =[ 4 ] ( h r s ) , I a R E C i m p =[ 4 ] ( h r s ) ,
4300 S M N =[ - 1 ] ( mm ) , S M A X =[ - 1 ] ( mm ) , S K =[ 0. 010 ] / ( mm ) ,
4301 I n t e r E v e n t T i m e =[ 18 ] ( h r s ) , E N D = - 1
4302 *%-----|-----|
4303 ADD HYD NHYD s u m =[ " A8T " ] , N H Y D s t o a d d =[ " A6 - M " + " A8 " ]
4304 *%-----|-----|
4305 COMPUTE DUALHYD NHYD i n =[ " A8T " ] , C I N L E T =[ 0. 238 ] ( c m s ) , N I N L E T =[ 1 ] ,
4306 M a j N H Y D =[ " A8 - M " ]
4307 M n N H Y D =[ " A8 - M N " ]
4308 T M S T O =[ 40 ] ( c u - m )
4309 *%-----|-----|
4310 ADD HYD NHYD s u m =[ " M H 105 " ] , N H Y D s t o
a d d =[ " 104 - 105 " + " B5 - M N " + " A8 - M N " + " T O D D _ M N 3 j " ]
4311 *%-----|-----|
4312 SAVE HYD NHYD =[ " M H 105 " ] , # O F P C Y C L E S =[ - 1 ] , I C A S E s h =[ 1 ]
4313 H Y D _ C O M M E N T =[ " T o t a l F l o w s a t M H 105 " ]

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4314 *%-----|
4315 DI VERT HYD      NHYDin=["A8-M"] NIDout=[2]max five,
4316                  outflow hydrographs (NHYDs)=["A8-M-JR" "A8-M-B6"]
4317                  flow distribution table: (modify as necessary)
4318                  Note: all flows are in (cms)
4319                  QIDi + QIDi = QTOTAL
4320                  [ 0 + 0 = 0 ]
4321                  [ 50 + 50 = 100 ] end
4322 *%-----|
4323 DI VERT HYD      NHYDin=["MHI05"] NIDout=[2]max five,
4324                  outflow hydrographs (NHYDs)=["MHI05-JR" "MHI05-B6"]
4325                  flow distribution table: (modify as necessary)
4326                  Note: all flows are in (cms)
4327                  QIDi + QIDi = QTOTAL
4328                  [ 0 + 0 = 0 ]
4329                  [ 0 + 3.0 = 3.0 ]
4330                  [ 96.9+ 3.1 = 100 ] end
4331 *%-----|
4332 CONTINUOUS STANDHYD NHYD=["B7"], DT=[1]min, AREA=[7.19](ha),
4333                  XI MP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4334                  SCS curve number CN=[75],
4335                  Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4336                  LGP=[40](m), MNP=[0.25], SCP=[0](min),
4337                  Impervious surfaces: I A imp=[1.57](mm), SLPI=[1](%),
4338                  LGI=[211](m), MNI=[0.013], SCI=[0](min),
4339                  Continuous simulation parameters:
4340                  I a REC per=[4](hrs), I a REC imp=[4](hrs),
4341                  SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4342                  Inter Event Time=[18](hrs), END=-1
4343 *%-----|
4344 ADD HYD          NHYDs um=["B7-B4M"], NHYDs to add=["B4-M"+"B7"]
4345 *%-----|
4346 COMPUTE DUALHYD NHYDin=["B7-B4M"], CINLET=[0.629](cms), NINLET=[1],
4347                  Maj NHYD=["B7R-M"]
4348                  MinNHYD=["B7R-MN"]
4349                  TMS TO=[311](cu-m)
4350 *%-----|
4351 ROUTE PIPE      PTYPE=[1]circ, NHYDout=["360-106A"], RNUMBER=[1.0], PDI AM=[1050](mm),
4352                  PLNGTH=[167](m), PROUGH=[0.013], PSLOPE=[0.001](m/m),
4353                  NHYDin=["B7R-MN"], RDT=[1]
4354 *%-----|
4355 * -JFSA 2021-01-19 change B6 to be developed as per geottawa website and apply the
4356 parameters of A7, the nearest sub-catchment to B6, LGI is calculated based on B6 area
4357 CONTINUOUS STANDHYD NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4358                  XI MP=[0.41], TIMP=[0.54], DWF=[0](cms), LOSS=[2],
4359                  SCS curve number CN=[75],
4360                  Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4361                  LGP=[40](m), MNP=[0.25], SCP=[0](min),
4362                  Impervious surfaces: I A imp=[1.57](mm), SLPI=[1](%),
4363                  LGI=[148.099](m), MNI=[0.013], SCI=[0](min),
4364                  Continuous simulation parameters:
4365                  I a REC per=[4](hrs), I a REC imp=[4](hrs),
4366                  SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4367                  Inter Event Time=[18](hrs), END=-1
4368 *%-----|
4369 * -JFSA 2021-01-25 add B1 DUALHYD as per Corrigan Report, IBI Group, 2008
4370 COMPUTE DUALHYD NHYDin=["B6"], CINLET=[0.064](cms), NINLET=[1],
4371                  Maj NHYD=["B6-M"]
4372                  MinNHYD=["B6-MN"]
4373                  TMS TO=[5484](cu-m)
4374 *%-----|
4375 *CONTINUOUS NASHYD NHYD=["B6"], DT=[1]min, AREA=[3.29](ha),
4376                  * DWF=[0](cms), CN/C=[75], IA=[2.5](mm),
4377                  * N=[3.0], TP=[0.36]hrs,

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4376 *          Continuous simulation parameters:
4377 *          IaRECper=[ 4](hrs),
4378 *          SMN=[- 1](mm),   SMAX=[- 1](mm),   SK=[ 0.010]/(mm),
4379 *          InterEventTime=[ 12](hrs)
4380 *          Baseflow simulation parameters:
4381 *          BaseFlowOption=[ 1] ,
4382 *          InitGWResVol=[ 50](mm),   GWResK=[ 0.96](mm/day/mm)
4383 *          VHydCond=[ 0.055](mm/hr),   END=- 1
4384 *%-----|-----
4385 *%   -EX-LAND is external land. It is a part of JOCKVA sub-catchment as per Corrigan
Report, IBI Group, 2008
4386 CONTINUOUS STANDHYD NHYD=["EX-LAND"], DT=[ 1]min, AREA=[ 32.5](ha),
4387 XI MP=[ 0.50], TI MP=[ 0.50], DWF=[ 0](cms), LOSS=[ 2],
4388 SCS curve number CN=[ 74],
4389 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4390 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4391 Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4392 LGI=[ 465.475](m), MNI=[ 0.013], SCI=[ 0](min),
4393 Continuous simulation parameters:
4394 IaRECper=[ 4](hrs),   IaRECImp=[ 4](hrs),
4395 SMN=[- 1](mm),   SMAX=[- 1](mm),   SK=[ 0.010]/(mm),
4396 InterEventTime=[ 18](hrs),   END=- 1
4397 *%-----|-----
4398 COMPUTE DUALHYD NHYDin=["EX-LAND"], CINLET=[ 2.275](cms), NINLET=[ 1],
4399 MajNHYD=["EX-LAND-M"]
4400 MinNHYD=["EX-LAND-MN"]
4401 TMSSTO=[ 1365](cu-m)
4402 *%-----|-----
4403 ADD HYD NHYDsum=["B6-B7ExM"], NHYDsto
add=["B7R-M"+"EX-LAND-M"+"B5-M"+"B6-M"+"B6-MN"+"A8-M-B6"]
4404 *%-----|-----
4405 COMPUTE DUALHYD NHYDin=["B6-B7ExM"], CINLET=[ 0.064](cms), NINLET=[ 1],
4406 MajNHYD=["B6R-M"]
4407 MinNHYD=["B6R-MN"]
4408 TMSSTO=[ 5484](cu-m)
4409 *%-----|-----
4410 ROUTE PIPE PTYPE=[ 1]circ, NHYDout=["105-106A"], RNUMBER=[ 1.0], PDIAM=[ 1800](mm),
4411 PLNGTH=[ 208](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
NHYDin=["MH105-B6"], RDT=[ 1]
4412 *%-----|-----
4413 ADD HYD NHYDsum=["MH106A"], NHYDsto
add=["360-106A"+"105-106A"+"B6R-MN"+"B6R-M"]
4414 *%-----|-----
4415 SAVE HYD NHYD=["MH106A"], # OF PCYCLES=[- 1], ICASEsh=[ 1]
4416 HYD_COMMENT=["Total Flows at MH106A"]
4417 *%-----|-----
4418 *%   -JFSA 2021-01-12 THE MANHOLE MH106 is called MH17/106 in Corrigan Report, IBI
Group, July 2008
4419 *%
4420 ROUTE PIPE PTYPE=[ 1]circ, NHYDout=["106A-106"], RNUMBER=[ 1.0], PDIAM=[ 1800](mm),
4421 PLNGTH=[ 190](m), PROUGH=[ 0.013], PSLOPE=[ 0.001](m/m),
NHYDin=["MH106A"], RDT=[ 1]
4422 *%-----|-----
4423 CONTINUOUS STANDHYD NHYD=["A9"], DT=[ 1]min, AREA=[ 2.44](ha),
4424 XI MP=[ 0.71], TI MP=[ 0.71], DWF=[ 0](cms), LOSS=[ 2],
4425 SCS curve number CN=[ 75],
4426 Pervious surfaces: IAper=[ 4.67](mm), SLPP=[ 1](%),
4427 LGP=[ 40](m), MNP=[ 0.25], SCP=[ 0](min),
4428 Impervious surfaces: IAimp=[ 1.57](mm), SLPI=[ 1](%),
4429 LGI=[ 262](m), MNI=[ 0.013], SCI=[ 0](min),
4430 Continuous simulation parameters:
4431 IaRECper=[ 4](hrs),   IaRECImp=[ 4](hrs),
4432 SMN=[- 1](mm),   SMAX=[- 1](mm),   SK=[ 0.010]/(mm),
4433 InterEventTime=[ 18](hrs),   END=- 1
4434 *%-----|-----
4435 COMPUTE DUALHYD NHYDin=["A9"], CINLET=[ 0.547](cms), NINLET=[ 1],

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4436      Mj NHYD=[ " A9- M " ]
4437      MnNHYD=[ " A9- MN" ]
4438      TM STO=[ 0 ] ( cu- m)
4439  *%-----|-----|
4440  ADD HYD      NHYDs um=[ " MHI06" ], NHYDs to add=[ " 106A- 106" + " A9- MN" ]
4441  *%-----|-----|
4442  SAVE HYD     NHYD=[ " MHI06" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
4443      HYD_ COMMENT=[ " Tot al Fl ows at MHI06" ]
4444  *%-----|-----|
4445  *% -JFSA 2021-01-12 THE MANHOLE MHI07 is called MHI18/107 in Corrigan Report, IBI
Group, July 2008
4446  *%
4447  ROUTE PIPE   PTYPE=[ 1 ] circ, NHYDout=[ " 106- 107" ], RNUMBER=[ 1. 0 ], PDI AM=[ 1800 ] ( mm),
4448      PLNGTH=[ 122. 5 ] ( m), PROUGH=[ 0. 013 ], PSLOPE=[ 0. 001 ] ( m/ m),
      NHYDin=[ " MHI06" ], RDT=[ 1 ]
4449  *%-----|-----|
4450  CONTINUOUS STANDHYD NHYD=[ " A10" ], DT=[ 1 ] mi n, AREA=[ 4. 14 ] ( ha),
4451      XI MP=[ 0. 35 ], TI MP=[ 0. 47 ], DWF=[ 0 ] ( cms), LOSS=[ 2 ],
4452      SCS curve number CN=[ 75 ],
4453      Pervious surfaces: IAper=[ 4. 67 ] ( mm), SLPP=[ 1 ] ( %),
4454      LGP=[ 40 ] ( m), MNP=[ 0. 25 ], SCP=[ 0 ] ( mi n),
4455      Impervious surfaces: IAi mp=[ 1. 57 ] ( mm), SLPI=[ 1 ] ( %),
4456      LGI=[ 183 ] ( m), MNI=[ 0. 013 ], SCI=[ 0 ] ( mi n),
4457      Continuous simulation parameters:
4458      IaRECper=[ 4 ] ( hr s), IaRECI mp=[ 4 ] ( hr s),
4459      SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0. 010 ] / ( mm),
4460      InterEvent Time=[ 18 ] ( hr s), END=- 1
4461  *%-----|-----|
4462  COMPUTE DUALHYD NHYDin=[ " A10" ], CI NLET=[ 0. 310 ] ( cms), NI NLET=[ 1 ],
4463      Mj NHYD=[ " A10- M " ]
4464      MnNHYD=[ " A10- MN" ]
4465      TM STO=[ 228 ] ( cu- m)
4466  *%-----|-----|
4467  CONTINUOUS STANDHYD NHYD=[ " A11" ], DT=[ 1 ] mi n, AREA=[ 10. 61 ] ( ha),
4468      XI MP=[ 0. 53 ], TI MP=[ 0. 62 ], DWF=[ 0 ] ( cms), LOSS=[ 2 ],
4469      SCS curve number CN=[ 75 ],
4470      Pervious surfaces: IAper=[ 4. 67 ] ( mm), SLPP=[ 1 ] ( %),
4471      LGP=[ 40 ] ( m), MNP=[ 0. 25 ], SCP=[ 0 ] ( mi n),
4472      Impervious surfaces: IAi mp=[ 1. 57 ] ( mm), SLPI=[ 1 ] ( %),
4473      LGI=[ 379 ] ( m), MNI=[ 0. 013 ], SCI=[ 0 ] ( mi n),
4474      Continuous simulation parameters:
4475      IaRECper=[ 4 ] ( hr s), IaRECI mp=[ 4 ] ( hr s),
4476      SMN=[ - 1 ] ( mm), SMAX=[ - 1 ] ( mm), SK=[ 0. 010 ] / ( mm),
4477      InterEvent Time=[ 18 ] ( hr s), END=- 1
4478  *%-----|-----|
4479  COMPUTE DUALHYD NHYDin=[ " A11" ], CI NLET=[ 0. 993 ] ( cms), NI NLET=[ 1 ],
4480      Mj NHYD=[ " A11- M " ]
4481      MnNHYD=[ " A11- MN" ]
4482      TM STO=[ 556 ] ( cu- m)
4483  *%-----|-----|
4484  ADD HYD      NHYDs um=[ " MHI07" ], NHYDs to add=[ " 106- 107" + " A10- MN" + " A11- MN" ]
4485  *%-----|-----|
4486  SAVE HYD     NHYD=[ " MHI07" ], # OF PCYCLES=[ - 1 ], I CASEs h=[ 1 ]
4487      HYD_ COMMENT=[ " Tot al Fl ows at MHI07" ]
4488  *%-----|-----|
4489  ROUTE PIPE   PTYPE=[ 1 ] circ, NHYDout=[ " 107- 119" ], RNUMBER=[ 1. 0 ], PDI AM=[ 1800 ] ( mm),
4490      PLNGTH=[ 114 ] ( m), PROUGH=[ 0. 013 ], PSLOPE=[ 0. 0012 ] ( m/ m),
      NHYDin=[ " MHI07" ], RDT=[ 1 ]
4491  *%-----|-----|
4492  *% -JFSA 2021-01-12 THE MANHOLE MHI08 is called MHI20/108 in Corrigan Report, IBI
Group, July 2008
4493  *%
4494  ROUTE PIPE   PTYPE=[ 1 ] circ, NHYDout=[ " 119- 108" ], RNUMBER=[ 1. 0 ], PDI AM=[ 1800 ] ( mm),
4495      PLNGTH=[ 65. 8 ] ( m), PROUGH=[ 0. 013 ], PSLOPE=[ 0. 0012 ] ( m/ m),
      NHYDin=[ " 107- 119" ], RDT=[ 1 ]
4496  *%-----|-----|

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4497 CONTINUOUS STANDHYD NHYD=["A12"], DT=[1] min, AREA=[12.29] (ha),
 4498 XI MP=[0.41], TI MP=[0.54], DWF=[0] (cms), LOSS=[2],
 4499 SCS curve number CN=[75],
 4500 Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
 4501 LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
 4502 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
 4503 LGI=[183] (m), MNI=[0.013], SCI=[0] (min),
 4504 Continuous simulation parameters:
 4505 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
 4506 SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010] / (mm),
 4507 Inter Event Time=[18] (hrs), END=- 1
 4508 *%-----|
 4509 COMPUTE DUALHYD NHYD i n=["A12"], CI NLET=[1.029] (cms), NI NLET=[1],
 4510 M a j NHYD=["A12- M"]
 4511 M n NHYD=["A12- MN"]
 4512 TM STO=[672] (cu-m)
 4513 *%-----|
 4514 CONTINUOUS STANDHYD NHYD=["A13"], DT=[1] min, AREA=[2.59] (ha),
 4515 XI MP=[0.71], TI MP=[0.71], DWF=[0] (cms), LOSS=[2],
 4516 SCS curve number CN=[75],
 4517 Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
 4518 LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
 4519 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
 4520 LGI=[379] (m), MNI=[0.013], SCI=[0] (min),
 4521 Continuous simulation parameters:
 4522 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
 4523 SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010] / (mm),
 4524 Inter Event Time=[18] (hrs), END=- 1
 4525 *%-----|
 4526 COMPUTE DUALHYD NHYD i n=["A13"], CI NLET=[0.571] (cms), NI NLET=[1],
 4527 M a j NHYD=["A13- M"]
 4528 M n NHYD=["A13- MN"]
 4529 TM STO=[0] (cu-m)
 4530 *%-----|
 4531 * -JFSA 2021-01-22 add the Corrigan pond area ("Pond-Block")
 4532 CONTINUOUS STANDHYD NHYD=["Pond-Block"], DT=[1] min, AREA=[2.94] (ha),
 4533 XI MP=[0.415], TI MP=[0.415], DWF=[0] (cms), LOSS=[2],
 4534 SCS curve number CN=[75],
 4535 Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
 4536 LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
 4537 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
 4538 LGI=[183] (m), MNI=[0.013], SCI=[0] (min),
 4539 Continuous simulation parameters:
 4540 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
 4541 SM N=[-1] (mm), SMAX=[-1] (mm), SK=[0.010] / (mm),
 4542 Inter Event Time=[18] (hrs), END=- 1
 4543 *%-----|
 4544 ADD HYD NHYDs um=["MHI08"], NHYDs to add=["119-108"+"A13-MN"+"A12-MN"]
 4545 *%-----|
 4546 SAVE HYD NHYD=["MHI08"], # OF PCYCLES=[-1], I CASEs h=[1]
 4547 HYD_COMMENT=["Total Flows at MHI08"]
 4548 *%-----|
 4549 ROUTE PIPE PTYPE=[1] circ, NHYDout=["108-116"], RNUMBER=[1.0], PDI AM=[1800] (mm),
 4550 PLNGTH=[76.6] (m), PROUGH=[0.013], PSLOPE=[0.0013] (m/m),
 NHYD i n=["MHI08"], RDT=[1]
 4551 *%-----|
 4552 ROUTE PIPE PTYPE=[1] circ, NHYDout=["116-corrigan"], RNUMBER=[1.0],
 4553 PDI AM=[1800] (mm),
 PLNGTH=[79.5] (m), PROUGH=[0.013], PSLOPE=[0.0013] (m/m),
 NHYD i n=["108-116"], RDT=[1]
 4554 *%-----|
 4555 ADD HYD NHYDs um=["Corrigan"], NHYDs to add=["116-corrigan"+"Pond-Block"]
 4556 *%-----|
 4557 SAVE HYD NHYD=["Corrigan"], # OF PCYCLES=[-1], I CASEs h=[1]
 4558 HYD_COMMENT=["Total Flows at Corrigan Pond"]
 4559 *%-----|

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4560 ROUTE RESERVOIR      NHYDout=["Co-P"],  NHYDin=["Corrigan"],
4561 RDT=[1](min),
4562          TABLE of ( OUTFLOW STORAGE ) values
4563                      (cms) - (ha-m)
4564                      [ 0.0 , 0.0 ]
4565                      [ 0.015 , 0.04118]
4566                      [ 0.030 , 0.08297]
4567                      [ 0.045 , 0.12537]
4568                      [ 0.060 , 0.16837]
4569                      [ 0.075 , 0.21199]
4570                      [ 0.090 , 0.27545]
4571                      [ 0.105 , 0.34650]
4572                      [ 0.120 , 0.42049]
4573                      [ 0.135 , 0.50188]
4574                      [ 0.186 , 0.60307]
4575                      [ 2.110 , 0.79083]
4576                      [ 5.874 , 1.00271]
4577                      [ 11.395 , 1.29643]
4578                      [ 18.770 , 1.62054]
4579                      [ 28.143 , 1.97516]
4580                      [ -1 , -1 ] (max twenty pts)
4581          NHYDovf=["Co-P-OVF"]
4582 *%-----|-----|
4583 ADD HYD          NHYDsum=["corrig"], NHYDs to
add=["Co-P-OVF"+"Co-P"+"N_TO"+"MH105-JR"+"A8-M-JR"+"A9-M-JR"+"A10-M-JR"+"A11-M-JR"+"A12-M-JR"+"A13-M-JR"]
4584 *%-----|-----|
4585 SAVE HYD        NHYD=["corrig"], # OF PCYCLES=[-1], ICASEsh=[1]
4586          HYD_COMMENT=["Total Flows at Corrigan Pond"]
4587 *%-----|-----|
4588 *#*****|*****|
4589 *#   Corrigan Pond 1
4590 *#   - Rating curve obtained from Barrhaven South M&S modeling
4591 *#   - Tributary Drainage Area to M&S Pond 1 = 145 ha
4592 *#*****|*****|
4593 *ROUTE RESERVOIR  NHYDout=["MS_P1"],  NHYDin=["CORRIG"],
4594 *          RDT=[1](min),
4595 *          TABLE of ( OUTFLOW STORAGE ) values
4596 *                      (cms) - (ha-m)
4597 *                      [ 0.0 , 0.0 ]
4598 *                      [ 0.06 , 0.58]
4599 *                      [ -1 , -1 ] (max twenty pts)
4600 *          NHYDovf=["P1-OVF"]
4601 *%-----|-----|
4602 *ADD HYD          NHYDsum=["SN_CO"], NHYDs to add=["N_TO"+"P1-OVF"+"MS_P1"]
4603 *%-----|-----|
4604 *SAVE HYD        NHYD=["SN_CO"], # OF PCYCLES=[-1], ICASEsh=[1]
4605 *          HYD_COMMENT=["Total Flows at Corrigan Drain"]
4606 *%-----|-----|
4607 *#
4608 *# Hydrograph from Corrigan Drain routed to Jockvale Road
4609 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 2462
4610 *#
4611 ROUTE CHANNEL    NHYDout=["N_M"] , NHYDin=["corrig"] ,
4612 RDT=[1](min),
4613 CHLGTH=[580](m),  CHSLOPE=[0.4448](%),
4614                      FPSLOPE=[0.4448](%),
4615 SECNUM=[1.0],     NSEG=[3]
4616 ( SEGROUGH, SEGDI ST (m))=
4617 [0.075, -17.72
4618 -0.045, 17.72
4619 0.075, 80.62] NSEG times
4620 ( DISTANCE (m), ELEVATION (m))=
4621 [-83.32, 90.00]
4622 [-81.36, 89.50]
4623 [-79.12, 89.00]

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4624 [- 76. 13, 88. 50]
4625 [- 20. 46, 88. 00]
4626 [- 19. 36, 87. 50]
4627 [- 18. 51, 87. 00]
4628 [- 17. 72, 86. 50]
4629 [- 11. 95, 85. 24]
4630 [- 0. 11, 85. 12]
4631 [11. 49, 85. 20]
4632 [17. 72, 86. 50]
4633 [19. 74, 87. 00]
4634 [21. 22, 87. 50]
4635 [22. 68, 88. 00]
4636 [24. 28, 88. 50]
4637 [26. 79, 89. 00]
4638 [71. 98, 90. 00]
4639 [80. 62, 90. 50]

4640 *%-----|-----|
4641 *#*****|*****|

4642 *# Catchment MLLS
4643 *# - To SWM Facility north of the Jock
4644 *# - Primarily residential development
4645 *#*****|*****|

4646 CONTINUOUS STANDHYD NHYD=["MLLS"], DT=[1] min, AREA=[175.99] (ha),
4647 XI MP=[0.38], TI MP=[0.38], DWF=[0] (cms), LOSS=[2],
4648 SCS curve number CN=[74],
4649 Pervious surfaces: I A per=[4.67] (mm), SLPP=[1] (%),
4650 LGP=[40] (m), MNP=[0.25], SCP=[0] (min),
4651 Impervious surfaces: I A i mp=[1.57] (mm), SLPI=[1] (%),
4652 LGI=[1118.123] (m), MNI=[0.013], SCI=[0] (min),
4653 Continuous simulation parameters:
4654 I a REC per=[4] (hrs), I a REC i mp=[4] (hrs),
4655 S M N=[-1] (mm), S M A X=[-1] (mm), S K=[0.010] / (mm),
4656 I n t e r E v e n t T i m e=[18] (hrs), E N D=-1

4657 *%-----|-----|
4658 *#*****|*****|

4659 *# Chapman Mills SWM Pond
4660 *# - Rating curve obtained from CCL hydraulic modeling
4661 *#*****|*****|

4662 ROUTE RESERVOIR NHYDout=["MLL_P"], NHYDin=["MLLS"],
4663 RDT=[1] (min),
4664 TABLE of (OUTFLOW STORAGE) values
4665 (cms) - (ha-m)
4666 [0.0 , 0.0]
4667 [0.01 , 0.01]
4668 [0.05 , 0.06]
4669 [0.09 , 0.11]
4670 [0.13 , 0.15]
4671 [0.18 , 0.19]
4672 [0.28 , 0.28]
4673 [0.37 , 0.34]
4674 [0.45 , 0.40]
4675 [0.51 , 0.44]
4676 [0.56 , 0.47]
4677 [0.64 , 0.52]
4678 [0.76 , 0.59]
4679 [0.86 , 0.65]
4680 [1.09 , 0.78]
4681 [1.44 , 0.96]
4682 [3.18 , 1.84]
4683 [4.05 , 2.31]
4684 [-1 , -1] (max twenty pts)
4685 NHYDovf=["ML-OV"]

4686 *%-----|-----|
4687 ADD HYD NHYDsum=["SN_M"], NHYDsto add=["N_M"+"ML-OV"+"MLL_P"]
4688 *%-----|-----|

4689 SAVE HYD NHYD=["SN_M"], # OF PCYCLES=[-1], I CASEs h=[1]


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4690 HYD_COMMENT=["Total Flows at Jockvale Road"]
4691 *%-----|-----|
4692 *#
4693 *# Hydrograph from Jockvale Road routed to Heart's Desire
4694 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 689
4695 *#
4696 ROUTE CHANNEL NHYDout=["N_DE"], NHYDin=["SN_M"],
4697 RDT=[1](min),
4698 CHLGTH=[1962](m), CHSLOPE=[0.2227](%),
4699 FPSLOPE=[0.2227](%),
4700 SECNUM=[1.0], NSEG=[3]
4701 (SEGROUGH, SEGDIST (m))=
4702 [0.075, -17.56
4703 -0.045, 18.27
4704 0.075, 32.51] NSEG times
4705 (DISTANCE (m), ELEVATION (m))=
4706 [-54.07, 85.00]
4707 [-39.43, 84.50]
4708 [-28.30, 84.00]
4709 [-24.12, 83.50]
4710 [-22.30, 83.00]
4711 [-20.55, 82.50]
4712 [-17.56, 82.00]
4713 [-12.63, 81.22]
4714 [-0.11, 80.75]
4715 [11.55, 81.22]
4716 [18.27, 82.00]
4717 [19.82, 82.50]
4718 [22.48, 83.00]
4719 [27.90, 83.50]
4720 [29.31, 84.00]
4721 [30.81, 84.50]
4722 [32.51, 85.00]
4723 *%-----|-----|
4724 *#*****|*****|
4725 *# Catchment DESIRE
4726 *# - To Jock River (north of the Jock)
4727 *# - Rural-estate subdivision (Heart's Desire Community)
4728 *#*****|*****|
4729 CONTINUOUS STANDHYD NHYD=["DESIRE"], DT=[1]min, AREA=[23.78](ha),
4730 XI MP=[0.25], TI MP=[0.25], DWF=[0](cms), LOSS=[2],
4731 SCS curve number CN=[77],
4732 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4733 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4734 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4735 LGI=[400](m), MNI=[0.013], SCI=[0](min),
4736 Continuous simulation parameters:
4737 I a REC per=[4](hrs), I a REC i mp=[4](hrs),
4738 SM N=[-1](mm), SMAX=[-1](mm), SK=[0.010]/(mm),
4739 Inter Event Time=[18](hrs), END=-1
4740 *%-----|-----|
4741 *#*****|*****|
4742 *# Catchment JOCKVA
4743 *# - To Jockvale SWM Facility
4744 *# - Residential development & golf course
4745 *# - JFSA 2021-01-11 update JOCKVA after updating CORRIG as per IBI GROUP, July 2008.
4746 *# JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two
4747 *# areas JOCKVA and EX-LAND 32.5 ha as per IBI GROUP, July 2008.
4748 *#*****|*****|
4749 CONTINUOUS STANDHYD NHYD=["JOCKVA"], DT=[1]min, AREA=[225.13](ha),
4750 XI MP=[0.50], TI MP=[0.50], DWF=[0](cms), LOSS=[2],
4751 SCS curve number CN=[74],
4752 Pervious surfaces: I A per=[4.67](mm), SLPP=[1](%),
4753 LGP=[40](m), MNP=[0.25], SCP=[0](min),
4754 Impervious surfaces: I A i mp=[1.57](mm), SLPI=[1](%),
4755 LGI=[1310.55](m), MNI=[0.013], SCI=[0](min),

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4755 Continuous simulation parameters:
4756 IaRECPper=[ 4] (hrs), IaRECImp=[ 4] (hrs),
4757 SMN=[ -1] (mm), SMAX=[ -1] (mm), SK=[ 0.010]/(mm),
4758 InterEventTime=[ 18] (hrs), END=- 1
4759 *%-----|-----|
4760 ADD HYD NHYDsum=[ "JOCKVA- TO" ], NHYDsto
add=[ "EX- LAND- MN" + "JOCKVA" + " B2- M" + " B3- M" ]
4761 *%-----|-----|
4762 SAVE HYD NHYD=[ "JOCKVA- TO" ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4763 HYD_COMMENT=[ "Total Flows at KB first pond" ]
4764 *%-----|-----|
4765 *#*****|*****|
4766 *# Jockvale SWM Facility
4767 *# - Rating curve obtained from Jockvale Servicing Study (CCL 1999)
4768 *#*****|*****|
4769 ROUTE RESERVOIR NHYDout=[ "JOCK_P" ], NHYDin=[ "JOCKVA- TO" ],
4770 RDT=[ 1] (min),
4771 TABLE of ( OUTFLOW STORAGE ) values
4772 (cms) - (ha-m)
4773 [ 0.0 , 0.0 ]
4774 [ 0.27 , 0.03]
4775 [ 0.28 , 0.55]
4776 [ 0.29 , 1.14]
4777 [ 0.30 , 1.80]
4778 [ 0.31 , 2.32]
4779 [ 1.12 , 2.87]
4780 [ 2.92 , 3.45]
4781 [ 4.64 , 4.07]
4782 [ 6.69 , 4.72]
4783 [ 9.02 , 5.39]
4784 [ 11.62 , 6.10]
4785 [ 14.42 , 6.85]
4786 [ 17.45 , 7.62]
4787 [ 20.69 , 8.44]
4788 [ 24.08 , 9.28]
4789 [ 27.68 , 10.17]
4790 [ -1 , -1 ] (max twenty pts)
4791 NHYDovf=[ "JO- OVF" ]
4792 *%-----|-----|
4793 ADD HYD NHYDsum=[ "SN_DE" ], NHYDsto add=[ "N_DE" + "DESIRE" + "JO- OVF" + "JOCK_P" ]
4794 *%-----|-----|
4795 SAVE HYD NHYD=[ "SN_DE" ], # OF PCYCLES=[ -1], ICASEsh=[ 1]
4796 HYD_COMMENT=[ "Total Flows at Heart's Desire" ]
4797 *%-----|-----|
4798 *#
4799 *# Hydrograph from Heart's Desire routed to Rideau River
4800 *# Channel X-Section obtained from RVCA Hydraulic Model - Station 0
4801 *#
4802 ROUTE CHANNEL NHYDout=[ "N1" ], NHYDin=[ "SN_DE" ],
4803 RDT=[ 1] (min),
4804 CHLGTH=[ 563] (m), CHSLOPE=[ 0.9668] (%),
4805 FPSLOPE=[ 0.9668] (%),
4806 SECNUM=[ 1.0 ], NSEG=[ 3]
4807 ( SEGROUGH, SEGDIST (m) ) =
4808 [ 0.075, -30.20
4809 -0.045, 30.20
4810 0.075, 48.48] NSEG times
4811 ( DISTANCE (m), ELEVATION (m) ) =
4812 [ -98.46, 81.50]
4813 [ -92.24, 81.00]
4814 [ -86.88, 80.50]
4815 [ -81.54, 80.00]
4816 [ -74.36, 79.50]
4817 [ -63.54, 79.00]
4818 [ -39.23, 78.50]
4819 [ -34.51, 78.00]

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4820 [- 33. 01, 77. 50]
4821 [- 30. 20, 77. 00]
4822 [- 13. 42, 76. 18]
4823 [- 1. 14, 76. 09]
4824 [17. 06, 76. 18]
4825 [30. 20, 77. 00]
4826 [32. 95, 77. 50]
4827 [34. 06, 78. 00]
4828 [35. 11, 78. 50]
4829 [36. 32, 79. 00]
4830 [37. 74, 79. 50]
4831 [48. 48, 81. 50]
4832 *%-----|-----|
4833 *#*****|*****|
4834 *# Catchment S-2
4835 *# - To Jock River (north and south)
4836 *# - Undeveloped floodplain and river
4837 *#*****|*****|
4838 CONTINUOUS NASHYD NHYD=["S-2"], DT=[1] min, AREA=[102.94] (ha),
4839 DWF=[0] (cms), CN/C=[72], IA=[4.67] (mm),
4840 N=[3], TP=[0.40] hrs,
4841 Continuous simulation parameters:
4842 IaRECper=[4] (hrs),
4843 SMN=[-1] (mm), SMAX=[-1] (mm), SK=[0.010]/(mm),
4844 InterEventTime=[12] (hrs)
4845 Baseflow simulation parameters:
4846 BaseFlowOption=[1],
4847 InitGWResVol=[50] (mm), GWResK=[0.96] (mm/day/mm)
4848 VHydCond=[0.055] (mm/hr), END=-1
4849 *%-----|-----|
4850 ADD HYD NHYDs um=["SN_NI"], NHYDs to add=["NI"+"S-2"]
4851 *%-----|-----|
4852 SAVE HYD NHYD=["SN_NI"], # OF PCYCLES=[-1], ICASEsh=[1]
4853 HYD_COMMENT=["Total Flows at Rideau River"]
4854 *%-----|-----|
4855 *#####|#####|
4856 *% 5 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4857 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[5]
4858 *% ["C24SC005.stm"] <--storm filename, one per line for NSTORMtime
4859 *%-----|-----|
4860 *% 10 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4861 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[10]
4862 *% ["C24SC010.stm"] <--storm filename, one per line for NSTORMtime
4863 *%-----|-----|
4864 *% 25 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4865 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[25]
4866 *% ["C24SC025.stm"] <--storm filename, one per line for NSTORMtime
4867 *%-----|-----|
4868 *% 50 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4869 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[50]
4870 *% ["C24SC050.stm"] <--storm filename, one per line for NSTORMtime
4871 *%-----|-----|
4872 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4873 *START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4874 *% ["100YC3H.STM"] <--storm filename, one per line for NSTORMtime
4875 *%-----|-----|
4876 *% 100 yr, 24 hr SCS storm based on OTTAWA CDA IDF Curves
4877 START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4878 *% ["C24SC100.stm"] <--storm filename, one per line for NSTORMtime
4879 *%-----|-----|
4880 *% 100 yr, 3 hr Chicago storm based on OTTAWA CDA IDF Curves
4881 *START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[100]
4882 *% ["C24SC100.stm"] <--storm filename, one per line for NSTORMtime
4883 *START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[101]
4884 *% ["A24SC100.stm"] <--storm filename, one per line for NSTORMtime
4885 *START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[102]

```

4886 *%
4887 FINISH
4888

["A24SCI00_60.stm"] <-- storm filename, one per line for NSTORMtime


```

00375# #
00376# R002:CO039 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00377# ADD HYD + 1.0 02:DM1 9506.00 7.379 No.date 33:12 11.30 n/a 000
00378# + 1.0 02:SW1 509.00 2.720 No.date 29:22 11.98 n/a 000
00379# + 1.0 02:NC_CK 1917.00 4.042 No.date 34:24 11.98 n/a 000
00380# SIMM 1.0 01:S_N1 11923.00 12.077 No.date 33:14 11.36 n/a 000
00381#
00382# Sum of hydrographs from Node 11 routed to Node 10
00383# Section 7
00384#
00385# R002:CO040 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00386# ROUTE CHANNEL -> 1.0 02:S_N1 11923.00 12.077 No.date 33:14 11.36 n/a 000
00387# [RFE:1.00] out_c 1.0 01:N1 11923.00 8.276 No.date 39:46 11.36 n/a 000
00388# [L/S= 4028. / 053/ 040]
00389# [Vmax =.462;Dmax=.886]
00390#
00391# Addition of Subwatershed 10 to Node 10
00392#
00393# R002:CO041 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00394# ADD HYD + 1.0 02:N1 11923.00 8.276 No.date 39:46 11.36 n/a 000
00395# + 1.0 02:SW1 509.00 2.720 No.date 29:22 11.98 n/a 000
00396# SIMM 1.0 01:S_N1 11923.00 12.077 No.date 33:14 11.36 n/a 000
00397# [L/S= 3982. / 053/ 040]
00398# [Vmax =.462;Dmax=.886]
00399#
00400# SAVE HYD 1.0 01:S_N1 17589.00 19.451 No.date 38:31 12.19 n/a 000
00401# frame :HLSND
00402# remark:flow at S_N1: N0 + SW10
00403#
00404# Addition of Kings Creek to S_N10
00405#
00406# R002:CO043 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00407# ADD HYD + 1.0 02:S_N10 17589.00 19.451 No.date 38:31 12.19 n/a 000
00408# + 1.0 02:NC_CK 8376.00 11.072 No.date 39:59 11.98 n/a 000
00409# SIMM 1.0 01:S_N10A 25965.00 30.328 No.date 39:58 12.12 n/a 000
00410#
00411# Sum of hydrographs from Node 10 routed to Node 9
00412# Section 2
00413#
00414# R002:CO044 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00415# ROUTE CHANNEL -> 1.0 02:S_N10A 25965.00 29.579 No.date 39:59 12.12 n/a 000
00416# [RFE:1.00] out_c 1.0 01:N1 25965.00 29.579 No.date 39:59 12.12 n/a 000
00417# [L/S= 3982. / 073/ 040]
00418# [Vmax =.595;Dmax=1.208]
00419#
00420# Addition of Subwatershed 9 and Nichols Creek to Node 9
00421#
00422# R002:CO045 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00423# ADD HYD + 1.0 02:N1 25965.00 29.579 No.date 39:59 12.12 n/a 000
00424# + 1.0 02:SW1 131.00 4.434 No.date 39:56 13.25 n/a 000
00425# + 1.0 02:NC_CK 4464.00 5.504 No.date 39:59 10.98 n/a 000
00426# SIMM 1.0 01:S_N1 31561.00 36.313 No.date 39:59 12.00 n/a 000
00427#
00428# Sum of hydrographs from Node 9 routed to Node 8
00429# Section 3
00430#
00431# R002:CO046 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00432# ROUTE CHANNEL -> 1.0 02:S_N1 31561.00 36.313 No.date 39:59 12.00 n/a 000
00433# [RFE:1.00] out_c 1.0 01:N1 31561.00 34.173 No.date 39:59 12.00 n/a 000
00434# [L/S= 3982. / 087/ 040]
00435# [Vmax =.418;Dmax=1.281]
00436#
00437# Addition of Subwatershed 8 and Hibb's Drain to Node 8
00438#
00439# R002:CO047 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00440# ADD HYD + 1.0 02:N1 31561.00 34.173 No.date 39:59 12.00 n/a 000
00441# + 1.0 02:SW1 131.00 4.434 No.date 39:56 13.25 n/a 000
00442# + 1.0 02:IB_DR 3854.00 6.242 No.date 38:46 11.98 n/a 000
00443# SIMM 1.0 01:S_N1 35546.00 40.474 No.date 39:59 12.00 n/a 000
00444#
00445# Sum of hydrographs from Node 8 routed to Node 7
00446# Section 4
00447#
00448# R002:CO048 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00449# ROUTE CHANNEL -> 1.0 02:S_N1 35546.00 40.474 No.date 39:59 12.00 n/a 000
00450# [RFE:1.00] out_c 1.0 01:N1 35546.00 32.891 No.date 44:30 12.00 n/a 000
00451# [L/S= 3750. / 053/ 070]
00452# [Vmax =.208;Dmax=1.651]
00453#
00454# Addition of Subwatershed 7 to Node 7
00455# Section 5
00456#
00457# R002:CO049 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00458# ADD HYD + 1.0 02:SW1 3197.00 4.651 No.date 36:31 9.85 n/a 000
00459# SIMM 1.0 01:S_N1 38743.00 35.071 No.date 43:33 11.82 n/a 000
00460# [L/S= 38743. / 053/ 040]
00461# [Vmax =.387;Dmax=.917]
00462#
00463# Insertion of a Reservoir to simulate the effects of the Richmond Fen.
00464# Storage area and volumes were estimated from available topography.
00465# Release rate from fen was assumed to be controlled by the downstream
00466# river cross section for various conditions. It is assumed that for up to
00467# 0.75 m of water, the main channel of the river provided the storage. Above
00468# this depth, the wetland starts to significantly store water.
00469#
00470# R002:CO050 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00471# ROUTE RESERVOIR -> 1.0 01:RES_RF 38743.00 23.265 No.date 55:09 11.82 n/a 000
00472# [M&S:out_c= 7.00]
00473# [L/S= 38743. / 053/ 040]
00474# [Vmax =.387;Dmax=.917]
00475#
00476# SAVE HYD 1.0 01:RES_RF 38743.00 23.265 No.date 55:09 11.82 n/a 000
00477# frame :HLSRF
00478# remark:outflow of Richmond Fen
00479#
00480# Sum of hydrographs from Node 7 routed to Node 6
00481# Section 5
00482#
00483# R002:CO051 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00484# ROUTE CHANNEL -> 1.0 02:RES_RF 38743.00 23.265 No.date 55:09 11.82 n/a 000
00485# [RFE:1.00] out_c 1.0 01:N1 38743.00 23.228 No.date 56:38 11.82 n/a 000
00486# [L/S= 38743. / 083/ 040]
00487# [Vmax =.432;Dmax=.808]
00488#
00489# Addition of Subwatershed 6 and Van Gual Drain to Node 6
00490#
00491# R002:CO052 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00492# ADD HYD + 1.0 02:N1 38743.00 23.228 No.date 56:38 11.82 n/a 000
00493# + 1.0 02:SW1 1332.00 3.148 No.date 35:23 13.94 n/a 000
00494# + 1.0 02:VG_DR 10320.00 23.318 No.date 39:59 11.89 n/a 000
00495# SIMM 1.0 01:S_N1 40240.00 23.318 No.date 39:59 11.89 n/a 000
00496#
00497# Sum of hydrographs from Node 6 routed to Node 5
00498# Section 6
00499#
00500# R002:CO053 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00501# ROUTE CHANNEL -> 1.0 02:S_N1 40240.00 23.318 No.date 39:59 11.89 n/a 000
00502# [RFE:1.00] out_c 1.0 01:N1 40240.00 23.285 No.date 56:09 11.89 n/a 000
00503# [L/S= 4630. / 053/ 040]
00504# [Vmax =.378;Dmax=.917]
00505#
00506# Addition of Subwatershed 5 and Flowing Creek to Node 5
00507#
00508# R002:CO054 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00509# ADD HYD + 1.0 02:N1 40240.00 23.285 No.date 56:09 11.89 n/a 000
00510# + 1.0 02:SW1 224.00 2.597 No.date 35:23 13.94 n/a 000
00511# + 1.0 02:FL_CK 4945.00 14.839 No.date 32:25 14.57 n/a 000
00512# SIMM 1.0 01:S_N1 44509.00 33.166 No.date 37:08 12.20 n/a 000
00513#
00514# Sum of hydrographs from Node 5 routed to Node 5A
00515# Section 7
00516#
00517# R002:CO055 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00518# ROUTE CHANNEL -> 1.0 02:RES_RF 45409.01 33.166 No.date 37:08 12.20 n/a 000
00519# [RFE:1.00] out_c 1.0 01:N1 45409.01 33.155 No.date 37:20 12.20 n/a 000
00520# [L/S= 556. / 090/ 040]
00521# [Vmax =.443;Dmax=.937]
00522#
00523# Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
00524#
00525# R002:CO056 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00526# ADD HYD + 1.0 02:SA 45409.01 33.155 No.date 37:23 12.20 n/a 000
00527# + 1.0 02:SWA2 20.00 .309 No.date 28:36 17.79 n/a 000
00528# + 1.0 02:SWA1 1412.00 3.090 No.date 38:04 15.22 n/a 000
00529# SIMM 1.0 01:S_N1A 46841.01 36.236 No.date 37:28 12.30 n/a 000
00530#
00531# Sum of hydrographs from Node 5A routed to Node 4
00532# Section 8
00533#
00534# R002:CO059 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00535# ROUTE CHANNEL -> 1.0 02:S_N1A 46841.01 36.236 No.date 37:28 12.30 n/a 000
00536# [RFE:1.00] out_c 1.0 01:N1 46841.01 35.288 No.date 39:22 12.30 n/a 000
00537# [L/S= 4630. / 043/ 035]
00538# [Vmax =.695;Dmax=2.444]
00539#
00540# Addition of Subwatershed 4 and Leary Creek to Node 4
00541#
00542# R002:CO060 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00543# ADD HYD + 1.0 02:SA 46841.01 35.288 No.date 39:22 12.30 n/a 000
00544# + 1.0 02:SWA 585.00 4.325 No.date 29:58 17.79 n/a 000
00545# + 1.0 02:LM_CK 1021.00 5.747 No.date 30:50 17.79 n/a 000
00546# SIMM 1.0 01:S_N1 48447.00 37.581 No.date 38:13 12.47 n/a 000
00547#
00548# SAVE HYD 1.0 01:S_N1 48447.00 37.581 No.date 38:13 12.47 n/a 000
00549# frame :S.N.002
00550# remark:flow at S_N1
00551#
00552# Sum of hydrographs from Node 4 routed to Node 2
00553# Section 9
00554#
00555# R002:CO062 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00556# ROUTE CHANNEL -> 1.0 02:S_N1 48447.00 37.581 No.date 38:13 12.47 n/a 000
00557# [RFE:1.00] out_c 1.0 01:N1 48447.00 37.455 No.date 38:49 12.47 n/a 000
00558# [L/S= 1667. / 060/ 040]
00559# [Vmax =.715;Dmax=2.845]
00560#
00561# Addition of Subwatershed 2 with Mbohan Drain and Smith Drain to Node 2
00562#
00563# R002:CO063 --- Dfm-n-ID-NND --- AREHA-QPEAKm-TpeakDate-hh:mm --- Rvmm-R-C --- Dwfcm
00564# ADD HYD + 1.0 02:N1 48447.00 37.455 No.date 38:49 12.47 n/a 000

```


Table listing project details for various road segments, including stationing, mileposts, and project descriptions. The table is organized into columns with headers like 'Stationing', 'Mileposts', 'Project Description', 'Construction Method', and 'Status'. It includes numerous rows for different road sections and projects, such as I-66, I-490, and I-275.

Table with multiple columns containing numerical data, text descriptions, and identifiers. The table is organized into several sections, with some rows grouped by a common identifier (e.g., 033770). The data includes various parameters and values, often with units or abbreviations.

Table with columns for station ID, description, and various parameters. The table contains multiple entries for stations such as 03741, 03742, 03743, etc., up to 03926. Each entry includes detailed technical specifications and data points.


```

048863 #   frame : SN RK 0005
048864 #   remark: Total Flow over Station 3633
048865 #   Hydrograph from Station 3633 to Node Todd
048866 #   Channel X Section obtained from RCEA Hydraulic Model Station 3633
048867 #   JFSA 2021-02-26 change the channel length (at model station 3633) from 650m to 608m and change the slope to 0.0489% to 0.2
048868 #
048869 # R005: C00309 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048870 # ROUTE CHANNEL > 1.0 02: SN BK 54681.13 70.342 NoDate 38.13 18.71 n/a 0.000
048871 # [RFS= 1.00] out c 0.01: TO 54681.13 70.065 NoDate 36.56 18.71 n/a 0.000
048872 # [L/S= 608. / 247.035]
048873 #
048874 #-----
048875 # Catchment Greenbank
048876 # To Greenbank Drain (south of the Jack)
048877 # - JFSA 2021-01-18 add Greenbank pond as per JFSA P598(04)-15, June 2016
048878 # - JFSA 2021-01-17 update area from 37.479 ha to 36.6 ha based on GIS measurements
048879 #
048880 # R005: C00310 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048881 # CONTNEX STANHD 1.0 01: TO 5487.73 70.171 NoDate 36.56 18.73 n/a 0.000
048882 # [LRS= 2 Cn= 77.0]
048883 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
048884 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 494.0: MNP= 013: SCL= 0]
048885 # [IARECmp= 4.00: IARECpe= 4.00]
048886 # [SM N= 31.15: SMW= 207.66: SKE= 010]
048887 #
048888 # ROUTE RESERVE R > 1.0 02: Greenbank 36.60 3.925 NoDate 28.02 42.59 n/a 0.000
048889 # out c 1.0 03: Greenbank 36.60 8.11 NoDate 28.35 42.58 n/a 0.000
048890 # over flow s 1.0 03: TO 5487.73 70.171 NoDate 28.35 42.58 n/a 0.000
048891 # [MkS otred= 76055E04 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
048892 #
048893 # R005: C00312 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048894 # ADD HYD + 1.0 02: TO 5487.13 70.065 NoDate 36.56 18.71 n/a 0.000
048895 # + 1.0 02: Greenbank 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
048896 # + 1.0 02: Greenbank 36.60 8.11 NoDate 28.35 42.58 n/a 0.000
048897 # SIM 1.0 01: TO 5477.73 70.171 NoDate 36.56 18.73 n/a 0.000
048898 # [L/S= 53.71: DM= 571]
048899 #
048900 # R005: C00313 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048901 # SAVE HYD 1.0 01: TO 5477.73 70.171 NoDate 36.56 18.73 n/a 0.000
048902 # frame : Greenbank 0005
048903 # remark: Total Flow at Greenbank Drain
048904 #
048905 #-----
048906 # Catchment TODD
048907 # - To Todd Drain (south of the Jack)
048908 # - Substn with 436imp as per Barhaven South MS
048909 # - 2020-11-30 increase imp. based on P598(04)-11
048910 # - 2020-11-30 update area from 146.016 ha to 146.015 ha based on P598(04)-11
048911 # - 2020-11-30 split TODD Drainage Area to MWLR, MNR, POND and ALL
048912 #
048913 # R005: C00314 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048914 # CONTNEX STANHD 1.0 01: TO 5487.73 70.171 NoDate 38.41 67.2 0.000
048915 # [LRS= 53.71: DM= 571]
048916 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
048917 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 118.0: MNP= 013: SCL= 0]
048918 # [IARECmp= 4.00: IARECpe= 4.00]
048919 # [SM N= 31.15: SMW= 207.66: SKE= 010]
048920 #
048921 # R005: C00315 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048922 # CONTNEX STANHD 1.0 01: TO 5487.73 70.171 NoDate 38.39 67.2 0.000
048923 # [LRS= 2 Cn= 77.0]
048924 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
048925 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 28.0: MNP= 013: SCL= 0]
048926 # [IARECmp= 4.00: IARECpe= 4.00]
048927 # [SM N= 31.15: SMW= 207.66: SKE= 010]
048928 #
048929 # R005: C00316 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048930 # CONTNEX STANHD 1.0 01: TO 5487.73 70.171 NoDate 38.39 67.2 0.000
048931 # [LRS= 52: TM= 64]
048932 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
048933 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 449.0: MNP= 013: SCL= 0]
048934 # [IARECmp= 4.00: IARECpe= 4.00]
048935 # [SM N= 31.15: SMW= 207.66: SKE= 010]
048936 #
048937 # R005: C00317 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048938 # CONTNEX STANHD 1.0 01: TO 5487.73 70.171 NoDate 38.41 66.9 0.000
048939 # [LRS= 52: TM= 57]
048940 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
048941 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 868.0: MNP= 013: SCL= 0]
048942 # [IARECmp= 4.00: IARECpe= 4.00]
048943 # [SM N= 31.15: SMW= 207.66: SKE= 010]
048944 #
048945 # R005: C00318 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048946 # CONTNEX STANHD 1.0 01: TO 5487.73 70.171 NoDate 38.39 67.2 0.000
048947 # [LRS= 52: TM= 63]
048948 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
048949 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 143.0: MNP= 013: SCL= 0]
048950 # [IARECmp= 4.00: IARECpe= 4.00]
048951 # [SM N= 31.15: SMW= 207.66: SKE= 010]
048952 #
048953 # R005: C00319 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048954 # ROUTE RESERVE R > 1.0 02: TODD 30.23 2.959 NoDate 28.02 39.47 n/a 0.000
048955 # + 1.0 02: TODD 30.23 2.959 NoDate 28.02 39.47 n/a 0.000
048956 # over flow s 1.0 03: TO 5487.73 70.171 NoDate 28.35 42.58 n/a 0.000
048957 # [MkS otred= 8949E04 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
048958 #
048959 # R005: C00320 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048960 # ROUTE RESERVE R > 1.0 02: TODD 2.10 2.238 NoDate 28.00 38.41 n/a 0.000
048961 # out c 1.0 03: TODD 2.10 2.238 NoDate 28.00 38.41 n/a 0.000
048962 # over flow s 1.0 03: TODD 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
048963 # [MkS otred= 062E04 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
048964 #
048965 # R005: C00321 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048966 # ROUTE RESERVE R > 1.0 02: TODD 12.04 1.414 NoDate 28.00 38.39 n/a 0.000
048967 # out c 1.0 03: TODD 12.04 1.414 NoDate 28.00 38.39 n/a 0.000
048968 # over flow s 1.0 03: TODD 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
048969 # [MkS otred= 8651E04 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
048970 #
048971 # R005: C00322 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048972 # CONTNEX STANHD 1.0 01: TO 5487.73 70.171 NoDate 38.41 66.9 0.000
048973 # [LRS= 42: TM= 52]
048974 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
048975 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 566.0: MNP= 013: SCL= 0]
048976 # [IARECmp= 4.00: IARECpe= 4.00]
048977 # [SM N= 31.15: SMW= 207.66: SKE= 010]
048978 #
048979 # R005: C00323 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048980 # COMPUTE DUALHD 1.0 01: A2 25.50 1.991 NoDate 28.03 34.66 n/a 0.000
048981 # Mjor System / 1.0 02: B2 25.50 1.991 NoDate 28.03 34.66 n/a 0.000
048982 # Mjor System V 1.0 03: A2-MN 25.50 1.818 NoDate 28.00 34.70 n/a 0.000
048983 # [M Sys= 3163E04 mb, TotOfVol= 0.000E+00 mb, N of A= 2, TotDurOf= 0 hrs]
048984 #
048985 # R005: C00324 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048986 # ADD HYD + 1.0 02: TODD 2.10 2.238 NoDate 28.00 38.41 n/a 0.000
048987 # + 1.0 02: TODD 2.10 2.238 NoDate 28.00 38.41 n/a 0.000
048988 # + 1.0 02: TODD 11.91 9.069 NoDate 28.05 38.19 n/a 0.000
048989 # + 1.0 02: CLAR.MN 1.77 1.192 NoDate 28.00 37.36 n/a 0.000
048990 # SIM 1.0 01: TO 5487.73 70.171 NoDate 36.56 18.73 n/a 0.000
048991 # [L/S= 53.71: DM= 571]
048992 #
048993 # R005: C00325 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
048994 # SAVE HYD 1.0 01: TO 5487.73 70.171 NoDate 36.56 18.73 n/a 0.000
048995 # frame : TODD 0005
048996 # remark: Total Flow at Todd Drain
048997 #
048998 #-----
048999 # Todd Pond 3
049000 # - Rater curve obtained from Barhaven South MS modeling
049001 # - stantec 2007, Tributary Drainage Area to MESS Pond 3 = 193 ha
049002 #
049003 # R005: C00326 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049004 # ROUTE RESERVE R > 1.0 02: TODD 119.95 9.538 NoDate 28.04 38.27 n/a 0.000
049005 # out c 1.0 03: TODD 119.95 9.538 NoDate 28.04 38.27 n/a 0.000
049006 # over flow s 1.0 03: TODD 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049007 # [MkS otred= 2251E04 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049008 #
049009 # R005: C00327 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049010 # ADD HYD + 1.0 02: M3.P3 119.95 3.765 NoDate 28.29 38.27 n/a 0.000
049011 # + 1.0 02: P3-OF 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049012 # + 1.0 02: TODD 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049013 # + 1.0 02: AC.M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049014 # + 1.0 02: TODD 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049015 # SIM 1.0 01: TO 5487.73 70.171 NoDate 36.56 18.73 n/a 0.000
049016 # [L/S= 53.71: DM= 571]
049017 #
049018 # R005: C00328 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049019 # SAVE HYD 1.0 01: SN TO 54837.69 70.373 NoDate 36.55 18.77 n/a 0.000
049020 # frame : SN TO 0005
049021 # remark: Total Flow at Todd Drain
049022 #
049023 #-----
049024 # Hydrograph from Todd Drain routed to Corrigan Drain
049025 # Channel X Section obtained from RCEA Hydraulic Model Station 2462
049026 # 2021-02-19 Change the slope from 0.033 % (as per Stantec Report 2007) to 0.05 % so the model will be more stable and g
049027 #
049028 # R005: C00329 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049029 # ROUTE CHANNEL > 1.0 02: SN BK 54837.69 70.373 NoDate 36.55 18.77 n/a 0.000
049030 # [RFS= 1.00] out c 0.01: TO 54837.69 70.373 NoDate 37.09 18.77 n/a 0.000
049031 # [L/S= 280. / 105.0]
049032 #
049033 # R005: C00330 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049034 # CONTNEX STANHD 1.0 01: TO 54837.69 70.373 NoDate 37.09 18.77 n/a 0.000
049035 # [LRS= 52: TM= 64]
049036 # [Pervious area: Iper= 4.67: SLP1= 0.0: LCP= 40.0: MNP= 250: SCP= 0]
049037 # [Impervious area: IAm= 1.57: SLP1= 0.0: LCP= 325.0: MNP= 013: SCL= 0]
049038 # [IARECmp= 4.00: IARECpe= 4.00]
049039 # [SM N= 31.15: SMW= 207.66: SKE= 010]
049040 #
049041 # R005: C00331 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049042 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.81 n/a 0.000
049043 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049044 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049045 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049046 #
049047 # R005: C00332 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049048 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049049 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049050 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049051 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049052 #
049053 # R005: C00333 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049054 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049055 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049056 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049057 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049058 #
049059 # R005: C00334 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049060 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049061 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049062 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049063 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049064 #
049065 # R005: C00335 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049066 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049067 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049068 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049069 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049070 #
049071 # R005: C00336 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049072 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049073 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049074 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049075 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049076 #
049077 # R005: C00337 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049078 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049079 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049080 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049081 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049082 #
049083 # R005: C00338 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049084 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049085 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049086 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049087 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049088 #
049089 # R005: C00339 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049090 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049091 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049092 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049093 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049094 #
049095 # R005: C00340 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049096 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049097 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049098 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049099 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049100 #
049101 # R005: C00341 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049102 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049103 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049104 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049105 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049106 #
049107 # R005: C00342 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049108 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049109 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049110 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049111 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049112 #
049113 # R005: C00343 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049114 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049115 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049116 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049117 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049118 #
049119 # R005: C00344 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049120 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049121 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049122 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049123 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049124 #
049125 # R005: C00345 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049126 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049127 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049128 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049129 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049130 #
049131 # R005: C00346 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049132 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049133 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049134 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049135 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049136 #
049137 # R005: C00347 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049138 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049139 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049140 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049141 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049142 #
049143 # R005: C00348 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049144 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049145 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049146 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049147 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049148 #
049149 # R005: C00349 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049150 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600 NoDate 28.00 41.58 n/a 0.000
049151 # Mjor System / 1.0 02: B1-M 0.00 0.00 NoDate 0.00 0.00 n/a 0.000
049152 # Mjor System V 1.0 03: corr1-M 15.18 0.806 NoDate 28.01 41.58 n/a 0.000
049153 # [M Sys= 0.000E+00 mb, TotOfVol= 0.000E+00 mb, N of A= 0, TotDurOf= 0 hrs]
049154 #
049155 # R005: C00350 ----- Dfna : ID NDD ----- AREHA-QPEAGm-TpeakDte:hh:mm ----- Rvmm R C ----- Dfcm
049156 # COMPUTE DUALHD 1.0 01: B1 15.18 0.600
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052375	[SMN= 33.81; SMWX=225.43; SKG= 010]	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
052388	REMARK: Total Flows at MH04	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
052389	COMPUTE DUALJHD / 1.0 01: A1	10.61	1.053	Ndate	28.02	34.82	n/a	0.00	
052400	Mjr System / 1.0 02: A1-M	0.00	0.00	Ndate	0.00	0.00	n/a	0.00	
052411	Mnor System / 1.0 03: A0-M	7.60	638	Ndate	28.02	34.82	n/a	0.00	
052425	[M SysSt= 0000E+00; TotOfVol= 0000E+00; NOf= 0; TotDirOf= 0. hrs]								
052428	ADD IHD / 1.0 02: B5-N	17.90	1,183	Ndate	28.06	34.84	n/a	0.00	
052435	Mjr System / 1.0 02: A1-M	7.20	638	Ndate	28.02	34.82	n/a	0.00	
052440	Mnor System / 1.0 03: A0-M	25.50	1,761	Ndate	28.05	34.83	n/a	0.00	
052478	SAVE IHD / 1.0 01: MH04	25.50	1,761	Ndate	28.05	34.83	n/a	0.00	
052490	frank: MH04.0005								
052511	REMARK: Total Flows at MH04	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
052512	COMPUTE DUALJHD / 1.0 01: A1	25.50	1,761	Ndate	28.05	34.83	n/a	0.00	
052528	Mjr System / 1.0 02: A1-M	25.50	1,698	Ndate	28.06	34.83	n/a	0.00	
052540	Mnor System / 1.0 03: A0-M	132.52	9,463	Ndate	28.04	35.27	n/a	0.00	
052585	[D= 1.65; Dused= 1.65]								
052588	ADD IHD / 1.0 02: 140-104	25.50	1,698	Ndate	28.06	34.83	n/a	0.00	
052600	SIMM / 1.0 01: MH04T	132.52	9,463	Ndate	28.04	35.27	n/a	0.00	
052611	REMARK: Total Flows at MH04	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
052612	COMPUTE DUALJHD / 1.0 01: A1	132.52	9,463	Ndate	28.04	35.27	n/a	0.00	
052628	Mjr System / 1.0 02: A1-M	132.52	9,463	Ndate	28.04	35.27	n/a	0.00	
052640	Mnor System / 1.0 03: A0-M	132.52	8,848	Ndate	28.06	35.27	n/a	0.00	
052685	[Vmax= 2.02; Dmax= 1.86]								
052688	ADD IHD / 1.0 01: B6-TM	132.52	8,848	Ndate	28.06	35.27	n/a	0.00	
052690	frank: MH05.0005								
052711	REMARK: Total Flows at MH05	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
052712	COMPUTE DUALJHD / 1.0 01: A1	2.20	248	Ndate	28.00	38.90	681	0.00	
052728	Mjr System / 1.0 02: A1-M	2.20	248	Ndate	28.00	38.90	681	0.00	
052740	Mnor System / 1.0 03: A0-M	2.20	248	Ndate	28.00	38.90	681	0.00	
052785	[D= 1.80; Dused= 1.80]								
052788	ADD IHD / 1.0 01: B6-TM	2.20	248	Ndate	28.00	38.90	681	0.00	
052800	frank: MH07.0005								
052811	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
052812	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
052828	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
052840	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
052885	[Vmax= 1.898; Dmax= 1.622]								
052888	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
052900	frank: MH07.0005								
052911	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
052912	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
052928	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
052940	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
052985	[Vmax= 1.898; Dmax= 1.622]								
052988	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053000	frank: MH07.0005								
053011	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053012	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053028	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053040	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053085	[Vmax= 1.898; Dmax= 1.622]								
053088	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053100	frank: MH07.0005								
053111	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053112	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053128	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053140	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053185	[Vmax= 1.898; Dmax= 1.622]								
053188	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053200	frank: MH07.0005								
053211	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053212	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053228	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053240	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053285	[Vmax= 1.898; Dmax= 1.622]								
053288	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053300	frank: MH07.0005								
053311	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053312	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053328	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053340	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053385	[Vmax= 1.898; Dmax= 1.622]								
053388	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053400	frank: MH07.0005								
053411	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053412	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053428	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053440	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053485	[Vmax= 1.898; Dmax= 1.622]								
053488	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053500	frank: MH07.0005								
053511	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053512	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053528	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053540	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053585	[Vmax= 1.898; Dmax= 1.622]								
053588	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053600	frank: MH07.0005								
053611	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053612	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053628	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053640	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053685	[Vmax= 1.898; Dmax= 1.622]								
053688	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053700	frank: MH07.0005								
053711	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053712	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053728	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053740	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053785	[Vmax= 1.898; Dmax= 1.622]								
053788	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053800	frank: MH07.0005								
053811	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053812	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053828	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053840	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053885	[Vmax= 1.898; Dmax= 1.622]								
053888	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053900	frank: MH07.0005								
053911	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
053912	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053928	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053940	Mnor System / 1.0 03: A0-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
053985	[Vmax= 1.898; Dmax= 1.622]								
053988	ADD IHD / 1.0 01: B6-TM	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
054000	frank: MH07.0005								
054011	REMARK: Total Flows at MH07	ARESHA-QPEAKM-TpeakDate-hh:mm--Rvmm R C--Dwfcm							
054012	COMPUTE DUALJHD / 1.0 01: A1	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
054028	Mjr System / 1.0 02: A1-M	138.28	5,061	Ndate	28.07	35.68	n/a	0.00	
054040	Mnor System / 1.0 03: A0-M								


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059855 # (Approximated cross-section - see cross-section 258)
059856 # Use method for routing conditions and node 025 for spring conditions
059857 #
059858 # Sum of hydrographs from Node 12 routed to Node 11 with Dumpy section 248
059859 #
059900 R010: C00038 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
059901 ROUTE CHANNEL -> 1.0 02: S_N2 9506.00 12.534 Ndate 22:45 19.65 n/a 0.00
059902 [RDE:1.00] out< 1.0 01: N0 9506.00 12.710 Ndate 33:02 19.65 n/a 0.00
059903 [L/S= 972 / 054/ 040]
059904 [Vmax = 680; Dmax = 2.98]
059905 #
059906 # Addition of Subwatershed 11 and No Name Creek to Node 11
059907 #
059908 R010: C00039 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
059909 ADD HYD + 1.0 02: N0 9506.00 12.710 Ndate 33:02 19.65 n/a 0.00
060000 + 1.0 02: SW10 500.00 5.639 Ndate 29:22 21.19 n/a 0.00
060001 + 1.0 02: NC_CK 1917.00 7.897 Ndate 34:28 21.19 n/a 0.00
060002 + 1.0 01: S_N1 11923.00 21.813 Ndate 33:05 19.96 n/a 0.00
060003 #
060004 # Sum of hydrographs from Node 11 routed to Node 10
060005 # Section 7
060006 R010: C00040 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060007 ROUTE CHANNEL -> 1.0 02: S_N1 11923.00 21.813 Ndate 33:05 19.96 n/a 0.00
060008 [RDE:1.00] out< 1.0 01: N0 11923.00 14.761 Ndate 39:58 19.96 n/a 0.00
060009 [L/S= 14028 / 157/ 040]
060010 [Vmax = 452; Dmax = 1.212]
060011 #
060012 # Addition of Subwatershed 10 to Node 10
060013 #
060014 R010: C00041 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060015 ADD HYD + 1.0 02: N0 11923.00 14.761 Ndate 39:58 19.96 n/a 0.00
060016 + 1.0 02: SW10 3066.00 21.255 Ndate 37:58 24.81 n/a 0.00
060017 + 1.0 01: S_N0 17589.00 35.808 Ndate 38:35 21.52 n/a 0.00
060018 + 1.0 02: NC_CK 4864.00 10.128 Ndate 39:59 19.29 n/a 0.00
060019 + 1.0 01: S_N0 31561.00 66.284 Ndate 39:59 21.20 n/a 0.00
060020 SAVE HYD 1.0 01: S_N0 31561.00 66.284 Ndate 39:59 21.20 n/a 0.00
060021 Frame: H_SNO
060022 remark: flow at S_N0: N0 + SW10
060023 # Addition of Kings Creek to S_N10
060024 #
060025 R010: C00043 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060026 ADD HYD + 1.0 02: N0 17589.00 35.808 Ndate 38:35 21.52 n/a 0.00
060027 + 1.0 02: NC_CK 8376.00 20.398 Ndate 39:59 21.19 n/a 0.00
060028 + 1.0 01: S_N10A 25965.00 55.807 Ndate 39:58 21.41 n/a 0.00
060029 #
060030 # Sum of hydrographs from Node 10 routed to Node 9
060031 # Section 7
060032 #
060033 R010: C00044 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060034 ROUTE CHANNEL -> 1.0 02: S_N0A 25965.00 54.076 Ndate 39:59 21.41 n/a 0.00
060035 [RDE:1.00] out< 1.0 01: N0 25965.00 54.076 Ndate 39:59 21.41 n/a 0.00
060036 [L/S= 3082 / 074/ 040]
060037 [Vmax = 682; Dmax = 1.095]
060038 #
060039 # Addition of Subwatershed 9 and Nichols Creek to Node 9
060040 #
060041 R010: C00045 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060042 ADD HYD + 1.0 02: N0 25965.00 54.076 Ndate 39:59 21.41 n/a 0.00
060043 + 1.0 02: SW10 1132.00 8.921 Ndate 30:54 23.73 n/a 0.00
060044 + 1.0 02: NC_CK 4864.00 10.128 Ndate 39:59 19.29 n/a 0.00
060045 + 1.0 01: S_N0 31561.00 66.284 Ndate 39:59 21.20 n/a 0.00
060046 #
060047 # Sum of hydrographs from Node 9 routed to Node 8
060048 # Section 7
060049 #
060050 R010: C00046 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060051 ROUTE CHANNEL -> 1.0 02: S_N 31561.00 66.284 Ndate 39:59 21.20 n/a 0.00
060052 [RDE:1.00] out< 1.0 01: N0 31561.00 61.483 Ndate 39:57 21.20 n/a 0.00
060053 [L/S= 2209 / 088/ 045]
060054 [Vmax = 363; Dmax = 1.619]
060055 #
060056 # Addition of Subwatershed 8 and Hibb's Drain to Node 8
060057 #
060058 R010: C00047 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060059 ADD HYD + 1.0 02: N0 31561.00 61.483 Ndate 39:57 21.20 n/a 0.00
060060 + 1.0 02: SW10 131.00 1.689 Ndate 30:54 23.73 n/a 0.00
060061 + 1.0 02: HB_DR 3854.00 11.811 Ndate 38:27 21.19 n/a 0.00
060062 + 1.0 01: S_N 35546.00 73.344 Ndate 39:57 21.19 n/a 0.00
060063 #
060064 # Sum of hydrographs from Node 8 routed to Node 7
060065 # Section 4
060066 #
060067 R010: C00048 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060068 ROUTE CHANNEL -> 1.0 02: S_N 35546.00 73.344 Ndate 39:57 21.19 n/a 0.00
060069 [RDE:1.00] out< 1.0 01: N0 35546.00 61.416 Ndate 45:01 21.19 n/a 0.00
060070 [L/S= 3750 / 053/ 070]
060071 [Vmax = 218; Dmax = 1.987]
060072 #
060073 # Addition of Subwatershed 7 to Node 7
060074 #
060075 R010: C00049 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060076 ADD HYD + 1.0 02: N0 35546.00 61.416 Ndate 45:01 21.19 n/a 0.00
060077 + 1.0 02: SW7 3197.00 8.899 Ndate 36:26 17.07 n/a 0.00
060078 + 1.0 01: S_N 38743.00 65.819 Ndate 44:06 20.85 n/a 0.00
060079 + 1.0 02: NC_CK 3947.00 65.819 Ndate 44:06 20.85 n/a 0.00
060080 SAVE HYD 1.0 01: S_N 38743.00 65.819 Ndate 44:06 20.85 n/a 0.00
060081 Frame: H_SNT
060082 remark: flow at S_N: N0 + SW7
060083 # Insertion of a reservoir to simulate the effects of the Richmond Fen.
060084 # Storage area and volumes were estimated from available top maps.
060085 # Release rate from Fen was assumed to be controlled by the downstream
060086 # river cross section for summer conditions. It is assumed that for up to
060087 # 0.75 m of water, the main channel of the river provided the storage. Above
060088 # this depth, the wetland starts to significantly store water.
060089 #
060090 R010: C00051 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060091 ROUTE RESERVOIR -> 1.0 02: S_N 38743.00 65.819 Ndate 44:06 20.85 n/a 0.00
060092 [RDE:1.00] out< 1.0 01: RES_RF 38743.00 31.796 Ndate 60:32 20.85 n/a 0.00
060093 [MS: 0.00; 25070; 0]
060094 R010: C00052 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060095 SAVE HYD 1.0 01: RES_RF 38743.00 31.796 Ndate 60:32 20.85 n/a 0.00
060096 Frame: H_RES_RF
060097 remark: outflow from Richmond Fen
060098 #
060099 # Sum of hydrographs from Node 7 routed to Node 6
060100 # Section 7
060101 #
060102 R010: C00053 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060103 ROUTE CHANNEL -> 1.0 02: S_N 38743.00 31.796 Ndate 60:32 20.85 n/a 0.00
060104 [RDE:1.00] out< 1.0 01: N0 38743.00 31.737 Ndate 62:00 20.85 n/a 0.00
060105 [L/S= 1852 / 083/ 040]
060106 [Vmax = 477; Dmax = 960]
060107 #
060108 # Addition of Subwatershed 6 and Van Gail Drain to Node 6
060109 #
060110 R010: C00054 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060111 ADD HYD + 1.0 02: N0 38743.00 31.737 Ndate 62:00 20.85 n/a 0.00
060112 + 1.0 02: SW5 224.00 2.246 Ndate 28:45 28.24 n/a 0.00
060113 + 1.0 02: ML_CR 1821.00 11.195 Ndate 30:48 20.72 n/a 0.00
060114 + 1.0 02: PFL_CK 4945.00 28.945 Ndate 33:21 25.81 n/a 0.00
060115 + 1.0 01: S_N 40240.00 31.737 Ndate 62:00 20.99 n/a 0.00
060116 #
060117 # Sum of hydrographs from Node 6 routed to Node 5
060118 # Section 6
060119 R010: C00055 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060120 ROUTE CHANNEL -> 1.0 02: S_N 40240.00 31.737 Ndate 62:00 20.99 n/a 0.00
060121 [RDE:1.00] out< 1.0 01: N0 40240.00 31.713 Ndate 62:48 20.99 n/a 0.00
060122 [L/S= 1852 / 054/ 055]
060123 [Vmax = 412; Dmax = 1.069]
060124 #
060125 # Addition of Subwatershed 5 and Flowing Creek to Node 5
060126 #
060127 R010: C00056 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060128 ADD HYD + 1.0 02: N0 40240.00 31.713 Ndate 62:48 20.99 n/a 0.00
060129 + 1.0 02: SW5 224.00 2.246 Ndate 28:45 28.24 n/a 0.00
060130 + 1.0 02: PFL_CK 4945.00 28.945 Ndate 33:21 25.81 n/a 0.00
060131 + 1.0 01: S_N 40240.00 31.713 Ndate 62:48 20.99 n/a 0.00
060132 #
060133 # Sum of hydrographs from Node 5 routed to Node 5A
060134 # Section 7
060135 #
060136 R010: C00057 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060137 ROUTE CHANNEL -> 1.0 02: S_NA 45409.01 51.312 Ndate 35:12 21.56 n/a 0.00
060138 [RDE:1.00] out< 1.0 01: N0A 45409.01 51.312 Ndate 35:12 21.56 n/a 0.00
060139 [L/S= 556 / 090/ 040]
060140 [Vmax = 485; Dmax = 1.131]
060141 #
060142 # Addition of Subwatershed 5A1 and Subwatershed 5A2 to Node 5A
060143 #
060144 R010: C00058 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060145 ADD HYD + 1.0 02: N0 45409.01 51.312 Ndate 35:12 21.56 n/a 0.00
060146 + 1.0 02: SW5A1 585.00 8.458 Ndate 29:57 31.37 n/a 0.00
060147 + 1.0 02: ML_CR 1821.00 11.195 Ndate 30:48 20.72 n/a 0.00
060148 + 1.0 02: SW5A2 1412.00 5.817 Ndate 37:54 27.06 n/a 0.00
060149 + 1.0 01: S_NA 46841.01 46.788 Ndate 35:22 21.73 n/a 0.00
060150 #
060151 # Sum of hydrographs from Node 5A routed to Node 4
060152 # Section 8
060153 R010: C00059 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060154 ROUTE CHANNEL -> 1.0 02: S_NA 46841.01 54.543 Ndate 35:22 21.73 n/a 0.00
060155 [RDE:1.00] out< 1.0 01: N0A 46841.01 54.543 Ndate 36:56 21.73 n/a 0.00
060156 [L/S= 4630 / 043/ 055]
060157 [Vmax = 793; Dmax = 3.295]
060158 #
060159 # Addition of Subwatershed 4 and Leamy Creek to Node 4
060160 #
060161 R010: C00060 -> DfIn-ID NDD -> AREQA-QPEAKm-TPeakDte-hh-mm-RvM-R-C--DfWm
060162 ADD HYD + 1.0 02: N0 46841.01 54.543 Ndate 36:56 21.73 n/a 0.00
060163 + 1.0 02: SW4 585.00 8.458 Ndate 29:57 31.37 n/a 0.00
060164 + 1.0 02: ML_CR 1821.00 11.195 Ndate 30:48 20.72 n/a 0.00
060165 + 1.0 02: SW5A1 1412.00 5.817 Ndate 37:54 27.06 n/a 0.00
060166 + 1.0 01: S_N 46841.01 46.788 Ndate 35:22 21.73 n/a 0.00
060167 #
060168 # Sum of hydrographs from Node 4 routed to Node 2
060169 # Section 8
060170 #
060171 # Sum of hydrographs from Node 4 routed to Node 2

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06359# R010:CO091.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06360# ROUTE RESERVOIR > 1.0 0:2:AcA 66.75 6.470 Ndate 28.10 48.24 n/a 000
06361# overFlow < 1.0 0:1:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06362# [MSStkEds:1835E:02 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06363# [InterEvent Time=12.00]
06364# R010:CO092.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06365# CONTINUES STANDBY 1.0 0:1:AcA 66.75 6.470 Ndate 29:04 22.40 346 000
06366# [XMP:68:TIMP:85]
06367# [InterEvent Time=12.00]
06368# [InterEvent Time=12.00]
06369# R010:CO093.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06370# ROUTE PIPE > 1.0 0:2:STR 3.51 0.75 Ndate 28:04 22.40 n/a 000
06371# [RDE:1.00] out < 1.0 0:1:BRND 60.55 852 Ndate 29:08 22.40 n/a 000
06372# [L/S=528 / 230] 0.013
06373# [Vms=1.080 Dms=441]
06374# [XMP:68:TIMP:85]
06375# R010:CO094.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06376# ADD HYD > 1.0 0:2:BRND 261.31 2.972 Ndate 28:31 17.14 n/a 000
06377# overFlow < 1.0 0:1:STR 3.51 0.75 Ndate 28:25 38.69 n/a 000
06378# + 1.0 0:2:AI:STR 2.50 0.98 Ndate 28:24 50.29 n/a 000
06379# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 28:12 50.29 n/a 000
06380# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06381# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06382# + 1.0 0:2:BRPIE 60.55 852 Ndate 29:08 22.40 n/a 000
06383# SIML 1.0 0:1:ST:LN 326.12 3.771 Ndate 28:40 18.45 n/a 000
06384# R010:CO095.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06385# CONTINUES STANDBY 1.0 0:1:AcA 3.51 538 Ndate 28:01 50.29 777 000
06386# [XMP:68:TIMP:85]
06387# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06388# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06389# [Impervious area:IAmp:1.57;SLPI:50;LGA:265;MNI:013;SCI:0]
06390# [IARECmp:4.00;IAREPwr:4.00]
06391# R010:CO096.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06392# ROUTE RESERVOIR > 1.0 0:2:AcA 3.51 538 Ndate 28:01 50.29 n/a 000
06393# overFlow < 1.0 0:1:STR 3.51 538 Ndate 28:01 50.29 n/a 000
06394# + 1.0 0:2:AI:STR 2.50 0.98 Ndate 28:01 50.29 n/a 000
06395# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 28:01 50.29 n/a 000
06396# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06397# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06398# SIML 1.0 0:1:ST:LN 326.12 3.771 Ndate 28:40 18.45 n/a 000
06399# [XMP:68:TIMP:85]
06400# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06401# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06402# [Impervious area:IAmp:1.57;SLPI:50;LGA:119;MNI:013;SCI:0]
06403# [IARECmp:4.00;IAREPwr:4.00]
06404# R010:CO098.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06405# ROUTE RESERVOIR > 1.0 0:2:ST 7.1 0.89 Ndate 28:00 38.69 n/a 000
06406# overFlow < 1.0 0:1:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06407# [MSStkEds:1988E:02 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06408# R010:CO099.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06409# ADD HYD > 1.0 0:2:ST:LN 326.12 3.771 Ndate 28:40 18.45 n/a 000
06410# overFlow < 1.0 0:1:STR 3.51 0.75 Ndate 28:25 38.69 n/a 000
06411# + 1.0 0:2:AI:STR 2.50 0.98 Ndate 28:24 50.29 n/a 000
06412# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 28:12 50.29 n/a 000
06413# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06414# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06415# SIML 1.0 0:1:PTSTS 330.34 3.914 Ndate 28:39 18.83 n/a 000
06416# R010:CO100.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06417# ROUTE CHANNEL > 1.0 0:1:PTSTS 330.34 3.914 Ndate 28:39 18.83 n/a 000
06418# [RDE:1.00] out < 1.0 0:1:BRND 330.34 3.914 Ndate 29:01 18.83 n/a 000
06419# [L/S=592 / 230] 0.043
06420# [Vms=582.0 Dms=1002]
06421# R010:CO101.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06422# CONTINUES STANDBY 1.0 0:1:AcA 2.28 0.46 Ndate 29:04 29.30 453 000
06423# [Cm:86.0;N:3.00;Tp=73]
06424# [IAREC:4.00;SMN:21.09;SMX:140.62;Ske:010]
06425# R010:CO102.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06426# CONTINUES STANDBY 1.0 0:1:AcA 12.04 1.618 Ndate 28:04 50.29 777 000
06427# [XMP:68:TIMP:85]
06428# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06429# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06430# [Impervious area:IAmp:1.57;SLPI:50;LGA:491;MNI:013;SCI:0]
06431# [IARECmp:4.00;IAREPwr:4.00]
06432# R010:CO103.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06433# ROUTE RESERVOIR > 1.0 0:2:AcA 12.04 1.618 Ndate 28:04 50.29 n/a 000
06434# overFlow < 1.0 0:1:AI:STR 3.51 0.75 Ndate 28:04 50.29 n/a 000
06435# + 1.0 0:2:AI:STR 2.50 0.98 Ndate 28:04 50.29 n/a 000
06436# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 28:04 50.29 n/a 000
06437# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06438# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06439# SIML 1.0 0:1:ST:LN 326.12 3.771 Ndate 28:40 18.45 n/a 000
06440# [XMP:46:TIMP:57]
06441# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06442# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06443# [Impervious area:IAmp:1.57;SLPI:50;LGA:84;MNI:013;SCI:0]
06444# [IARECmp:4.00;IAREPwr:4.00]
06445# R010:CO105.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06446# ROUTE RESERVOIR > 1.0 0:2:ST 3.51 0.46 Ndate 28:00 38.69 n/a 000
06447# overFlow < 1.0 0:1:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06448# [MSStkEds:2771E:00 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06449# R010:CO106.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06450# CONTINUES STANDBY 1.0 0:1:AcA 5.30 780 Ndate 28:02 50.29 777 000
06451# [XMP:68:TIMP:85]
06452# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06453# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06454# [Impervious area:IAmp:1.57;SLPI:50;LGA:326;MNI:013;SCI:0]
06455# [IARECmp:4.00;IAREPwr:4.00]
06456# R010:CO107.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06457# ROUTE RESERVOIR > 1.0 0:2:AcA 5.30 780 Ndate 28:02 50.29 n/a 000
06458# overFlow < 1.0 0:1:AI:STR 3.51 0.75 Ndate 28:02 50.29 n/a 000
06459# + 1.0 0:2:AI:STR 2.50 0.98 Ndate 28:02 50.29 n/a 000
06460# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 28:02 50.29 n/a 000
06461# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06462# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06463# ADD HYD > 1.0 0:2:ST 2.28 0.46 Ndate 29:04 29.30 n/a 000
06464# overFlow < 1.0 0:1:STSTR 12.04 1.618 Ndate 28:31 50.29 n/a 000
06465# + 1.0 0:2:AI:STR 3.51 0.75 Ndate 28:12 50.29 n/a 000
06466# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 28:04 50.29 n/a 000
06467# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06468# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06469# SIML 1.0 0:1:PTSTS 350.31 4.213 Ndate 28:37 20.48 n/a 000
06470# R010:CO109.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06471# ROUTE CHANNEL > 1.0 0:1:PTSTS 350.31 4.213 Ndate 28:37 20.48 n/a 000
06472# [RDE:1.00] out < 1.0 0:1:BRND 350.31 4.037 Ndate 29:17 20.48 n/a 000
06473# [L/S=528 / 230] 0.043
06474# [Vms=606.0 Dms=1049]
06475# R010:CO110.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06476# CONTINUES STANDBY 1.0 0:1:AcA 7.59 759 Ndate 28:11 49.05 758 000
06477# [Cm:86.0;N:3.00;Tp=73]
06478# [IAREC:4.00;SMN:21.09;SMX:116.21;Ske:010]
06479# [InterEvent Time=12.00]
06480# [InterEvent Time=12.00]
06481# R010:CO111.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06482# CONTINUES STANDBY 1.0 0:1:AcA 3.41 523 Ndate 28:01 50.29 777 000
06483# [XMP:68:TIMP:85]
06484# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06485# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06486# [Impervious area:IAmp:1.57;SLPI:50;LGA:261;MNI:013;SCI:0]
06487# [IARECmp:4.00;IAREPwr:4.00]
06488# R010:CO112.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06489# ROUTE RESERVOIR > 1.0 0:2:ST 3.41 523 Ndate 28:01 50.29 n/a 000
06490# overFlow < 1.0 0:1:CL:STR 3.41 133 Ndate 28:25 50.29 n/a 000
06491# + 1.0 0:2:CL:STR 2.50 0.98 Ndate 28:01 50.29 n/a 000
06492# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06493# [MSStkEds:7930E:01 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06494# CONTINUES STANDBY 1.0 0:1:ST:LN 326.12 3.771 Ndate 28:40 18.45 n/a 000
06495# [XMP:46:TIMP:57]
06496# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06497# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06498# [Impervious area:IAmp:1.57;SLPI:50;LGA:95;MNI:013;SCI:0]
06499# [IARECmp:4.00;IAREPwr:4.00]
06500# R010:CO113.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06501# ROUTE RESERVOIR > 1.0 0:2:ST 4.5 058 Ndate 28:00 38.69 n/a 000
06502# overFlow < 1.0 0:1:STSTR 4.5 040 Ndate 28:04 38.69 n/a 000
06503# [MSStkEds:1552E:02 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06504# ADD HYD > 1.0 0:2:BRND 350.31 4.037 Ndate 29:17 20.48 n/a 000
06505# overFlow < 1.0 0:1:STR 3.51 0.75 Ndate 28:12 50.29 n/a 000
06506# + 1.0 0:2:AI:STR 2.50 0.98 Ndate 28:25 50.29 n/a 000
06507# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 28:04 50.29 n/a 000
06508# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06509# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06510# SIML 1.0 0:1:PTSTS 356.68 4.181 Ndate 29:15 20.87 n/a 000
06511# R010:CO116.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06512# ROUTE CHANNEL > 1.0 0:1:PTSTS 356.68 4.181 Ndate 29:15 20.87 n/a 000
06513# [RDE:1.00] out < 1.0 0:1:BRND 356.68 4.181 Ndate 29:15 20.87 n/a 000
06514# [L/S=528 / 230] 0.043
06515# CONTINUES STANDBY 1.0 0:1:STRAND 7.59 759 Ndate 28:11 49.05 758 000
06516# [XMP:68:TIMP:85]
06517# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06518# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06519# [Impervious area:IAmp:1.57;SLPI:50;LGA:1230;MNI:013;SCI:0]
06520# [IARECmp:4.00;IAREPwr:4.00]
06521# R010:CO117.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06522# ROUTE RESERVOIR > 1.0 0:2:STRAND 7.59 759 Ndate 28:11 49.05 n/a 000
06523# overFlow < 1.0 0:1:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06524# [MSStkEds:5930E:00 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06525# R010:CO118.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06526# ADD HYD > 1.0 0:2:ST:LN 356.68 4.181 Ndate 29:15 20.87 n/a 000
06527# overFlow < 1.0 0:1:AI:STR 3.51 0.75 Ndate 29:15 20.87 n/a 000
06528# + 1.0 0:2:AI:STR 2.50 0.98 Ndate 29:15 20.87 n/a 000
06529# + 1.0 0:2:CL:STR 1.51 0.59 Ndate 29:15 20.87 n/a 000
06530# + 1.0 0:2:STSTR 59 052 Ndate 28:05 38.69 n/a 000
06531# + 1.0 0:2:STWOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06532# SIML 1.0 0:1:SSACUT 364.27 4.242 Ndate 29:15 21.45 n/a 000
06533# R010:CO119.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06534# remark:SSACUT 0.010
06535# R010:CO120.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06536# CONTINUES STANDBY 1.0 0:1:AcA 66.75 6.470 Ndate 28:10 48.24 746 000
06537# [XMP:64:TIMP:80]
06538# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06539# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06540# [Impervious area:IAmp:1.57;SLPI:50;LGA:115;MNI:013;SCI:0]
06541# [IARECmp:4.00;IAREPwr:4.00]
06542# R010:CO121.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06543# SAVE HYD 1.0 0:1:AcA 66.75 6.470 Ndate 28:10 48.24 n/a 000
06544# [XMP:64:TIMP:80]
06545# remark:SMW:AFInflw
06546# R010:CO122.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06547# ROUTE RESERVOIR > 1.0 0:2:AcA 66.75 6.470 Ndate 28:10 48.24 n/a 000
06548# overFlow < 1.0 0:1:SWMOW 0.00 0.00 Ndate 0:00 0.00 n/a 000
06549# [MSStkEds:2350E:00 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06550# R010:CO123.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06551# SAVED HYD 1.0 0:1:SWMOW 66.75 6.465 Ndate 29:54 48.24 n/a 000
06552# [XMP:64:TIMP:80]
06553# remark:SMW:AFInflw
06554# R010:CO124.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06555# ADD HYD > 1.0 0:2:SSACUT 364.27 4.242 Ndate 29:15 21.45 n/a 000
06556# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06557# [Impervious area:IAmp:1.57;SLPI:50;LGA:180;MNI:013;SCI:0]
06558# [IARECmp:4.00;IAREPwr:4.00]
06559# R010:CO125.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06560# ROUTE RESERVOIR > 1.0 0:2:CF 1.87 0.74 Ndate 28:01 50.29 n/a 000
06561# overFlow < 1.0 0:3:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06562# CONTINUES STANDBY 1.0 0:1:CF 1.87 0.74 Ndate 28:01 50.29 777 000
06563# [XMP:68:TIMP:85]
06564# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06565# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06566# [Impervious area:IAmp:1.57;SLPI:50;LGA:193;MNI:013;SCI:0]
06567# [IARECmp:4.00;IAREPwr:4.00]
06568# R010:CO126.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06569# ROUTE RESERVOIR > 1.0 0:2:CF 1.87 0.74 Ndate 28:01 50.29 n/a 000
06570# overFlow < 1.0 0:3:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06571# [MSStkEds:4578E:02 mb, TotOfVol=0.0000E+00 mb, NOf=0, TotDurOf=0 h:rs]
06572# R010:CO127.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06573# CONTINUES STANDBY 1.0 0:1:CF 1.62 0.262 Ndate 28:01 50.29 777 000
06574# [XMP:68:TIMP:85]
06575# [Horton parameters: Fw:76.20;Fc:13.20;ICAV:14;Fa:00]
06576# [Previous area:IApwr:4.67;SLPP:50;LGP:50;MNP:250;SCP:0]
06577# [Impervious area:IAmp:1.57;SLPI:50;LGA:180;MNI:013;SCI:0]
06578# [IARECmp:4.00;IAREPwr:4.00]
06579# R010:CO128.....DfIn n ID NND.....AREhA-QPEAKm-TpEakht-hh:mm.....RvM R C.....DfWcm
06580# ROUTE RESERVOIR > 1.0 0:2:CF 1.62 0.262 Ndate 28:01 50.29 n/a 000
06581# overFlow < 1.0 0:2:CF:STR 4.1 0.656 Ndate 28:04 38.69 n/a 000
06582# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06583# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06584# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06585# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06586# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06587# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06588# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06589# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06590# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06591# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06592# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06593# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06594# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06595# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06596# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06597# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06598# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06599# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06600# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06601# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06602# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06603# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06604# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06605# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06606# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06607# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06608# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06609# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06610# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06611# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06612# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06613# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06614# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06615# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06616# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06617# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06618# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06619# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06620# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06621# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06622# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06623# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06624# + 1.0 0:2:CF:STR 1.51 0.59 Ndate 28:04 38.69 n/a 000
06625# + 1.0 0:2:CF:WOF 0.00 0.00 Ndate 0:00 0.00 n/a 000
06626# + 1.0 0:2:CF:STR 2.50 0.98 Ndate 28:04 38.69 n/a 000
06627# + 1.0
```



```

071075 R0100:CD0125.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071085 SAVE IVD.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071095 Iname=WCLAR_0010
071105 # Rating curve obtained from West Clarke
071115 *****
071125 # West Clarke Pond 2
071135 # Rating curve obtained from Barhaven South MS modeling
071145 # Tributary Drainage Area to MS Pond 2 = 241 ha
071155 *****
071165 R0100:CD0126.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071175 ROUTE RBSURF.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071185 out sw 1.0 0:02:18.128 119.40 3.959 29.02 28.33 47.94 n/a 0.00
071195 overflow sw 1.0 0:02:18.128 119.40 3.959 29.02 28.33 47.94 n/a 0.00
071205 [MfSto=0.000E+00, TotOfVol=0.000E+00, NOf=0, TotDirOf=0.0hrs]
071215 *****
071225 R0100:CD0127.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071235 ADD IVD.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071245 + 1.0 0:02:1-DIR 3.28 29.02 29.02 29.12 48.75 n/a 0.00
071255 + 1.0 0:02:1-DR 12.75 48.75 48.75 48.75 48.75 n/a 0.00
071265 + 1.0 0:02:1-DR 2.03 48.75 48.75 48.75 48.75 n/a 0.00
071275 + 1.0 0:02:1-DR 0.00 0.00 0.00 0.00 0.00 n/a 0.00
071285 + 1.0 0:02:1-DR 0.00 0.00 0.00 0.00 0.00 n/a 0.00
071295 + 1.0 0:02:1-DR 0.00 0.00 0.00 0.00 0.00 n/a 0.00
071305 SIMM 1.0 0:01:SN,CE 54253.88 90.199 29.02 33.44 22.88 n/a 0.00
071315 R0100:CD0128.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071325 SAVE IVD.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071335 Iname=SN,CE 0010
071345 remark:Total Flow before Station 5737 on Jack River
071355 # Channel X-Section obtained from RCV4 Hydraulic Model - Station 5737
071365 # 2021-02-25 add station 5737 before station 5002. Station 5737 was extracted from the HEC-RAS model T:\PRA\1474-1616.s
071375 # FJSA 2021-03-02 change the slope to 0.01756 instead of 0.02593 to stabilize the model
071385 R0100:CD0129.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071395 ROUTE CHANNEL.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071405 + [RDR=1.00] out sw 1.0 0:01:57.377 54253.88 85.563 29.02 36.21 22.88 n/a 0.00
071415 [L/Sma=2.0] [L/Smas=2.0]
071425 [L/Sma=7.00] [L/Smas=4.78]
071435 R0100:CD0130.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071445 ADD IVD.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071455 + 1.0 0:02:1-DR 21.67 191.29 29.12 29.23 48.83 n/a 0.00
071465 + 1.0 0:02:1-DR 1.85 48.83 48.83 48.83 48.83 n/a 0.00
071475 + 1.0 0:02:1-DR 2.03 48.75 48.75 48.75 48.75 n/a 0.00
071485 + 1.0 0:02:1-DR 0.00 0.00 0.00 0.00 0.00 n/a 0.00
071495 + 1.0 0:02:1-DR 0.00 0.00 0.00 0.00 0.00 n/a 0.00
071505 SIMM 1.0 0:02:1-DR 54279.34 85.716 29.02 36.21 22.89 n/a 0.00
071515 R0100:CD0131.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071525 SAVE IVD.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071535 Iname=5002.010
071545 remark:Total Flow before Station 5002 on Jack River
071555 # Hydrograph from Node 0 at Node at West Clarke Drain
071565 # Channel X-Section obtained from RCV4 Hydraulic Model - Station 5002
071575 # FJSA 2021-02-19 change the slope to 0.01756 instead of 0.02593 to stabilize the model
071585 # FJSA 2021-02-19 change to three ROUTE CHANNEL with length 275 m each instead of one with 825 m length so the model will
071595 # FJSA 2021-02-19 change the slope to 0.01756 instead of 0.02593 to stabilize the model because of adding station 5737 be
071605 R0100:CD0132.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071615 ROUTE CHANNEL.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071625 + [RDR=1.00] out sw 1.0 0:02:50.2 54279.34 85.716 29.02 36.21 22.89 n/a 0.00
071635 [L/Sma=2.0] [L/Smas=2.0]
071645 [L/Sma=245 / 095 / 05]
071655 [L/Sma=1.13] [L/Smas=2.0]
071665 R0100:CD0133.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071675 ROUTE CHANNEL.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071685 + [RDR=1.00] out sw 1.0 0:01:NB,WD 54279.34 85.716 29.02 36.25 22.89 n/a 0.00
071695 [L/Sma=245 / 095 / 05]
071705 [L/Sma=1.13] [L/Smas=2.0]
071715 R0100:CD0134.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071725 ROUTE CHANNEL.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071735 + [RDR=1.00] out sw 1.0 0:01:NB,WC 54279.34 85.716 29.02 36.25 22.89 n/a 0.00
071745 [L/Sma=245 / 095 / 05]
071755 [L/Sma=1.13] [L/Smas=2.0]
071765 # Hydrograph from Node 0 at Node at Kennedy Burn Drain
071775 # Channel X-Section obtained from RCV4 Hydraulic Model - Station 4534
071785 *****
071795 R0100:CD0135.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071805 ROUTE CHANNEL.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071815 + [RDR=1.00] out sw 1.0 0:02:NC 54279.34 85.716 29.02 36.25 22.89 n/a 0.00
071825 [L/Sma=1020 / 050 / 05]
071835 [L/Sma=1.13] [L/Smas=2.0]
071845 # Catchment Runoff
071855 # To Kennedy Burnett SWM Facility
071865 # Outlets to Fraser-Clarke drain (north of the Jack)
071875 # Mid-Reach
071885 *****
071895 R0100:CD0136.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071905 Existing Kennedy Burnett SWM Facility
071915 # Rating curve obtained from LRRBP
071925 # Tributary Drainage Area to MS Pond 160 ha
071935 *****
071945 R0100:CD0137.....Dfwn-ID NDD.....AREHA-QPEAKm-TPeakDate-hh:mm.....Rvmm R C.....Dfwmc
071955 CINTLNKX STANHYD 1.0 0:01:KB,01A 40.82 3.338 29.02 28.12 24.14 37.3 n/a 0.00
071965 [MfSto=10.0] [MfSto=10.0]
071975 [MfSto=10.0] [MfSto=10.0]
071985 [MfSto=10.0] [MfSto=10.0]
071995 [MfSto=10.0] [MfSto=10.0]
072005 [MfSto=10.0] [MfSto=10.0]
072015 [MfSto=10.0] [MfSto=10.0]
072025 [MfSto=10.0] [MfSto=10.0]
072035 [MfSto=10.0] [MfSto=10.0]
072045 [MfSto=10.0] [MfSto=10.0]
072055 [MfSto=10.0] [MfSto=10.0]
072065 [MfSto=10.0] [MfSto=10.0]
072075 [MfSto=10.0] [MfSto=10.0]
072085 [MfSto=10.0] [MfSto=10.0]
072095 [MfSto=10.0] [MfSto=10.0]
072105 [MfSto=10.0] [MfSto=10.0]
072115 [MfSto=10.0] [MfSto=10.0]
072125 [MfSto=10.0] [MfSto=10.0]
072135 [MfSto=10.0] [MfSto=10.0]
072145 [MfSto=10.0] [MfSto=10.0]
072155 [MfSto=10.0] [MfSto=10.0]
072165 [MfSto=10.0] [MfSto=10.0]
072175 [MfSto=10.0] [MfSto=10.0]
072185 [MfSto=10.0] [MfSto=10.0]
072195 [MfSto=10.0] [MfSto=10.0]
072205 [MfSto=10.0] [MfSto=10.0]
072215 [MfSto=10.0] [MfSto=10.0]
072225 [MfSto=10.0] [MfSto=10.0]
072235 [MfSto=10.0] [MfSto=10.0]
072245 [MfSto=10.0] [MfSto=10.0]
072255 [MfSto=10.0] [MfSto=10.0]
072265 [MfSto=10.0] [MfSto=10.0]
072275 [MfSto=10.0] [MfSto=10.0]
072285 [MfSto=10.0] [MfSto=10.0]
072295 [MfSto=10.0] [MfSto=10.0]
072305 [MfSto=10.0] [MfSto=10.0]
072315 [MfSto=10.0] [MfSto=10.0]
072325 [MfSto=10.0] [MfSto=10.0]
072335 [MfSto=10.0] [MfSto=10.0]
072345 [MfSto=10.0] [MfSto=10.0]
072355 [MfSto=10.0] [MfSto=10.0]
072365 [MfSto=10.0] [MfSto=10.0]
072375 [MfSto=10.0] [MfSto=10.0]
072385 [MfSto=10.0] [MfSto=10.0]
072395 [MfSto=10.0] [MfSto=10.0]
072405 [MfSto=10.0] [MfSto=10.0]
072415 [MfSto=10.0] [MfSto=10.0]
072425 [MfSto=10.0] [MfSto=10.0]
072435 [MfSto=10.0] [MfSto=10.0]
072445 [MfSto=10.0] [MfSto=10.0]
072455 [MfSto=10.0] [MfSto=10.0]
072465 [MfSto=10.0] [MfSto=10.0]
072475 [MfSto=10.0] [MfSto=10.0]
072485 [MfSto=10.0] [MfSto=10.0]
072495 [MfSto=10.0] [MfSto=10.0]
072505 [MfSto=10.0] [MfSto=10.0]
072515 [MfSto=10.0] [MfSto=10.0]
072525 [MfSto=10.0] [MfSto=10.0]
072535 [MfSto=10.0] [MfSto=10.0]
072545 [MfSto=10.0] [MfSto=10.0]
072555 [MfSto=10.0] [MfSto=10.0]
072565 [MfSto=10.0] [MfSto=10.0]
072575 [MfSto=10.0] [MfSto=10.0]
072585 [MfSto=10.0] [MfSto=10.0]
072595 [MfSto=10.0] [MfSto=10.0]
072605 [MfSto=10.0] [MfSto=10.0]
072615 [MfSto=10.0] [MfSto=10.0]
072625 [MfSto=10.0] [MfSto=10.0]
072635 [MfSto=10.0] [MfSto=10.0]
072645 [MfSto=10.0] [MfSto=10.0]
072655 [MfSto=10.0] [MfSto=10.0]
072665 [MfSto=10.0] [MfSto=10.0]
072675 [MfSto=10.0] [MfSto=10.0]
072685 [MfSto=10.0] [MfSto=10.0]
072695 [MfSto=10.0] [MfSto=10.0]
072705 [MfSto=10.0] [MfSto=10.0]
072715 [MfSto=10.0] [MfSto=10.0]
072725 [MfSto=10.0] [MfSto=10.0]
072735 [MfSto=10.0] [MfSto=10.0]
072745 [MfSto=10.0] [MfSto=10.0]
072755 [MfSto=10.0] [MfSto=10.0]
072765 [MfSto=10.0] [MfSto=10.0]
072775 [MfSto=10.0] [MfSto=10.0]
072785 [MfSto=10.0] [MfSto=10.0]
072795 [MfSto=10.0] [MfSto=10.0]
072805 [MfSto=10.0] [MfSto=10.0]
072815 [MfSto=10.0] [MfSto=10.0]
072825 [MfSto=10.0] [MfSto=10.0]
072835 [MfSto=10.0] [MfSto=10.0]
072845 [MfSto=10.0] [MfSto=10.0]
072855 [MfSto=10.0] [MfSto=10.0]
072865 [MfSto=10.0] [MfSto=10.0]
072875 [MfSto=10.0] [MfSto=10.0]
072885 [MfSto=10.0] [MfSto=10.0]
072895 [MfSto=10.0] [MfSto=10.0]
072905 [MfSto=10.0] [MfSto=10.0]
072915 [MfSto=10.0] [MfSto=10.0]
072925 [MfSto=10.0] [MfSto=10.0]
072935 [MfSto=10.0] [MfSto=10.0]

```


074811 + 1.0 0:2:KB-Pflow 00 000 N:date 0:00 00 n/a 0:00

074828 * SLM 254.24 15.083 N:date 28:10 35.46 n/a 0:00

074830 R010:0276 Df:n-ID NND AREA:QPEAKm-TpEakTdt-h:m-- Rvm:R C...DWfcm

074848 * SLM 254.24 15.083 N:date 28:10 35.46 n/a 0:00

074851 # name:KB-Pond3.0010

074860 remark:Total Flow at KB second pond

074870 CONTINUOUS STANDHD 1.0 0:1:KB-P3 2.80 4.65 N:date 28:01 52.17 8.06 0:00

074880 # [X.M: 75.TiM: 75]

074900 # (Perov area: I:Apr: 4.67.SLPP: 0.LG: 40.MN: 250.SCP: 0)

074910 # (Imperv area: I:Apr: 4.67.SLPP: 0.LG: 137.MN: 013.SCI: 0)

074920 # (I:Apr: 4.00.I:Apr: 4.00)

074930 # (I:Apr: 4.00.I:Apr: 4.00)

074940 # (I:Apr: 4.00.I:Apr: 4.00)

074950 # (I:Apr: 4.00.I:Apr: 4.00)

074960 # (I:Apr: 4.00.I:Apr: 4.00)

074970 # (I:Apr: 4.00.I:Apr: 4.00)

074980 R010:0276 Df:n-ID NND AREA:QPEAKm-TpEakTdt-h:m-- Rvm:R C...DWfcm

074990 # ROUTE RESERVOIR > 1.0 0:2:KB-P3 257.04 15.295 N:date 28:08 35.64 n/a 0:00

075000 # overlow < 1.0 0:1:KB-P3 257.04 15.295 N:date 28:08 35.64 n/a 0:00

075010 # (I:Apr: 4.00.I:Apr: 4.00)

075020 # (I:Apr: 4.00.I:Apr: 4.00)

075030 # (I:Apr: 4.00.I:Apr: 4.00)

075040 # (I:Apr: 4.00.I:Apr: 4.00)

075050 # (I:Apr: 4.00.I:Apr: 4.00)

075060 # (I:Apr: 4.00.I:Apr: 4.00)

075070 R010:0281 Df:n-ID NND AREA:QPEAKm-TpEakTdt-h:m-- Rvm:R C...DWfcm

075080 # SAVE HD 1.0 0:1:KB-Pond3 257.04 15.294 N:date 28:09 35.64 n/a 0:00

075090 # (I:Apr: 4.00.I:Apr: 4.00)

075100 # (I:Apr: 4.00.I:Apr: 4.00)

075110 # (I:Apr: 4.00.I:Apr: 4.00)

075120 # (I:Apr: 4.00.I:Apr: 4.00)

075130 # (I:Apr: 4.00.I:Apr: 4.00)

075140 # (I:Apr: 4.00.I:Apr: 4.00)

075150 # (I:Apr: 4.00.I:Apr: 4.00)

075160 # (I:Apr: 4.00.I:Apr: 4.00)

075170 # (I:Apr: 4.00.I:Apr: 4.00)

075180 # (I:Apr: 4.00.I:Apr: 4.00)

075190 # (I:Apr: 4.00.I:Apr: 4.00)

075200 # (I:Apr: 4.00.I:Apr: 4.00)

075210 # (I:Apr: 4.00.I:Apr: 4.00)

075220 # (I:Apr: 4.00.I:Apr: 4.00)

075230 # (I:Apr: 4.00.I:Apr: 4.00)

075240 # (I:Apr: 4.00.I:Apr: 4.00)

075250 # (I:Apr: 4.00.I:Apr: 4.00)

075260 # (I:Apr: 4.00.I:Apr: 4.00)

075270 # (I:Apr: 4.00.I:Apr: 4.00)

075280 # (I:Apr: 4.00.I:Apr: 4.00)

075290 # (I:Apr: 4.00.I:Apr: 4.00)

075300 # (I:Apr: 4.00.I:Apr: 4.00)

075310 R010:0282 Df:n-ID NND AREA:QPEAKm-TpEakTdt-h:m-- Rvm:R C...DWfcm

075320 # CONTINUOUS STANDHD 1.0 0:1:FRASER-D 28.01 59.76 9.24 0:00

075330 # [X.M: 93.TiM: 93]

075340 # (Perov area: I:Apr: 4.67.SLPP: 0.LG: 40.MN: 250.SCP: 0)

075350 # (Imperv area: I:Apr: 4.67.SLPP: 0.LG: 137.MN: 013.SCI: 0)

075360 # (I:Apr: 4.00.I:Apr: 4.00)

075370 # (I:Apr: 4.00.I:Apr: 4.00)

075380 # (I:Apr: 4.00.I:Apr: 4.00)

075390 # (I:Apr: 4.00.I:Apr: 4.00)

075400 # (I:Apr: 4.00.I:Apr: 4.00)

075410 # (I:Apr: 4.00.I:Apr: 4.00)

075420 # (I:Apr: 4.00.I:Apr: 4.00)

075430 # (I:Apr: 4.00.I:Apr: 4.00)

075440 # (I:Apr: 4.00.I:Apr: 4.00)

075450 # (I:Apr: 4.00.I:Apr: 4.00)

075460 # (I:Apr: 4.00.I:Apr: 4.00)

075470 # (I:Apr: 4.00.I:Apr: 4.00)

075480 # (I:Apr: 4.00.I:Apr: 4.00)

075490 # (I:Apr: 4.00.I:Apr: 4.00)

075500 # (I:Apr: 4.00.I:Apr: 4.00)

075510 # (I:Apr: 4.00.I:Apr: 4.00)

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075600 # (I:Apr: 4.00.I:Apr: 4.00)

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076100 # (I:Apr: 4.00.I:Apr: 4.00)

076110 # (I:Apr: 4.00.I:Apr: 4.00)

076120 # (I:Apr: 4.00.I:Apr: 4.00)

076130 # (I:Apr: 4.00.I:Apr: 4.00)

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076170 # (I:Apr: 4.00.I:Apr: 4.00)

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076190 # (I:Apr: 4.00.I:Apr: 4.00)

076200 # (I:Apr: 4.00.I:Apr: 4.00)

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076240 # (I:Apr: 4.00.I:Apr: 4.00)

076250 # (I:Apr: 4.00.I:Apr: 4.00)

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076270 # (I:Apr: 4.00.I:Apr: 4.00)

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076290 # (I:Apr: 4.00.I:Apr: 4.00)

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076370 # (I:Apr: 4.00.I:Apr: 4.00)

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076390 # (I:Apr: 4.00.I:Apr: 4.00)

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076670 # (I:Apr: 4.00.I:Apr: 4.00)


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082259 remark:Total Flow at MH06
082300 R010:CH0417-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082310 ROUTE PIPE > 1.0 02:MH07 119.82 3.962 NoDate 28:02 41.87 n/a 000
082320 * [RDS=1.00 out-c 1.0 01:MH107 119.82 3.962 NoDate 28:03 41.87 n/a 000
      [L/S/n= 123./100/0.03]
082330 * [Vmax=1.666;Dmax=1.530]
082340 [Dmax=1.80;Dbase=1.34]
082350 CONTINX STANDHD 1.0 01:MH107 4.14 .416 NoDate 28:01 38.46 .595 000
082360 [XfM=35;TfM=47]
082370 [LGRS=2 Cn=75.0]
082400 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
082410 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 183.;MNI= 013;SCI= 0]
082420 [IARECmp= 4.00; IARECPR= 4.00]
082430 [SM N= 33.81; SMM=225.43; Sfk= 010]
082440 R010:CH0418-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082450 COMPUTE DIALHD 1.0 01:MH107 4.14 .416 NoDate 28:01 38.46 n/a 000
082460 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
082470 Minor System \ 1.0 02:AO-M 4.14 .310 NoDate 27:55 38.48 n/a 000
082480 [M Syst=on 14172E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
082490 CONTINX STANDHD 1.0 01:MH107 10.61 1.251 NoDate 28:01 45.36 7.01 000
082500 [LGRS=2 Cn=75.0]
082510 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
082520 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 183.;MNI= 013;SCI= 0]
082530 [IARECmp= 4.00; IARECPR= 4.00]
082540 [SM N= 33.81; SMM=225.43; Sfk= 010]
082550 R010:CH0419-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082560 COMPUTE DIALHD 1.0 01:MH107 10.61 1.251 NoDate 28:01 45.36 n/a 000
082570 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
082580 Minor System \ 1.0 02:AO-M 10.61 .993 NoDate 27:56 45.38 n/a 000
082590 [M Syst=on 10161E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
082600 CONTINX STANDHD 1.0 01:MH107 10.61 .993 NoDate 27:56 45.38 n/a 000
082610 [LGRS=2 Cn=75.0]
082620 R010:CH0420-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082630 COMPUTE DIALHD 1.0 01:MH107 10.61 .993 NoDate 27:56 45.38 n/a 000
082640 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
082650 Minor System \ 1.0 02:AO-M 10.61 .993 NoDate 27:56 45.38 n/a 000
082660 [M Syst=on 14172E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
082670 CONTINX STANDHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
082680 [LGRS=2 Cn=75.0]
082690 R010:CH0421-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082700 COMPUTE DIALHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
082710 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
082720 Minor System \ 1.0 02:AO-M 134.57 5.265 NoDate 28:03 42.04 n/a 000
082730 [M Syst=on 114./120/0.03]
082740 [L/S/n= 114./120/0.03]
082750 [Vmax=1.012;Dmax=2.00]
082760 [Dmax=1.80;Dbase=2.00]
082770 R010:CH0422-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082780 ROUTE PIPE > 1.0 01:MH108 134.57 5.265 NoDate 28:03 42.04 n/a 000
082790 * [RDS=1.00 out-c 1.0 01:119-108 134.57 5.268 NoDate 28:06 42.04 n/a 000
      [L/S/n= 66./120/0.03]
082800 [Vmax=1.011;Dmax=1.639]
082810 [Dmax=1.80;Dbase=1.20]
082820 R010:CH0423-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082830 COMPUTE DIALHD 1.0 01:MH108 12.29 1.385 NoDate 28:01 41.19 .637 000
082840 [LGRS=2 Cn=75.0]
082850 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
082860 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 183.;MNI= 013;SCI= 0]
082870 [IARECmp= 4.00; IARECPR= 4.00]
082880 [SM N= 33.81; SMM=225.43; Sfk= 010]
082890 R010:CH0424-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082900 COMPUTE DIALHD 1.0 01:MH108 12.29 1.385 NoDate 28:01 41.19 n/a 000
082910 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
082920 Minor System \ 1.0 02:AO-M 12.29 1.029 NoDate 27:54 41.27 n/a 000
082930 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
082940 CONTINX STANDHD 1.0 01:MH107 12.29 1.029 NoDate 27:54 41.27 n/a 000
082950 [LGRS=2 Cn=75.0]
082960 R010:CH0425-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
082970 COMPUTE DIALHD 1.0 01:MH108 12.29 1.029 NoDate 27:54 41.27 n/a 000
082980 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
082990 Minor System \ 1.0 02:AO-M 12.29 1.029 NoDate 27:54 41.27 n/a 000
083000 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083010 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083020 [LGRS=2 Cn=75.0]
083030 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
083040 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 183.;MNI= 013;SCI= 0]
083050 [IARECmp= 4.00; IARECPR= 4.00]
083060 [SM N= 33.81; SMM=225.43; Sfk= 010]
083070 R010:CH0426-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083080 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083090 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083100 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083110 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083120 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083130 [LGRS=2 Cn=75.0]
083140 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
083150 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 183.;MNI= 013;SCI= 0]
083160 [IARECmp= 4.00; IARECPR= 4.00]
083170 [SM N= 33.81; SMM=225.43; Sfk= 010]
083180 R010:CH0427-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083190 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083200 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083210 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083220 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083230 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083240 [LGRS=2 Cn=75.0]
083250 R010:CH0428-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083260 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083270 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083280 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083290 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083300 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083310 [LGRS=2 Cn=75.0]
083320 R010:CH0429-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083330 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083340 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083350 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083360 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083370 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083380 [LGRS=2 Cn=75.0]
083390 R010:CH0430-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083400 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083410 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083420 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083430 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083440 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083450 [LGRS=2 Cn=75.0]
083460 R010:CH0431-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083470 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083480 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083490 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083500 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083510 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083520 [LGRS=2 Cn=75.0]
083530 R010:CH0432-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083540 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083550 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083560 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083570 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083580 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083590 [LGRS=2 Cn=75.0]
083600 R010:CH0433-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083610 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083620 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083630 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083640 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083650 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083660 [LGRS=2 Cn=75.0]
083670 R010:CH0434-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083680 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083690 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083700 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083710 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083720 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083730 [LGRS=2 Cn=75.0]
083740 R010:CH0435-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083750 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083760 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083770 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083780 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083790 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083800 [LGRS=2 Cn=75.0]
083810 R010:CH0436-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083820 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083830 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083840 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083850 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083860 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083870 [LGRS=2 Cn=75.0]
083880 R010:CH0437-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083890 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083900 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083910 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083920 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
083930 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083940 [LGRS=2 Cn=75.0]
083950 R010:CH0438-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
083960 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
083970 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
083980 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
083990 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
084000 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
084010 [LGRS=2 Cn=75.0]
084020 R010:CH0439-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084030 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
084040 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
084050 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
084060 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
084070 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
084080 [LGRS=2 Cn=75.0]
084090 R010:CH0440-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084100 COMPUTE DIALHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
084110 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
084120 Minor System \ 1.0 02:AO-M 149.45 6.598 NoDate 28:04 42.14 n/a 000
084130 [M Syst=on 1401E03; TotOfV=0.0000E+00; NOf= 0; TotDurOf= 0 hrs]
084140 CONTINX STANDHD 1.0 01:MH108 149.45 6.598 NoDate 28:04 42.14 n/a 000
084150 [LGRS=2 Cn=75.0]

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084160 ROUTE CHANNEL > 1.0 02:SNM 55194.85 86.233 NoDate 37:23 23.16 n/a 000
084170 [RFR=1.00 out-c 1.0 01:R1_V_E 55194.85 86.067 NoDate 37:44 23.16 n/a 000
      [L/S/n= 1962./233/045]
084180 [Vmax=1.303;Dmax=2.211]
084190 [Dmax=1.80;Dbase=1.20]
084200 *****
084210 # Catchment DESIRE
084220 # To Jack River (north of the Jack)
084230 # Rural-estate subdivision (Heart's Desire Community)
084240 *****
084250 R010:CH0429-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084260 CONTINX STANDHD 1.0 01:DSRBE 23.78 1.694 NoDate 28:03 32.93 5.09 000
084270 [LGRS=2 Cn=74.0]
084280 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
084290 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 400.;MNI= 013;SCI= 0]
084300 [IARECmp= 4.00; IARECPR= 4.00]
084310 [SM N= 31.15; SMM=207.66; Sfk= 010]
084320 *****
084330 # Catchment JMW
084340 # To Jackville SWM Facility
084350 # Residential development & golf course
084360 # JFSA 2021-01-11 update JOCKVA after updating CDBRI as per IB GROUP July 2008.
084370 # JOCKVA area became 225.13 ha instead of 237.63 ha. JOCKVA separated into two areas JOCKVA and EX LAND 32.5 ha as
084380 *****
084390 R010:CH0430-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084400 CONTINX STANDHD 1.0 01:JOCKVA 225.13 17.670 NoDate 28:08 41.86 6.47 000
084410 [LGRS=2 Cn=74.0]
084420 [XfM=50;TfM=50]
084430 *****
084440 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
084450 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 1311.;MNI= 013;SCI= 0]
084460 [IARECmp= 4.00; IARECPR= 4.00]
084470 [SM N= 36.67; SMM=244.49; Sfk= 010]
084480 R010:CH0431-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084490 ADD HYD 1.0 02:EX-LAND 32.50 2.275 NoDate 27:55 41.99 n/a 000
084500 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
084510 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 400.;MNI= 013;SCI= 0]
084520 [IARECmp= 4.00; IARECPR= 4.00]
084530 [SM N= 31.15; SMM=207.66; Sfk= 010]
084540 R010:CH0432-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084550 CONTINX STANDHD 1.0 01:JOCKVA 225.13 17.670 NoDate 28:08 41.86 n/a 000
084560 [LGRS=2 Cn=74.0]
084570 # Catchment S-2
084580 # To Jack River (north and south)
084590 # Undeveloped floodplain and river
084600 *****
084610 R010:CH0433-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084620 CONTINX STANDHD 1.0 01:SR 102.94 2.957 NoDate 28:20 23.57 3.64 000
084630 [IAREC= 4.00; SM N= 39.75; SMM=264.99; Sfk= 010]
084640 [InterEvent Time= 12.00]
084650 R010:CH0434-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084660 ADD HYD 1.0 02:NI 55476.26 86.683 NoDate 37:44 23.25 n/a 000
084670 [Cn= 64.0; N= 3.00; T= 7.13]
084680 [Vmax=1.789;Dmax=1.038]
084690 [Dmax=1.80;Dbase=2.00]
084700 *****
084710 [Pervious area: IPer= 4.67;SLP=1.00;LGP= 40.;MNP= 250;SCP= 0]
084720 [Impervious area: IAp= 1.57;SLP=1.00;LGA= 400.;MNI= 013;SCI= 0]
084730 [IARECmp= 4.00; IARECPR= 4.00]
084740 [SM N= 33.81; SMM=225.43; Sfk= 010]
084750 R010:CH0435-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084760 COMPUTE DIALHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
084770 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
084780 Minor System \ 1.0 02:AO-M 134.57 5.265 NoDate 28:03 42.04 n/a 000
084790 [M Syst=on 114./120/0.03]
084800 [L/S/n= 114./120/0.03]
084810 [Vmax=1.012;Dmax=2.00]
084820 R010:CH0436-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084830 COMPUTE DIALHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
084840 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
084850 Minor System \ 1.0 02:AO-M 134.57 5.265 NoDate 28:03 42.04 n/a 000
084860 [M Syst=on 114./120/0.03]
084870 [L/S/n= 114./120/0.03]
084880 [Vmax=1.011;Dmax=1.639]
084890 [Dmax=1.80;Dbase=1.20]
084900 R010:CH0437-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084910 COMPUTE DIALHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
084920 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
084930 Minor System \ 1.0 02:AO-M 134.57 5.265 NoDate 28:03 42.04 n/a 000
084940 [M Syst=on 114./120/0.03]
084950 [L/S/n= 114./120/0.03]
084960 [Vmax=1.012;Dmax=2.00]
084970 [Dmax=1.80;Dbase=2.00]
084980 R010:CH0438-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
084990 COMPUTE DIALHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
085000 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
085010 Minor System \ 1.0 02:AO-M 134.57 5.265 NoDate 28:03 42.04 n/a 000
085020 [M Syst=on 114./120/0.03]
085030 [L/S/n= 114./120/0.03]
085040 [Vmax=1.012;Dmax=2.00]
085050 [Dmax=1.80;Dbase=2.00]
085060 R010:CH0439-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
085070 COMPUTE DIALHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
085080 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
085090 Minor System \ 1.0 02:AO-M 134.57 5.265 NoDate 28:03 42.04 n/a 000
085100 [M Syst=on 114./120/0.03]
085110 [L/S/n= 114./120/0.03]
085120 [Vmax=1.012;Dmax=2.00]
085130 [Dmax=1.80;Dbase=2.00]
085140 R010:CH0440-----DfIn-ID-NND-----AREHA-QPEAKm-TPeakDte,hb,mm-----RvM R C-----DfWcm
085150 COMPUTE DIALHD 1.0 01:MH107 134.57 5.265 NoDate 28:03 42.04 n/a 000
085160 Major System / 1.0 02:AO-M 0.00 .000 NoDate 0:00 .00 n/a 000
085170 Minor System \ 1.0 02:AO-M 134.57 5.265 NoDate 28:03 42.04 n/a 000
085180 [M Syst=on 114./120/0.03]
085190 [L/S/n
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08603# [Cm 66.0 No 3.00; Tpr= 2.41]
08604# [IAREC 4.00; SMN# 52.62; SMM#350.79; SK#_010]
08605# [InterEventTime= 12.00]
08606#
08607# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08608# # of 1.80
08609# ROUTES: C00011.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08610# CONTI NLUK NASHID 1.0 01:SW_2 1917.00 10.351 No.date 34:27 27.01 363 0000
08611# [Cm 66.0 No 3.00; Tpr= 2.41]
08612# [IAREC 4.00; SMN# 52.62; SMM#350.79; SK#_010]
08613# [InterEventTime= 12.00]
08614#
08615# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08616# # of 1.52
08617# ROUTES: C00102.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08618# CONTI NLUK NASHID 1.0 01:SW_10 5666.00 27.457 No.date 37:54 31.50 423 0000
08619# [Cm 72.0 No 3.00; Tpr= 8.00]
08620# [IAREC 4.00; SMN# 39.75; SMM#264.99; SK#_010]
08621# [InterEventTime= 12.00]
08622#
08623# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08624# # of 1.75
08625# ROUTES: C00103.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08626# CONTI NLUK NASHID 1.0 01:ML_CK 8376.00 26.276 No.date 39:59 27.01 363 0000
08627# [Cm 66.0 No 3.00; Tpr=11.661]
08628# [IAREC 4.00; SMN# 52.62; SMM#350.79; SK#_010]
08629# [InterEventTime= 12.00]
08630#
08631# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08632# # of 1.80
08633# ROUTES: C00104.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08634# CONTI NLUK NASHID 1.0 01:HB_LR 1132.00 11.752 No.date 30:54 30.18 406 0000
08635# [Cm 66.0 No 3.00; Tpr= 2.41]
08636# [IAREC 4.00; SMN# 43.07; SMM#287.10; SK#_010]
08637# [InterEventTime= 12.00]
08638#
08639# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08640# # of 1.82
08641# ROUTES: C00105.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08642# CONTI NLUK NASHID 1.0 01:NC_CK 4464.00 13.075 No.date 39:59 24.61 331 0000
08643# [Cm 62.0 No 3.00; Tpr=1.32]
08644# [IAREC 4.00; SMN# 52.62; SMM#412.66; SK#_010]
08645# [InterEventTime= 12.00]
08646#
08647# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08648# # of 1.80
08649# ROUTES: C00106.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08650# CONTI NLUK NASHID 1.0 01:SW_8 131.00 2.266 No.date 28:57 25.20 339 0000
08651# [Cm 62.0 No 3.00; Tpr= 90.8]
08652# [IAREC 4.00; SMN# 59.42; SMM#396.11; SK#_010]
08653# [InterEventTime= 12.00]
08654#
08655# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08656# # of 1.80
08657# ROUTES: C00107.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08658# CONTI NLUK NASHID 1.0 01:HB_LR 3854.00 15.333 No.date 38:34 27.01 363 0000
08659# [Cm 66.0 No 3.00; Tpr= 2.41]
08660# [IAREC 4.00; SMN# 52.62; SMM#350.79; SK#_010]
08661# [InterEventTime= 12.00]
08662#
08663# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08664# # of 1.82
08665# ROUTES: C00108.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08666# CONTI NLUK NASHID 1.0 01:SW_7 3197.00 11.663 No.date 36:24 21.75 292 0000
08667# [Cm 57.0 No 3.00; Tpr= 6.51]
08668# [IAREC 4.00; SMN# 50.81; SMM#508.18; SK#_010]
08669# [InterEventTime= 12.00]
08670#
08671# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08672# # of 1.75
08673# ROUTES: C00109.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08674# CONTI NLUK NASHID 1.0 01:SW_6 165.00 1.076 No.date 33:03 27.63 371 0000
08675# [Cm 67.0 No 3.00; Tpr= 1.50]
08676# [IAREC 4.00; SMN# 50.55; SMM#336.97; SK#_010]
08677# [InterEventTime= 12.00]
08678#
08679# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08680# # of 1.80
08681# ROUTES: C00110.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08682# CONTI NLUK NASHID 1.0 01:HB_LR 1532.00 7.882 No.date 35:14 31.50 423 0000
08683# [Cm 72.0 No 3.00; Tpr= 5.95]
08684# [IAREC 4.00; SMN# 39.75; SMM#264.99; SK#_010]
08685# [InterEventTime= 12.00]
08686#
08687# ROUTES: C00111.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08688# CONTI NLUK NASHID 1.0 01:SW_1 224.00 6.882 No.date 28:45 35.66 479 0000
08689# [Cm 77.0 No 3.00; Tpr= 7.51]
08690# [IAREC 4.00; SMN# 31.15; SMM#207.66; SK#_010]
08691# [InterEventTime= 12.00]
08692#
08693# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08694# # of 1.20
08695# ROUTES: C00112.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08696# CONTI NLUK NASHID 1.0 01:FL_CK 4945.00 37.664 No.date 33:18 32.85 442 0000
08697# [Cm 74.0 No 3.00; Tpr= 4.45]
08698# [IAREC 4.00; SMN# 34.94; SMM#244.49; SK#_010]
08699# [InterEventTime= 12.00]
08700#
08701# ROUTES: C00113.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08702# CONTI NLUK NASHID 1.0 01:SW_2 20.00 0.798 No.date 28:35 39.36 529 0000
08703# [Cm 81.0 No 3.00; Tpr= 6.21]
08704# [IAREC 4.00; SMN# 25.21; SMM#168.09; SK#_010]
08705# [InterEventTime= 12.00]
08706#
08707# The Tp was modified according to a Peak Reduction factor (MO-Chart B2-4)
08708# # of 1.81
08709# ROUTES: C00114.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08710# CONTI NLUK NASHID 1.0 01:SW_SAI 1412.00 7.480 No.date 37:50 34.24 1460 0000
08711# [Cm 72.0 No 3.00; Tpr= 4.00]
08712# [IAREC 4.00; SMN# 33.81; SMM#225.43; SK#_010]
08713# [InterEventTime= 12.00]
08714#
08715# ROUTES: C00115.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08716# CONTI NLUK NASHID 1.0 01:ML_CK 585.00 10.942 No.date 29:56 39.36 529 0000
08717# [Cm 81.0 No 3.00; Tpr= 1.75]
08718# [IAREC 4.00; SMN# 25.21; SMM#168.09; SK#_010]
08719# [InterEventTime= 12.00]
08720#
08721# ROUTES: C00116.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08722# CONTI NLUK NASHID 1.0 01:SW_3 1021.00 14.476 No.date 30:46 38.60 519 0000
08723# [Cm 80.0 No 3.00; Tpr= 2.46]
08724# [IAREC 4.00; SMN# 26.32; SMM#175.50; SK#_010]
08725# [InterEventTime= 12.00]
08726#
08727# ROUTES: C00117.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08728# CONTI NLUK NASHID 1.0 01:ML_CK 1122.00 13.229 No.date 31:45 39.36 529 0000
08729# [Cm 81.0 No 3.00; Tpr= 1.25]
08730# [IAREC 4.00; SMN# 25.21; SMM#168.09; SK#_010]
08731# [InterEventTime= 12.00]
08732#
08733# ROUTES: C00118.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08734# CONTI NLUK NASHID 1.0 01:ML_LR 2737.00 39.508 No.date 31:30 34.94 470 0000
08735# [Cm 76.0 No 3.00; Tpr= 3.01]
08736# [IAREC 4.00; SMN# 25.46; SMM#216.39; SK#_010]
08737# [InterEventTime= 12.00]
08738#
08739# Routing hydrographs
08740# Starting with the addition of Jack River Headwater and Subwatershed 13
08741#
08742# ROUTES: C00130.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08743# ADD HYD + 1.0 02:JR_HV 3680.00 15.500 No.date 36:56 25.80 n/a 0000
08744# + 1.0 02:SW_13 971.00 5.778 No.date 32:34 24.02 n/a 0000
08745# SIM# 1.0 01:S_N3 4651.00 19.777 No.date 35:26 25.80 n/a 0000
08746#
08747# Sum of hydrographs from Node 13 routed to Node 13A
08748# (Approximated cross-section - see cross-section 258)
08749# Use m=0.04 for summer conditions and m=0.025 for spring conditions
08750#
08751# ROUTES: C00131.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08752# ROUTE CHANNEL -> 1.0 02:S_N3 4651.00 19.777 No.date 35:26 25.80 n/a 0000
08753# [RFD= 1.00] out c. 1.0 01:S_N3A 4651.00 15.935 No.date 39:17 25.42 n/a 0000
08754# [L/S= 9074. / 0231 040] [Vmax= 548.Dmax= 3.659]
08755#
08756#
08757# Addition of Subwatershed Jack River at Goodwood Marsh to Node 13A
08758#
08759# ROUTES: C00132.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08760# ADD HYD + 1.0 02:NA 4651.00 15.935 No.date 39:17 25.42 n/a 0000
08761# + 1.0 02:JR_OHM 7725.00 3.678 No.date 60:27 23.52 n/a 0000
08762# SIM# 1.0 01:SN13A 7725.00 23.402 No.date 39:59 23.52 n/a 0000
08763#
08764# Insertion of a reservoir to simulate the effects of the Goodwood Marsh
08765#
08766# ROUTES: C00133.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08767# ROUTE RESERVOIR -> 1.0 02:SN13A 7725.00 23.402 No.date 39:59 23.52 n/a 0000
08768# [MS oledn= 11920d] out c. 1.0 01:RES_GM 7725.00 3.678 No.date 60:27 23.52 n/a 0000
08769# [MS oledn= 11920d]
08770#
08771# ROUTES: C00134.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08772# SAVE HYD 1.0 01:RES_GM 7725.00 3.678 No.date 60:27 23.52 n/a 0000
08773# [Fname= H.RES.GM]
08774# remark: Outflow from Res GM
08775# Output Reservoir Goodwood Marsh routed from Node 13A to Node 12
08776# (Approximated cross-section - see cross-section 258)
08777# Use m=0.04 for summer conditions and m=0.025 for spring conditions
08778# ROUTES: C00135.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08779# ROUTE CHANNEL -> 1.0 02:RES_GM 7725.00 3.678 No.date 60:27 23.52 n/a 0000
08780# [RFD= 1.00] out c. 1.0 01:S_N2 7725.00 3.675 No.date 63:05 23.52 n/a 0000
08781# [L/S= 5926. / 0761 040]
08782# [Vmax= 552.Dmax= 1.524]
08783#
08784# Addition of Subwatershed Jack River at Athton to Node 12
08785#
08786# ROUTES: C00136.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08787# ADD HYD + 1.0 02:JR_ASH 1781.00 14.166 No.date 32:40 31.50 n/a 0000
08788# + 1.0 01:S_N2 9506.00 16.182 No.date 32:43 25.02 n/a 0000
08789# SIM# 1.0 01:S_N2 9506.00 16.182 No.date 32:43 25.02 n/a 0000
08790#
08791# ROUTES: C00137.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08792# SAVE HYD 1.0 02:S_N2 9506.00 16.182 No.date 32:43 25.02 n/a 0000
08793# [Fname= H.SN2]
08794# remark: Flow at S_N2 near Athton
08795#
08796# # of 1.80
08797# Sum of hydrographs from Node 12 routed to Node 11
08798# (Approximated cross-section - see cross-section 258)
08799# Use m=0.04 for summer conditions and m=0.025 for spring conditions
08800#
08801# ROUTES: C00138.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08802# ROUTE CHANNEL -> 1.0 02:Don1 2956.00 69.032 No.date 39:59 27.26 n/a 0000
08803# [RFD= 1.00] out c. 1.0 01:Don1 2956.00 69.032 No.date 39:59 27.26 n/a 0000
08804# [L/S= 972. / 034 040]
08805# [Vmax= 721.Dmax= 2.847]
08806#
08807# Addition of Subwatershed 11 and No Name Creek to Node 11
08808#
08809# ROUTES: C00139.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08810# ADD HYD + 1.0 02:SW_1 500.00 7.521 No.date 29:22 27.01 n/a 0000
08811# + 1.0 02:SW_1 1917.00 10.351 No.date 34:27 27.01 n/a 0000
08812# SIM# 1.0 01:S_N1 11923.00 27.908 No.date 33:04 25.42 n/a 0000
08813#
08814#
08815# Sum of hydrographs from Node 11 routed to Node 10
08816# Section 1
08817#
08818# ROUTES: C00140.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08819# ROUTE CHANNEL -> 1.0 02:S_N1 11923.00 27.908 No.date 33:04 25.42 n/a 0000
08820# [RFD= 1.00] out c. 1.0 01:N10 11923.00 10.039 No.date 40:01 25.42 n/a 0000
08821# [L/S= 1028. / 137 040]
08822# [Vmax= 464.Dmax= 1.329]
08823#
08824# Addition of Subwatershed 10 to Node 10
08825#
08826# ROUTES: C00141.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08827# ADD HYD 11923.00 10.039 No.date 40:01 25.42 n/a 0000
08828# + 1.0 02:SW_10 5666.00 27.457 No.date 37:54 31.50 n/a 0000
08829# SIM# 17858.00 45.026 No.date 38:35 27.38 n/a 0000
08830#
08831# ROUTES: C00142.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08832# SAVE HYD 1.0 01:S_N10 17589.00 45.026 No.date 38:35 27.38 n/a 0000
08833# [Fname= H.SN10]
08834# remark: Flow at S_N10: N10 + SW_10
08835# Addition of Leaky Creek to S_N10
08836#
08837# ROUTES: C00143.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08838# ADD HYD + 1.0 02:S_N10 17589.00 45.026 No.date 38:35 27.26 n/a 0000
08839# + 1.0 01:S_N10A 25965.00 47.812 No.date 39:59 27.69 n/a 0000
08840# SIM# 1.0 01:S_N10 43554.00 70.812 No.date 39:59 27.69 n/a 0000
08841#
08842# Sum of hydrographs from Node 10 routed to Node 9
08843# Section 2
08844#
08845# ROUTES: C00144.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08846# ROUTE CHANNEL -> 1.0 02:S_N10A 25965.00 70.812 No.date 39:59 27.69 n/a 0000
08847# [RFD= 1.00] out c. 1.0 01:N9 25965.00 69.032 No.date 39:59 27.26 n/a 0000
08848# [L/S= 3982. / 0751 040]
08849# [Vmax= 718.Dmax= 1.889]
08850#
08851# Addition of Subwatershed 9 and Nichols Creek to Node 9
08852#
08853# ROUTES: C00145.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08854# ADD HYD 25965.00 69.032 No.date 39:59 27.26 n/a 0000
08855# + 1.0 02:SW_9 1132.00 11.752 No.date 30:54 30.18 n/a 0000
08856# + 1.0 01:REZ_CK 4464.00 13.075 No.date 39:59 24.61 n/a 0000
08857# SIM# 1.0 01:S_N9 31561.00 84.884 No.date 39:59 26.99 n/a 0000
08858#
08859# Sum of hydrographs from Node 9 routed to Node 8
08860# Section 3
08861#
08862# ROUTES: C00146.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08863# ROUTE CHANNEL -> 1.0 02:S_N9 31561.00 84.884 No.date 39:59 26.99 n/a 0000
08864# [RFD= 1.00] out c. 1.0 01:N8 31561.00 79.245 No.date 39:59 26.99 n/a 0000
08865# [L/S= 2269. / 0881 045]
08866# [Vmax= 363.Dmax= 743]
08867#
08868# Addition of Subwatershed 8 and Hobb's Drain to Node 8
08869#
08870# ROUTES: C00147.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08871# ADD HYD 31561.00 79.245 No.date 39:59 26.99 n/a 0000
08872# + 1.0 02:SW_8 1376.00 13.676 No.date 28:57 25.20 n/a 0000
08873# + 1.0 01:REZ_CK 4464.00 13.075 No.date 39:59 24.61 n/a 0000
08874# SIM# 1.0 01:S_N8 33546.00 91.927 No.date 39:59 26.99 n/a 0000
08875#
08876# Sum of hydrographs from Node 8 routed to Node 7
08877# Section 4
08878#
08879# ROUTES: C00148.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08880# ROUTE CHANNEL -> 1.0 02:SW_8 1376.00 13.676 No.date 28:57 25.20 n/a 0000
08881# [RFD= 1.00] out c. 1.0 01:N7 33546.00 80.337 No.date 45:08 26.98 n/a 0000
08882# [L/S= 3760. / 0531 070]
08883# [Vmax= 226.Dmax= 2.161]
08884#
08885# Addition of Subwatershed 7 to Node 7
08886#
08887# ROUTES: C00149.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08888# ADD HYD 33546.00 80.337 No.date 45:08 26.98 n/a 0000
08889# + 1.0 02:SW_7 35546.00 80.337 No.date 45:08 26.98 n/a 0000
08890# + 1.0 02:SW_7 3197.00 11.663 No.date 36:24 21.75 n/a 0000
08891# SIM# 1.0 01:S_N7 38743.00 86.331 No.date 44:08 26.55 n/a 0000
08892#
08893# ROUTES: C00150.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08894# SAVE HYD 1.0 01:S_N7 38743.00 86.331 No.date 44:08 26.55 n/a 0000
08895# [Fname= H.SN7]
08896# remark: Flow at S_N7: N7 + SW_7
08897# Insertion of a reservoir to simulate the effects of the Richmond Fen.
08898# Storage area and volumes were estimated from available topo maps.
08899# Release rate from Fen was assumed to be controlled by the downstream
08900# river cross-section for summer conditions. It was assumed that for up to
08901# 0.75 of winter water, the rate of change of the min of storage. Above
08902# this depth, the wetland starts to significantly store water.
08903#
08904# ROUTES: C00151.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08905# ROUTE RESERVOIR -> 1.0 02:SN 38743.00 86.331 No.date 44:08 26.55 n/a 0000
08906# [MS oledn= 367E+03] out c. 1.0 01:RES_RF 38743.00 42.032 No.date 60:05 26.55 n/a 0000
08907#
08908# ROUTES: C00152.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08909# SAVE HYD 1.0 01:RES_RF 38743.00 42.032 No.date 60:05 26.55 n/a 0000
08910# [Fname= H.RES_RF]
08911# remark: outflow of Richmond Fen
08912#
08913# Sum of hydrographs from Node 7 routed to Node 6
08914# Section 5
08915#
08916# ROUTES: C00153.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08917# ROUTE CHANNEL -> 1.0 02:RES_RF 38743.00 42.032 No.date 60:05 26.55 n/a 0000
08918# [RFD= 1.00] out c. 1.0 01:RES_RF 38743.00 41.826 No.date 61:20 26.72 n/a 0000
08919# [L/S= 3056. / 0321 025]
08920# [Vmax= 514.Dmax= 1.120]
08921#
08922# Addition of Subwatershed 6 and Van Gul Drain to Node 6
08923#
08924# ROUTES: C00154.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08925# ADD HYD + 1.0 02:SW_6 165.00 1.076 No.date 33:03 27.63 n/a 0000
08926# + 1.0 02:ML_CK 1332.00 7.882 No.date 35:14 31.50 423 0000
08927# SIM# 1.0 01:S_N6 16240.01 41.832 No.date 61:20 26.72 n/a 0000
08928#
08929# Sum of hydrographs from Node 6 routed to Node 5
08930# Section 6
08931#
08932# ROUTES: C00155.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08933# ROUTE CHANNEL -> 1.0 02:S_N6 40240.01 41.832 No.date 61:20 26.72 n/a 0000
08934# [RFD= 1.00] out c. 1.0 01:N5 40240.01 41.717 No.date 62:14 26.72 n/a 0000
08935# [L/S= 1852. / 0541 035]
08936# [Vmax= 444.Dmax= 1.222]
08937#
08938# Addition of Subwatershed 5 and Flowing Creek to Node 5
08939#
08940# ROUTES: C00156.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08941# ADD HYD 40240.01 41.717 No.date 62:14 26.72 n/a 0000
08942# + 1.0 02:SW_5 224.00 6.882 No.date 28:45 35.66 n/a 0000
08943# + 1.0 02:FL_CK 4945.00 37.664 No.date 33:18 32.85 n/a 0000
08944# SIM# 1.0 01:S_N5 45409.01 62.834 No.date 34:27 27.43 n/a 0000
08945#
08946# Sum of hydrographs from Node 5 routed to Node 5A
08947# Section 7
08948#
08949# ROUTES: C00157.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08950# ROUTE CHANNEL -> 1.0 02:SW_5A 45409.01 62.834 No.date 34:27 27.43 n/a 0000
08951# [RFD= 1.00] out c. 1.0 01:SA 45409.01 62.847 No.date 34:43 27.43 n/a 0000
08952# [L/S= 536. / 0001 040]
08953# [Vmax= 511.Dmax= 1.222]
08954#
08955# Addition of Subwatershed 5A and Subwatershed 5A2 to Node 5A
08956#
08957# ROUTES: C00158.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08958# ADD HYD + 1.0 02:SA 45409.01 62.847 No.date 34:43 27.43 n/a 0000
08959# + 1.0 02:SW_5A 224.00 6.882 No.date 28:45 35.66 n/a 0000
08960# + 1.0 02:FL_CK 4945.00 37.664 No.date 33:18 32.85 n/a 0000
08961# SIM# 1.0 01:S_SA 46841.01 69.334 No.date 35:01 27.64 n/a 0000
08962#
08963# Sum of hydrographs from Node 5A routed to Node 4
08964# Section 8
08965#
08966# ROUTES: C00159.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08967# ROUTE CHANNEL -> 1.0 02:S_SA 46841.01 69.334 No.date 35:01 27.64 n/a 0000
08968# [RFD= 1.00] out c. 1.0 01:NA 46841.01 66.496 No.date 36:27 27.64 n/a 0000
08969# [L/S= 437. / 0431 035]
08970# [Vmax= 840.Dmax= 3.530]
08971#
08972# Addition of Subwatershed 4 and Leamy Creek to Node 4
08973#
08974# ROUTES: C00160.....DfIn n-1D NND.....AREhA-QPEAGm-TPeakDate-hh-mm-RvM R.C-DFWcm
08975# ADD HYD + 1.0 02:NA 46841.01 66.496 No.date 36:27 27.64 n/a 0000
08976# + 1.0 02:SW_4 582.00 10.294 No.date 32:40 31.50 n/a 0000
08977# + 1.0 02:LMLCK 1021.00


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097255 ROUTE RESERVOIR -> 1.0 02:51-DIS 5.27 672 NoDate 27:55 42:30 n/a .000
097256 overflow curve 1.0 02:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
097257 [MSYSto=1440E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
097298 ROUTE25:CO0219-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097300 CONTINUOUS STANDBY 1.0 01:51-A 75.88 2,541 NoDate 28:36 34:14 .459 .000
097318 [InterEventTime 12.00]
097319 [InterEventTime 3.00]
097320 [InterEventTime 12.00]
097321 [InterEventTime 3.00]
097322 [InterEventTime 12.00]
097323 [InterEventTime 3.00]
097324 ROUTE25:CO0186-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097325 CONTINUOUS STANDBY 1.0 01:SN:CLAR 35.65 .789 NoDate 29:10 34:14 .459 .000
097366 [InterEventTime 12.00]
097367 [InterEventTime 3.00]
097368 [InterEventTime 12.00]
097369 [InterEventTime 3.00]
097370 ROUTE25:CO0187-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097371 COMPUTE DUAL/D 1.0 02:51-DI 537.77 109.02 NoDate 28:32 28:72 n/a .000
097411 ADD HYD + 1.0 02:52-OUT 335.49 7,032 NoDate 29:23 52:43 n/a .000
097422 + 1.0 02:51-P10 73.29 1,168 NoDate 29:06 56:82 n/a .000
097436 + 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
097448 + 1.0 02:W:CLAR 35.65 789 NoDate 29:10 34:14 n/a .000
097465 [MSYSto=65-TiM=65] 0.00 NoDate 0:00 0.00 n/a .000
097466 + 1.0 02:51-DFP-ED 14.96 153 NoDate 29:30 56:85 n/a .000
097474 + 1.0 02:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
097484 + 1.0 02:51-DIR 5.27 672 NoDate 28:47 42:30 n/a .000
097499 + 1.0 02:51-A 75.88 2,541 NoDate 28:36 34:14 n/a .000
097500 [InterEventTime 12.00]
097501 [InterEventTime 3.00]
097502 ROUTE25:CO0188-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097528 SAVE HYD frame :SN:FO 0025 54118.96 114.198 NoDate 33:27 28.94 n/a .000
097533 [InterEventTime 12.00]
097534 [InterEventTime 3.00]
097535 # remark: Total Flow at Foster Drain
097536 # Hydrograph from Node Foster routed to Node at Cedarvow Road
097537 # Channel X-Section obtained from RWCA Hydraulic Model - Station 6016
097578 ROUTE25:CO0189-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097579 ROUTE CHANNEL 1.0 01:SN:CLAR 35.65 789 NoDate 29:10 34:14 .459 .000
097600 + [RDI=1.00] out c 1.0 01:NCE 54118.96 114.014 NoDate 33:32 28.94 n/a .000
097618 [L/S=area 159.082/.035]
097628 [L/S=area 425.218/.035]
097638 Catchment S1 1.0 02:51-DIR 21.67 225 NoDate 29:22 57.58 n/a .000
097653 # To Jock River (north and south of Jock)
097654 # Primary agricultural portion of sand quarry
097674 # Catchment S1 1.0 02:51-DIR 21.67 225 NoDate 29:22 57.58 n/a .000
097688 ROUTE25:CO0190-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097689 [InterEventTime 12.00]
097700 [InterEventTime 3.00]
097701 [InterEventTime 12.00]
097702 [InterEventTime 3.00]
097703 # JFSA 2021-02-24 change the name from S1-RDC to S1-A and S1-B. Change their TP values based on the new areas
097724 # JFSA 2021-02-24 change the name from S1-RDC to S1-B. Change their TP values based on the new areas
097725 # JFSA 2021-01-19, after adding Greenbank pond, S1-RDC3 is not existing anymore
097766 ROUTE25:CO0191-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097775 CONTINUOUS STANDBY 1.0 01:51-DI 21.67 3,385 NoDate 28:01 57.38 771 .000
097788 [L/S=2 C=CN 75.0]
097800 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
097818 [InterEventTime 12.00]
097819 [InterEventTime 3.00]
097820 [InterEventTime 12.00]
097821 [InterEventTime 3.00]
097822 ROUTE25:CO0192-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097823 COMPUTE DUAL/D 1.0 02:51-DI 21.33 424 NoDate 27:55 57.38 n/a .000
097860 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
097878 Mjor System / 1.0 03:51-DIN 21.67 2,409 NoDate 27:55 57.38 n/a .000
097889 [MSYSto=594E+00, TotOfVol=0.0000E+00, N.Of=1, TotDrOf=0 hrs.]
097890 ROUTE25:CO0193-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097900 ADD HYD + 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
097911 + 1.0 02:51-DIN 21.67 2,409 NoDate 27:55 57.38 n/a .000
097928 [MSYSto=1.21E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
097939 ROUTE25:CO0194-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097948 ROUTE RESERVOIR -> 1.0 02:51-DIS 21.67 2,409 NoDate 27:55 57.38 n/a .000
097966 overflow curve 1.0 03:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
097977 [MSYSto=749E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
097988 ROUTE25:CO0195-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
097999 CONTINUOUS STANDBY 1.0 01:51-DI 3.28 581 NoDate 28:00 57.38 771 .000
098000 [L/S=2 C=CN 75.0]
098012 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
098013 [InterEventTime 12.00]
098014 [InterEventTime 3.00]
098015 [InterEventTime 12.00]
098016 [InterEventTime 3.00]
098017 ROUTE25:CO0196-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098018 COMPUTE DUAL/D 1.0 02:51-DI 21.33 424 NoDate 27:55 57.38 n/a .000
098088 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
098099 Mjor System / 1.0 03:51-DIN 21.67 2,409 NoDate 27:55 57.38 n/a .000
098110 [MSYSto=6427E+02, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098120 ROUTE25:CO0197-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098129 ADD HYD + 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
098133 + 1.0 02:51-DIN 3.28 421 NoDate 27:54 57.72 n/a .000
098148 [MSYSto=1.21E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098159 ROUTE25:CO0198-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098168 ROUTE RESERVOIR -> 1.0 02:51-DIS 3.28 421 NoDate 27:54 57.72 n/a .000
098178 overflow curve 1.0 03:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
098189 [MSYSto=1440E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098190 ROUTE25:CO0199-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098200 CONTINUOUS STANDBY 1.0 01:51-DI 12.84 2,083 NoDate 28:01 57.38 771 .000
098201 [L/S=2 C=CN 75.0]
098225 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
098226 [InterEventTime 12.00]
098227 [InterEventTime 3.00]
098228 [InterEventTime 12.00]
098229 [InterEventTime 3.00]
098230 ROUTE25:CO0200-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098239 COMPUTE DUAL/D 1.0 02:51-DI 12.84 2,083 NoDate 28:01 57.38 n/a .000
098300 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
098318 Mjor System / 1.0 03:51-DIN 12.84 1,500 NoDate 27:55 57.52 n/a .000
098328 [MSYSto=2624E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098339 ROUTE25:CO0201-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098348 ADD HYD + 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
098355 + 1.0 02:51-DI 12.84 1,500 NoDate 27:55 57.52 n/a .000
098370 ROUTE25:CO0202-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098388 ROUTE RESERVOIR -> 1.0 02:51-DIS 12.84 1,500 NoDate 27:55 57.52 n/a .000
098398 overflow curve 1.0 03:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
098410 [MSYSto=4437E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098421 ROUTE25:CO0203-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098429 CONTINUOUS STANDBY 1.0 01:51-DI 2.28 324 NoDate 28:00 57.38 771 .000
098448 [L/S=2 C=CN 75.0]
098466 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
098477 [InterEventTime 12.00]
098478 [InterEventTime 3.00]
098479 [InterEventTime 12.00]
098480 [InterEventTime 3.00]
098481 ROUTE25:CO0204-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098482 COMPUTE DUAL/D 1.0 02:51-DI 2.03 265 NoDate 27:53 57.80 n/a .000
098499 Mjor System / 1.0 03:51-DIN 2.03 265 NoDate 27:53 57.80 n/a .000
098511 [MSYSto=1.21E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098522 ROUTE25:CO0205-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098531 ADD HYD + 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
098538 + 1.0 02:51-DI 2.03 265 NoDate 27:53 57.80 n/a .000
098550 ROUTE25:CO0206-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098559 ROUTE RESERVOIR -> 1.0 02:51-DIS 1.75 232 NoDate 27:53 57.55 n/a .000
098568 out c 1.0 02:51-DIR 1.75 232 NoDate 27:53 57.55 n/a .000
098587 overflow curve 1.0 03:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
098598 ROUTE25:CO0207-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098600 ROUTE RESERVOIR -> 1.0 02:51-DIS 1.75 232 NoDate 27:53 57.55 n/a .000
098618 out c 1.0 02:51-DIR 1.75 232 NoDate 27:53 57.55 n/a .000
098629 overflow curve 1.0 03:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
098640 [MSYSto=6057E+01, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098649 ROUTE25:CO0208-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098655 CONTINUOUS STANDBY 1.0 01:51-DI 2.03 369 NoDate 28:00 57.38 771 .000
098666 [L/S=2 C=CN 75.0]
098677 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
098689 [InterEventTime 12.00]
098690 [InterEventTime 3.00]
098691 [InterEventTime 12.00]
098692 [InterEventTime 3.00]
098693 ROUTE25:CO0209-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098694 COMPUTE DUAL/D 1.0 01:51-DI 2.03 369 NoDate 28:00 57.38 n/a .000
098730 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
098749 Mjor System / 1.0 03:51-DIN 2.03 265 NoDate 27:53 57.80 n/a .000
098760 [MSYSto=4431E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098771 ROUTE25:CO0210-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098780 ADD HYD + 1.0 02:51-DIN 2.03 265 NoDate 27:53 57.80 n/a .000
098787 + 1.0 02:51-DIN 2.03 265 NoDate 27:53 57.80 n/a .000
098799 [MSYSto=3427E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098800 ROUTE25:CO0211-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098801 ROUTE RESERVOIR -> 1.0 02:51-DIS 2.03 265 NoDate 27:53 57.80 n/a .000
098828 overflow curve 1.0 03:51-DIRBovf 0.00 0.00 NoDate 0:00 0.00 n/a .000
098839 [MSYSto=749E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
098840 ROUTE25:CO0212-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098848 CONTINUOUS STANDBY 1.0 01:51-DI 119.44 15,305 NoDate 28:04 56.82 764 .000
098855 [L/S=2 C=CN 75.0]
098856 # Catchment W:CLAR
098857 # To West Clarke Drain (south of the Jock)
098858 # Subdiver with Barhaven South MS
098889 # - 2020-11-30 update CLAR25 Tributary Drainage Area to = 121 ha based on P598(04)-11
098900 # - 2020-10-30 update CLAR25 Tributary Drainage Area to = 121 ha based on P598(04)-11
098911 #*****
098921 ROUTE25:CO0213-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098922 CONTINUOUS STANDBY 1.0 01:51-CLAR 119.44 15,305 NoDate 28:04 56.82 764 .000
098948 [L/S=2 C=CN 75.0]
098949 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
098950 [InterEventTime 12.00]
098951 [InterEventTime 3.00]
098952 [InterEventTime 12.00]
098953 [InterEventTime 3.00]
098954 ROUTE25:CO0214-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
098955 COMPUTE DUAL/D 1.0 01:51-DI 2.03 265 NoDate 27:53 57.80 n/a .000
098966 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
098984 Mjor System / 1.0 03:51-DIN 2.03 265 NoDate 27:53 57.80 n/a .000
098995 [MSYSto=1173E-03, TotOfVol=2205E-02, N.Of=0, TotDrOf=0 hrs.]
099006 ROUTE25:CO0215-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099007 CONTINUOUS STANDBY 1.0 01:51-CLAR 119.44 15,305 NoDate 28:04 56.82 764 .000
099010 [L/S=2 C=CN 75.0]
099011 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099012 [InterEventTime 12.00]
099013 [InterEventTime 3.00]
099014 [InterEventTime 12.00]
099015 [InterEventTime 3.00]
099016 ROUTE25:CO0216-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099017 COMPUTE DUAL/D 1.0 01:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099018 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099019 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099020 [MSYSto=7689E+03, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099021 ROUTE25:CO0217-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099022 ADD HYD + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099023 + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099024 [MSYSto=4458E+04, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099025 ROUTE25:CO0218-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099026 CONTINUOUS STANDBY 1.0 01:51-CLAR 6.77 1,124 NoDate 28:04 65.79 n/a .000
099027 [L/S=2 C=CN 75.0]
099028 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099029 [InterEventTime 12.00]
099030 [InterEventTime 3.00]
099031 [InterEventTime 12.00]
099032 [InterEventTime 3.00]
099033 ROUTE25:CO0219-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099034 COMPUTE DUAL/D 1.0 01:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099035 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099036 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099037 [MSYSto=1.21E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099038 ROUTE25:CO0220-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099039 ADD HYD + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099040 + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099041 [MSYSto=3173E+03, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099042 ROUTE25:CO0221-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099043 CONTINUOUS STANDBY 1.0 01:51-CLAR 6.77 1,124 NoDate 28:04 65.79 n/a .000
099044 [L/S=2 C=CN 75.0]
099045 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099046 [InterEventTime 12.00]
099047 [InterEventTime 3.00]
099048 [InterEventTime 12.00]
099049 [InterEventTime 3.00]
099050 ROUTE25:CO0222-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099051 COMPUTE DUAL/D 1.0 01:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099052 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099053 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099054 [MSYSto=1.21E+00, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099055 ROUTE25:CO0223-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099056 ADD HYD + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099057 + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099058 [MSYSto=4458E+04, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099059 ROUTE25:CO0224-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099060 CONTINUOUS STANDBY 1.0 01:51-CLAR 6.77 1,124 NoDate 28:04 65.79 n/a .000
099061 [L/S=2 C=CN 75.0]
099062 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099063 [InterEventTime 12.00]
099064 [InterEventTime 3.00]
099065 [InterEventTime 12.00]
099066 [InterEventTime 3.00]
099067 ROUTE25:CO0225-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099068 COMPUTE DUAL/D 1.0 01:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099069 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099070 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099071 [MSYSto=7689E+03, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099072 ROUTE25:CO0226-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099073 CONTINUOUS STANDBY 1.0 01:51-CLAR 6.77 1,124 NoDate 28:04 65.79 n/a .000
099074 [L/S=2 C=CN 75.0]
099075 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099076 [InterEventTime 12.00]
099077 [InterEventTime 3.00]
099078 [InterEventTime 12.00]
099079 [InterEventTime 3.00]
099080 ROUTE25:CO0227-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099081 COMPUTE DUAL/D 1.0 01:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099082 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099083 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099084 [MSYSto=7689E+03, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099085 ROUTE25:CO0228-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099086 ADD HYD + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099087 + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099088 [MSYSto=4458E+04, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099089 ROUTE25:CO0229-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099090 CONTINUOUS STANDBY 1.0 01:51-CLAR 6.77 1,124 NoDate 28:04 65.79 n/a .000
099091 [L/S=2 C=CN 75.0]
099092 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099093 [InterEventTime 12.00]
099094 [InterEventTime 3.00]
099095 [InterEventTime 12.00]
099096 [InterEventTime 3.00]
099097 ROUTE25:CO0230-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099098 COMPUTE DUAL/D 1.0 01:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099099 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099100 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099101 [MSYSto=7689E+03, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099102 ROUTE25:CO0231-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099103 ADD HYD + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099104 + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099105 [MSYSto=4458E+04, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099106 ROUTE25:CO0232-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099107 CONTINUOUS STANDBY 1.0 01:51-CLAR 6.77 1,124 NoDate 28:04 65.79 n/a .000
099108 [L/S=2 C=CN 75.0]
099109 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099110 [InterEventTime 12.00]
099111 [InterEventTime 3.00]
099112 [InterEventTime 12.00]
099113 [InterEventTime 3.00]
099114 ROUTE25:CO0233-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099115 COMPUTE DUAL/D 1.0 01:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099116 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099117 Mjor System / 1.0 02:51-DI 0.00 0.00 NoDate 0:00 0.00 n/a .000
099118 [MSYSto=7689E+03, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099119 ROUTE25:CO0234-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099120 ADD HYD + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099121 + 1.0 02:51-DI 6.77 1,124 NoDate 28:04 65.79 n/a .000
099122 [MSYSto=4458E+04, TotOfVol=0.0000E+00, N.Of=0, TotDrOf=0 hrs.]
099123 ROUTE25:CO0235-----DfIn-ID NND-----AREHA-QPEAKm-TPeakDte,hh:mm-----RvMn R,C-----DWfmC
099124 CONTINUOUS STANDBY 1.0 01:51-CLAR 6.77 1,124 NoDate 28:04 65.79 n/a .000
099125 [L/S=2 C=CN 75.0]
099126 [Previous area: I=per: 4.67-SLPP=0.0-LGP=40.0-MNP=250-SCP=0]
099127 [InterEventTime 
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004773 ROUTE CHANNEL  ->  1.00:02:4241      54658.48 103.518 No.date 38:57 29.12 n/a .000
004774 |  
104774 | L/R/S= 294 / 1009 / 0351  
104775 |  
104776 |  
004777 ROUTE2: CO0307-----DfMn-ID NDDND-----AREAhA-QPEAKm-TPeakDtE-hBm--Rvmm R.C.-DfWcm  
004780 ADD HYD          + 1.00:02:4241-out    54658.48 103.664 No.date 38:56 29.12 n/a .000  
004781 |  
104781 | L/R/S= 294 / 1009 / 0351  
004782 |  
004783 |  
004784 |  
004785 |  
004786 |  
004787 |  
004788 |  
004789 |  
004790 |  
004791 |  
004792 |  
004793 |  
004794 |  
004795 |  
004800 ROUTE2: CO0308-----DfMn-ID NDDND-----AREAhA-QPEAKm-TPeakDtE-hBm--Rvmm R.C.-DfWcm  
004801 |  
104801 | L/R/S= 320 / 1160 / 0351  
004802 |  
004803 |  
004804 |  
004805 |  
004810 ROUTE2: CO0309-----DfMn-ID NDDND-----AREAhA-QPEAKm-TPeakDtE-hBm--Rvmm R.C.-DfWcm  
004811 |  
104811 | L/R/S= 320 / 1160 / 0351  
004812 |  
004813 |  
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084747 ROUTE PIPE   ->  1.0:02:335-338    17.90  1.490 N,date      28:26  49.83 n/a      0.000
084848 [RD=1.0] out s >  1.0:02:MH04  25.50  2.104 N,date      28:18  49.88 n/a      0.000
084849 [L/S= 233 / 100] 0:13:338-340    17.90  1.456 N,date      28:26  49.83 n/a      0.000
084850 [Vmax = 1.31; Dmax = 1.35]
085151 [L/S= 1.35; Dmax = 1.35]
085252 R0205:CO1366.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
085353 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:02  49.75 669
085454 [L/S= 2. CN= 75.0]
085555 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
085656 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 188; MN= 013; SCL= 0]
085757 [IARG=mp 4.00; IAREC= 4.00]
085858 [SM= 33.81; SMOV=225.43; SK= 010]
085959 remrk: Total Flows at M106
086060 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
086161 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:02  49.75 n/a      0.000
086262 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:02  49.75 n/a      0.000
086363 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:02  49.75 n/a      0.000
086464 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:02  49.75 n/a      0.000
086565 [M SysSt= 1011E+02, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
086666 R0205:CO1388.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
086767 ADD HYD  + 1.0:02:338-340    17.90  1.456 N,date      28:26  49.83 n/a      0.000
086868 [L/S= 1.02:MH04  25.50  2.104 N,date      28:18  49.88 n/a      0.000
086969 [IARG=mp 4.00; IAREC= 4.00]
087070 R0205:CO1369.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
087171 SAVE HYD  + 1.0:01:MH04  25.50  2.104 N,date      28:18  49.88 n/a      0.000
087272 frame: M104.0025
087373 remrk: Total Flows at M104
087474 R0205:CO1370.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
087575 ROUTE PIPE   ->  1.0:02:MH04  25.50  2.104 N,date      28:18  49.88 n/a      0.000
087676 [RD=1.0] out s >  1.0:01:340-104    25.50  2.086 N,date      28:18  49.88 n/a      0.000
087777 [L/S= 240 / 150] 0:13:104-105    25.50  2.086 N,date      28:18  49.88 n/a      0.000
087878 [Vmax = 1.722; Dmax = 1.97]
087979 [L/S= 1.65; Dmax = 1.97]
088080 R0205:CO1371.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
088181 ADD HYD  + 1.0:02:340-104    25.50  2.086 N,date      28:18  49.88 n/a      0.000
088282 [L/S= 240 / 150] 0:13:104-105    25.50  2.086 N,date      28:18  49.88 n/a      0.000
088383 R0205:CO1372.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
088484 SIMM  + 1.0:01:MH04T  131.77  11.233 N,date      28:08  50.09 n/a      0.000
088585 ROUTE PIPE   ->  1.0:01:104-105    131.77  11.233 N,date      28:08  50.09 n/a      0.000
088686 [RD=1.0] out s >  1.0:01:104-105    131.77  11.233 N,date      28:08  50.09 n/a      0.000
088787 [L/S= 131.77 / 100] 0:13:104-105    131.77  11.233 N,date      28:08  50.09 n/a      0.000
088888 [Vmax = 2.100; Dmax = 1.992]
088989 [L/S= 2.35; Dmax = 1.992]
089090 R0205:CO1373.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
089191 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:02  49.75 674
089292 [L/S= 2. CN= 75.0]
089393 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
089494 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 187; MN= 013; SCL= 0]
089595 [IARG=mp 4.00; IAREC= 4.00]
089696 [SM= 33.81; SMOV=225.43; SK= 010]
089797 R0205:CO1374.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
089898 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
089999 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:02  49.75 n/a      0.000
090000 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:02  49.75 n/a      0.000
090101 [M SysSt= 3304E+02, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
090202 R0205:CO1375.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
090303 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:02  49.75 674
090404 [L/S= 2. CN= 75.0]
090505 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
090606 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 186; MN= 013; SCL= 0]
090707 [IARG=mp 4.00; IAREC= 4.00]
090808 [SM= 33.81; SMOV=225.43; SK= 010]
090909 R0205:CO1376.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
091010 ADD HYD  + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
091111 [L/S= 1.01:AT  96  177 N,date      28:00  60.02 n/a      0.000
091212 [Vmax = 1.00; Dmax = 1.91]
091313 R0205:CO1377.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
091414 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
091515 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
091616 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
091717 [M SysSt= 0.000E+00, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
091818 R0205:CO1378.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
091919 ADD HYD  + 1.0:02:340-104    25.50  2.086 N,date      28:08  50.09 n/a      0.000
092020 [L/S= 240 / 150] 0:13:104-105    25.50  2.086 N,date      28:08  50.09 n/a      0.000
092121 [RD=1.0] out s >  1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
092222 [L/S= 240 / 150] 0:13:104-105    25.50  2.086 N,date      28:08  50.09 n/a      0.000
092323 [Vmax = 1.00; Dmax = 1.91]
092424 [L/S= 1.01:AT  96  177 N,date      28:00  60.02 n/a      0.000
092525 R0205:CO1379.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
092626 SAVE HYD  + 1.0:01:MH05  134.93  11.775 N,date      28:08  50.23 n/a      0.000
092727 frame: M105.0025
092828 remrk: Total Flows at M105
092929 R0205:CO1380.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
093030 DIVERTED SW  + 1.0:01:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
093131 diverted sw  + 1.0:01:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
093232 diverted sw  + 1.0:01:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
093333 R0205:CO1381.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
093434 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
093535 Mjr System / 1.0:02:MH05 JR  134.93  11.775 N,date      28:08  50.23 n/a      0.000
093636 Mjr System / 1.0:02:MH05 JR  134.93  11.775 N,date      28:08  50.23 n/a      0.000
093737 R0205:CO1382.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
093838 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:01  49.75 669
093939 [L/S= 2. CN= 75.0]
094040 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
094141 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 211; MN= 013; SCL= 0]
094242 [IARG=mp 4.00; IAREC= 4.00]
094343 [SM= 33.81; SMOV=225.43; SK= 010]
094444 R0205:CO1383.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
094545 ADD HYD  + 1.0:02:BA-M  7.60  960 N,date      28:01  49.75 n/a      0.000
094646 [L/S= 1.01:AT  96  177 N,date      28:00  60.02 n/a      0.000
094747 [Vmax = 1.00; Dmax = 1.91]
094848 R0205:CO1384.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
094949 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
095050 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
095151 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
095252 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
095353 [M SysSt= 7020E+00, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
095454 R0205:CO1385.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
095555 ROUTE PIPE   ->  1.0:01:360-106A    7.19  629 N,date      28:12  50.01 n/a      0.000
095656 [RD=1.0] out s >  1.0:01:360-106A    7.19  629 N,date      28:12  50.01 n/a      0.000
095757 [L/S= 1.01:MH05  134.93  11.775 N,date      28:08  50.23 n/a      0.000
095858 [Vmax = 1.089; Dmax = 1.95]
095959 [L/S= 2.35; Dmax = 1.95]
096060 R0205:CO1386.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
096161 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:00  49.75 669
096262 [L/S= 2. CN= 75.0]
096363 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
096464 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 148; MN= 013; SCL= 0]
096565 [IARG=mp 4.00; IAREC= 4.00]
096666 [SM= 33.81; SMOV=225.43; SK= 010]
096767 R0205:CO1387.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
096868 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
096969 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
097070 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
097171 [M SysSt= 5165E+00, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
097272 R0205:CO1388.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
097373 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:02  50.08 673
097474 [L/S= 30.50; Dmax = 1.91]
097575 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
097676 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 465; MN= 013; SCL= 0]
097777 [IARG=mp 4.00; IAREC= 4.00]
097878 [SM= 36.67; SMOV=244.49; SK= 010]
097979 R0205:CO1389.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
098080 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
098181 Mjr System / 1.0:01:EX-LAND  32.50  4.063 N,date      28:02  50.08 n/a      0.000
098282 Mjr System / 1.0:01:EX-LAND  32.50  4.063 N,date      28:02  50.08 n/a      0.000
098383 Mjr System / 1.0:01:EX-LAND  32.50  4.063 N,date      28:02  50.08 n/a      0.000
098484 Mjr System / 1.0:01:EX-LAND  32.50  4.063 N,date      28:02  50.08 n/a      0.000
098585 [M SysSt= 1224E+00, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
098686 R0205:CO1390.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
098787 ADD HYD  + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
098888 + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
098989 + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
099090 + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
099191 + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
099292 + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
099393 [L/S= 1.01:BA-M  3.29  465 N,date      28:14  49.75 n/a      0.000
099494 R0205:CO1391.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
099595 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
099696 Mjr System / 1.0:02:BA-M  3.29  465 N,date      28:14  49.75 n/a      0.000
099797 Mjr System / 1.0:02:BA-M  3.29  465 N,date      28:14  49.75 n/a      0.000
099898 [M SysSt= 0.000E+00, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
099999 R0205:CO1392.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
100000 ROUTE PIPE   ->  1.0:01:MH05 BA  100.98  3.008 N,date      28:08  50.23 n/a      0.000
100101 [RD=1.0] out s >  1.0:01:105-106A    100.98  3.018 N,date      28:02  50.23 n/a      0.000
100202 [L/S= 208 / 100] 0:13:105-106A    100.98  3.018 N,date      28:02  50.23 n/a      0.000
100303 [Vmax = 1.597; Dmax = 1.949]
100404 [L/S= 1.80; Dmax = 1.80]
100505 R0205:CO1393.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
100606 ADD HYD  + 1.0:02:360-106A    7.19  629 N,date      28:21  50.01 n/a      0.000
100707 + 1.0:02:360-106A    7.19  629 N,date      28:21  50.01 n/a      0.000
100808 + 1.0:02:BA-M  7.60  960 N,date      28:14  49.75 n/a      0.000
100909 + 1.0:02:BA-M  7.60  960 N,date      28:14  49.75 n/a      0.000
101010 SIMM  + 1.0:01:MH05  134.93  11.775 N,date      28:02  50.23 n/a      0.000
101111 R0205:CO1394.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
101212 SAVE HYD  + 1.0:01:MH06A  111.46  3.702 N,date      28:02  50.20 n/a      0.000
101313 frame: M106A.0025
101414 remrk: Total Flows at M106A
101515 R0205:CO1395.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
101616 ROUTE PIPE   ->  1.0:02:MH06A  111.46  3.702 N,date      28:02  50.20 n/a      0.000
101717 [RD=1.0] out s >  1.0:01:106A-106  111.46  3.699 N,date      28:16  50.20 n/a      0.000
101818 [L/S= 190 / 100] 0:13:106-106  111.46  3.699 N,date      28:16  50.20 n/a      0.000
101919 [Vmax = 1.635; Dmax = 1.81]
102020 [L/S= 1.80; Dmax = 1.81]
102121 R0205:CO1396.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
102222 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:00  60.02 807
102323 [L/S= 2. CN= 75.0]
102424 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
102525 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 262; MN= 013; SCL= 0]
102626 [IARG=mp 4.00; IAREC= 4.00]
102727 [SM= 33.81; SMOV=225.43; SK= 010]
102828 R0205:CO1397.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
102929 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
103030 Mjr System / 1.0:01:BA-M  2.44  433 N,date      28:00  60.02 n/a      0.000
103131 Mjr System / 1.0:01:BA-M  2.44  433 N,date      28:00  60.02 n/a      0.000
103232 Mjr System / 1.0:01:BA-M  2.44  433 N,date      28:00  60.02 n/a      0.000
103333 R0205:CO1398.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
103434 ADD HYD  + 1.0:02:106A-106  111.46  3.699 N,date      28:16  50.20 n/a      0.000
103535 [RD=1.0] out s >  1.0:02:106A-106  111.46  3.699 N,date      28:16  50.20 n/a      0.000
103636 [L/S= 121.46 / 100] 0:13:106A-106  111.46  3.699 N,date      28:16  50.20 n/a      0.000
103737 [Vmax = 1.676; Dmax = 1.544]
103838 [L/S= 1.80; Dmax = 1.81]
103939 R0205:CO1400.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
104040 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:01  46.81 629
104141 [L/S= 1.01:AT  96  177 N,date      28:00  60.02 n/a      0.000
104242 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
104343 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 183; MN= 013; SCL= 0]
104444 [IARG=mp 4.00; IAREC= 4.00]
104545 [SM= 33.81; SMOV=225.43; SK= 010]
104646 R0205:CO1401.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
104747 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:01  46.81 629
104848 [L/S= 1.01:AT  96  177 N,date      28:00  60.02 n/a      0.000
104949 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
105050 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 183; MN= 013; SCL= 0]
105151 [IARG=mp 4.00; IAREC= 4.00]
105252 [SM= 33.81; SMOV=225.43; SK= 010]
105353 R0205:CO1402.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
105454 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
105555 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
105656 Mjr System / 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
105757 [M SysSt= 1196E+03, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
105858 R0205:CO1403.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
105959 CONTINUS STANHD ID  1.0:01:BA  7.60  960 N,date      28:01  54.09 727
106060 [L/S= 1.01:AT  96  177 N,date      28:00  60.02 n/a      0.000
106161 [RD=1.0] out s >  1.0:01:AT  96  177 N,date      28:00  60.02 n/a      0.000
106262 [L/S= 23.33; Dmax = 1.97]
106363 [Previous area: Iper= 4.67; SLLP=1.0; LQ= 40; MNP= 250; SCP= 0]
106464 [Impervious area: Iper= 1.57; SLLP=1.0; LQ= 179; MN= 013; SCL= 0]
106565 [IARG=mp 4.00; IAREC= 4.00]
106666 [SM= 33.81; SMOV=225.43; SK= 010]
106767 R0205:CO1404.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
106868 COMPUTE DUALIND .....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
106969 Mjr System / 1.0:01:AT  1.01  1.01 N/date      28:01  54.09 n/a      0.000
107070 Mjr System / 1.0:01:AT  1.01  1.01 N/date      28:01  54.09 n/a      0.000
107171 [M SysSt= 279E+03, TotOfVol= 0.000E+00, NOf= 0, TotDrOf= 0 hrs]
107272 R0205:CO1405.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
107373 ADD HYD  + 1.0:02:106-107  113.90  4.067 N,date      28:03  50.41 n/a      0.000
107474 [L/S= 113.90 / 100] 0:13:106-107  113.90  4.067 N,date      28:03  50.41 n/a      0.000
107575 [Vmax = 2.100; Dmax = 1.992]
107676 + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
107777 + 1.0:02:BA-M  7.60  960 N,date      28:00  0.00 n/a      0.000
107878 R0205:CO1406.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
107979 SAVE HYD  + 1.0:01:MH07  128.65  5.370 N,date      28:03  50.61 n/a      0.000
108080 frame: M107.0025
108181 remrk: Total Flows at M107
108282 R0205:CO1407.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
108383 ROUTE PIPE   ->  1.0:02:MH07  128.65  5.370 N,date      28:03  50.61 n/a      0.000
108484 [RD=1.0] out s >  1.0:01:117-119  128.65  5.374 N,date      28:04  50.61 n/a      0.000
108585 [L/S= 128.65 / 100] 0:13:117-119  128.65  5.374 N,date      28:04  50.61 n/a      0.000
108686 [Vmax = 1.922; Dmax = 1.633]
108787 [L/S= 1.92; Dmax = 1.63]
108888 R0205:CO1408.....Df=n-ID NND.....ARE/A..QPE/AgC...Tpe/AtD..hh:mm.....Rv/m R.C.....Df/wcm
108989 ROUTE PIPE   ->  1.0:02:117-119  128.65  5.374 N,date      28:04  50.61 n/a      0.000
109090 [RD=1.0] out s >  1.0:01:117-119  128.65  5.374 N,date      28:04  50.61 n/a      0.000
109191 [L/S= 66.7 / 120] 0:13:117-119  128.65  5.374 N,date      28:04  50.61 n/a      0.000
109292 [Vmax = 922; Dmax = 1.633]
10
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122121 # remark:Total Flow at Jockvale Road
122122 # Hydrograph from Jockvale Road routed to Heart's Desire
122123 # Channel X-Section obtained from RCVIA Hydraulic Model - Station 689
122124 #
122125 #
122126 # R0025:CO0428..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122127 # ROUTE CHANNEL > 1.0 0:2:SN,DE 55194.86 104.159 No,Date 39.29 50.23 n/a 0.00
122128 # [RFD=1.0] out < 1.0 0:1:N,DE 55194.86 104.140 No,Date 39.45 29.33 n/a 0.00
122129 # [L/S=1062.7 / 227.7] SN:24.49; SK=0.010
122130 # [Vmax=1.483; Dmax=2.264]
122131 #
122132 # Catchment DESIRE
122133 # To Jock River (north of the Jock)
122134 # Rural-estate subdivision, Heart's Desire Community
122135 #
122136 # R0025:CO0429..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122137 # CONTINUS STANDRD 1.0 0:1:DESI:RE 23.78 2.161 No,Date 28.03 40.77 548 000
122138 # [L/S=25; Timp=25]
122139 # [CN=70.0; No: 3.00; Tpe: 5.29]
122140 # [AREC=4.00; SM=36.67; SMX=264.99; SK= 0.010]
122141 # Perovous area: IArea:4.67; SLP:n=0.10; LQR= 40. ; MPN=250; SCPN= 0]
122142 # [IMPERVUS area: IArea:1.57; SLP:n=0.10; LQR= 400. ; MN: 0.15; SCPN= 0]
122143 # [ARECmp= 4.00; IARECmp= 4.00]
122144 # [SM=31.15; SMOX=207.66; SK= 0.010]
122145 #
122146 # Catchment JOCKVA
122147 # To Jockvale SWM Facility
122148 # Residential development & golf course
122149 # JFSA 2021-01-11 update JOCKVA after updating CORRGA per IRI GROUP July 2008.
122150 # JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX.LAND 32.5 ha as
122151 #
122152 # R0025:CO0430..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122153 # CONTINUS STANDRD 1.0 0:1:JOCKVA 225.13 21.797 No,Date 28.07 50.08 673 000
122154 # [L/S=106.2; Vmax=50]
122155 # [L/S=25; CN=74.0]
122156 # Perovous area: IArea:4.67; SLP:n=0.10; LQR= 40. ; MPN=250; SCPN= 0]
122157 # [IMPERVUS area: IArea:1.57; SLP:n=0.10; LQR=1311. ; MN: 0.15; SCPN= 0]
122158 # [ARECmp= 4.00; IARECmp= 4.00]
122159 # [SM=36.67; SMOX=244.49; SK= 0.010]
122160 # R0025:CO0431..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122161 # ADD HND + 1.0 0:2:JOCKVA 225.13 21.797 No,Date 28.07 50.08 n/a 0.00
122162 # + 1.0 0:2:JOCKVA 225.13 21.797 No,Date 28.07 50.08 n/a 0.00
122163 # + 1.0 0:2:RESI 0.00 0.00 No,Date 0.00 0.00 n/a 0.00
122164 # + 1.0 0:2:RI:M 0.00 0.00 No,Date 0.00 0.00 n/a 0.00
122165 # SLM 1.0 0:1:JOCKVA:TO 257.63 24.072 No,Date 28.07 50.10 n/a 0.00
122166 # R0025:CO0432..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122167 # SAVE HND 1.0 0:1:JOCKVA:TO 257.63 24.072 No,Date 28.07 50.10 n/a 0.00
122168 # Frame: JFSA:TO 0025
122169 # remark:Total Flow at KB first pond
122170 #
122171 # Jockvale SWM Facility
122172 # Rating curve obtained from Jockvale Servicing Study (CCL 1999)
122173 #
122174 # R0025:CO0433..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122175 # ROUTE RESERVOIR 1.0 0:1:JOCKVA 257.63 24.072 No,Date 28.07 50.10 n/a 0.00
122176 # out < 1.0 0:1:JOCK:P 257.63 24.072 No,Date 28.37 50.10 n/a 0.00
122177 # overflow < 1.0 0:2:JO-DFP 0.00 0.00 No,Date 0.00 0.00 n/a 0.00
122178 # [MKS:0.03; SLSZ:0] 1.0 0:1:JOCKVA 257.63 24.072 No,Date 28.07 50.10 n/a 0.00
122179 # R0025:CO0434..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122180 # ADD HND + 1.0 0:2:RESI 23.78 2.161 No,Date 28.03 40.77 n/a 0.00
122181 # + 1.0 0:2:DESI:RE 23.78 2.161 No,Date 28.03 40.77 n/a 0.00
122182 # + 1.0 0:2:JOCK:P 257.63 24.072 No,Date 28.37 50.10 n/a 0.00
122183 # SLM 1.0 0:1:JOCKVA:TO 257.63 24.072 No,Date 28.07 50.10 n/a 0.00
122184 # R0025:CO0435..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122185 # SAVE HND 1.0 0:1:SN,DE 55476.27 104.765 No,Date 39.44 29.43 n/a 0.00
122186 # Frame: SN,DE:0025
122187 # remark:Total Flow at Heart's Desire
122188 #
122189 # Hydrograph from Heart's Desire routed to Rdeau River
122190 # Channel X-Section obtained from RCVIA Hydraulic Model - Station 0
122191 #
122192 # R0025:CO0436..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122193 # ROUTE CHANNEL > 1.0 0:1:SN,DE 55476.27 104.765 No,Date 39.44 29.43 n/a 0.00
122194 # [RFD=1.0] out < 1.0 0:1:SN,DE 55476.27 104.757 No,Date 39.48 29.43 n/a 0.00
122195 # [L/S=563.7 / 967.045]
122196 # [Vmax=1.941; Dmax=2.72]
122197 #
122198 # Catchment S 2
122199 # To Jock River (north and south)
122200 # Developed floodplain and river
122201 #
122202 # R0025:CO0437..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122203 # CONTINUS STANDRD 1.0 0:1:SN,DE 102.94 3.971 No,Date 28.20 30.13 408 000
122204 # [CN=72.0; No: 3.00; Tpe: 4.51]
122205 # [AREC=4.00; SM=36.67; SMX=264.99; SK= 0.010]
122206 # [L/S=25; Timp=25]
122207 # [CN=70.0; No: 3.00; Tpe: 5.29]
122208 # Perovous area: IArea:4.67; SLP:n=0.10; LQR= 40. ; MPN=250; SCPN= 0]
122209 # [IMPERVUS area: IArea:1.57; SLP:n=0.10; LQR= 104.958; MN: 0.15; SCPN= 0]
122210 # [ARECmp= 4.00; IARECmp= 4.00]
122211 # [SM=36.67; SMOX=244.49; SK= 0.010]
122212 # R0025:CO0438..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122213 # ADD HND + 1.0 0:2:SN,DE 55476.27 104.757 No,Date 39.48 29.43 n/a 0.00
122214 # + 1.0 0:2:SN,DE 102.94 3.971 No,Date 28.20 30.13 n/a 0.00
122215 # SLM 1.0 0:1:SN,DE 55579.21 104.958 No,Date 39.48 29.43 n/a 0.00
122216 # R0025:CO0439..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
122217 # SAVE HND 1.0 0:1:SN,DE 55579.21 104.958 No,Date 39.48 29.43 n/a 0.00
122218 # Frame: SN,NI:0025
122219 # remark:Total Flow at Rdeau River
122220 #
122221 # *****
122222 # ** END OF RIN : 49
122223 #
122224 #
122225 #
122226 #
122227 #
122228 #
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122400 #

14408 # CONTINUS NASHDD 1.0 0:1:JR,ASH 1781.00 16.834 No,Date 32.39 36.85 452 000
14409 # [CN=75.0; No: 3.00; Tpe: 8.11]
14410 # [AREC=4.00; SM=39.75; SMX=264.99; SK= 0.010]
14411 # [InterEventTime=12.00]
14412 # R0050:CO0010..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14413 # CONTINUS NASHDD 1.0 0:1:SW11 500.00 9.061 No,Date 29.21 31.73 389 000
14414 # [CN=66.0; No: 3.00; Tpe: 1.51]
14415 # [AREC=4.00; SM=52.62; SMX=350.79; SK= 0.010]
14416 # [InterEventTime=12.00]
14417 # #
14418 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14419 # of 1.80
14420 # R0050:CO0011..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14421 # CONTINUS NASHDD 1.0 0:1:SN,CK 1917.00 12.142 No,Date 34.26 31.73 389 000
14422 # [CN=66.0; No: 3.00; Tpe: 5.29]
14423 # [AREC=4.00; SM=52.62; SMX=350.79; SK= 0.010]
14424 # [InterEventTime=12.00]
14425 # #
14426 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14427 # of 1.52
14428 # R0050:CO0012..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14429 # CONTINUS NASHDD 1.0 0:1:SW10 5666.00 32.402 No,Date 37.52 36.85 452 000
14430 # [CN=72.0; No: 3.00; Tpe: 8.00]
14431 # [AREC=4.00; SM=39.75; SMX=264.99; SK= 0.010]
14432 # [InterEventTime=12.00]
14433 # #
14434 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14435 # of 1.25
14436 # R0050:CO0013..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14437 # CONTINUS NASHDD 1.0 0:1:RL,CK 4376.00 31.024 No,Date 39.59 31.73 389 000
14438 # [CN=66.0; No: 3.00; Tpe:1]
14439 # [AREC=4.00; SM=52.62; SMX=350.79; SK= 0.010]
14440 # [InterEventTime=12.00]
14441 # #
14442 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14443 # of 1.68
14444 # R0050:CO0014..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14445 # CONTINUS NASHDD 1.0 0:1:SN,CK 1132.00 14.039 No,Date 30.53 38.35 424 000
14446 # [CN=70.0; No: 3.00; Tpe: 2.51]
14447 # [AREC=4.00; SM=36.67; SMX=287.10; SK= 0.010]
14448 # [InterEventTime=12.00]
14449 # #
14450 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14451 # of 1.82
14452 # R0050:CO0015..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14453 # CONTINUS NASHDD 1.0 0:1:N,CK 4464.00 15.472 No,Date 39.59 28.95 355 000
14454 # [CN=72.0; No: 3.00; Tpe: 8.00]
14455 # [AREC=4.00; SM=61.90; SMX=412.66; SK= 0.010]
14456 # [InterEventTime=12.00]
14457 # #
14458 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14459 # of 1.50
14460 # R0050:CO0016..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14461 # CONTINUS NASHDD 1.0 0:1:SW,8 131.00 2.740 No,Date 28.57 29.64 364 000
14462 # [CN=63.0; No: 3.00; Tpe: 5.90]
14463 # [AREC=4.00; SM=36.67; SMX=396.11; SK= 0.010]
14464 # [InterEventTime=12.00]
14465 # #
14466 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14467 # of 1.65
14468 # R0050:CO0017..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14469 # CONTINUS NASHDD 1.0 0:1:HL,RL 3854.00 18.180 No,Date 38.32 31.73 389 000
14470 # [CN=66.0; No: 3.00; Tpe: 8.42]
14471 # [AREC=4.00; SM=52.62; SMX=350.79; SK= 0.010]
14472 # [InterEventTime=12.00]
14473 # #
14474 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14475 # of 1.32
14476 # R0050:CO0018..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14477 # CONTINUS NASHDD 1.0 0:1:SW,7 3197.00 19.937 No,Date 36.23 25.61 314 000
14478 # [CN=57.0; No: 3.00; Tpe: 6.61]
14479 # [AREC=4.00; SM=76.32; SMX=508.81; SK= 0.010]
14480 # [InterEventTime=12.00]
14481 # #
14482 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14483 # of 1.67
14484 # R0050:CO0019..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14485 # CONTINUS NASHDD 1.0 0:1:SW,6 165.00 1.285 No,Date 33.02 32.44 398 000
14486 # [CN=67.0; No: 3.00; Tpe: 4.18]
14487 # [AREC=4.00; SM=30.55; SMX=336.97; SK= 0.010]
14488 # [InterEventTime=12.00]
14489 # #
14490 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14491 # of 1.67
14492 # R0050:CO0020..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14493 # CONTINUS NASHDD 1.0 0:1:VL,RL 1332.00 9.332 No,Date 35.12 36.85 452 000
14494 # [CN=72.0; No: 3.00; Tpe: 5.91]
14495 # [AREC=4.00; SM=39.75; SMX=264.99; SK= 0.010]
14496 # [InterEventTime=12.00]
14497 # R0050:CO0021..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14498 # CONTINUS NASHDD 1.0 0:1:SW,5 224.00 8.187 No,Date 28.45 41.51 509 000
14499 # [CN=75.0; No: 3.00; Tpe: 7.51]
14500 # [AREC=4.00; SM=31.15; SMX=207.66; SK= 0.010]
14501 # [InterEventTime=12.00]
14502 # #
14503 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14504 # of 1.20
14505 # R0050:CO0022..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14506 # CONTINUS NASHDD 1.0 0:1:FL,CK 4945.00 44.623 No,Date 33.18 38.37 471 000
14507 # [CN=74.0; No: 3.00; Tpe: 4.45]
14508 # [AREC=4.00; SM=36.67; SMX=244.49; SK= 0.010]
14509 # [InterEventTime=12.00]
14510 # R0050:CO0023..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14511 # CONTINUS NASHDD 1.0 0:1:SW,4 20.00 0.943 No,Date 28.35 45.60 560 000
14512 # [CN=81.0; No: 3.00; Tpe: 6.21]
14513 # [AREC=4.00; SM=35.21; SMX=168.09; SK= 0.010]
14514 # [InterEventTime=12.00]
14515 # #
14516 # The Tp was modified according to a Peak Reduction factor (MFO Chart B2-4)
14517 # of 1.61
14518 # R0050:CO0024..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14519 # CONTINUS NASHDD 1.0 0:1:SW,SAJ 1412.00 8.794 No,Date 37.48 39.93 490 000
14520 # [CN=75.0; No: 3.00; Tpe: 8.00]
14521 # [AREC=4.00; SM=33.81; SMX=225.43; SK= 0.010]
14522 # [InterEventTime=12.00]
14523 # R0050:CO0025..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14524 # CONTINUS NASHDD 1.0 0:1:SW,4 585.00 12.896 No,Date 29.55 45.60 560 000
14525 # [CN=70.0; No: 3.00; Tpe: 3.21]
14526 # [AREC=4.00; SM=36.67; SMX=168.09; SK= 0.010]
14527 # [InterEventTime=12.00]
14528 # R0050:CO0026..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14529 # CONTINUS NASHDD 1.0 0:1:L,ML,CK 1021.00 17.059 No,Date 30.46 44.77 549 000
14530 # [CN=80.0; No: 3.00; Tpe: 2.46]
14531 # [AREC=4.00; SM=36.67; SMX=175.50; SK= 0.010]
14532 # [InterEventTime=12.00]
14533 # R0050:CO0027..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14534 # CONTINUS NASHDD 1.0 0:1:SW,2 177.00 6.469 No,Date 28.45 41.51 509 000
14535 # [CN=70.0; No: 3.00; Tpe: 7.51]
14536 # [AREC=4.00; SM=31.15; SMX=207.66; SK= 0.010]
14537 # [InterEventTime=12.00]
14538 # R0050:CO0028..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14539 # CONTINUS NASHDD 1.0 0:1:SL,RL 1122.00 15.544 No,Date 31.43 45.60 560 000
14540 # [CN=81.0; No: 3.00; Tpe: 3.21]
14541 # [AREC=4.00; SM=25.21; SMX=168.09; SK= 0.010]
14542 # [InterEventTime=12.00]
14543 # R0050:CO0029..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14544 # CONTINUS NASHDD 1.0 0:1:ML,RL 2737.00 34.946 No,Date 31.29 40.72 500 000
14545 # [CN=76.0; No: 3.00; Tpe: 3.63]
14546 # [AREC=4.00; SM=32.46; SMX=216.39; SK= 0.010]
14547 # [InterEventTime=12.00]
14548 # #
14549 # Routing hydrographs
14550 # #
14551 # Starting with the addition of Jock River Headwater and Subwatershed 13
14552 #
14553 # R0050:CO0030..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14554 # ADD HND + 1.0 0:2:JR,HW 3680.00 18.440 No,Date 36.55 30.33 n/a 0.00
14555 # + 1.0 0:2:SW,13 971.00 6.937 No,Date 32.34 28.20 n/a 0.00
14556 # SLM 1.0 0:1:SN,13 4651.00 23.559 No,Date 35.24 29.90 n/a 0.00
14557 # #
14558 # Sum of hydrographs from Node 13 routed to Node 13A
14559 # (Approximated cross-section - see cross-section 258)
14560 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
14561 #
14562 # R0050:CO0031..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14563 # ROUTE CHANNEL > 1.0 0:2:SN,13 4651.00 23.559 No,Date 35.24 29.90 n/a 0.00
14564 # [RFD=1.0] out < 1.0 0:1:SN,13A 4651.00 19.136 No,Date 39.54 27.61 n/a 0.00
14565 # [L/S=9074.7 / 022/040]
14566 # [Vmax=574; Dmax=920]
14567 #
14568 # Addition of Subwatershed Jock River at Goodwood Marsh to Node 13A
14569 #
14570 # R0050:CO0032..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14571 # ADD HND + 1.0 0:2:SN,13A 3074.00 8.912 No,Date 39.59 24.31 298 000
14572 # + 1.0 0:2:JR,GM 3074.00 2.932 No,Date 39.59 24.31 n/a 0.00
14573 # SLM 1.0 0:1:SN,13A 7725.00 3.808 No,Date 61.35 27.67 n/a 0.00
14574 # #
14575 # Insertion of a reservoir to simulate the effects of the Goodwood Marsh
14576 #
14577 # R0050:CO0033..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14578 # ROUTE RESERVOIR > 1.0 0:2:SN,13A 4651.00 19.136 No,Date 39.54 27.67 n/a 0.00
14579 # out < 1.0 0:1:RES,GM 7725.00 3.808 No,Date 61.35 27.67 n/a 0.00
14580 # [MFO:0.46; 1481E903 m]
14581 # #
14582 # R0050:CO0034..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14583 # SAVE HND + 1.0 0:1:RES,GM 7725.00 3.808 No,Date 61.35 27.67 n/a 0.00
14584 # frame: H,RESGM
14585 # remark:Outflow from Res.GM
14586 # Output of Reservoir Goodwood Marsh routed from Node 13A to Node 12
14587 # (Approx. cross-section 258)
14588 # Use n=0.04 for summer conditions and n=0.025 for spring conditions
14589 # R0050:CO0035..... Dfm:n-1D:NDD..... AREA:h-QPEA:GM-TpeakDate:hh:mm... Rvm:n-R.C...-DFWCM
14590 # CONTINUS NASHDD 1.0 0:1:RES,GM 7725.00 3.808 No,Date 61.35 27.67 n/a 0.00
14591 # [RFD=1.0] out < 1.0 0:1:RES,GM 7725.00 3.804 No,Date 64.19 27.68 n

Main data table with columns: ID, Code, Description, Material, etc. Contains thousands of rows of technical specifications and material codes.

Table with columns for project ID (e.g., 121717, 121718), name (e.g., R0505:CO0213), description, and various numerical data points (e.g., Dtm=1.0, L/S=19.47, etc.). The table contains multiple entries for different sections and stations, including details for 'R0505:CO0213' through 'R0505:CO0272'.

13091	+	1.0	02:KB-12.5	4.86	867	Ndate	28:09	68.93	n/a	0.00
13092	+	1.0	02:KB-11.9	5.47	873	Ndate	27:53	60.02	n/a	0.00
13093	+	1.0	02:KB-14.5	5.47	873	Ndate	27:53	60.02	n/a	0.00
13094	+	1.0	02:KB-14.5	5.47	873	Ndate	27:53	60.02	n/a	0.00
13095	SLM	1.0	01:KB-P2	254.24	26.339	Ndate	28:02	46.87	n/a	0.00
13096	ROUTE RESERVOIR	1.0	01:ID-NDD	AREHA-QPEAKm-TpeakDte	hh:mm	RvM R C	DFWcm			
13097	out	1.0	01:KB-P2	253.10	17.634	Ndate	28:09	46.87	n/a	0.00
13098	overFlow	1.0	01:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13099	MSI	1.0	01:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13100	ADD IVD	1.0	02:KB-P2	253.10	17.634	Ndate	28:09	46.87	n/a	0.00
13101	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13102	SLM	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13103	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13104	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13105	ROUTE RESERVOIR	1.0	01:ID-NDD	AREHA-QPEAKm-TpeakDte	hh:mm	RvM R C	DFWcm			
13106	out	1.0	01:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13107	overFlow	1.0	01:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13108	MSI	1.0	01:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13109	ADD IVD	1.0	02:KB-P2	253.10	17.634	Ndate	28:09	46.87	n/a	0.00
13110	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13111	SLM	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13112	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13113	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13114	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13115	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13116	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13117	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13118	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13119	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13120	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13121	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13122	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13123	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13124	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13125	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13126	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a	0.00
13127	+	1.0	02:KB-P2	254.24	26.339	Ndate	28:09	46.87	n/a </tr	

Table with multiple columns containing alphanumeric codes, system names, and numerical values. The table is organized into several vertical sections, each starting with a header row like '134655' or '134656'. The rows contain detailed system information, including component names and various numerical parameters.


```

145877 ROUTE CHANNEL -> 1.0 02:5:NSA 4644.01 89.756 N_date 34:38 37.51 n/a .000
145878 |RDE 1.001 out-> 1.0 01:2:NR 4644.01 85.943 N_date 36:10 37.51 n/a .000
145879 |L/S na= 4630 / 043/ 051
145880 |Vmax = .944;Dmax = 3.866
145901 #
145902 # Addition of Subwatershed 4 with Leamy Creek to Node 4
145903 #
145904 ROUTE0100 C00060 -> Dtmn 1-D NND -> AREHA-QPEAKm-TpeakDate-hh:mm- Rvmm R C -Dwfm
145905 |ADD HYD + 1.0 02:SWZ 4644.01 85.943 N_date 36:10 37.51 n/a .000
145906 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145907 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145908 |IARECmp=4.00;IARECp=4.00
145909 ROUTE RESEVEROR > 1.0 02:AI 2.50 .572 N_date 28:01 70.42 n/a .000
145910 |over flow out-> 1.0 03:AI-OPF 0.00 0.00 N_date 0:00 n/a .000
145911 |MSStOkEd= 17764.00 mt. TotOfVol= 0.000E+00 m. NOfv= 0. TotDurOfv= 0 hrs
145912 #
145913 ROUTE0100 C00090 -> Dtmn 1-D NND -> AREHA-QPEAKm-TpeakDate-hh:mm- Rvmm R C -Dwfm
145914 |CONTINUOUS STANDIHD 1.0 01:ST-2 59 .115 N_date 28:00 55.78 630 .000
145915 |IARECmp=4.00;IARECp=4.00
145916 #
145917 ROUTE RESEVEROR > 1.0 02:AI 2.50 .572 N_date 28:01 70.42 n/a .000
145918 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145919 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145920 |IARECmp=4.00;IARECp=4.00
145921 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145922 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145923 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145924 |IARECmp=4.00;IARECp=4.00
145925 #
145926 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145927 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145928 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145929 |IARECmp=4.00;IARECp=4.00
145930 #
145931 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145932 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145933 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145934 |IARECmp=4.00;IARECp=4.00
145935 #
145936 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145937 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145938 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145939 |IARECmp=4.00;IARECp=4.00
145940 #
145941 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145942 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145943 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145944 |IARECmp=4.00;IARECp=4.00
145945 #
145946 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145947 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145948 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145949 |IARECmp=4.00;IARECp=4.00
145950 #
145951 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145952 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145953 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145954 |IARECmp=4.00;IARECp=4.00
145955 #
145956 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145957 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145958 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145959 |IARECmp=4.00;IARECp=4.00
145960 #
145961 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145962 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145963 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145964 |IARECmp=4.00;IARECp=4.00
145965 #
145966 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145967 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145968 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145969 |IARECmp=4.00;IARECp=4.00
145970 #
145971 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145972 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145973 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145974 |IARECmp=4.00;IARECp=4.00
145975 #
145976 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145977 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145978 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145979 |IARECmp=4.00;IARECp=4.00
145980 #
145981 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145982 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145983 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145984 |IARECmp=4.00;IARECp=4.00
145985 #
145986 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145987 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145988 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145989 |IARECmp=4.00;IARECp=4.00
145990 #
145991 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145992 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145993 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145994 |IARECmp=4.00;IARECp=4.00
145995 #
145996 ROUTE RESEVEROR > 1.0 02:ST-2 59 .115 N_date 28:00 55.78 n/a .000
145997 |Previous area: IPer=4.0;SLPP=0.0;LGP=0.0;MMP=250;SCP=0
145998 |Impervious area: IAm=1.57;SLPI=0.50;LGR=224;MNI=013;SCI=0
145999 |IARECmp=4.00;IARECp=4.00
146000 #

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Table with multiple columns containing technical data such as IDs, descriptions, and numerical values.


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168313 ROUTE RESERVOIR -> 1.0 02: MLLS 175.99 20.390 No.date 28:06 56.87 n/a .000
168328 out <= 1.0 01: MLLP 146.22 4.050 No.date 28:08 56.87 n/a .000
168333 overflow <= 1.0 03: MLL-OW 20.77 16.228 No.date 28:08 56.87 n/a .000
168349 [MKS]obled: 23100601 ob. No.Off= 2. TotDur(Off) 1. hr(s)
168355 R0100: CO0426 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168360 ADD HYD + 1.0 02: N.M 5520.07 145.768 No.date 36:59 39.46 n/a .000
168375 + 1.0 02: MLL-OW 29.77 16.228 No.date 28:08 56.87 n/a .000
168388 + 1.0 02: MLLP 146.22 4.050 No.date 28:08 56.87 n/a .000
168399 SLM 55196.05 146.399 No.date 36:58 39.51 n/a .000
168400 R0100: CO0427 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168413 SAVE HYD 1.0 01: SN,NI 55196.05 146.399 No.date 36:58 39.51 n/a .000
168425 Inam: SN,M 0100
168433 remark:Total Flow at Jockvale Road
168445 #
168455 # Hydrograph from Jockvale Road routed to Hart's Desire
168460 # Channel X-Section obtained from RCV4 Hydraulic Model - Station 689
168478 #
168488 R0100: CO0428 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168499 ROUTE CHANNEL -> 1.0 02: SN,M 55196.05 146.399 No.date 36:58 39.51 n/a .000
168500 [RDF= 1.00] out <= 1.0 01: NI,DE 55196.05 146.071 No.date 37:13 39.51 n/a .000
168510 [L/S= 1962 / 221.045]
168520 [Vmax= 1.642; Dmax= 2.661]
168530 *****
168540 # Catchment DESIRE
168555 # - To Jock River (north of the Jock)
168560 # Rural estate subdivision (Hart's Desire Community)
168575 #
168580 R0100: CO0429 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168590 CONTINUOUS STANBYD 1.0 01: DESIRE 23.78 3.004 No.date 28:03 53.11 600 .000
168600 [M P= 25; TSM= 25]
168610 [LRS= 2; C= 77.0]
168620 [Pervious area: IArea 4.67; SLP(=) 0; LCP= 40; MPM= 250; SC(=) 0]
168630 [Impervious area: IArea 1.57; SLP(=) 0; LCP= 400; MN= 013; SC(=) 0]
168640 [IARECmp= 4.00; IARECPer= 4.00]
168650 [SM N= 31.35; SSM= 24.49; SKE= 010]
168660 *****
168670 # Catchment JOCKVA
168680 # - To Jockvale SWM Facility
168690 # - Residential development & golf course
168700 # - JESA CO0411 update JOCKVA after updating CORPG as per HR GROUP July 2008.
168710 # JOCKVA area became 225.13 ha instead of 257.63 ha. JOCKVA separated into two areas JOCKVA and EX LAND 32.5 ha as
168720 #
168730 R0100: CO0430 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168740 CONTINUOUS STANBYD 1.0 01: JOCKVA 225.13 28.623 No.date 28:07 62.70 708 .000
168750 [M P= 50; TSM= 50]
168760 [LRS= 2; C= 74.0]
168770 [Pervious area: IArea 4.67; SLP(=) 0; LCP= 40; MPM= 250; SC(=) 0]
168780 [Impervious area: IArea 1.57; SLP(=) 0; LCP= 400; MN= 013; SC(=) 0]
168790 [IARECmp= 4.00; IARECPer= 4.00]
168800 [SM N= 36.67; SSM= 24.49; SKE= 010]
168810 R0100: CO0431 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168820 ADD HYD + 1.0 02: JOCKVA 225.13 28.623 No.date 28:07 62.70 n/a .000
168830 + 1.0 02: JOCKVA 225.13 28.623 No.date 28:07 62.70 n/a .000
168840 + 1.0 02: NI 36.820 No.date 28:05 62.88 n/a .000
168850 + 1.0 02: NI 19.402 No.date 28:04 62.88 n/a .000
168860 SLM 31.850 No.date 28:06 62.71 n/a .000
168870 R0100: CO0432 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168880 SAVE HYD 1.0 01: JOCKVA,TO 256.41 31.850 No.date 28:06 62.71 n/a .000
168890 Inam: JOCKVA,TO 0100
168900 remark:Total Flow at KB first pond
168910 *****
168920 # Jockvale SWM Facility
168930 # Rating curve obtained from Jockvale Servicing Study (CC, 1999)
168940 *****
168950 R0100: CO0433 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
168960 ROUTE RESERVOIR -> 1.0 02: JOCKP 256.41 12.850 No.date 28:35 62.71 n/a .000
168970 out <= 1.0 01: JOCKP 256.41 12.850 No.date 28:35 62.71 n/a .000
168980 overflow <= 1.0 03: JO-OW 0.00 0.00 No.date 0:00 60.00 n/a .000
168990 [MKS]obled: 64301001 ob. No.Off= 0. TotDur(Off) 0. hr(s)
169000 R0100: CO0434 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
169010 ADD HYD + 1.0 02: NI,DE 55196.05 146.071 No.date 37:13 39.51 n/a .000
169020 + 1.0 02: DESIRE 23.78 3.004 No.date 28:03 53.11 n/a .000
169030 + 1.0 02: JO-OW 0.00 0.00 No.date 0:00 60.00 n/a .000
169040 + 1.0 02: JOCKP 256.41 12.850 No.date 28:35 62.71 n/a .000
169050 SLM 55476.25 147.027 No.date 37:12 39.63 n/a .000
169060 R0100: CO0435 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
169070 SAVE HYD 1.0 01: SN,NI 55476.25 147.027 No.date 37:12 39.63 n/a .000
169080 Inam: SN,NI 0100
169090 remark:Total Flow at Hart's Desire
169100 *****
169110 # Hydrograph from Hart's Desire routed to Rideau River
169120 # Channel X-Section obtained from RCV4 Hydraulic Model - Station 0
169130 #
169140 R0100: CO0436 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
169150 ROUTE CHANNEL -> 1.0 02: SN,NI 55476.25 147.027 No.date 37:12 39.63 n/a .000
169160 [RDF= 1.00] out <= 1.0 01: NI 55476.25 147.014 No.date 37:15 39.63 n/a .000
169170 [L/S= 563 / 967.045]
169180 [Vmax= 2.19; Dmax= 1.324]
169190 *****
169200 # Catchment S
169210 # - To Jock River (north and south)
169220 # Underlapped floodplain and river
169230 #
169240 R0100: CO0437 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
169250 CONTINUOUS STANBYD 1.0 01: S,2 1.02 94 5.685 No.date 28:20 40.95 142 .000
169260 [C= 72.0; N= 3.00; Tpm= 40]
169270 [IAREC= 4.00; SLM N= 39.75; SSM= 264.99; SKE= 010]
169280 [InterEventTime= 12.00]
169290 R0100: CO0438 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
169300 ADD HYD + 1.0 02: NI 55476.25 147.014 No.date 37:15 39.63 n/a .000
169310 + 1.0 02: S,2 1.02 94 5.685 No.date 28:20 40.95 n/a .000
169320 SLM 55579.19 147.276 No.date 37:15 39.63 n/a .000
169330 R0100: CO0439 ----- Dfm n-1D NMYD ----- AREba-QPEAgcm-TpeakDtte:hh:mm----- Rvmm R.C.-----Dfwm
169340 SAVE HYD 1.0 01: SN,NI 55579.19 147.276 No.date 37:15 39.63 n/a .000
169350 Inam: SN,NI 0100
169360 remark:Total Flow at Rideau River
169370 *****
169380 R0100: CO0002 -----
169390 FIN SH
169400 *****
169410 *****
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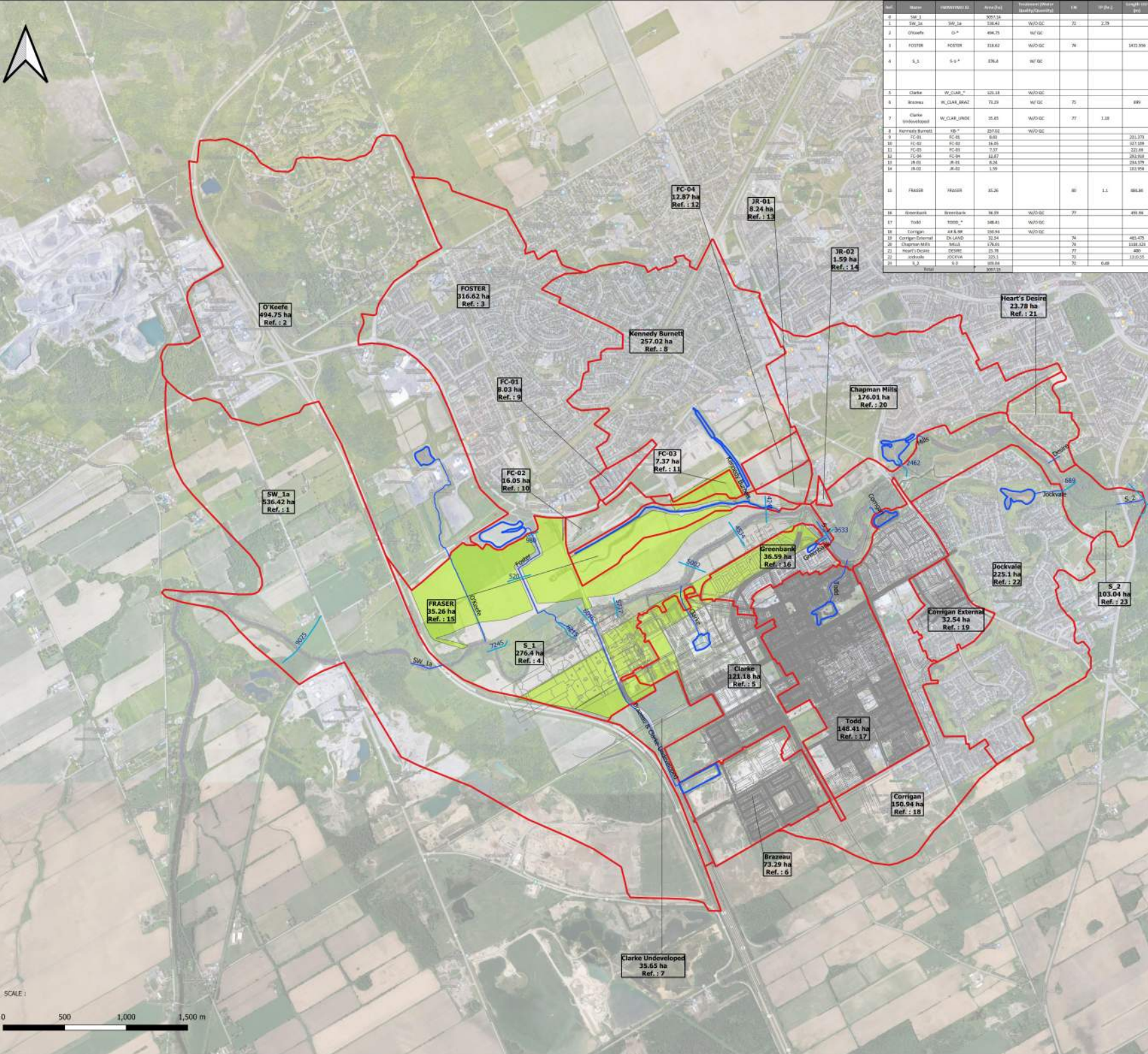
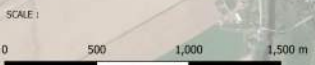
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Attachment F

Updated Subcatchment Schematics & Tables

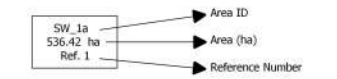


Ref.	Area ID	Area (ha)	Sub-catchment	Area (ha)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)	Flow (m ³ /s)
1	SW_1a	536.42	SW_1a	536.42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	O'Keefe	494.75	O'Keefe	494.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	FOSTER	315.62	FOSTER	315.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	Kennedy Burnett	257.02	Kennedy Burnett	257.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	Clarke	121.8	Clarke	121.8	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
6	Brazeno	73.29	Brazeno	73.29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
7	Clarke Undeveloped	35.65	Clarke Undeveloped	35.65	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
8	Kennedy Burnett	257.02	Kennedy Burnett	257.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
9	FC-01	9.03	FC-01	9.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
10	FC-02	16.05	FC-02	16.05	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
11	FC-03	7.37	FC-03	7.37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
12	FC-04	12.87	FC-04	12.87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
13	JR-01	8.24	JR-01	8.24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
14	JR-02	1.59	JR-02	1.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
15	FRASER	35.26	FRASER	35.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
16	Greenbank	36.59	Greenbank	36.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
17	Todd	148.41	Todd	148.41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
18	Corrigan	150.94	Corrigan	150.94	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
19	Corrigan External	32.54	Corrigan External	32.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
20	Chagman Mills	176.01	Chagman Mills	176.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
21	Heart's Desire	23.79	Heart's Desire	23.79	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
22	Jockvale	225.1	Jockvale	225.1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
23	S_2	103.04	S_2	103.04	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
24	S_1	276.4	S_1	276.4	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25	S_3	103.04	S_3	103.04	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26	Total	3091.15	Total	3091.15	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

- Legend
- Sub-catchments
 - SW_1
 - Channel Cross Section
 - SWM Drains
 - SWM ponds
 - Approved Developments
 - Additional Future Developments
 - Google Hybrid

File name: [Figure 3 - Overall Jock River Lower Reach one Sub-catchments.pdf](#)

[XS 4534](#) Cross Section at station 4534



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www.jfsa.com

DSEL
david schaeffer engineering ltd

PROJECT :
BCDC - Quantity Control Study

TITLE :
Figure 3 - Overall Jock River Lower Reach one Sub-catchments
Table 3 - Overall Jock River Lower Reach one Sub-catchments

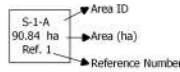
PROJECT NO.	1474-16
DRAWN:	MM
DATE:	Mar. 2021



- Legend**
- Channel Cross Sections
 - S-1 Sub-catchments and Fraser Sub-catchments
 - S-1 Sub-catchments
 - FRASER-DRN
 - FRASER-D
 - Google Hybrid

File name:
Figure 4A - S-1 & Fraser Clarke Sub-catchments.pdf

XS 3633 Cross Section at station 3633



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PROJECT :
BCDC - Quantity Control Study

TITLE :
Figure 4A - S-1 & Fraser Clarke Sub-catchments
Table 4A - S-1 & Fraser Clarke Sub-catchments

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021

Station	Area ID	Area (ha)	Reference	Channel	Channel ID	Channel Name	Channel Type	Channel Material	Channel Slope	Channel Width	Channel Depth	Channel Velocity	Channel Discharge	Channel Capacity	Channel Status
3633	S-1-A	90.84	Ref. 1	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-B	55.36	Ref. 2	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D1	21.67	Ref. 3	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D2	18.67	Ref. 4	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D3	6.79	Ref. 5	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D4	3.28	Ref. 6	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D5	12.84	Ref. 7	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D6	1.75	Ref. 8	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D7	2.03	Ref. 9	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-D8	5.27	Ref. 10	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-FO-D1	5.11	Ref. 11	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-FO-D2	4.94	Ref. 12	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-FO-F-D	14.96	Ref. 13	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-FO-D	21.61	Ref. 16	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	S-1-Okeefe	44.93	Ref. 14	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633
3633	FRASER-DRN	13.65	Ref. 15	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633	3633





Legend

- Channel Cross Sections
- S-1 Sub-catchments and Fraser Sub-catchments
- S-1 Sub-catchments
- FRASER-DRN
- FRASER-D
- Google Hybrid

File name:
Figure 4B - S-1 & Fraser Clarke Sub-catchments.pdf

XS 3633 Cross Section at station 3633

S-1-A	▼ Area ID
90.84 ha	► Area (ha)
Ref. 1	► Reference Number

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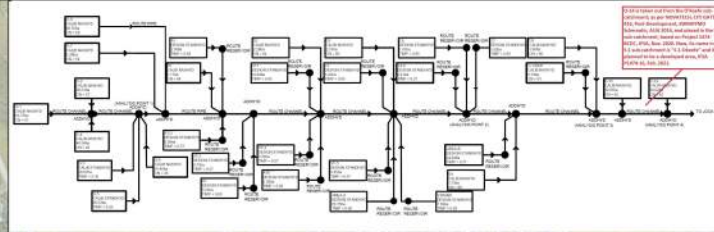
PROJECT :
BCDC - Quantity Control Study

TITLE :
Figure 4B - S-1 & Fraser Clarke Sub-catchments
Table 4B - S-1 & Fraser Clarke Sub-catchments

PROJECT NO.	1474-16
DRAWN:	MM
DATE:	Mar. 2021

Station	Channel	Area (ha)	Perimeter (m)	Length (m)	Flow (m³/s)	Velocity (m/s)	Time (min)	Volume (m³)	Notes
1	S-1-A	75.88	1284	1284	1000	0.5	10	1000	
2	S-1-D1	21.67	2167	2167	1000	0.5	10	1000	
3	S-1-D2	18.67	1867	1867	1000	0.5	10	1000	
4	S-1-D3	6.79	679	679	1000	0.5	10	1000	
5	S-1-D4	3.28	328	328	1000	0.5	10	1000	
6	S-1-D5	12.84	1284	1284	1000	0.5	10	1000	
7	S-1-D6	1.75	175	175	1000	0.5	10	1000	
8	S-1-D7	2.03	203	203	1000	0.5	10	1000	
9	S-1-D8	5.27	527	527	1000	0.5	10	1000	
10	S-1-D9	14.96	1496	1496	1000	0.5	10	1000	
11	S-1-D10	5.11	511	511	1000	0.5	10	1000	
12	S-1-D11	4.94	494	494	1000	0.5	10	1000	
13	S-1-D12	44.93	4493	4493	1000	0.5	10	1000	
14	S-1-D13	21.61	2161	2161	1000	0.5	10	1000	
15	S-1-D14	13.65	1365	1365	1000	0.5	10	1000	





Area ID	Area (ha)	Reference Number	Area ID	Area (ha)	Reference Number
O-1	47.34	1	O-14	5	31
O-2	26.61	2	O-15	11.76	30
O-3	43.68	5	O-16	0.44	16
O-4	43	3	O-17	11.99	15
O-5	64.2	6	O-18	0.72	10
O-6	16.14	4	O-19	3.42	19
O-7	3.67	7	O-20	0.59	20
O-8	48.69	11	O-21	5.69	21
O-9	2.6	9	O-22	68.04	22
O-10	0.99	13	O-23	1.98	23
O-11	3.51	12	O-24	1.78	24
O-12	0.72	10	O-25	0.42	25
O-13	0.99	13	O-26	1.56	26
O-14	5	31	O-27	23.18	27
O-15	11.99	15	O-28	1.94	28
O-16	0.44	16	O-29	8.68	29
O-17	11.99	15	O-30	11.76	30
O-18	0.72	10	O-31	5	31
O-19	3.42	19			
O-20	0.59	20			
O-21	5.69	21			
O-22	68.04	22			
O-23	1.98	23			
O-24	1.78	24			
O-25	0.42	25			
O-26	1.56	26			
O-27	23.18	27			
O-28	1.94	28			
O-29	8.68	29			
O-30	11.76	30			
O-31	5	31			



SCALE :

File name: Figure F1 - O'Keefe Sub-catchments.pdf

Legend

20210129-O'Keefe Sub-catchment Boundaries XS 7245 Cross Section at station 7245

O'Keefe Sub-catchment Boundaries

Google Hybrid

Legend

HYDROLOGIC MODELING (SWHYMO) ANALYSIS POINT ENVIRONMENTAL MANAGEMENT (EMP) ANALYSIS POINT

Area ID

Area (ha)

Reference Number

STA. 1840.6 CROSS SECTION AND STATION

J.F. Sabourin and Associates Inc.
 WATER RESOURCES AND ENVIRONMENTAL CONSULTANTS
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 Ottawa, ON, K2S 1B9 www.jfsa.com



PROJECT : BCDC - Quantity Control Study

TITLE : Figure F1 - O'Keefe Sub-catchments
 Table F1 - O'Keefe Sub-catchments
 Schematic F1 - O'Keefe Sub-catchments

PROJECT NO.	1474
DRAWN:	MM
DATE:	Mar. 2021



Ref.	Water ID	Area (ha)	Major System In	Minor System To	I. Ingress	KIMP	C/N	ICD Length (m) (ICD) (Area*100000.1)	Slope (%)		NWDic	NWDout	RESERVE RESERVOIR		RELIEF CHANNEL (Station (m))						
									SLP (horizontal)	SLP (topographic)			Outflow (m)	Storage (ha m)	NWDin	NWDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)	
1	FOSTER	325.44	FOSTER OUT - ("FOSTER" + "DWP")		0.55	0.05	74	347.94021	0.5	0.5	FOSTER	F_FOS	FO-DWP	18.34	10	SN_FO (Total Flow at Foster Drain)	N_CS	309	0.0818	440.23	93.5
																				392.2	93.2
																				-91	93.5
																				-85.52	93.5
																				-23.48	89.4
																				-9.79	88.11
																				-3.22	86.24
																				1.22	85.07
																				10.96	82.79
																				16.44	86.49
																				26.55	88.45
																				29.93	90.27
																				35.76	90.67
																				36.87	91
																				108.08	91
																				189.82	90.5
																				112.04	90.5
																				114.62	91
																				116.78	93.5



FOSTER
325.44 ha
Ref. 1

File name:
Figure F2 - Foster Sub-catchment.pdf

Legend

- Channel Cross Sections
- SWMF Drains
- Foster Drain
- SWMF ponds
- Foster Pond
- Foster Sub-catchment
- FOSTER
- Google Hybrid

XS 6016 Cross Section at station 6016

FOSTER 325.44 ha Area ID
Ref. 1 Area (ha)
Reference Number

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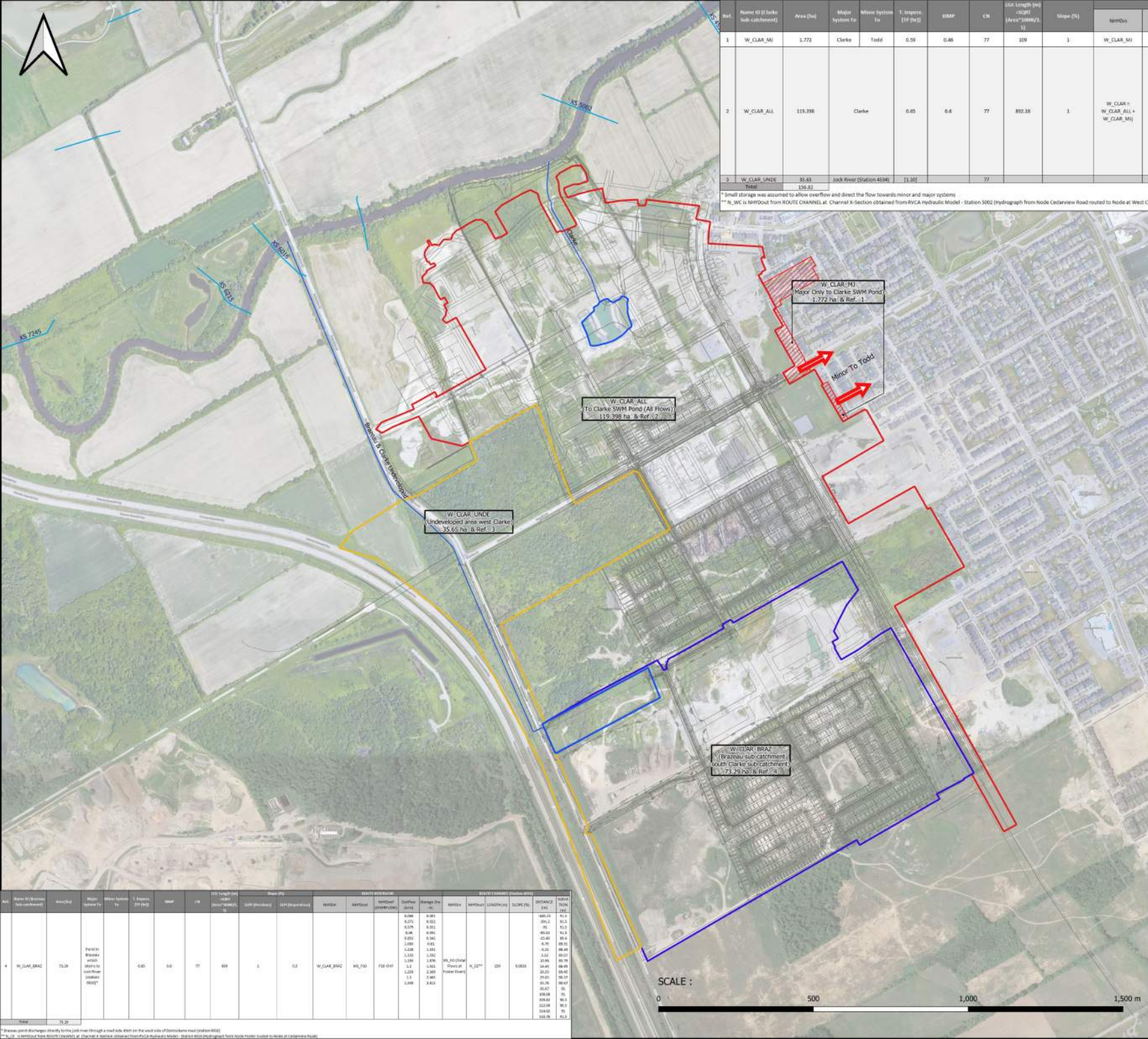
DSEL
david schaeffer engineering ltd

PROJECT :
BCDC - Quantity Control Study

TITLE :
Figure F2 - Foster Sub-catchment
Table F2 - Foster Sub-catchment

PROJECT NO.	1474-16
DRAWN:	MM
DATE:	Mar. 2021

Ref.	Name ID	Area (ha)	Major System To	Minor System To	T. Imperv.	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)		ROUTE RESERVOIR					ROUTE CHANNEL (Station 6016)					
									SLPP (Pervious)	SLPI (Impervious)	NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)
1	FOSTER	325.44	FOSTER-OUT = ["P_FOS"+"FO-OVF"]		0.55	0.55	74	1472.956211	0.5	0.5	FOSTER	P_FOS	FO-OVF	10.34	10	SN_FO (Total Flows at Foster Drain)	N_CE	159	0.0818	-645.23	91.5
																				-391.2	91.5
																				-91	91.5
																				-85.52	91.5
																				-15.46	89.4
																				-9.79	89.31
																				-3.22	86.24
																				3.22	85.07
																				10.96	85.79
																				16.44	86.49
																				26.55	89.45
																				29.03	90.27
																				35.76	90.67
																				36.67	91
																				108.08	91
																				109.82	90.5
																				112.04	90.5
																				114.62	91
																				116.76	91.5



Ref.	Route ID (Linker Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. FIP (%)	XIMP	C.R.	Slope (%)	ROUTE RESERVOIR			ROUTE CHANNEL (Station #346)												
									WYIDin	WYIDout	WYIDout (OVERFLOW)	WYIDin	WYIDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)							
1	W_CLEAR_MI	1.772	Clarke	Todd	0.50	0.80	77	100																
2	W_CLEAR_ALL	119.398	Clarke		0.05	0.4	77	892.18	1	W_CLEAR = W_CLEAR_MI	MS_P2	P2-OVF												
3	W_CLEAR_UNDE	35.65	Jock River (Station 4534)		1.30		77																	

* Small storage was assumed to allow overflow and direct the flow towards minor and major systems
 ** N_WC is AFFIDout from ROUTE CHANNEL at Channel X-Section obtained from R/WCA Hydraulic Model - Station 5062 (Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain)

Legend

- Channel Cross Sections
- SWM Drains
 - Brazeau & Clarke Undeveloped
 - Clarke
- SWM ponds
 - West Clarke
 - Brazeau
- Clarke Sub-catchment & Brazeau Sub-catchment
 - W_CLEAR_MI
 - W_CLEAR_MJ (Major Only to Clarke SWM Pond)
 - W_CLEAR_ALL
 - W_CLEAR_BRAZ
 - W_CLEAR_UNDE
 - Clarke-Brazeau-CAD
 - Google Hybrid

File name: Figure F3 - Clarke & Brazeau Sub-catchments.pdf

XS 4534 Cross Section at station 4534 ➔ Minor System

W_CLEAR_MJ (Major Only to Clarke SWM Pond) 1.772 ha & Ref. 1

Area ID ➔

Reference Number ➔

Area (ha) ➔

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PROJECT : BCDC - Quantity Control Study

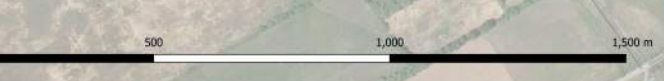
TITLE : Figure F3 - Clarke & Brazeau Sub-catchments
 Table F3-1 - Clarke Sub-catchments
 Figure F3-2 - Brazeau Sub-catchment

PROJECT NO. 1474-16

DRAWN: MM

DATE: Mar. 2021

Area	Route ID (Linker Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. FIP (%)	XIMP	C.R.	Slope (%)	Distance (m)	Elevation (m)	ROUTE RESERVOIR		ROUTE CHANNEL (Station #346)											
											WYIDin	WYIDout	WYIDout (OVERFLOW)	WYIDin	WYIDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)					
4	W_CLEAR_BRAZ	73.29	Clarke		0.05	0.8	77	100																



Ref.	Name ID (Clarke Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. [TP (hr)]	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)	ROUTE RESERVOIR					ROUTE CHANNEL (Station 4534)																	
										NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)												
1	W_CLAR_MJ	1.772	Clarke	Todd	0.59	0.46	77	109	1	W_CLAR_MJ	W_CLAR_MJn	W_CLAR_MJj	0.213	0.0001*																		
2	W_CLAR_ALL	119.398	Clarke		0.65	0.6	77	892.18	1	W_CLAR = W_CLAR_ALL + W_CLAR_MJj	MS_P2	P2-OVF	0.128	0.161																		
													0.138	0.409																		
													0.148	0.68																		
													0.227	0.931																		
													0.354	1.223																		
													0.505	1.52																		
													0.666	1.821																		
													0.831	2.123																		
													0.995	2.434																		
													1.069	2.583																		
													1.51	2.647																		
													4.904	2.861																		
													13.048	3.188																		
													23.745	3.523																		
36.474	3.871																															
45.938	4.127																															
61.652	4.539																															
3	W_CLAR_UNDE	35.65	Jock River (Station		[1.10]		77																									
Total		156.82																														

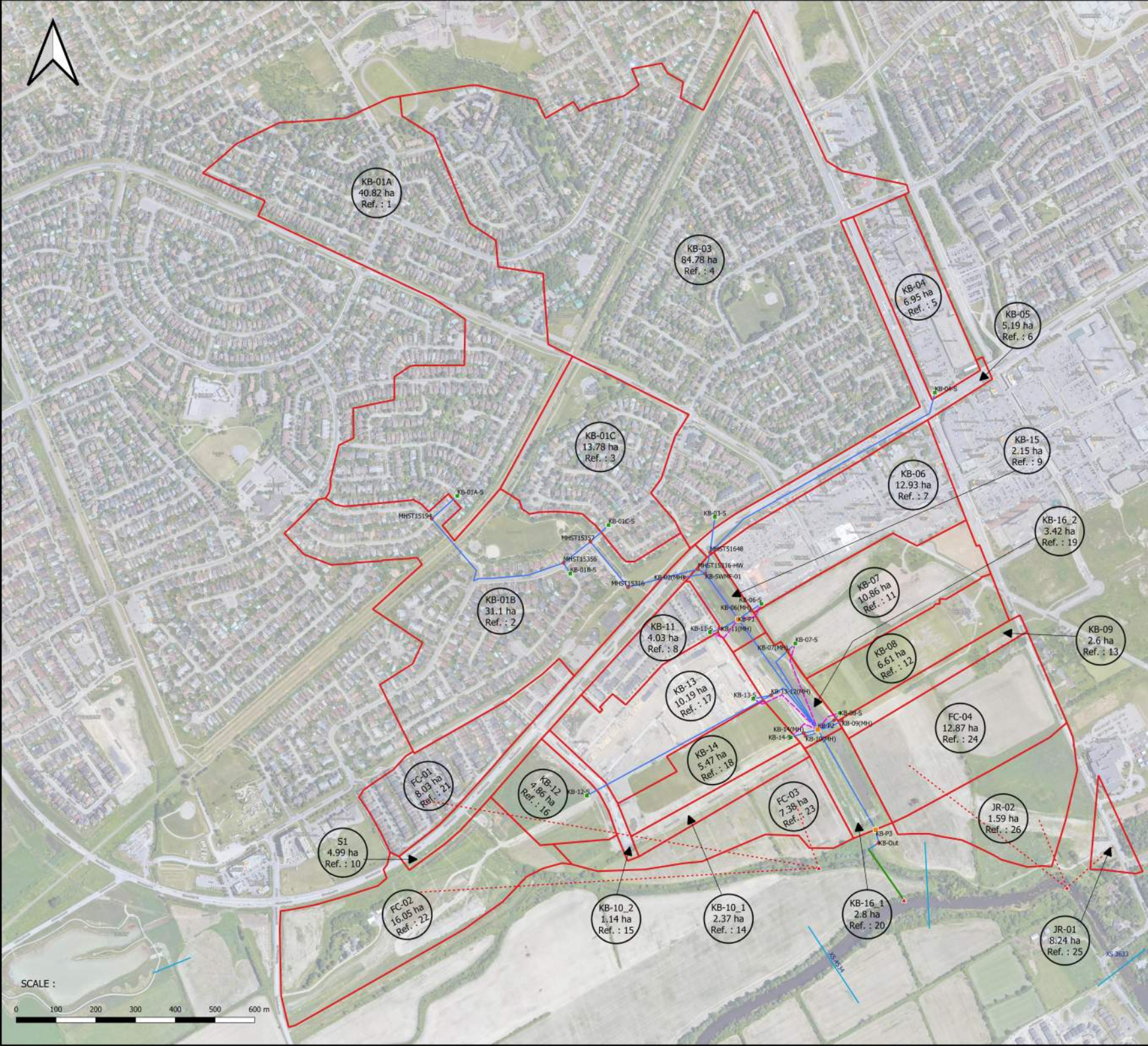
* Small storage was assumed to allow overflow and direct the flow towards minor and major systems

** N_WC is NHYDout from ROUTE CHANNEL at Channel X-Section obtained from RVCA Hydraulic Model - Station 5002 (Hydrograph from Node Cedarview Road routed to Node at West Clarke Drain)

Ref.	ID (Brazeau Sub-catchment)	Area (ha)	Major System To	Minor System To	T. Imperv. [TP (hr)]	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)		ROUTE RESERVOIR					ROUTE CHANNEL (Station 6016)					
									SLPP (Pervious)	SLPI (Impervious)	NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)
4	W_CLAR_BRAZ	73.29	Pond in Brazeau which drains to Jock River (station 6016)*		0.65	0.6	77	699	1	0.5	W_CLAR_BRAZ	MS_P10	P10-OVF	0.068	0.001	SN_FO (Total Flows at Foster Drain)	N_CE**	159	0.0818	-645.23	91.5
														0.271	0.022					-391.2	91.5
														0.379	0.051					-91	91.5
														0.48	0.091					-85.52	91.5
														0.853	0.341					-15.46	89.4
														1.005	0.61					-9.79	89.31
														1.128	1.231					-3.22	86.24
														1.155	1.592					3.22	85.07
														1.194	1.876					10.96	85.79
														1.2	1.921					16.44	86.49
														1.259	2.369					26.55	89.45
														1.3	2.665					29.03	90.27
														1.349	2.813					35.76	90.67
																				36.67	91
																				108.08	91
		109.82	90.5																		
		112.04	90.5																		
		114.62	91																		
		116.76	91.5																		
Total		73.29																			

* Brazeau pond discharges directly to the jock river through a road side ditch on the west side of Borrissokane road (station 6016)

** N_CE is NHYDout from ROUTE CHANNEL at Channel X-Section obtained from RVCA Hydraulic Model - Station 6016 (Hydrograph from Node Foster routed to Node at Cedarview Road)



Area ID	Area (ha)	Reference Number
KB-01A	40.82	1
KB-01B	31.1	2
KB-01C	13.78	3
KB-03	84.78	4
KB-04	6.95	5
KB-05	5.19	6
KB-06	12.93	7
KB-07	10.86	11
KB-08	6.61	12
KB-09	2.6	13
KB-10_1	2.37	14
KB-10_2	1.14	15
KB-11	4.03	8
KB-12	4.86	16
KB-13	10.19	17
KB-14	5.47	18
KB-15	2.15	9
KB-16_1	2.8	20
KB-16_2	3.42	19
FC-01	8.03	21
FC-02	16.05	22
FC-03	7.36	23
FC-04	12.87	24
JR-01	8.24	25
JR-02	1.59	26
S1	4.99	10

- Legend**
- Ken-BU storage
 - Ken-BU MH
 - ▲ Ken-BU Pond
 - ▲ Ken-BU Outfalls
 - Ken-BU Pipe
 - Ken-BU Major System
 - Ken-BU Channel
 - Ken-BU to FC (Fraser Clarke) and JR (Jock River)
 - Kennedy Burnett Sub-catchment
 - Kennedy Burnett (Ken-BU)
 - Google Hybrid

File name: Figure F4 - Kennedy Burnett Sub-catchments.pdf



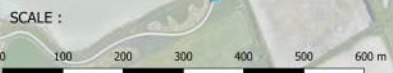
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PROJECT :
 BCDC - Quantity Control Study

TITLE :
 Figure F4 - Kennedy Burnett Sub-catchments
 Table F4 - Kennedy Burnett Sub-catchments

PROJECT NO.	1474
DRAWN:	MM
DATE:	Mar. 2021



Ref.	Name ID (Todd)	Area (ha)	Major System To	Minor System To	T. Imperv.	XIMP	CN	LGI: Length (m) =SQRT (Area*10000/1.5)	Slope (%)	ROUTE RESERVOIR					ROUTE CHANNEL (Station 2462)					
										NHYDin	NHYDout	NHYDovf (OVERFLOW)	Outflow (cms)	Storage (ha-m)	NHYDin	NHYDout	LENGTH (m)	SLOPE (%)	DISTANCE (m)	ELEVATION (m)
1	TODD_MN2	2.1	Jock River (Station 2462)	Todd	0.57	0.53	77	118.322	1	TODD_MN2	TODD_MN2n	TODD_MN2j	0.268	0.0001*						
2	TODD_MN3	0.117	Corrigan	Todd	0.57	0.53	77	27.928	1	TODD_MN3	TODD_MN3n	TODD_MN3j	0.016	0.0001*						
3	TODD_MJ	30.23	Todd	Corrigan	0.64	0.52	77	448.925	1	TODD_MJ	TODD_MJn	TODD_MJj	3.314	0.0001*						
4	TODD_ALL	112.908	Todd	Todd	0.57	0.52	77	867.594	1											
5	TODD_P	3.055	Todd	Todd	0.63	0.63	77	142.712	1	"TODD" = ["TODD_MN2n"+ "TODD_MN3n"+ "TODD_MJj"+ "TODD_P"+ "TODD_ALL"+ "W_CLAR_MJn"***]	MS_P3	P3-OVF	0.014 0.048 0.061 0.08 0.088 0.109 0.118 0.122 1.972 9.135 15.608 19.256 27.282 40.957 56.372 73.349 85.469 104.771	0.155 0.394 0.56 0.909 1.089 1.652 1.952 2.099 2.269 2.598 2.826 2.942 3.181 3.55 3.929 4.317 4.579 4.977	SN_TO = ["GreenB" (Greenbank Pond)+ "MS_P3"+"P3-OVF"+ "TODD_MN2j"+ "A2-MJ"***]	N_TO	280	0.033	-83.32 -81.36 -79.12 -76.13 -20.46 -19.36 -18.51 -17.72 -11.95 -0.11 11.49 17.72 19.74 21.22 22.68 24.28 26.79 71.98 80.62	90 89.5 89 88.5 88 87.5 87 86.5 85.24 85.12 85.2 86.5 87 87.5 88 88.5 89 90 90.5
Total		148.41																		

* Small storage was assumed to allow overflow and direct the flow towards minor and major systems

** "W_CLAR_MJn" is the minor system from the major system area(Area = 1.772 ha & TIMP = 0.59 & XIMP = 0.46 & CN = 75 & Slope = 1% & Outflow = 0.213 cms)in Clarke sub-catchment (Schematic 3) to Jock River (Station 2462)

*** "A2-MJ" is the major system from A2 area (Area = 25.5 ha & TIMP = 0.52 & XIMP = 0.42 & CN = 75 & Slope = 1% & Storage = 924 cu-m & Flow rate = 1.818 cms) in Corrigan sub-catchment (Schematic 1) to Todd sub-catchment



SCALE :



Area ID	Area (ha)	Reference Number	Notes
A1	15.75	1	
A2	25.5	6	
A3	1.27	5	
A4	1.27	5	
A5	1.6	7	
A6	1.56	8	
A8	0.96	13	
A10	4.14	18	
A12	12.29	20	
A11	6.27	19	
A11	4.34	19	
A13	2.59	21	
A9	2.44	17	
B1	2.77	4	
B2	12.31	9	
B3	5.59	10	
B4	7.6	11	
B6	3.29	15	
B7	7.19	14	
corr1	15.87	2	
corr2	12.47	3	
corr	2.94	22	
corr-external	32.25	16	



File name:
Figure F6 - Corrigan Sub-catchments.pdf

Major System Overflow
Legend
 Channel Cross Section
 Corrigan-Pipe Line
 Corrigan-MH
 Corrigan Drainage Boundaries
 Corrigan Drainage Boundaries
 XS 0 Cross Section at station 0
 Area ID
 Area (ha)
 Reference Number

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 david schaeffer engineering ltd

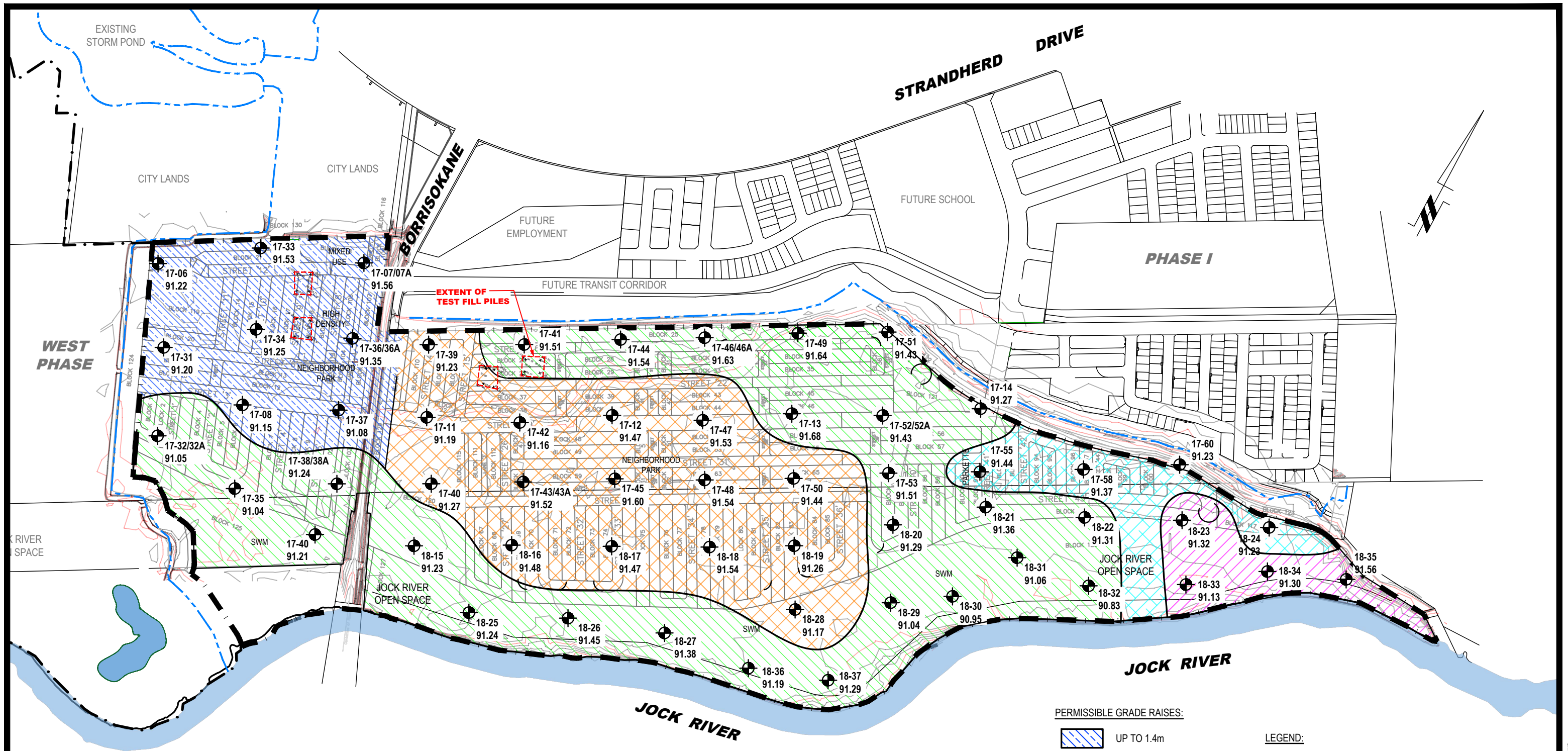
PROJECT :
BCDC - Quantity Control Study

TITLE :
Figure F6 - Corrigan Sub-catchments
Table F6 - Corrigan Sub-catchments




PROJECT NO.	1474
DRAWN:	MM
DATE:	Mar. 2021

APPENDIX E


GEO TECHNICAL



PERMISSIBLE GRADE RAISES:

-  UP TO 1.4m
-  UP TO 1.6m
-  UP TO 1.8m
-  UP TO 2.0m
-  UP TO 2.2m

LEGEND:

-  APPROXIMATE BOREHOLE LOCATION (GOLDER ASSOCIATES, 04/2019)
- 91.56 GROUND SURFACE ELEVATION (m)
- BOREHOLE LOCATIONS WERE SURVEYED BY OTHERS AND ARE REFERENCED TO A GEODETIC DATUM.

patersongroup
consulting engineers

154 Colonnade Road South
Ottawa, Ontario K2E 7J5
Tel: (613) 226-7381 Fax: (613) 226-6344

NO.	REVISIONS	DATE	INITIAL
2	REVISED CONCEPTUAL PLAN, UPDATED GRADE RAISE RESTRICTIONS	04/02/2021	OC
1	REVISED CONCEPTUAL PLAN, ADDED TEST FILL PILE INFO AND UPDATED GRADE RAISE RESTRICTIONS	14/05/2020	DJG

CAIVAN COMMUNITIES
GEOTECHNICAL INVESTIGATION
PROP. RESIDENTIAL DEVELOPMENT - CONSERVANCY LANDS EAST
OTTAWA, ONTARIO

Title: **PERMISSIBLE GRADE RAISE PLAN**

Scale:	1:6000	Date:	09/2019
Drawn by:	MPG	Report No.:	PG5036-1
Checked by:	OC	PG5036-2	Revision No.: 2
Approved by:	DJG		

re: Road Grade Exceedance Review
Proposed Residential Development - Conservancy Lands
Borrisokane Road - Ottawa

to: Caivan Communities - **Mr. Hugo Lalonde** - hugo.lalonde@caivan.com
David Schaeffer Engineering Ltd. - **Mr. Kevin Murphy** - KMurphy@dsel.ca

date: March 8, 2021

file: PG5036-MEMO.10

Paterson Group (Paterson) prepared the following memo to provide a geotechnical review of permissible grade raise exceedances of the proposed roadway grading at various locations throughout the aforementioned development. This memorandum should be read in conjunction with Paterson Group Report PG5036-1 Revision 1 dated February 3, 2021.

The following drawings prepared by David Schaeffer Engineering Ltd. were reviewed from a geotechnical perspective:

- Barrhaven Conservancy - Maximum Grade Raise Exceedance - Project No. 16-891 - Drawing No. 6 and 7 - dated March 2021
- Latest road grading AutoCAD file - 891_Grad_PS_Lowered_Mar5-21

Based on the available drawings, it is understood that minor permissible grade raise exceedances occur within the proposed roadway grades at various locations throughout the subject development.

Geotechnical Review

Based on our detailed review, the grading exceedances are considered acceptable from a geotechnical perspective and lightweight fill will not be required within the City of Ottawa right-of-way(s).

We trust that this information satisfies your immediate requirements.

Paterson Group Inc.



Owen Canton, E.I.T.



David J. Gilbert, P.Eng.

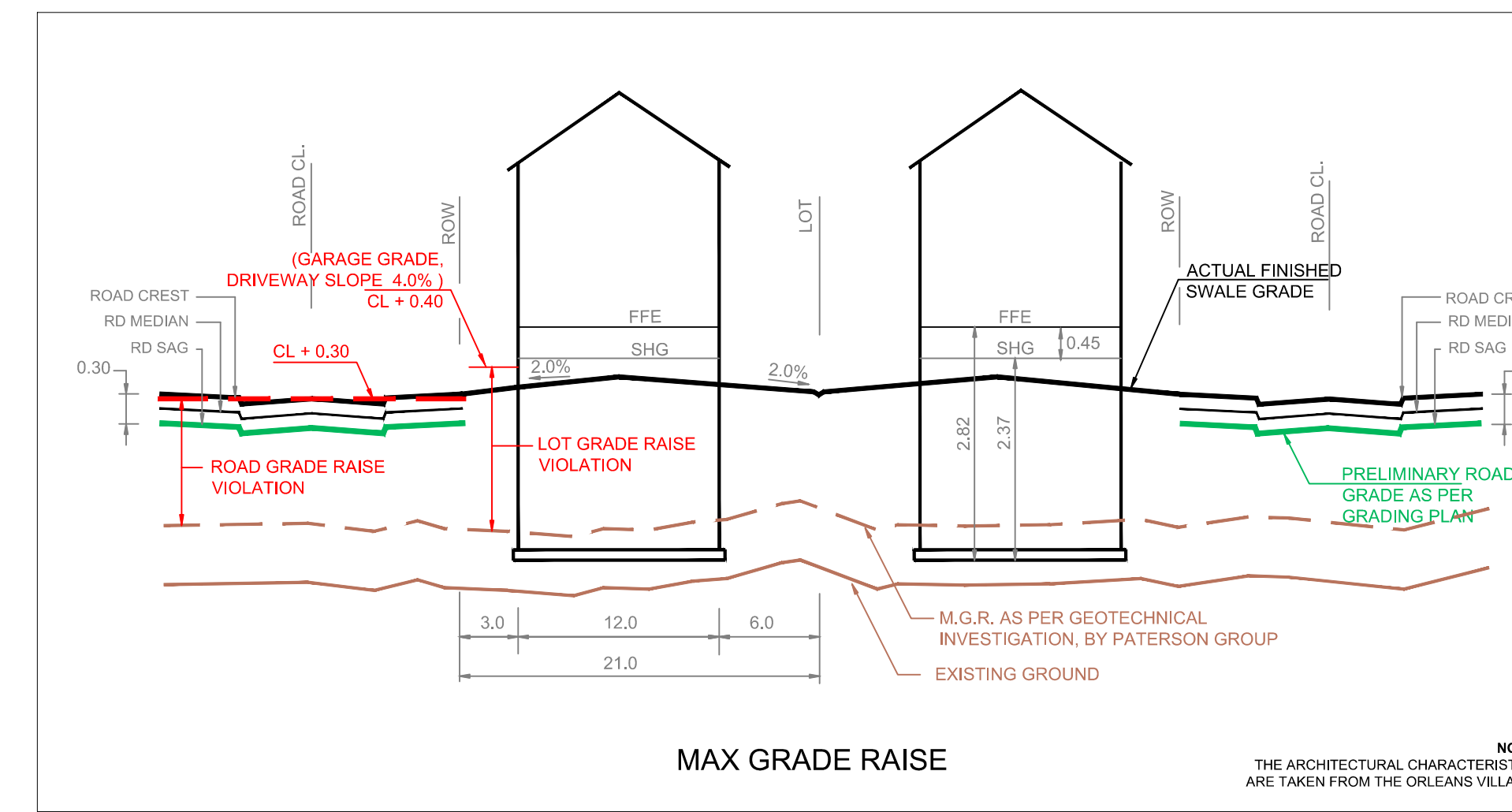
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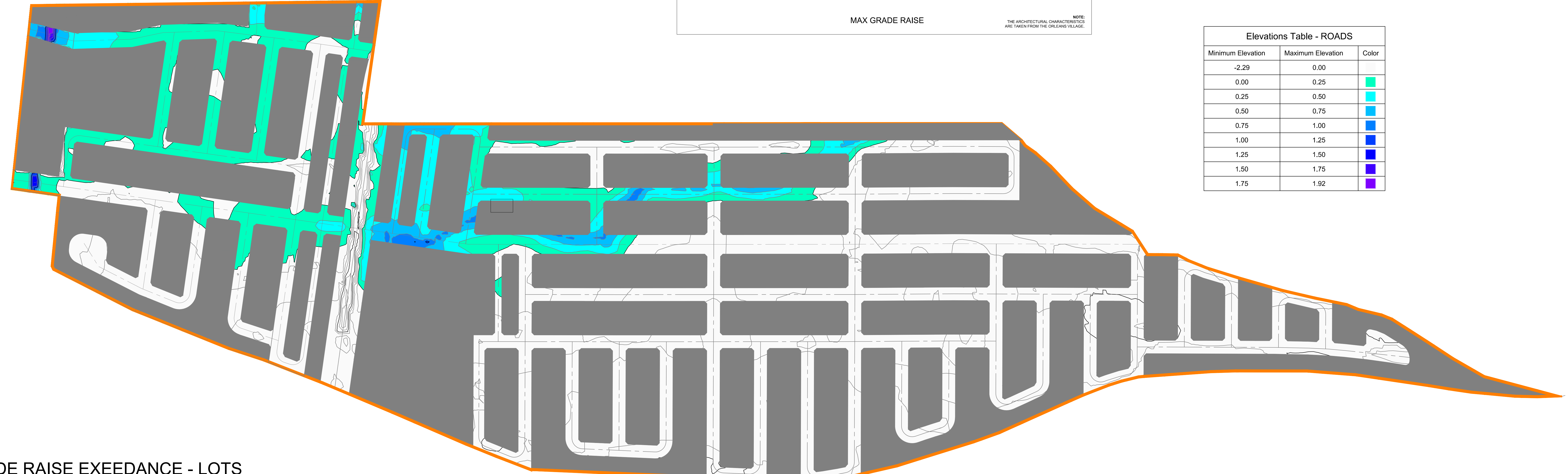
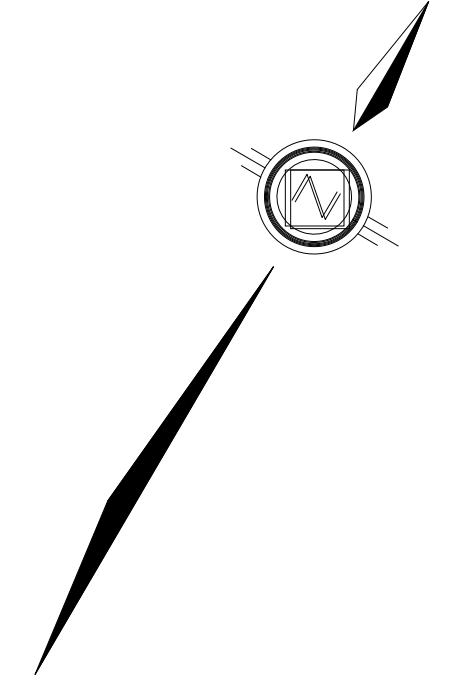
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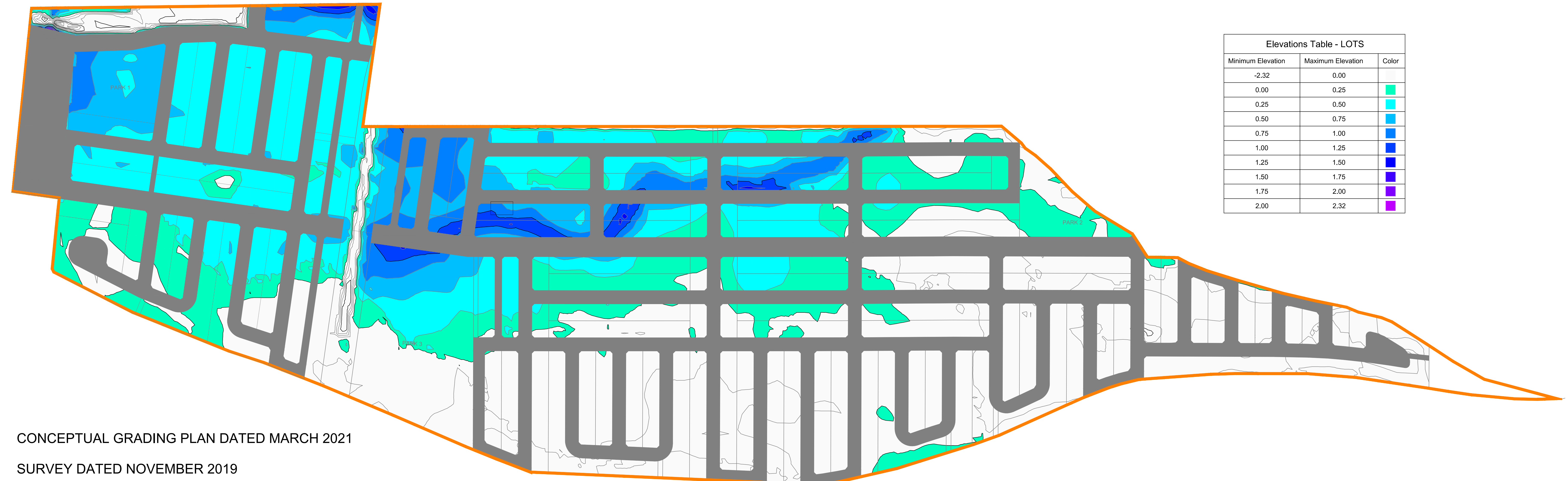
MAXIMUM GRADE RAISE EXCEEDANCE - ROADS



NOTES:
 1. LOW FLOW CHANNEL ELEVATION TO BE VERIFIED IN THE FIELD AT ALL ROAD AND SEWER CROSSING LOCATIONS.
 2. DETAILED DESIGN ROAD GRADES REPRESENT LOW POINTS. ACTUAL DESIGN GRADES WILL RISE AND FALL BY APPROXIMATELY 0.3m ABOVE THE TROUGH/SAG GRADES PRESENTED ON THIS PLAN.
 3. EARTHWORKS VOLUMES ARE BASED ON THE MEDIAN ROAD GRADES, WHICH ARE ESTIMATED TO BE 0.09m HIGHER THAN THE TROUGH/SAG GRADES PRESENTED ON THIS PLAN.
 EARTHWORKS DEPTHS AND VOLUMES ARE APPROXIMATE BASED ON USING ASSUMED MEDIAN VALUES FOR THE MANY VARIABLES THAT AFFECT EARTHWORKS. WHILE THE MEDIAN DEPTH AND VOLUME ARE GENERALLY REFLECTIVE OF THE SITE CHARACTERISTICS, THE ACTUAL DEPTH AND VOLUME AT ANY PRECISE LOCATION WILL VARY (UP OR DOWN) BASED ON THE LOCATION OF HIGH POINTS AND LOW POINTS IN THE ROAD AND ON THE LOTS.
 THE SITE GRADING PLAN HAS BEEN PREPARED IN GENERAL CONFORMANCE WITH CITY OF OTTAWA DESIGN CRITERIA AND HISTORIC PRACTICES. THE GRADING PLAN HAS NOT BEEN REVIEWED BY CITY OF OTTAWA AND IS SUBJECT TO CHANGE FOLLOWING CITY REVIEW. ANY CHANGE IN THE GRADING PLAN WILL HAVE A CORRESPONDING CHANGE IN EARTHWORKS.

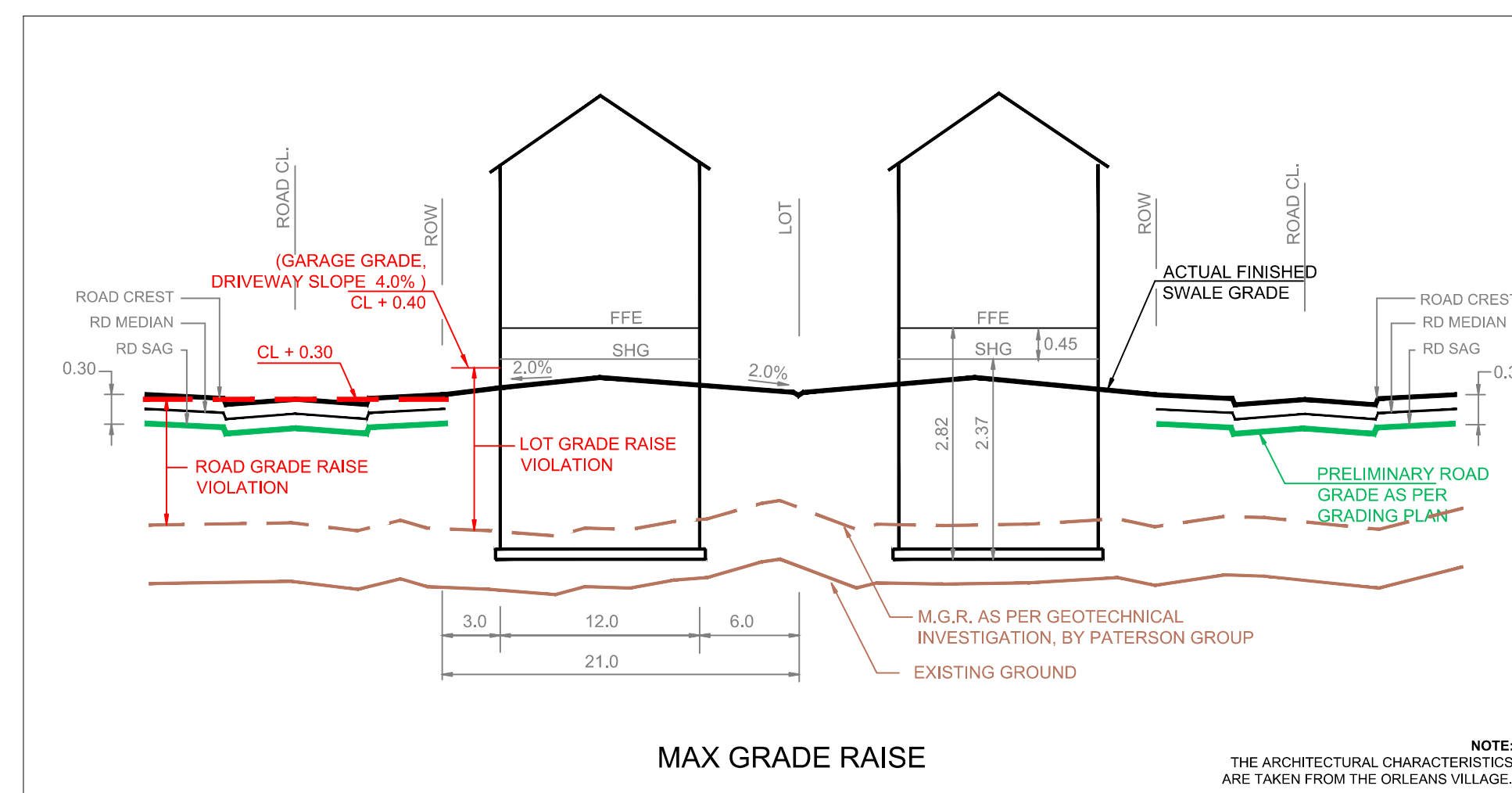
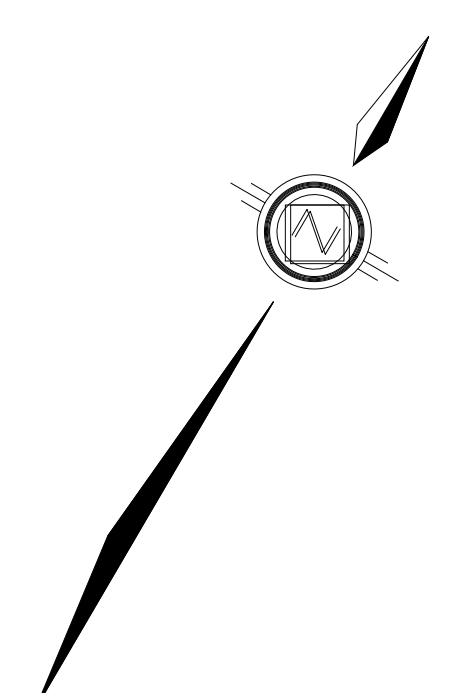


MAXIMUM GRADE RAISE EXCEEDANCE - LOTS

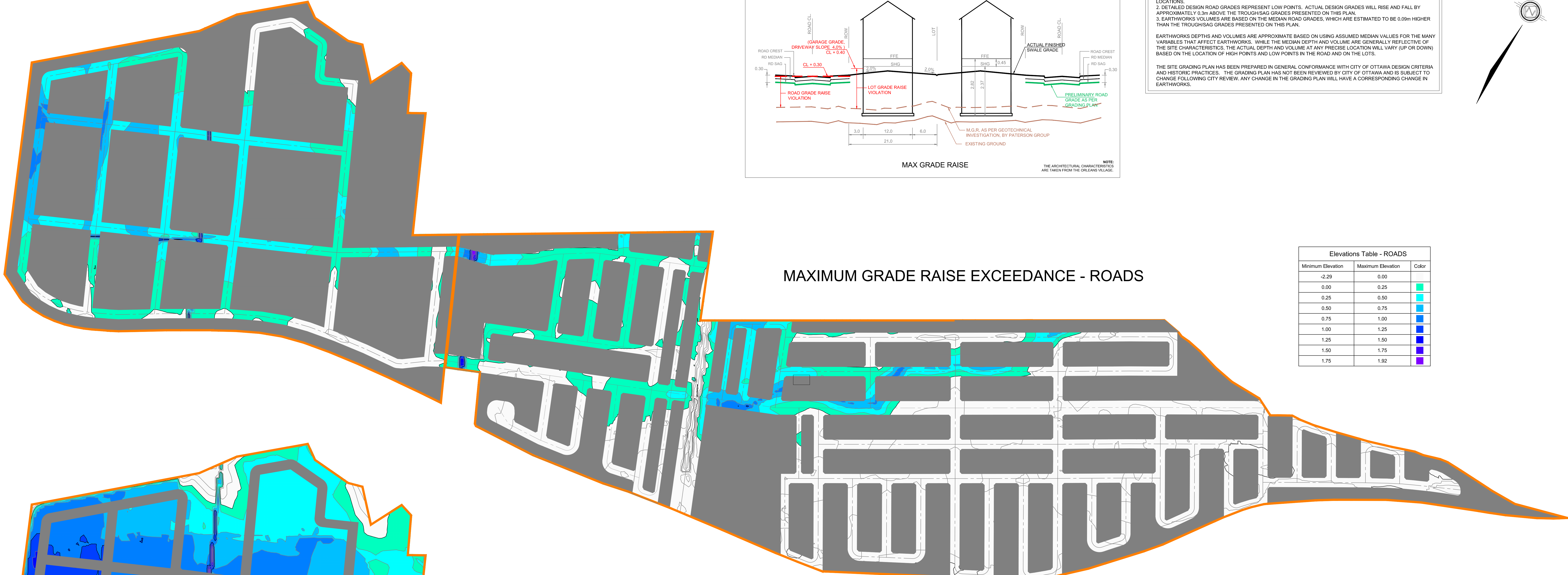


Minimum Elevation	Maximum Elevation	Color
-2.32	0.00	Green
0.00	0.25	Light Green
0.25	0.50	Yellow
0.50	0.75	Orange
0.75	1.00	Red
1.00	1.25	Dark Red
1.25	1.50	Purple
1.50	1.75	Dark Purple
1.75	2.00	Black
2.00	2.32	Black

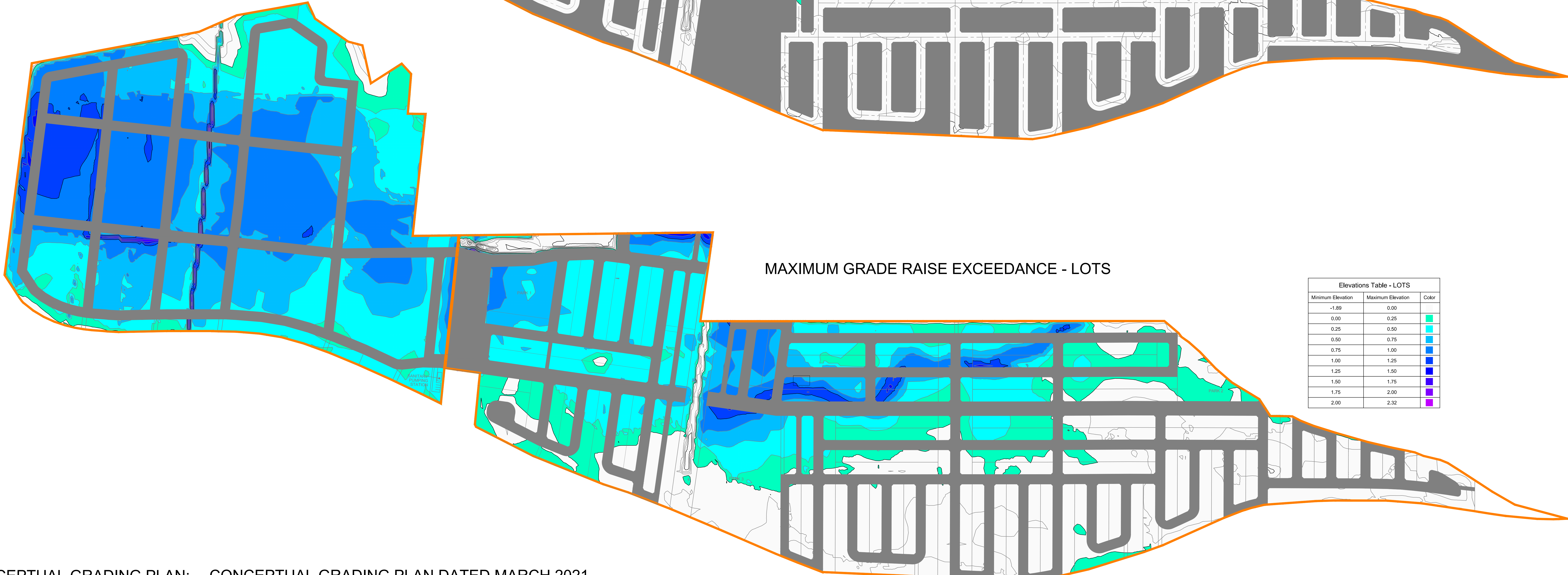
CONCEPTUAL GRADING PLAN: CONCEPTUAL GRADING PLAN DATED MARCH 2021
 ORIGINAL GROUND: SURVEY DATED NOVEMBER 2019
 MAXIMUM GRADE RAISE AS PER : GEOTECHNICAL INVESTIGATION, BY PATERSON GROUP DATED SEPTEMBER 27, 2019 - REVISED MAY 15, 2020



NOTES:
 1. LOW FLOW CHANNEL ELEVATION TO BE VERIFIED IN THE FIELD AT ALL ROAD AND SEWER CROSSING LOCATIONS.
 2. DETAILED DESIGN ROAD GRADES REPRESENT LOW POINTS. ACTUAL DESIGN GRADES WILL RISE AND FALL BY APPROXIMATELY 0.3m ABOVE THE TROUGH/SAG GRADES PRESENTED ON THIS PLAN.
 3. EARTHWORKS VOLUMES ARE BASED ON THE MEDIAN ROAD GRADES, WHICH ARE ESTIMATED TO BE 0.09m HIGHER THAN THE TROUGH/SAG GRADES PRESENTED ON THIS PLAN.
 EARTHWORKS DEPTHS AND VOLUMES ARE APPROXIMATE BASED ON USING ASSUMED MEDIAN VALUES FOR THE MANY VARIABLES THAT AFFECT EARTHWORKS. WHILE THE MEDIAN DEPTH AND VOLUME ARE GENERALLY REFLECTIVE OF THE SITE CHARACTERISTICS, THE ACTUAL DEPTH AND VOLUME AT ANY PRECISE LOCATION WILL VARY (UP OR DOWN) BASED ON THE LOCATION OF HIGH POINTS AND LOW POINTS IN THE ROAD AND ON THE LOTS.
 THE SITE GRADING PLAN HAS BEEN PREPARED IN GENERAL CONFORMANCE WITH CITY OF OTTAWA DESIGN CRITERIA AND HISTORIC PRACTICES. THE GRADING PLAN HAS NOT BEEN REVIEWED BY CITY OF OTTAWA AND IS SUBJECT TO CHANGE FOLLOWING CITY REVIEW. ANY CHANGE IN THE GRADING PLAN WILL HAVE A CORRESPONDING CHANGE IN EARTHWORKS.

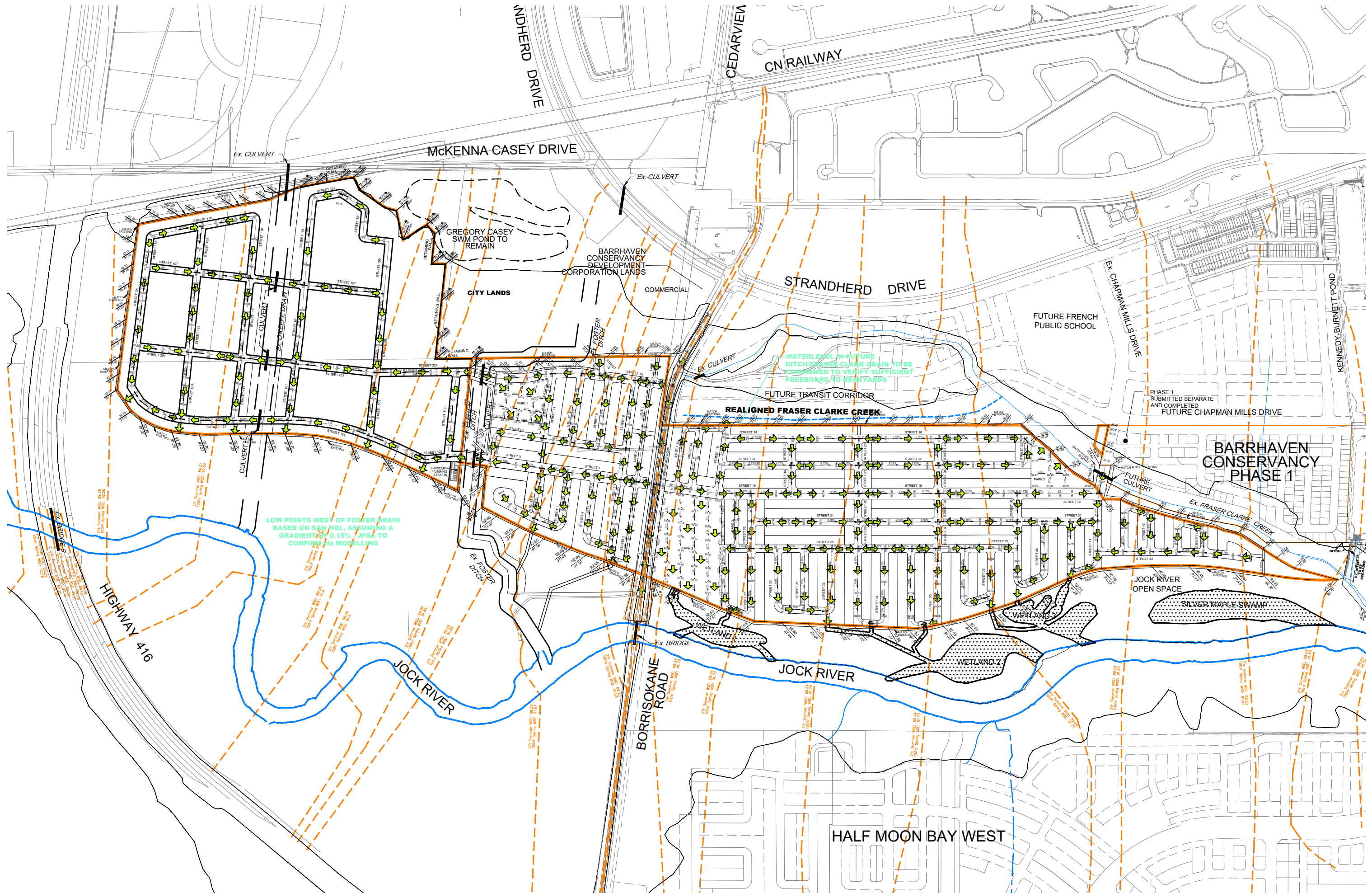


Minimum Elevation	Maximum Elevation	Color
-2.29	0.00	Light Green
0.00	0.25	Light Blue
0.25	0.50	Medium Blue
0.50	0.75	Dark Blue
0.75	1.00	Very Dark Blue
1.00	1.25	Dark Purple
1.25	1.50	Medium Purple
1.50	1.75	Light Purple
1.75	1.92	Dark Purple



Minimum Elevation	Maximum Elevation	Color
-1.89	0.00	Light Green
0.00	0.25	Light Blue
0.25	0.50	Medium Blue
0.50	0.75	Dark Blue
0.75	1.00	Very Dark Blue
1.00	1.25	Dark Purple
1.25	1.50	Medium Purple
1.50	1.75	Light Purple
1.75	2.00	Dark Purple
2.00	2.32	Dark Purple

CONCEPTUAL GRADING PLAN: CONCEPTUAL GRADING PLAN DATED MARCH 2021
 ORIGINAL GROUND: SURVEY DATED NOVEMBER 2019
 MAXIMUM GRADE RAISE AS PER : GEOTECHNICAL INVESTIGATION, BY PATERSON GROUP
 DATED SEPTEMBER 27, 2019 - REVISED MAY 15, 2020



McKENNA CASEY DRIVE

CEDARVIEW
CN RAILWAY

GREGORY CASEY
SWM POND TO
REMAIN

BARRHAVEN
CONSERVANCY
DEVELOPMENT
CORPORATION LANDS

STRANDHERD DRIVE

FUTURE FRENCH
PUBLIC SCHOOL

Ex. CHAPMAN MILLS DRIVE

KENNEDY-BURNETT POND

CITY LANDS

COMMERCIAL

REALIGNED FRASER CLARKE CREEK

PHASE 1
SUBMITTED SEPARATE
AND COMPLETED
FUTURE CHAPMAN MILLS DRIVE

BARRHAVEN
CONSERVANCY
PHASE 1

LOW POINTS WEST OF FOSTER DRAIN
BASED ON SAN HGL, ASSUMING A
GRADIENT OF 0.15% - JFSA TO
CONFIRM via MODELLING

FUTURE CULVERT

Ex. FRASER CLARKE CREEK

JOCK RIVER
OPEN SPACE

SILVER MAPLE SWAMP

HIGHWAY 416

JOCK RIVER

BORRISOKANE ROAD

JOCK RIVER

WETLAND 1

WETLAND 2

HALF MOON BAY WEST