PETRIES LANDING III
Functional Servicing and Stormwater Management

March 6, 2024

Prepared for:
11034936 Canada Inc.

Prepared by:
Stantec Consulting Ltd.

Stantec Project Number:
160401751

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Prepared by: Michael Wu

Signature

Michael Wu, E.I.T.

Printed Name

Reviewed by:

Signature

Rob Brandrick, P.Eng.

Printed Name

LICENSED PROFESSIONAL ENGINEER
R. J. B. BRANDRICK
100570025
March 6 2024
PROVINCE OF ONTARIO

Approved by:

Signature

Kris Kilborn

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1.0 Introduction

1.1 Project Information

This report is prepared to demonstrate the Functional Servicing and Stormwater Management in support of a Draft Plan of Subdivision application for the proposed development located at 8600 Jeanne d'Arc Boulevard N. in the City of Ottawa. The site is 10.43 ha in size and located in the Chatelaine Village neighbourhood of the City of Ottawa.

The site location is illustrated in **Figure 1.1** below.

Figure 1.1: Key Plan of Site

Current zoning is O1 and DR. The site is currently vacant and is farmed. The site is bound by Jeanne D'Arc Boulevard N. to the north, institutional development to the east, Regional Road 174 to the south, and residential development to the east.

A copy of the proposed Concept Plan (from an April 21, 2023 draft Design Brief) and preliminary building statistics prepared by BDP Quadrangle architects is provided in **Appendix A**. The proposed plan consists of a public road, park dedication, and four private development blocks. Private development blocks are anticipated to contain multi-storey residential apartment units with private open space and internal roadways (where needed).

The current anticipated unit counts per private development block are listed in **Table 1.1** below.

Table 1.1: Unit Count

Development Block	Apartment Units
A	392
B	1029
C	476
D	1111
Total	**3008**

A unit type breakdown for each of the buildings is not yet confirmed. Subsequent applications through the development process can confirm unit types as needed.

1.2 Regulatory Framework

The development of the Petries Landing III site is governed by the City of Ottawa's current Official Plan, the Orleans Corridor Secondary Plan, and applicable development application requirements.

The Rideau Valley Conservation Authority (RVCA) administers development regulations in areas subject to natural hazards (such as flooding, erosion, and unstable slopes) and in environmentally sensitive areas (such as wetlands, shorelines, and waterways). The RVCA also reviews development proposals and municipal planning applications within or adjacent to natural areas.

The pre-application consultation process with the City of Ottawa and the RVCA establishes the initial design criteria associated with demonstrating the suitability of servicing and stormwater management on the site.

1.2.1 REFERENCE DOCUMENTS

Documents referenced in support of this report include:

- *City of Ottawa Sewer Design Guidelines* (SDG), City of Ottawa, October 2012, including all subsequent technical bulletins

- *City of Ottawa Design Guidelines – Water Distribution*, City of Ottawa, July 2010, including all subsequent technical bulletins
- *Design Guidelines for Drinking Water Systems*, Ministry of the Environment, Conservation, and Parks (MECP), 2008
- *Fire Protection Water Supply Guideline* for Part 3 in the Ontario Building Code, Office of the Fire Marshal (OFM), October 2020
- *Water Supply for Public Fire Protection*, Fire Underwriters Survey (FUS), 2020
- *Fire Code*, National Fire Protection Agency, 2012
- Pre-Application Consultation meeting notes and related correspondence with City of Ottawa and RVCA staff (see **Appendix B**).
- *Geotechnical Investigation Petrie's Landing III 8600 Jeanne D'Arc Boulevard*, Paterson Group, Report PG6414-1, December 23, 2022
- Site topographic survey data provided to Stantec.

Information on infrastructure located within the adjacent public roads are obtained from available City of Ottawa as-built records.

It is noted that there is no Master Drainage Plan or Sub-watershed study available to support the stormwater management (SWM) objectives.

1.3 Objective

This Functional Servicing and Stormwater Management report assesses and identifies preliminary servicing and stormwater management (SWM) conditions which are generally consistent with City of Ottawa Design Guidelines and considers related pre-application consultation advice provided by City of Ottawa and RVCA staff. Deviations from existing reference documents or pre-consultation advice is identified with an explanation for the change in relation to site specific circumstances.

Preliminary general and applicable site-specific objectives considered are summarized below. Specific technical design criteria details are described in the associated servicing sections of this report.

Potable Water Servicing

- Develop a functional assessment of the potable water and fire flow demand for the site.
- Identify that the City of Ottawa water distribution system can supply adequate water pressure to the site for typical operational and emergency conditions.

Wastewater (Sanitary Sewer) Servicing

- Develop a functional assessment of the wastewater flow projected for the site.

- Identify that the City of Ottawa sanitary sewer system can support the project wastewater flow from the site.

Storm Sewer Servicing and Stormwater Management

- Identify allowable flow contributions from the site to the adjacent receiving water bodies.
- Identify applicable water quality control targets.
- Develop a functional assessment of the SWM system for the site to achieve applicable water quantity (minor and major system) control and water quality control targets.

Site Grading Plan

- Prepare a preliminary grading plan to support the servicing assessments and identify compatibility with surrounding existing ground conditions.

The accompanying figures and drawings illustrate the key components of the functional servicing assessments.

To reflect changes in design conditions, related objectives and/or assessment findings may be adjusted as needed through subsequent stages of the development application process.

2.0 Potable Water Servicing

2.1 Background

The site is within Pressure Zone ‘1E’ of the City of Ottawa water distribution system.

The existing watermains along the boundaries of the site consist of a 400 mm diameter PVC watermain within Jeanne D’Arc Boulevard N.

Existing fire hydrants are located along Jeanne D’Arc Boulevard N. immediately adjacent to the site.

2.2 Design Criteria

The following design criteria are considered with the assessment of the potable water and fire protection servicing for the site.

2.2.1 WATER DEMAND AND ALLOWABLE PRESSURE

Preliminary potable water demand and allowable water pressure are assessed using the City of Ottawa Guidelines – Water Distribution (2010) as amended, and the ISTB 2021-03 Technical Bulletin.

Residential Apartment Population Rate	
Average Apartment	1.8 persons / unit
Residential Apartment Demand	
Average Daily (AVDY)	280 L/cap/day
Maximum Daily (MXDY)	2.5 x AVDY
Peak Hour (PKHR)	2.2 x MXDY
Allowable Water Pressure	
MXDY Flow	345 kPa (50 psi) to 552 kPa (80 psi)
PKHR Flow Minimum	276 kPa (40 psi.)
MXDY + Fire Flow	140 kPa (20 psi.)
Maximum Allowable for Occupied Area	552 kPa (80 psi)

2.2.2 FIRE FLOW AND HYDRANT CAPACITY

Preliminary fire flow requirements are assessed using the Fire Underwriters Survey (FUS) methodology (2020). Site specific criteria considered are noted in Section 2.3.2.

Fire hydrant capacity is assessed based on Table 18.5.4.3 of the National Fire Protection Agency (NFPA) Fire Code document. A hydrant situated less than 76 m away from a building can supply a maximum capacity of 5,678 L/min, and a hydrant 76 to less than 152 m away can supply a maximum capacity of 3,785 L/min.

2.2.3 WATERMAIN SERVICING

The preliminary watermain network is considered in general accordance with Ministry of Environment, Conservation and Parks (MECP) Guidelines, City of Ottawa Design Guidelines – Water Distribution (2010), Ministry of Environment, Conservation and Parks (MECP) Guidelines, and the pre-application meeting notes.

2.3 Water Demand

2.3.1 DOMESTIC WATER DEMAND

The domestic water demand is assessed based on the proposed development conditions described in **Table 1.1** and the design criteria described in **Section 2.2**.

The assessed domestic water demand for the site is summarized in **Table 2.1**. Supporting calculations are provided in **Appendix C.1**.

Table 2.1: Estimated Domestic Water Demand

Demand Type	Population	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Block A - Residential Apartment	706	2.3	5.7	12.6
Block B - Residential Apartment	1852	6.0	15.0	33.0
Block C - Residential Apartment	857	2.8	6.9	15.3
Block D - Residential Apartment	2000	6.5	16.2	35.6
Total	**5414**	**17.6**	**43.9**	**96.5**

2.3.2 FIRE FLOW DEMAND

The fire flow demand is assessed based on:

- Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction (i.e., building construction materials with a 1-hour fire resistance rating).
- Total effective building area is the gross floor area of the two largest floor plus 50% of the floor area for eight adjoining floors.
    - Vertical openings are not protected.

- Occupancy and contents factor considering non-combustible materials.
- A fully supervised automatic sprinkler system that conforms to the NFPA 13 standard supplied by a standard water supply.
- Exposure distances based on the proposed adjacent structures having Type **I-II** (fire resistive or non-combustible rating) construction with unprotected openings.

The highest fire flow is assessed to be approximately 10,000 L/min (167 L/s) for the proposed site plan. Supporting calculations per the FUS methodology are provided in **Appendix C.2**.

2.4 Available Level of Service

2.4.1 BOUNDARY CONDITIONS

The assessed domestic water and fire flow demands are used to confirm the level of servicing available to the proposed development from the adjacent municipal watermain and hydrants. The associated hydraulic grade line (HGL) elevation boundary conditions provided by the City of Ottawa (see **Appendix C.3** for correspondence) are summarized in **Table 2.2**.

Table 2.2: Boundary Conditions

HGL Condition	Elevation (m)
Minimum HGL	106.1
Maximum HGL	113.7
Max. Day + Fire Flow (167 L/s) HGL	102.1

2.4.2 ALLOWABLE DOMESTIC PRESSURE

Finished elevations across the site will vary. To review the anticipated pressure conditions, a low elevation and high elevation are considered as reference for the calculation of residual pressures at ground level. The low elevation selected is 50.5 m. The high elevation selected is 54.0m.

From the boundary condition HGL elevations, the pressures under normal operating conditions are anticipated as:

- Low elevation (50.5 m) = 545 kPa to 620 kPa (79 psi to 90 psi).
- High elevation (54.0 m) = 511 kPa to 585 kPa (74 psi to 85 psi).

The anticipated pressures may exceed the maximum allowable for occupied areas under the potential maximum pressure condition. Pressure reducing measures may be required.

To ensure adequate water pressure above the first-floor elevation of the apartment buildings, booster pump requirements are to be confirmed by the mechanical engineering consultant during subsequent stages of the development application process.

2.4.3 ALLOWABLE FIRE FLOW PRESSURE

From the boundary condition HGL elevations, the existing watermain can provide the required fire flow while maintaining the minimum residual pressure of 138 kPa (20 psi).

2.4.4 FIRE HYDRANT COVERAGE

The buildings are to be sprinklered and a Siamese (fire department) connection provided. The Siamese connections are to be within 45 m of a fire hydrant.

Fire hydrant coverage will be developed and confirmed within the site during the subsequent stages of the development application process.

2.5 Proposed Water Servicing

The development is to be serviced with connections to the existing 400 mm watermain in Jeanne D’Arc Boulevard N. The proposed water servicing is shown on **Drawing WTR-1**. Connections and service requirements are to be consistent with City of Ottawa guidelines and specifications.

Service connections from the development blocks are made to a 250 mm looped watermain within the proposed public road. Block A is anticipated to have a 200 mm water service connected to the existing water main in Jeanne D’Arc Boulevard N. and the proposed 250 mm watermain within the site. Block B, C, and D are anticipated to have paired 200mm service laterals extended into each development block from the 250 mm watermain within the site.

For each proposed building, a mechanical engineering consultant is responsible to confirm the service size required and that the water pressure within each building is adequate to meet building code requirements. This confirmation is to occur during subsequent stages of the development application process.

2.6 Hydraulic Assessment

The proposed watermains within the Petries Landing III site are modeled in the H2OMAP hydraulic model to test the service pressure under average day, peak hour, and maximum day plus fire flow conditions.

The hydraulic model uses the boundary conditions provided by the City of Ottawa, as described in Section 2.4.1. The boundary conditions are applied to a fixed head reservoir simulated at the eastern and western connections from the Petries Landing III site to the existing 400 mm watermain in Jeanne D’Arc Boulevard N. Demand values are applied within the modeled system based on the values for each development block as described in Section 2.3.1. The demand values are applied at a node that approximates the anticipated service location for each development block.

Hazen-Williams coefficients ("C-Factors") are applied to the simulated watermains in accordance with the City of Ottawa's Water Distribution Design Guidelines and as shown in **Table 2.3** below.

Table 2.3: Proposed Watermain C-Factors

Pipe Diameter (mm)	C-Factor
150	100
200 to 250	110
300 to 600	120
> 600	130

2.6.1 AVERAGE DAY DEMAND (AVDY)

The hydraulic modeling results indicate that under the average day demand, the pressure in the proposed watermain ranges from 587 kPa to 621 kPa (85.1 psi to 90.1 psi). These pressures exceed the serviceable limit of 276 kPa to 550 kPa (40 psi to 80 psi) as specified in the City of Ottawa Design Guidelines – Water Distribution. Results are shown in **Figure 2.1** below.

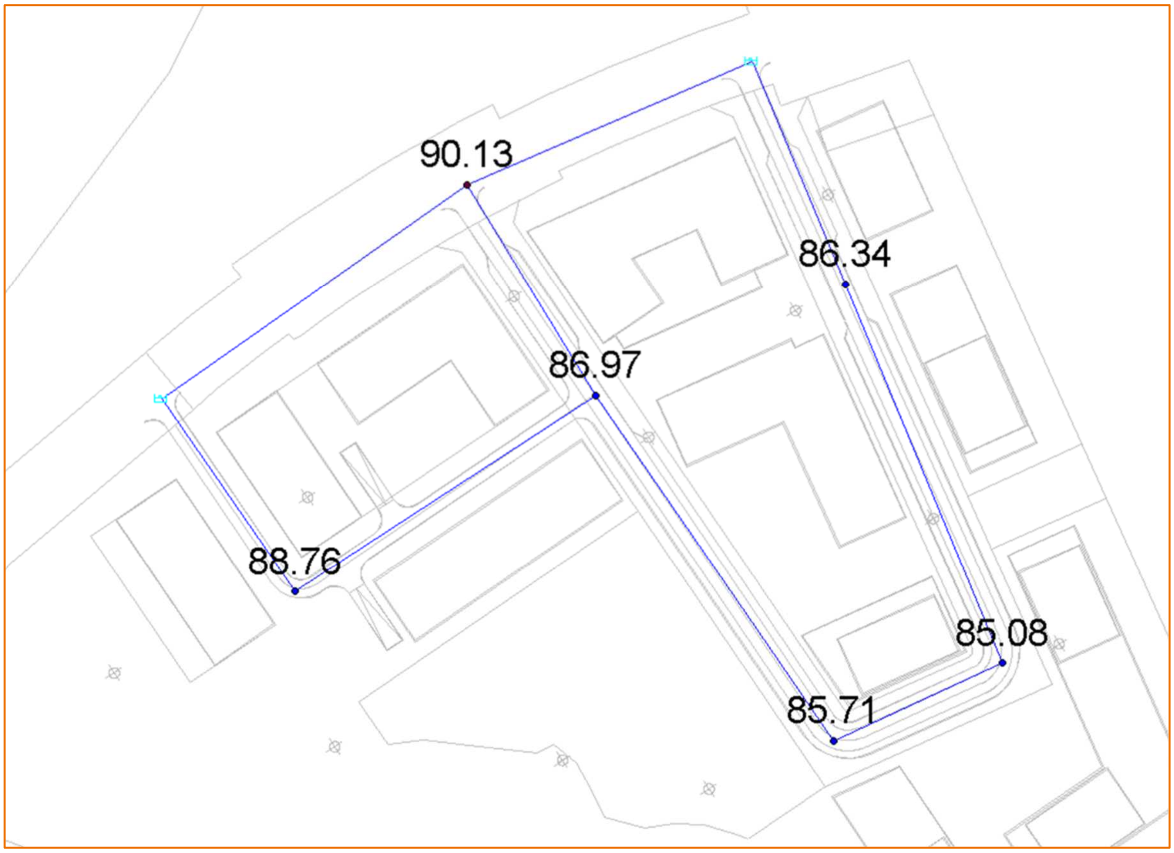


Figure 2.1: AVDY Pressure Results (psi)

As noted in Section 2.4.2, the anticipated pressures under the average day demand (AVDY) may exceed the maximum allowable for occupied areas. Pressure reducing measures may be required.

2.6.2 PEAK HOUR DEMAND (PKHR)

The hydraulic modeling results indicate that under the peak hour demands, the pressure in the proposed watermain ranges from 508 kPa to 547 kPa (73.7 psi to 79.3 psi). These pressures are within the serviceable limit of 276 kPa to 552 kPa (40 psi to 80 psi) as specified in the City of Ottawa Design Guidelines – Water Distribution. Results are shown in **Figure 2.2** below.

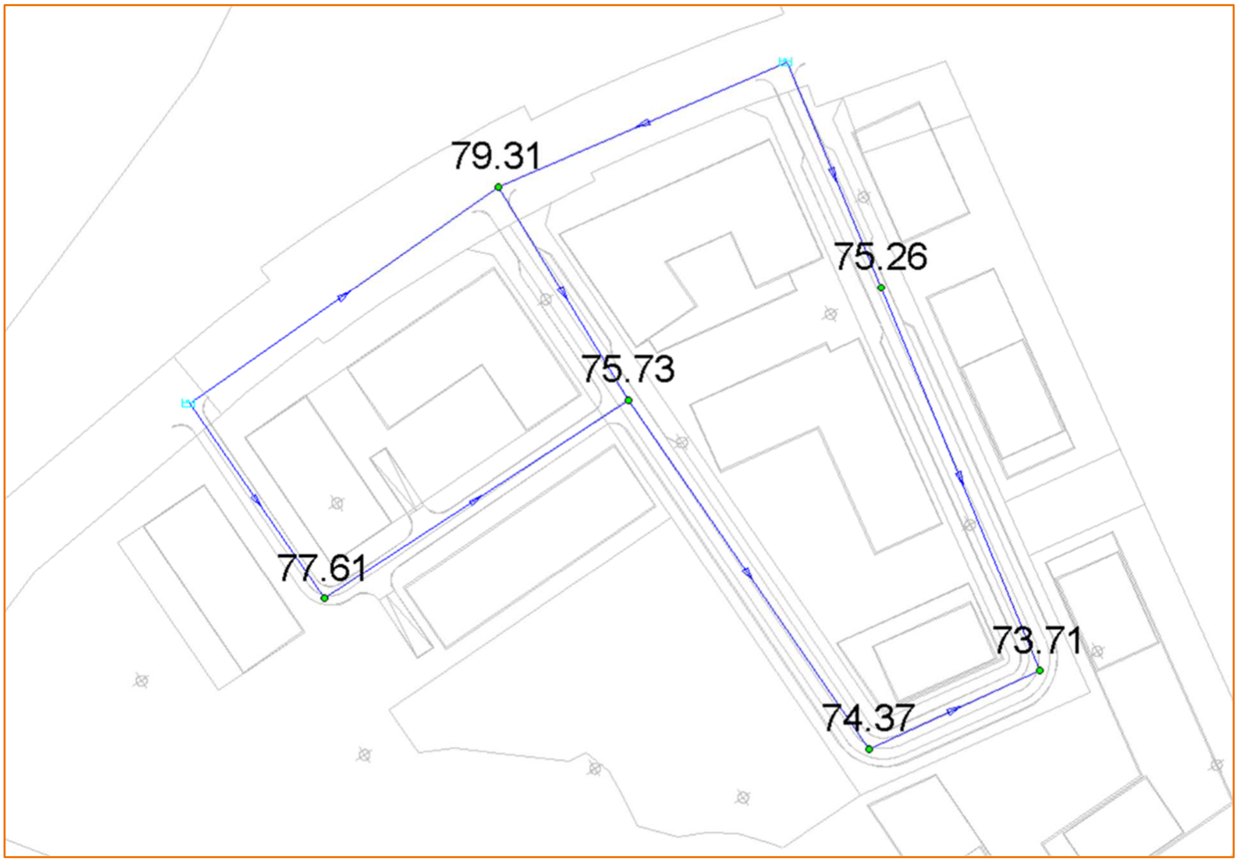


Figure 2.2: PKHR Pressure Results (psi)

2.6.3 MAXIMUM DAY DEMAND + FIRE FLOW (MXDY+FF)

The hydraulic modeling is also used to assess the maximum day and fire flow demands while maintaining a residual pressure of 138 kPa (20 psi), per the City of Ottawa Design Guidelines – Water Distribution. The modeling is conducted using a steady-state maximum day demand scenario along with the automated fire flow simulation feature of H2OMAP. The fire flow demand is set to 167 L/s as per the value noted in Section 2.3.2.

Figure 2.3 illustrates that the proposed watermain can deliver flows exceeding 10,000 L/min (167 L/s) while maintaining the required residual pressure of 138 kPa (20 psi).

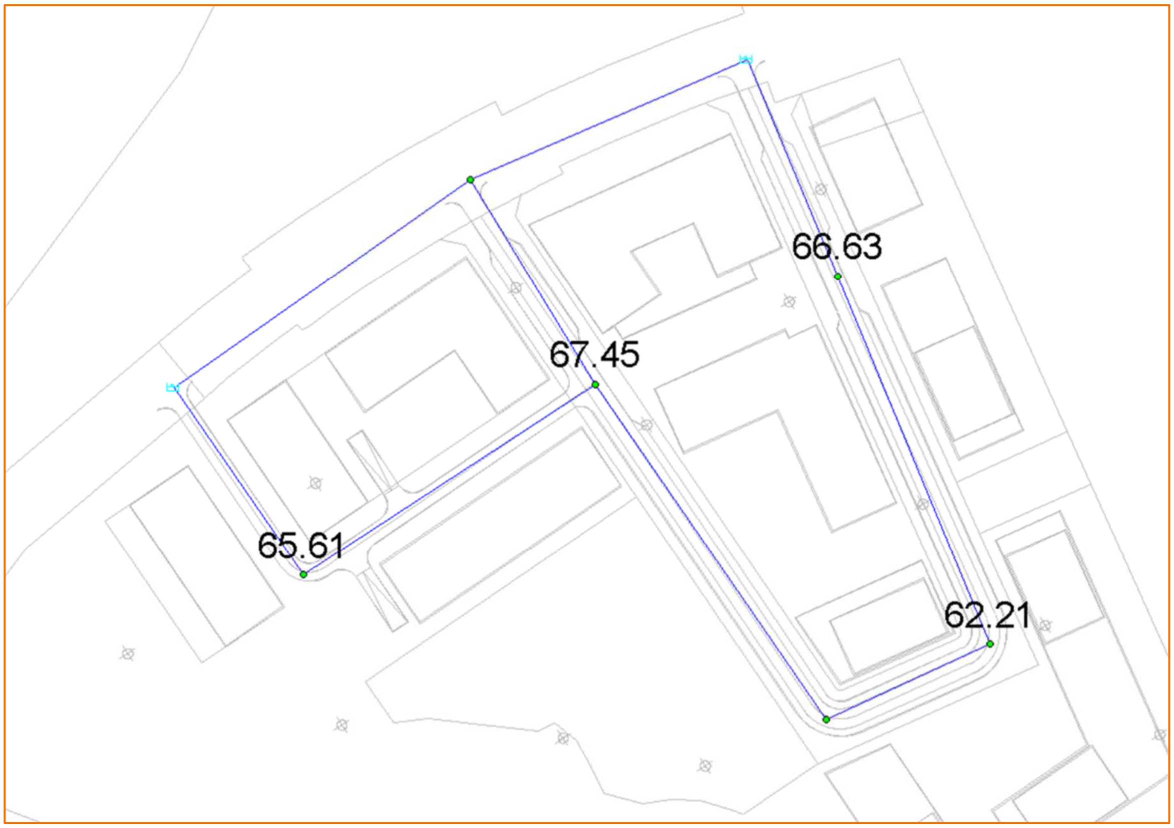


Figure 2.3: Fire Flow Results – Residual Pressure (psi)

2.6.4 HYDARULIC ASSESSMENT SUMMARY

Based on the boundary conditions provided by the City of Ottawa and the hydraulic assessment using H2OMAP, the water distribution system provides adequate pressure to satisfy the potable water and fire flow needs of the proposed development.

A model schematic and summary results from the H2OMAP hydraulic assessment of the water distribution systems are included in **Appendix C.4**.

3.0 Wastewater Servicing

3.1 Background

The existing sanitary sewers along the boundaries of the site consist of a 900 mm diameter trunk sewer along Jeanne D'Arc Boulevard N.

3.2 Design Criteria

Preliminary wastewater servicing is assessed using the City of Ottawa Sewer Design Guidelines (2012) as amended, and the MECP Design Guidelines for Sewage Works. The following design criteria are considered with the assessment of wastewater servicing for the site.

Population criteria are the same as that applied for the water demand analysis (see **Section 2.2.1**).

Residential Wastewater Flow	
Average Flow Generation	280 L/cap/day
Peaking Factor	Harmon Equation (max. residential = 4.0)
Harmon Correction Factor	0.80
Infiltration Allowance	0.33 L/s/ha

Sewer Design	
Minimum Velocity	0.6 m/s (0.8 m/s for upstream sections)
Maximum Velocity	3.0 m/s
Minimum Service Size	135 mm
Manning Roughness Coefficient	0.013
Minimum Service Slope	1.0 % (2.0 % preferred)
Minimum Service Cover	2.0 m

3.3 Wastewater Generation and Servicing Design

The peak wastewater flow is assessed based on the proposed development conditions described in **Table 1.1** and the design criteria described in **Section 3.2**.

The assessed peak wastewater flow for the site is summarized in **Table 3.1**. Supporting calculations are provided in **Appendix D.1**.

Table 3.1: Estimated Peak Wastewater Flow

Location Reference	Peak Residential Wastewater Flow			Infiltration Flow (L/s)	Total Peak Flow (L/s)
	Population	Peak Factor	Peak Flow (L/s)		
West Connection	2557	3.00	24.9	1.1	26.0
East Connection	2857	2.97	27.5	0.7	28.2
Total	**5414**	**-**	**52.4**	**1.8**	**54.2**

The anticipated peak wastewater flows for the proposed development were provided to the City of Ottawa staff to evaluate the adequacy of the receiving municipal sanitary sewer system in the vicinity of the site and downstream network. At the time of the writing of this report, the City has not yet provided confirmation of the sanitary sewer capacity (see correspondence in **Appendix D.2**).

3.4 Proposed Sanitary Servicing

The development is to be serviced with connections to the existing sanitary sewer in Jeanne D’Arc Boulevard N. The proposed sanitary servicing is shown on **Drawing SAN-1**. Related preliminary sanitary sewer design calculations are provided in **Appendix D.1**. Connections and service requirements are to be consistent with City of Ottawa guidelines and specifications.

Service connections from the development blocks are made to 250mm sewers within the proposed public road. Block A is anticipated to have a 250mm sanitary service lateral, and Block B, C, and D are anticipated to have 200mm service laterals.

For each proposed building, a mechanical engineering consultant is responsible to confirm the service size required and that the appropriate backwater valve requirements are satisfied. This confirmation is to occur during subsequent stages of the development application process.

4.0 Stormwater Management and Servicing

4.1 Background

The existing storm drainage system along the boundaries of the site consists of a ditch and culvert drainage system along Jeanne D’Arc Boulevard N. A portion of the ditch drains to Taylor Creek and the remainder drains to the Ottawa River. There are two culvert systems conveying runoff from the south side of Jeanne D’Arc Boulevard N. to the Ottawa River to the north. Subject to confirmation from field investigation, the existing culverts are currently considered 600mm diameter corrugated steel pipe (CSP) material.

As noted in Section 1.2.1, there is no Master Drainage Plan or Sub-watershed study available to support the stormwater management (SWM) objectives.

4.2 Design Criteria

Preliminary stormwater management (SWM) and storm sewer servicing is assessed using the City of Ottawa Sewer Design Guidelines (2012) as amended, and the pre-application consultation notes provided by the City of Ottawa and RVCA staff. The following design criteria are considered with the assessment of SWM and storm sewer servicing for the site.

General

- Use of the dual drainage principle.
- Wherever feasible and practical, site-level measures should be used to reduce and control the volume and rate of runoff.
- Consider the impact of 100-year event outlined in the City of Ottawa Sewer Design Guidelines on the major and minor drainage systems.

Storm Sewer & Inlet Controls

- Surcharge in the storm sewer system shall not occur for the 2-year design storm on local roads and the 5-year design storm for collector roads.
- Within private development blocks, peak flows generated from events greater than the 5-year and including the 100-year storm must be detained on-site.

Surface Storage & Overland Flow

- Building openings to be a minimum of 0.30 m above the 100-year water level.

- Maximum depth of flow under either static or dynamic conditions shall be less than 0.35 for the 100-year design storm. Within the private development blocks, runoff greater than the 100-year design would spill to the city right-of-way.

- Provide adequate emergency overflow conveyance off-site with a minimum vertical clearance of 0.15 m between the spill elevation and the ground elevation at the building envelope in the proximity of the flow route or ponding area.

Water Quality Control

- An enhanced level of water quality control - 80% Total Suspended Solids (TSS) removal.

Taylor Creek

- Considerations for a new storm sewer outlet to Taylor need to include geotechnical, environmental (terrestrial and aquatic), geomorphological (erosion), and runoff elements.

4.3 Existing Conditions

As noted in Section 1.1, the site is currently vacant and is farmed. All runoff from the site currently drains uncontrolled to the adjacent water bodies of Taylor Creek and the Ottawa River. There is no external drainage area draining into this property. The current pre-development drainage pattern is illustrated on **Figure E1.1 Pre-Development Storm Drainage Area** (located in **Appendix E.1**). A summary of pre-development drainage areas and runoff coefficients are provided in **Table 4.2**. Supporting calculations are provided in **Appendix E.1**.

Table 4.1: Summary of Pre-Development Drainage Areas

Drainage Areas	Area (ha)	Runoff Coefficient, C	Time of Concentration (min)	Outlet
PRE1	1.08	0.30	25	Taylor Creek
PRE1A	0.31	0.43	10	Taylor Creek
PRE2	1.33	0.30	26	Taylor Creek
PRE3	2.56	0.30	41	Ottawa River
PRE3A	0.23	0.53	10	Ottawa River
PRE3B	0.16	0.42	10	Ottawa River
PRE4	1.65	0.30	45	Ottawa River

Given the intended SWM design for the site, pre-development runoff rates are developed from Rational Method calculations. The existing condition rational method runoff coefficient is assessed based on the existing surface condition (e.g., asphalt, concrete, gravel, grass, etc.), or assigned a minimum value of C = 0.30. Time of Concentration values are developed based on a review of the site topography and existing conditions. Supporting calculations are provided in **Appendix E.1**.

4.4 Stormwater Management Design

Based on the proposed Site Plan, drainage area boundaries are defined as illustrated on **DrawingSTM-1**. Overall, the proposed SWM design intent is to direct storm runoff from the site to the Ottawa River. The only runoff intended to be direct to Taylor Creek is from the proposed public park and from landscaped portions of Block A and Block D not anticipated to be intercepted by the storm drainage system.

No new storm sewer outlet to Taylor Creek is proposed so no additional assessments of Taylor Creek are completed.

Preliminary runoff coefficient values for storm sewer design calculations and imperviousness allocations are assigned to each drainage area based on the anticipated finished surface condition (e.g., asphalt, concrete, gravel, grass, etc.) typically associated with the associated land use. A summary of drainage areas and runoff coefficients are provided in **Table 4.2**. Supporting calculations are provided in **Appendix E.2**.

Table 4.2: Summary of Post-Development Drainage Areas

Drainage Areas	Area (ha)	Runoff Coefficient, C	Outlet
C101A (Block B)	1.36	0.83	Ottawa River
C101B	0.54	0.80	Ottawa River
C102A (Block A)	1.52	0.82	Ottawa River
C201A	0.54	0.80	Ottawa River
C201B (Block C)	0.61	0.82	Ottawa River
C201C (Block D)	0.95	0.83	Ottawa River
UNC-1	0.22	0.35	Taylor Creek
UNC-2	0.66	0.35	Taylor Creek
UNC-3	0.22	0.35	Taylor Creek
PRE3A	*0.23*	*0.53*	*Ottawa River*
PRE3B	*0.16*	*0.42*	*Ottawa River*

The areas ‘PRE3A’, and ‘PRE3B’ represent contributing areas associated with the existing Jeanne D’Arc Boulevard N. They are expected to be the same in both pre-development and post-development conditions. They are not shown on **DrawingSTM-1** but are included in the assessments of this report to allow for reasonable pre-development to post-development comparisons.

4.4.1 ALLOWABLE RELEASE RATE

With no applicable watershed or sub-watershed study, allowable release rates are considered relative to the respective receiving water body or connection to the proposed storm sewer system.

Ottawa River

For runoff directed to the Ottawa River, no specific allowable release rate is applied. The City of Ottawa design guideline of preventing surcharge conditions in the storm sewer with the 5-year design storm event is considered.

Taylor Creek

The allowable release to Taylor Creek should be less than the pre-development peak flow for both the 5-year and 100-year design storm events. A summary of the applicable pre-development peak flows is provided below.

Table 4.3: Allowable Target Release Rate to Taylor Creek

Area	Pre-Development Flow Rate (L/s)	
	5-Year	100-Year
PRE1	54.9	93.5
PRE1A	38.2	65.5
PRE2	65.8	112.2
Total	**158.9**	**271.3**

Supporting calculations are provided in **Appendix E.1**.

Public Road to Storm Sewer

For runoff from the public roadway (both within the site and from Jeanne D’Arc Boulevard N.), flow is directed to the Ottawa River so no specific allowable release rate is applied. As noted previously, the City of Ottawa design guideline of preventing surcharge conditions in the storm sewer with the 5-year design storm event is considered. This approach eliminates the need for underground storage to be provided in the public road system to control the storm runoff to an equivalent pre-development flow rate.

Development Block to Storm Sewer

Each development block will have an allowable release rate set as per the 5-year design storm event associated with the site storm sewer design. A summary of the applicable allowable release rates is provided in the following table. Supporting calculations are provided in **Appendix E.2**.

Table 4.4: Allowable Target Release Rate for Development Blocks

Development Block	Allowable 5-Year Release Rate (L/s)
A (C102A)	365.8
B (C101A)	328.0
C (C201B)	144.2
D (C201C)	229.0

The target allowable release rate for each development block is used to assess water quantity control conditions anticipated for each block.

4.4.1.1 Uncontrolled Areas

Portions of the proposed site plan are anticipated to have drainage areas that will not be intercepted by the storm drainage systems. These areas are the western edge of Block A (UNC-1), the public park space (UNC-2), and the western and southern portion of Block D (UNC-3). These areas are illustrated on **DrawingSTM-1**. These areas are anticipated as effectively 'undeveloped' landscape areas with a 'sheet flow' condition comparable to the pre-development drainage area condition. Consideration for some impervious elements integrated into the finished landscape condition is accommodated by the impervious areas selected and listed in **Table 4.2**.

The data summarized in **Table 4.5** indicates that the proposed SWM plan reduces the overall site storm runoff release rate to Taylor Creek by approximately 5% compared to the pre-development design storm events. Compensating for an increase in uncontrolled areas as part of the allowable release rate condition is not required.

Table 4.5: Comparison of Pre- and Post-Development Release Rates to Taylor Creek

Drainage Area	5-Year Discharge (L/s)	100-Year Discharge (L/s)
Pre-Development Total (2.72 ha)	158.9	271.3
Post-Development		
PRE1A (0.31 ha)	38.2	65.5
UNC-1 (0.22 ha)	22.3	38.2
UNC-2 (0.66 ha)	66.9	114.7
UNC-3 (0.22 ha)	22.3	38.2
Post-Development Total (1.41 ha)	149.8	256.6
Difference (Post minus Pre)	**-9.1 (-5.7%)**	**-14.7 (-5.4%)**

Additionally, with the net reduction in peak flow anticipated to Taylor Creek, no additional assessment of the conditions within Taylor Creek or the associated valley lands is considered.

4.4.2 QUANTITY CONTROL

With no specified allowable release rate target for discharge to the Ottawa River, no overall site water quantity control measure (i.e., a stormwater management pond) is considered.

The only water quantity control measures applicable are for each development block to ensure that only the 5-year storm sewer design flow is released from each block during the 100-year design storm event. A summary of the current anticipated storage requirements for each development block is provided with the discussion of the proposed stormwater servicing strategy in **Section 4.5**.

Initial outlines of the ponding areas anticipated within the low points of the public roads is illustrated on **Drawing STM-1**. In coordination with the functional grading plan as illustrated on **Drawing GP-1**, an effective overland conveyance and emergency overland escape routes for stormwater management and flood protection is available. Potential ponding areas within each development block are subject to the final site design details are not available at this stage of the development application process.

The analytical assessment of the ponding areas in the public roads and within the development blocks is to be completed during the subsequent stages of the development application process.

4.4.3 QUALITY CONTROL

Water quality control is to be provided with oil-grit separator (OGS) units. A single OGS unit associated with each of the two storm sewer outlets is to be provided within the site boundary. Each OGS unit will provide the required water quality treatment for both the public roads and the development blocks.

Information on conceptual OGS unit sizing is provided in Section 4.5.2.

4.5 Proposed Stormwater Servicing

The proposed stormwater servicing approach is illustrated on **Drawing STM-1**. Related preliminary storm sewer design calculations are provided in **Appendix E.2**. The western roadway segment, Block A, and Block B will discharge to the Ottawa River via an upgrade to the nearest associated existing culvert system (Outlet 1). The eastern roadway segment, Block C, and Block D will discharge to the Ottawa River with an outlet to the existing ditch on the south side of Jeanne D’Arc Boulevard N. and then through the nearest associated existing culvert system (Outlet 2).

Given the nature of the proposed stormwater servicing system, PCSWMM is used to assess the design criterion of preventing surcharge in the storm sewer system during the 5-year design storm event. Only the minor system condition is considered at this functional assessment stage. Major system considerations will be included as needed during subsequent stages of the development application process. The PCSWMM analysis is also used to provide a preliminary site storage value for each development block. Information on the methodology and project specific data with the PCSWMM analysis is provided in **Appendix E.3** and **Appendix E.4**.

The following describes the conditions proposed for the stormwater servicing associated with the public roads and the private development blocks.

4.5.1 PUBLIC ROADS

4.5.1.1 Outlet 1

The storm system for Outlet 1 intends to replace the existing culverts under Jeanne D’Arc Boulevard N. and the adjacent pathway. The culverts will be replaced with a 900mm storm sewer to facilitate a storm sewer system at a lower elevation which offers a preferred design condition to support the development of Block A. To maintain the existing drainage pattern of Jeanne D'Arc Boulevard N., ditch inlets will be connected to the new 900mm storm sewer.

The outlet elevation of the new 900 mm storm sewer will generally match the outlet of the existing culverts at an elevation of approximately 47.0 m. This elevation is above the Ottawa River flood elevation in the area which is at approximately 44.0 m.

In the western segment of the proposed road a 600mm storm sewer will support the road drainage and the service connection for development Block B. Based on the initial PCSWMM review, it is anticipated that the road drainage could be directed to the proposed storm sewer without the use of ICDs in the catch basins. This approach can be confirmed through the subsequent stages of the development application process.

From the preliminary PCSWMM review, the following summarizes the outlet peak flow and HGL elevations anticipated in the public storm sewer system for Outlet 1.

Table 4.6: Outlet 1 HGL and Peak Flow

Node ID	Elevation (m)				Peak Flow (L/s)	
	Rim	Pipe Obvert	5-Year HGL	100-Year HGL	5-Year	100-Year
101	53.38	49.52	49.38	49.64	467	582
100	50.53	48.02	47.79	47.87	890	1,065
HW1	48.00	47.90	47.56	47.61	890	1,064

From the intimal PCSWMM review, there are no surcharge conditions anticipated in the storm sewer system for the 5-year design storm event. Surcharge conditions for the 100-year design storm event are to be considered with respect to future development block connections through the subsequent stages of the development application process.

The location of Outlet 1 on the north side of Jeanne D'Arc Boulevard N is within land owned by the applicant (11034936 Canada Inc.). Outlet 1 is intended to be outside the Ottawa River floodplain, have access via Jeanne D'Arc Boulevard N and the adjacent pathway, and have appropriate erosion protection downstream of the headwall.

The design details for Outlet 1 are to be established through the subsequent stages of the development application process.

4.5.1.2 Outlet 2

The storm system for Outlet 2 intends to maintain the existing culverts under Jeanne D'Arc Boulevard N. and the adjacent pathway.

The outlet elevation of the new 675 mm storm sewer will generally match the inlet elevation of the existing culverts at approximately 48.9 m. This elevation is also above the Ottawa River flood elevation in the area which, as noted with Outlet 1, is at approximately 44.0 m.

In the eastern segment of the proposed road a 600mm storm sewer will support the road drainage and the service connection for development Block C and Block D. As with the road segment supported through Outlet 1, it is anticipated that the road drainage could be directed to the proposed storm sewer without the use of ICDs in the catch basins. This approach can be confirmed through the subsequent stages of the development application process.

From the preliminary PCSWMM review, the following summarizes the outlet peak flow and HGL elevations anticipated in the public storm sewer system for Outlet 2.

Table 4.7: Outlet 2 HGL and Peak Flow

Node ID	Elevation (m)				Peak Flow (L/s)	
	Rim	Pipe Obvert	5-Year HGL	100-Year HGL	5-Year	100-Year
201	53.60	51.03	50.99	51.52	507	622
200	50.91	49.62	49.42	49.46	492	568
HW2	51.00	49.53	49.19	49.32	524	606

From the intimal PCSWMM review, there are no surcharge conditions anticipated in the storm sewer system for the 5-year design storm event. Surcharge conditions for the 100-year design storm event are to be considered with respect to future development block connections through the subsequent stages of the development application process.

The location of Outlet 2 on the south side of Jeanne D’Arc Boulevard N is within the public road boundary. Outlet 2 is intended to be outside the Ottawa River floodplain, have access via Jeanne D’Arc Boulevard N, and have appropriate erosion protection downstream of the headwall.

The design details for Outlet 2 are to be established through the subsequent stages of the development application process.

4.5.2 CONCEPTUAL OGS UNIT SIZING

The *‘PCSWMM for Stormceptor’* online tool provide by Imbrium Systems is used to develop an initial approximation of the OGS unit size at Outlet 1 and Outlet 2. The runoff coefficient and area for the applicable drainage areas, on which the sizing is based, is listed in the following table.

Table 4.8: OGS Sizing Parameters

Drainage Area	Runoff Coefficient, C	Area (ha)
Outlet 1		
C101A	0.83	1.36
C101B	0.80	0.54
C102A	0.83	1.52
Outlet 1 Total	**0.83**	**3.42**
Outlet 2		
C201A	0.80	0.54
C201B	0.82	0.61
C201C	0.83	0.95
East Outlet Total	**0.82**	**2.10**

Using a fine particle size distribution, a Stormceptor model EFO10 achieves 84% TSS removal for Outlet 1. A Stormceptor model EFO8 achieves 85% TSS removal for Outlet 2. Both OGS models exceed the minimum required TSS removal level of 80%. The Stormceptor sizing report for each unit is included in **Appendix E.5**. Preliminary locations for each OGS unit are illustrated on **Drawing STM-1**.

The OGS unit sizes are considered conceptual for the purposes of validating the proposed stormwater servicing condition. Alternative OGS products or treatment systems with equivalent TSS removal capabilities may be selected based on the associated site design conditions and water quality objectives. The final OGS unit size and type is to be confirmed through subsequent stages of the development application process.

4.5.3 DEVELOPMENT BLOCKS

For this assessment, the focus for the development blocks is to summarize the anticipated storage requirements within each block. The storage requirements are needed to ensure that only the 5-year storm sewer design flow is released from each block during the 100-year design storm event. The following is the current summary of the design parameters applicable to the development blocks.

Table 4.9: Site Plan Block Design Parameters

Development Block	Area (ha)	Reference MH	Allowable Release Rate (m^3/s)	Design Storage Volume (m^3)
A (C102A)	1.52	100	365.8	184
B (C101A)	1.36	101	328.0	170
C (C201B)	0.61	201	144.2	78
D (C201C)	0.95	201	229.0	119

The water quantity control storage volumes may be accommodated with the development blocks through a combination of techniques. This may include, but not be limited to, any combination of roof top, cisterns internal to the buildings, underground storage external to the buildings, surface storage, Low Impact Development (LID) measures, etc.

For each proposed building, a mechanical engineering consultant is responsible to confirm the service size required, that the appropriate backwater valve requirements are satisfied, the nature of the foundation drainage system, and that any roof drainage systems (including internal storage systems, roof drains, scuppers, etc.) are adequate for accommodating the 100-year design storm conditions. It is noted that the 100-year SWM design condition is more stringent than the design condition associated with the typical building code requirements. This confirmation is to occur during subsequent stages of the development application process.

5.0 Site Grading

A functional grading plan is illustrated on **Drawing GP-1**. The overall grading strategy serves to:

- Match existing grades along adjacent existing property, existing roadway, and proposed/required development setback boundaries.
- Respect recommended grade raise restrictions.
- Provide suitable cover conditions for sanitary sewer, storm sewer, and watermain servicing.
- Establish effective overland conveyance and emergency overland escape routes for stormwater management and flood protection.

During subsequent stages of the development application process, adjustments to grading conditions may be made as needed. The associated servicing and stormwater management conditions will be considered and may also be adjusted as needed to maintain consistency with the related design criteria.

6.0 Other Considerations

6.1 Geotechnical

Geotechnical conditions for the site are investigated by Paterson Group with findings presented in the supporting investigation report PG6414-1 dated December 23, 2022 (provided under separate cover in support of the development application process). Recommendations from the geotechnical report are intended to be followed as they relate to the proposed servicing strategy for the site.

It is noted that shallow ground water conditions at select locations across the site may limit the implementation of infiltration-focused LID measures. Subsequent review of groundwater conditions for future block development will confirm the applicability of LID measures relative to local groundwater conditions as needed.

Additional geotechnical investigation to support the proposed storm sewer outlets is to be prepared and provided through the subsequent stages of the development application process.

6.2 Utilities

Existing utilities from Hydro Ottawa, Bell, Rogers, and Enbridge are anticipated to be used to service this site. The exact size, location, and routing of utilities is to be finalized during subsequent stages of the development application process.

6.3 Erosion and Sediment Control During Construction

To protect downstream water quality and prevent sediment build-up in catch basins and storm sewers, erosion and sediment control measures must be implemented during construction. Erosion and sediment control (ESC) measures are the responsibility of the contractor. Recommendations for ESC implementation will be included with subsequent submissions through the development application process.

6.4 Regulatory Approvals

Information on anticipated regulatory approvals associated with the site will be confirmed and provided with subsequent submissions through the development application process.

7.0 Closing

The water, wastewater, and storm water servicing conditions assessed in this report indicate that the existing public services immediately adjacent to the project site are adequate to support the proposed development and that a suitable design condition can be created to support the development plan.

The details of the block development and the associated confirmations from the mechanical engineering consultant are to occur during subsequent stages of the development application process.

APPENDICES

Appendix A Site Information

A.1 Concept Plan

Concept Plan

1 | Design Principles & Planning Strategy

Petries Landing III has potential to create a mixed-use walkable development that introduces commercial and residential areas, open landscape areas, and create a variety of public spaces that foster a community atmosphere. The edges of the site have the opportunity to create frontages along Jeanne-D'Arc Boulevard and active the streetscape. Within the site itself new blocks and buildings are organized with higher density on the south by the Queensway and transition to mid-rise buildings along Jeanne-D'Arc Boulevard. The massing strives to maximize frontage and create a hierarchy in the site. The towers are arranged to provide generous separations which ensure views and natural light for both the residents of the towers and to allow sun light and airflow to adequately pass through the towers to the public realm. The network of sidewalks and various open spaces and parks encourage pedestrian movement, which generates more commercial activity for new commercial spaces and frontages which connect and attract pedestrians to the new developments within the site.

├─┤ Tower Separation Dimensions

BDP.
Quadrangle

A.2 Site Information

BLOCK	NO. OF STOREYS	TOTAL GBA SM	TOTAL GBA SM (less ground floor)	TOTAL GBA SF (less ground floor)	SUITE COUNT @ 700 SF average)
A1	4	6,000	4,500	48,438	18
A2	6	7,770	6,475	69,696	88
A3	6	12,540	10,450	112,483	141
A4	6	12,840	10,700	115,174	145
B1	9	25,137	22,344	240,509	302
B2	9	23,904	21,248	228,711	288
B3 PODIUM	6	7,812	6,510	70,073	88
B3 TOWER	34	25,942	25,942	279,237	351
C1	9	9,180	8,160	87,833	110
C2 PODIUM	6	9,960	8,300	89,340	112
C2 TOWER	24	18,768	18,768	202,017	254
D1 PODUM	6	21,018	17,515	188,530	237
D1 TOWERS	24	37,536	37,536	404,034	508
D2 PODIUM	6	9,948	8,290	89,233	112
D2 TOWER	24	18,768	18,768	202,017	254
TOTAL		**247,123**	**225,506**	**2,427,324**	**3,009**

Appendix B Pre-Application Consultation

From: Murshid, Shoma <Shoma.Murshid@ottawa.ca>
Sent: Thursday, August 4, 2022 2:54 PM
To: Lisa Dalla Rosa; Patricia Warren
Cc: Philip Thibert; Wang, Randolph; Maloney, David; Curry, William; Wildman, Geraldine; Giampa, Mike; Wood, Mary Ellen; Rehman, Sami; Jamie Batchelor
Subject: 8600 Jeanne d'Arc Blvd. N. - Pre-Consultation follow-up for Subdivision & Zoning Amendment
Attachments: design_brief_submission requirements_8600 Jeanne D'arc.pdf; 8600 Jeanne D'Arc_Preconsult Subdivision Park Comments.pdf; Summary of Pre-application Consultation Meeting - 8600 Jeanne d'Arc Boulevard (PC2020-0258); 20220511 PETRIES III Site plan for zoning package.pdf

Good afternoon FoTenn,

Thank you for meeting with us on June 30, 2022 to discuss a revised master plan/draft plan of subdivision at 8600 Jeanne d'Arc Boulevard North for Brigil. This meeting allowed the City to review a recently revised concept plan (attached and entitled "20220211 PETRIES III Site plan for zoning package.pdf") and further outline subdivision requirements. I have also attached the previous pre-consultation follow-up for the pre-consultation meeting held in February 2022 with Michael Boughton.

Please note there is one amendment to the list of required reports within the attached follow-up from Michael Boughton and that is regarding the servicing report – the servicing report requirement has now been replaced by a Functional Servicing Report and Modeling report requirement.

The current proposal triggers an Application for New Development, 251 or More Dwelling Units Plus Non-Residential Uses, with a submission fee of $108,446.83 + Initial Engineering Design Review and Inspection Fee of $10,000.00 + Initial Conservation Authority Fee of $3,920.00 for this Plan of Subdivision Application. Along with the identified submission fee and a completed application form, the following plans and reports will be required to deem the application complete:

Plan of Subdivision, including CAD file
Survey Plan
Topographical Plan of Survey with published Benchmarks
4R-Plan, if applicable
Planning Rationale
Design Brief
Demonstration Plan
Concept Plan showing Ultimate Use of Lands
UDRP submission Package
Archeological Resource Assessment
EIS with TCR
Functional Servicing Report and Modeling
Geotechnical Landslide Risk Assessment
Geotechnical Report with Limit of Hazard Land
TIA
Noise Impact Study
Phase 1 ESA

*Community Benefits By-law charge will apply to this proposal.

Urban design comments on behalf of PRUD:

From **Randolph Wang**
Urban Designer | Concepteur Urbain
Planning, Real Estate and Economic Development | Services de la planification, de l’immobilier et du développement économique
City of Ottawa | Ville d'Ottawa
City of Ottawa | Ville d'Ottawa
☎ 613.580.2424 ext./poste 16391
ottawa.ca/planning / ottawa.ca/urbanisme

1. A Design Brief is required for a site plan control application. A scoped Design Brief is required for an OPA and a rezoning. A Design Brief is not required for a standalone plan of subdivision. However, since the plan of subdivision is guided by a master site plan, it will be a good idea to require the submission of a Design Brief. The Terms of Reference is attached for convenience.
    a. Please prepare a shadow study.
    b. Please engage a wind engineer in the master plan process. While the master plan exercise may not warrant a detailed quantitative wind study, opinions of a wind engineer will be very helpful to determine the appropriate build form and public realm design, with respect to pedestrian comfort and livability of the residents.
2. The site is not within a Design Priority Area in the current OP. Neither is it within a Design Priority Area of the new OP. However, due to the complexity of the proposal the applicant can benefit from input of the UDRP, perhaps in form of a “focused review”. Please reach out the City’s UDRP coordinator for further information.
3. With respect to the master plan concept presented, the urbanistic approach is appreciated. The master plan concept has clearly demonstrated the intent to follow the general principles of a TOD. Here are a few comments that aim to support further advancement of the plan.
    a. The current built form approach appears to follow the general principles of a TOD closely with respect to the distribution of density and height. It clearly shows the potential highest uses of the site. However, it has not yet clearly responded to the unique conditions of this particularly site, including being at waterfront, in close proximity to a major waterfront public amenity, and surrounded by natural features.
    b. With respect to the public realm, the pairing of the park and the square concept shown in the current master plan concept is interesting. However, the overall impression of the public realm is that it is quite generic and does not capture the opportunities of the unique setting, including opportunities for views. Considerations may be given to creating a central public space (park and square) that can clearly be the anchor of the new community. Given the site context, it would be interesting to explore the integration between the Ruisseau Taylers Creek and this central public space.
    c. Overall, the master plan should be guided by multi-facet principles and should reflect the unique opportunities offered by the site, in addition to following the principles of a TOD and the pursuant of highest and best of uses.
    d. The master plan should employ principles of sustainable design and design with nature to guide both the built form and public realm design and explore opportunities for creative integration between built form and public realm through sustainable design features such as storm water management facilities.
    e. The master plan should strive to optimize micro climate conditions at pedestrian levels and create optimal living conditions in private spaces by carefully oriented buildings to maximize exposure to sun light and minimize wind impacts.
    f. The master plan should strive to create a more inclusive community with options for different ownership and tenancy being contemplated.
    g. The master plan should continue to study the location, scale and design of commercial spaces to ensure their viability.
    h. The master plan should continue to explore and demonstrate the relationship, transition, and connection with the immediate abutting properties and the broader community.
    i. The master plan should study the characteristics of Jeanne D’arc, and explore how the street may transition from a parkway like setting towards a more urban typology as one approaches the LRT.

Hope these are helpful.

PIED Policy Comments:

From David Maloney

Demonstration Plans

Large development blocks in the Plan Area require coordination and preparation of a Demonstration Plan. A Demonstration Plan illustrates the functionality of development proposed for a large parcel or group of parcels, allowing for the coordination of phasing and development in keeping with the policies of this Plan.

1. A demonstration plan is intended to outline conceptually how development can be coordinated. However, through the development process, substantial flexibility exists to respond to site considerations, the market for housing types, and the design and height of buildings. Demonstration plans prepared under this plan require submission of:
    a. conceptual layout of buildings
    b. height schedule
    c. street network plan
    d. a pedestrian and cycling plan demonstrating priority for pedestrian and cycling movements, and connectivity to transit.
    e. public realm plan including parks, open space, street furniture and public art
    f. height schedule with setbacks
    g. calculation of unit density for the purposes of compliance with minimum density requirements
    h. private financing agreements
    i. a plan for at-grade pedestrian movement
2. Demonstration plans required for areas as identified on Schedule B will require a detailed Servicing Study that determines the capacity requirements for the entirety of the proposed development, measured against existing capacity constraints, and shall include defined solutions, phasing of works and the financing of works to address any capacity constraints.

8600 Jeanne d’Arc Boulevard

1) Development of this parcel will require submission of a Demonstration Plan.
2) Highest buildings should be located on the east side of the site with the most direct pedestrian and cycling access to Trim Station.
3) A multi-use pathway (MUP) will be constructed to link Tweddle Road, connecting the future AT bridge to the future street network in the master planned development site at 8600 Jeanne d’Arc Boulevard. The pathway will cross the watercourse west of Tweddle Road, utilize the Highway 174 right-of-way, and may traverse the Collège La Cité campus, linking the station with both the campus, and the future street network of the master planned development. The MUP will be designed, funded and constructed by the proponent of the master planned development at 8600 Jeanne d’Arc Boulevard, as a condition of development approval and completed prior to occupancy of the first phase.
4) A future public park will be located adjacent to Taylor Creek ravine.
5) A street functioning as a community activity centre and designed as a woonerf, will run north-south, connecting Jeanne d’Arc Boulevard to the MUP that leads to the Tweddle Road ROW. This street will act as the focal point of the neighbourhood, with a concentration of uses and activity. Non-residential active frontages are required along this street.
6) A series of POPS and courtyards will be designed to connect the community activity centre to the interior of the site, with a well-defined public realm that allows for comfortable pedestrian circulation between Jeanne d’Arc Boulevard, the park, and blocks with mixed-use and high-density residential buildings.

Station Core

The Station Core designation represents the heart of transit supportive, 15-minute neighbourhoods in the Orléans Corridor. Development in this area will fulfill the two-fold goals of achieving the highest densities of mixed-use and the lowest level of automobile dependency. Pedestrian convenience and safety will contribute to a more urban streetscape while maintaining access for slow moving vehicles in the core. Surface parking will be minimized, with the exception of parking for accessibility and emergency services.

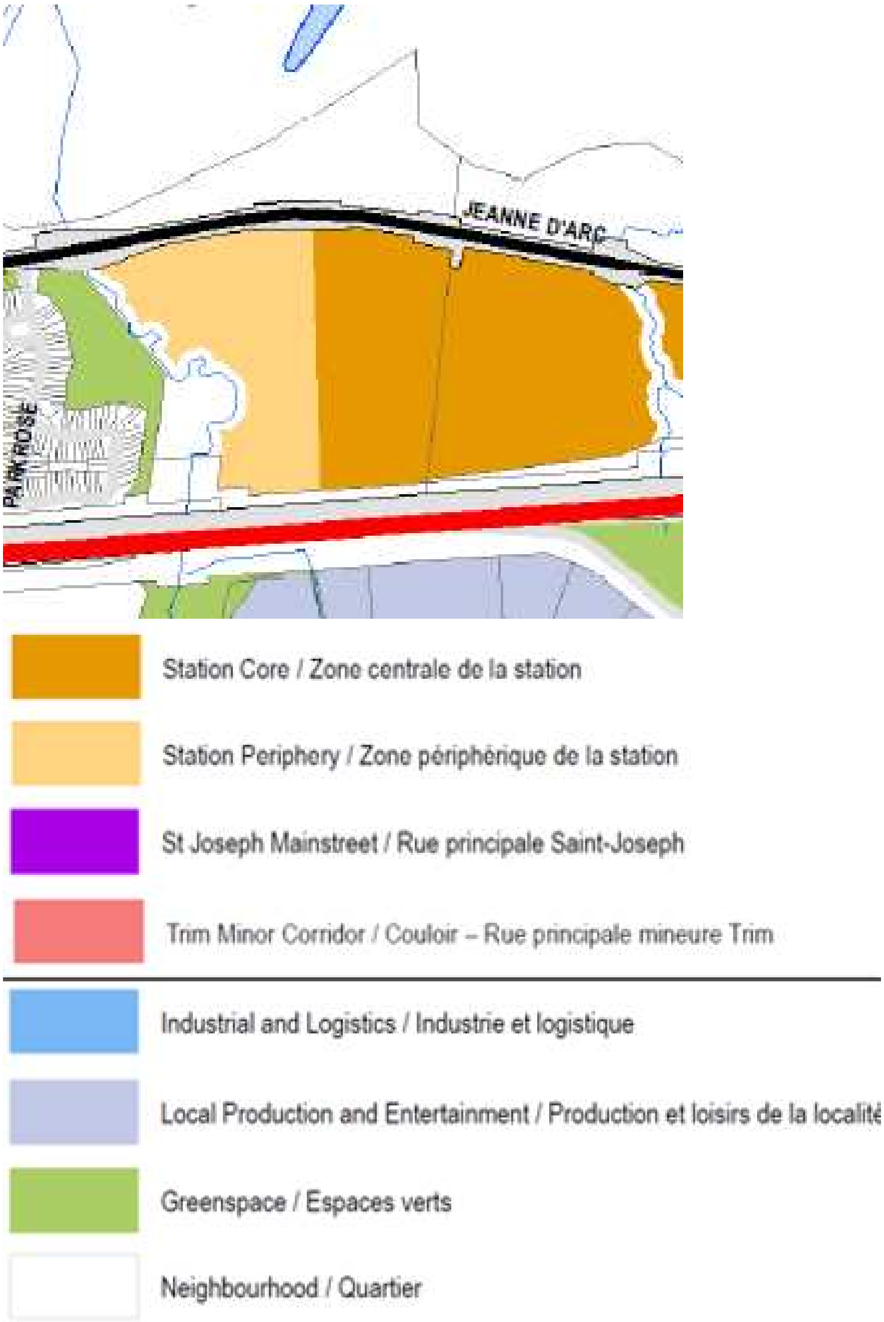
The Station Core designation serves as a focal point for services and amenities in the wider catchment area of O-Train stations. Permitted uses include all forms of mid-rise and high-rise residential, mixed-use buildings, and non-residential uses compatible with sensitive land uses, like residential and institutional uses. With the increase in jobs and people in the Station core there will also be new parks, greenspaces and street trees.

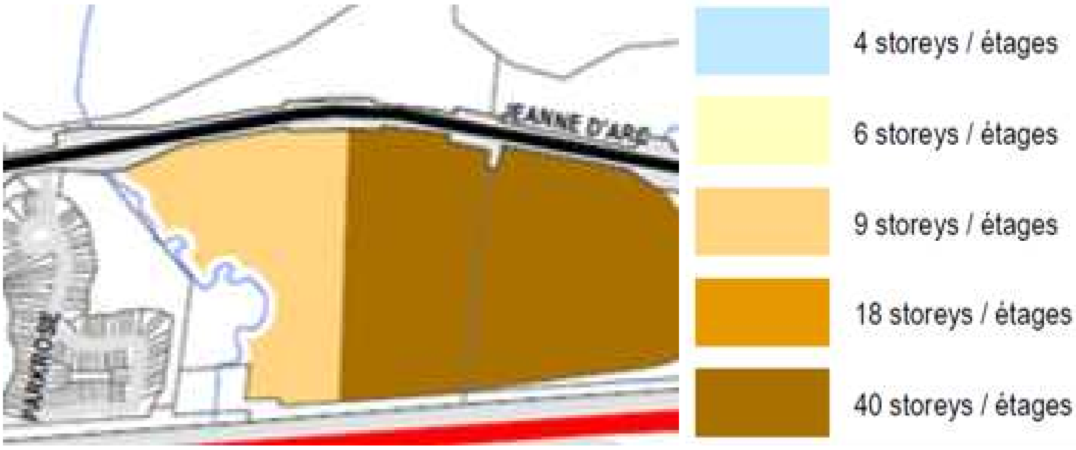
Station Periphery

The vision for the station periphery is to provide for high density pedestrian-oriented development of neighbourhoods in close proximity to the station in a 15-minute neighbourhood. The Station Periphery designation principally supports residential development at lesser heights than the mixed-use, high-density and high-rise context of the Station Core designation.

Within the Station Periphery district, residents will benefit from the features of a 15-minute neighbourhood. Consistent with the nearby Station Core designation, pedestrian and cycling movements will be highly prioritized and automobile movements typified by slow design speeds allowing for safe mixed traffic on local streets. Housing in the district will be predominantly mid-rise however low-rise apartments, stacked townhouses and traditional rowhouses will also be common.

In some areas of the secondary plan, the immediate area around stations have been designated Station Periphery, rather than Station Core. This is due to constraints on achieving greater densities and heights of buildings, like existing lot and street layouts and neighbourhoods. In those areas, such as the Convent Glen Station, the Station Periphery designation only is used to provide transit-supportive densities around the station, while minimizing potential impacts on abutting Neighbourhoods.





Engineering Requirements and Comments

Submission request: Water Boundary condition requests must include the location of the service and the expected loads required by the proposed development.

Please provide the following information:

Location of service connections (MAP)

Type of development and the amount of fire flow required (as per FUS).

Average daily demand: ___ l/s.

Maximum daily demand: ___l/s.

Maximum hourly daily demand: ___ l/s.

Subdivision – engineering documents required at Detailed Design

Title Page and Index Plan

Road Cross Sections

Erosion & Sediment Control Plan

General Plan of Services

Grading & Drainage Plan

Plan and profile

CUP

Post Catchment Plans

Details Plan

Landscape Plans

Design Brief and Stormwater Management Report with Modeling

Geotechnical Report with Limit of Hazard Land

Topographical Plan of Survey Plan with a published Bench Mark

Design Criteria

Storm Post to Pre. Consider LID within this application.

Onsite, 2-year pipe minimum and store up to 100-year on site. No 2-year ponding on site.

Permissible ponding of 350mm for 100-year, then at 100-year ponding elevation you must spill to City ROW

Geoinformation Centre - geoinformation@ottawa.ca for as-built plans.

Standard City of Ottawa Details, request from .... standardssection@ottawa.ca

Draft Plan of Subdivision; FSR to include:

Macro Catchment Area Plans (Post)

Marco Sanitary Servicing Plan with inverts and TOG

Macro Water Servicing Plan

Macro Storm Servicing Plan with inverts and TOG

Macro Storm Ponding Plan

Dry Pond Plan with sizes if required

Macro Grading Plan

Road Allowance Cross sections

Modeling____

*The City reserves the right to make changes to any decisions made herein should new information or data present itself.

Transportation Comments

The requirements for the Plan of Subdivision submission will be the same as what was requested under the Official Plan Amendment and Zoning By-law Amendment pre-consultation follow-up (pre-application consultation meeting held on 4 February 2021):

Transportation/Noise. The following comments are provided by Mike Giampa, Senior Engineer, Infrastructure Applications.

- The submission of a Screening Form is required. A TIA is warranted, therefore, please proceed to scoping.
  - The application will not be deemed complete until the submission of the draft Steps 1-4, including the functional draft RMA package (if applicable) and/or monitoring report (if applicable).
  - Although a full review of the TIA Strategy report (Step 4) is not required prior to an application, it is strongly recommended. Synchro files are required with Step 4.
- The R.O.W. protection along Jeanne d'Arc Boulevard North is 26.0m as per Annex 1 of the Official Plan (North Service Road).
- Geometric Road Design (GRD) drawings for the internal public roads will be required with the first submission of underground infrastructure and grading drawings. These drawings should include such items as, but not limited to:
  - Road Signage and Pavement Marking for the subdivision;
  - Intersection control measure at new internal intersections; and
  - Location of depressed curbs and TWSIs.
- Traffic calming measures on roads are to be included within the limits of the future subdivision to limit vehicular speed to 30 kph and improve pedestrian safety. These measures may include either vertical or horizontal features.
- A Noise Impact Study is required.

Park Comments Attached.

Environmental Policy Review Comments:

From Sami Rehman

Here are my comments from the pre-development consultation for the Plan of Subdivision at 8600 Jeanne d'Arc Blvd on 30 June 2022.

Part of the subject property is within the Natural Heritage Feature Overlay and is adjacent to a watercourse and a Provincially Significant Wetland (PSW). As such, an Environmental Impact Study (EIS) will be required. The EIS should address:

-the Petrie Island PSW
-Potential significant habitat for threatened or endangered species – I recommend initiating the Information Gathering Form (IGF) process with the MECP because there has been significant Species at Risk (SAR) habitat identified with the Petrie Island PSW.
-Significant woodlands
-Significant valleylands
-Significant wildlife habitat
-Establishing the appropriate setbacks from a surface water feature based on OP policies and zoning; Please note that these setbacks have changed since the previous Official Plan
-If the stormwater on-site and its outlet will be directed into the watercourse (Taylor Creek), then the impacts associated with this outlet will need to be considered in the EIS.
-the proposed development will need to consider and recommend design elements from the City's Bird Safe Design guidelines into their proposal. While the actual incorporation of these design elements will likely be more appropriate at the Site Plan Control applications, the EIS should draw the fundamental principles and components to guide the proposal.

I think a Tree Conservation Report (TCR) will be required for the plan of subdivision proposal and the TCR can be combined with the EIS to avoid any duplications. Further details on the TCR will be provided by the forestry planner.

I would recommend the applicant consult with the RVCA to determine if any permits or approvals are required under their regulations.

I'm happy to discuss these matters in further detail if you wish. Best regards, sami

OTHER MATTERS:
Note: A 10% reduction in the planning fee component of each application type will be applied if both applications are filed concurrently.

Line to Plan of Subdivision Application: https://app06.ottawa.ca/online_services/forms/ds/subdivision_en.pdf
Link to Official Plan Amendment Application:
https://app06.ottawa.ca/online_services/forms/ds/official_plan_amendment_en.pdf
Link to Zoning Amendment
Application: https://app06.ottawa.ca/online_services/forms/ds/zoning_amendment_en.pdf

- It is recommended that you contact the RVCA asap and pre-consult with them on their requirements for the upcoming development review applications.

- It is also recommended you contact the Ward Councillor, Matthew Luloff, in advance of submitting your applications to briefly describe your proposal. His telephone no. is 613-580-2471.

Best wishes,

Shoma Murshid, MCIP, RPP
(she/ her/ elle)
File Lead, Planner II
Responsable de dossier, urbaniste II
City of Ottawa/ Ville d'Ottawa
Development Review (Suburban Services, East)/ Examen des projets d'aménagement (Services suburbains Est)
Planning, Real Estate and Economic Development Department / Direction générale de la planification, des biens immobiliers et du développement économique
110 Laurier Avenue West, 4th Floor, Ottawa ON K1P 1J1/ 110, avenue Laurier Ouest, 4^{e} étage, Ottawa (Ontario) K1P 1J1
Mail Code/ Code de courrier : 01-14

Tel/ Tél: (613) 580-2424 ext. 15430
Fax/ Téléc. : (613) 580-4751
e-mail/ courriel : shoma.murshid@ottawa.ca
www.ottawa.ca

Please note I will be out on annual leave starting August 5, 2022 and shall return August 16, 2022

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Meeting Minutes
8600 Jeanne d'Arc

Meeting/Project Name: Petrie Island III – RVCA consultation

Date of Meeting: August 3rd, 2022 **Time:** 9am – 10am

Meeting Facilitator: Lisa Dalla Rosa / Jamie Batchelor **Location:** MS Teams

Meeting Objective

This meeting is a pre-consultation meeting with RVCA prior to the submission of planning applications to the City of Ottawa.

Attendees

Name	**Representing**
Philip Thibert	Brigil
Kris Kilborn	Stantec
Faisal Abou-Seido	Paterson Group
Michelle Lavictoire	CIMA+
Lisa Dalla Rosa	Fotenn
Patricia Warren	Fotenn
Jamie Batchelor	RVCA – Planner
Jennifer Lamoureux	RVCA – Aquatic Ecologist
Claire Milloy	RVCA – Hydrogeologist
Evelyn Liu	RVCA – Engineer

Meeting Minutes

Topic

Lisa – gave an overview of the concept plan, discussed locating height at the southern end and the park along the ravine
Phil – wants to start the project off on the right foot with the RVCA
Jamie

- Two things related to slopes:
  - Normal slope stability and the limit of hazard land Reports
  - Very close to an escarpment that has historically had largescale landslides – will be looking for Landslide Hazard Assessment (whether it pertains to the escarpment or the ravine as well… will get confirmation on this)
- Is it anticipated Paterson Group will undertake the Landslide Hazard Assessment or co-authoring it?
  - Yes, Paterson will be taking it
  - Trigger point is the escarpment for this assessment
- May be helpful to have a pre-con with BCG and Paterson Group prior to landslide hazard assessment
- Need to confirm if meander belt will be required
  - Paterson confirms that this will be looked at
- Will be looking at SWM on this – are you proposing any outlets or will it all be going to existing storm sewers
  - Kris – capacity analysis has not been completed yet but does not believe there is existing storm sewers. SWM would probably be on site, likely a wet pond depending on where we can put it

    - One thing to consider, if there are alterations proposed for an outlet, this needs to be incorporated into the Geotech studies
- Storm – will be emphasizing water budget
    - Jennifer – clarifying that the ravine is Taylor Creek, so there is a lot of background information about that
        - Phil – does that change anything for an outlet into Taylor Creek?
            - Jennifer – not necessarily. The City has done a lot of restoration work along Taylor Creek. There is a lot of old infrastructure that would not really be permitted now (i.e. Gabian mattress). There is a lot of erosion and invasive species, so would suggest choosing an area with invasive species with minimal erosion for an outlet location.
    - Claire – do you know if the site has had boreholes done yet?
        - Faisal – not specifically on site, but knows that it's 200 kpa clay
        - Recommend a geomorphology assessment be done to confirm the small landslides
        - If there is an outlet, RVCA will need to see that flows are understood pre and post development – need to start flow monitoring very soon if looking to get a development application shortly.
            - If the pre-development is already highly erosive, we know we will need to do a lot to control the flows on site for post-development
        - A lot of the solutions can be related to evapotranspiration (i.e. the park with trees)
        - Would like pre-development flow measured in the clay plain
    - Jennifer – showed map of erosion locations, erosion protection required
    - Kris – are there any background reports for flows under the 174 or Jeanne d'Arc culverts?
        - Jennifer – provided a City of Ottawa contact to get this information, RVCA has some, but not exactly what we would need
    - Claire – storage, loss of evapotranspiration and overall increase in flows to the creek that may cause additional erosion are important items to consider
    - Evelyn – also checking MOECP guidelines
    - Claire – sometimes just the generic mapping is used, you should create a map for the site
    - Jamie – geomorph will be key, we want to make sure we're not exceed the erosion in the post development and even better if we lessen the erosion and reduce the hazard.
    - Consideration for snow melt
    - Provincial LID guidelines
- Jamie – what can be done in the constraint lands
    - Lot lines not allowed in the constraint lands
    - From a Geotech perspective, usually nothing in the limit of hazard lands.
    - For an environmental setback, have most of the pathways outside that setback as well
    - Jennifer – nicely treed on both sides right now, so want to maintain that
        - Keep pathways out of the riparian buffer area – keep the pathways on the edge of the trees
        - If you want to poke in and see Taylor Creek with a lookout spot, pick an area with invasive species
    - Lisa – do we need fencing?
        - Jamie – only would ask for fencing if the constraint lands are adjacent to parking lot or roadway. If the proposed land is a passive space, we can have a conversation about fencing and if it's needed (depends on the context)
        - Michelle – there would be a turtle fence needed
        - Jennifer – sometimes pathways can be a nice barrier for people to not garden and encroach on the space. Really depends on the people who are using the space
        - Lisa – confirmed that this won't be a development where there are individual backyards, it will be controlled by a condo corp or property manager
        - Jamie – once the design is narrowed down, we can have further discussions about fencing

    - Jamie – fencing doesn't have to be chain link fencing either, it can be something that fits better with the area.
- Jennifer – do you have a sense now of what the city is looking for in the park?
  - Lisa – they want programmable space, but the City's new parkland by-law has come out and we might need to provide more park. We might do something where we have a transition from a field to a more passive space towards Taylor Creek. City is on board with a park that buffers that existing corridor.
    - Once we get into the park design we would loop the RVCA in to discuss with City staff
- Michelle – most of the work was done in 2015 and 2018, going to go back and take a look at a few things
  - Just an EIS and a tree report for this site
  - The corridor for the Taylor Creek is trying to be protected, which has been identified previously
  - Jamie – did the city indicate that EIS is being triggered because of Taylor Creek or did they mention lands within 120m of a PSW? Not seeing it being a big concern since Jeanne d'Arc bisects the area, but just want the context for what the city has asked for.
    - Lisa – Michelle wasn't at the pre-app, but the City did bring up what was going on the north side of the Jeanne d'Arc
    - Michelle – in the other developments, we haven't had to deal with the PSW
    - Jennifer – on Taylor Creek itself is there a wetland polygon? If so you protect Taylor Creek, you protect this feature as well
  - Jennifer – will send the 2018 report and headwaters data from 2012 to have as background to inform the EIS
  - Michelle – not planning on doing a headwater study, which is fine
- Kris – outlet would probably be around where the existing culvert is crossing Jeanne d'Arc. As far as the geomorph and upstream studies, it looks like more of a linear outlet than a meander, is geomorph still required?
  - Jamie – will check with Geotech
  - Jennifer – typically would want the outlet to be upstream
    - Kris – would the RVCA permit multiple smaller outlets at the upstream end?
  - Michelle – fish habitat here as well, so we will need to consider this, which is outside of the RVCA area. DFO considerations.

Appendix C Water

C.1 Domestic Water Demand

Petries Landing III - Domestic Water Demand Estimates
Site Plan provided by Quandrangle (2023-04-24)
Project Number: 160401751

Population densities as per Table 4.1 of the City of Ottawa Water Design Guidelines:		
Average Apartment	1.8	ppu

Demand conversion factors as per Table 4.2 of the City of Ottawa Water Design Guidelines:		
Residential	280	L/cap/day

Building ID	Number of Apt Units^2	Estimated Population	Avg. Day Demand		Max. Day Demand [1]		Peak Hour Demand [1]	
			(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Plot A	392	706	137.2	2.3	343.0	5.7	754.6	12.6
Plot B	1,029	1,852	360.2	6.0	900.4	15.0	1980.8	33.0
Plot C	476	857	166.6	2.8	416.5	6.9	916.3	15.3
Plot D	1,111	2,000	388.9	6.5	972.1	16.2	2138.7	35.6
Total Site :	**3,008**	**5,414**	**1,052.8**	**17.5**	**2,632.0**	**43.9**	**5,790.4**	**96.5**

Notes:
1 Water demand criteria used to estimate peak demand rates for residential areas are as follows:
maximum day demand rate = 2.5 x average day demand rate
peak hour demand rate = 2.2 x maximum day demand rate (as per Technical Bulletin ISD-2010-02)
2 Number of apartment units as per Quandrangle Preliminary Plan development statistics table (April 21, 2023).

C.2 Fire Flow Demand (2020 FUS)

Stantec

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 1
Description: Plot 'A1', 4-Storey Medium-Rise Building
Building Area: 1500 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes							Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction							0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?		NO	-
		1500	1500	1500	1500				4500	-
3	Determine Required Fire Flow	(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min							-	12000
4	Determine Occupancy Charge	Limited Combustible							-15%	10200
5	Determine Sprinkler Reduction	Conforms to NFPA 13							-30%	-5100
		Standard Water Supply							-10%	
		Fully Supervised							-10%	
		% Coverage of Sprinkler System							100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	Firewall / Sprinklered ?	-	-
		North	> 30	0	0	0-20	Type V	NO	0%	0
		East	10.1 to 20	46.4	4	> 100	Type I-II - Unprotected Openings	YES	0%	
		South	> 30	0	0	0-20	Type V	NO	0%	
		West	> 30	0	0	0-20	Type V	NO	0%	
7	Determine Final Required Fire Flow	**Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min**								**5000**
		Total Required Fire Flow in L/s								**83.3**
		Required Duration of Fire Flow (hrs)								**1.75**
		Required Volume of Fire Flow (m^3)								**525**

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 2
Description: Plot 'A2', 6-Storey Medium-Rise Building
Building Area: 1295 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

<table>
<tr><th>Step</th><th>Task</th><th colspan="10">Notes</th><th>Value Used</th><th>Req'd Fire Flow (L/min)</th></tr>
<tr><td>1</td><td>Determine Type of Construction</td><td colspan="10">Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction</td><td>0.8</td><td>-</td></tr>
<tr><td rowspan="2">2</td><td rowspan="2">Determine Effective Floor Area</td><td colspan="5">Sum of Two Largest Floors + 50% of Eight Additional Floors</td><td colspan="5">Vertical Openings Protected?</td><td>NO</td><td>-</td></tr>
<tr><td>1295</td><td>1295</td><td>1295</td><td>1295</td><td>1295</td><td>1295</td><td></td><td></td><td></td><td></td><td>5180</td><td>-</td></tr>
<tr><td>3</td><td>Determine Required Fire Flow</td><td colspan="10">(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min</td><td>-</td><td>13000</td></tr>
<tr><td>4</td><td>Determine Occupancy Charge</td><td colspan="10">Limited Combustible</td><td>-15%</td><td>11050</td></tr>
<tr><td rowspan="4">5</td><td rowspan="4">Determine Sprinkler Reduction</td><td colspan="10">Conforms to NFPA 13</td><td>-30%</td><td rowspan="4">-5525</td></tr>
<tr><td colspan="10">Standard Water Supply</td><td>-10%</td></tr>
<tr><td colspan="10">Fully Supervised</td><td>-10%</td></tr>
<tr><td colspan="10">% Coverage of Sprinkler System</td><td>100%</td></tr>
<tr><td rowspan="5">6</td><td rowspan="5">Determine Increase for Exposures (Max. 75%)</td><td>Direction</td><td>Exposure Distance (m)</td><td>Exposed Length (m)</td><td>Exposed Height (Stories)</td><td>Length-Height Factor (m x stories)</td><td colspan="2">Construction of Adjacent Wall</td><td colspan="3">Firewall / Sprinklered ?</td><td>-</td><td>-</td></tr>
<tr><td>North</td><td>> 30</td><td>0</td><td>0</td><td>0-20</td><td colspan="2">Type V</td><td colspan="3">NO</td><td>0%</td><td rowspan="4">0</td></tr>
<tr><td>East</td><td>10.1 to 20</td><td>51.8</td><td>6</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td>South</td><td>10.1 to 20</td><td>9.2</td><td>6</td><td>41-60</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td>West</td><td>10.1 to 20</td><td>46.4</td><td>4</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td rowspan="4">7</td><td rowspan="4">Determine Final Required Fire Flow</td><td colspan="11">Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min</td><td>6000</td></tr>
<tr><td colspan="11">Total Required Fire Flow in L/s</td><td>100.0</td></tr>
<tr><td colspan="11">Required Duration of Fire Flow (hrs)</td><td>2.00</td></tr>
<tr><td colspan="11">Required Volume of Fire Flow (m^3)</td><td>720</td></tr>
</table>

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 3
Description: Plot 'A3', 6-Storey Medium-Rise Building
Building Area: 2089.6 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction										0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?					NO	-
		2089.6	2089.6	2089.6	2089.6	2089.6	2089.6					8358.4	-
3	Determine Required Fire Flow	(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min										-	16000
4	Determine Occupancy Charge	Limited Combustible										-15%	13600
5	Determine Sprinkler Reduction	Conforms to NFPA 13										-30%	-6800
		Standard Water Supply										-10%	
		Fully Supervised										-10%	
		% Coverage of Sprinkler System										100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall		Firewall / Sprinklered ?			-	-
		North	> 30	0	0	0-20	Type V		NO			0%	0
		East	20.1 to 30	46.3	6	> 100	Type I-II - Unprotected Openings		YES			0%	
		South	10.1 to 20	60	6	> 100	Type I-II - Unprotected Openings		YES			0%	
		West	10.1 to 20	51.8	6	> 100	Type I-II - Unprotected Openings		YES			0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min											7000
		Total Required Fire Flow in L/s											116.7
		Required Duration of Fire Flow (hrs)											2.00
		Required Volume of Fire Flow (m^3)											840

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 4
Description: Plot 'A4', 6-Storey Medium-Rise Building
Building Area: 2140 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction										0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?					NO	-
		2140	2140	2140	2140	2140	2140					8560	-
3	Determine Required Fire Flow	(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min										-	16000
4	Determine Occupancy Charge	Limited Combustible										-15%	13600
5	Determine Sprinkler Reduction	Conforms to NFPA 13										-30%	-6800
		Standard Water Supply										-10%	
		Fully Supervised										-10%	
		% Coverage of Sprinkler System										100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall		Firewall / Sprinklered ?			-	-
		North	10.1 to 20	69.2	6	> 100	Type I-II - Unprotected Openings		YES			0%	0
		East	20.1 to 30	25	6	> 100	Type I-II - Unprotected Openings		YES			0%	
		South	> 30	0	0	0-20	Type V		NO			0%	
		West	> 30	0	0	0-20	Type V		NO			0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min											7000
		Total Required Fire Flow in L/s											116.7
		Required Duration of Fire Flow (hrs)											2.00
		Required Volume of Fire Flow (m^3)											840

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 5
Description: Plot 'B1', 9-Storey Medium-Rise Building
Building Area: 2792.6 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction										0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?					NO	-
		2792.6	2792.6	2792.6	2792.6	2792.6	2792.6	2792.6	2792.6	2792.6		15359.3	-
3	Determine Required Fire Flow	(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min										-	22000
4	Determine Occupancy Charge	Limited Combustible										-15%	18700
5	Determine Sprinkler Reduction	Conforms to NFPA 13										-30%	-9350
		Standard Water Supply										-10%	
		Fully Supervised										-10%	
		% Coverage of Sprinkler System										100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	Firewall / Sprinklered ?				-	-
		North	> 30	0	0	0-20	Type V	NO				0%	0
		East	20.1 to 30	34.4	9	> 100	Type I-II - Unprotected Openings	YES				0%	
		South	20.1 to 30	61.1	9	> 100	Type I-II - Unprotected Openings	YES				0%	
		West	20.1 to 30	46.3	6	> 100	Type I-II - Unprotected Openings	YES				0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min											9000
		Total Required Fire Flow in L/s											150.0
		Required Duration of Fire Flow (hrs)											2.00
		Required Volume of Fire Flow (m^3)											1080

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 6
Description: Plot 'B2', 9-Storey Medium-Rise Building
Building Area: 2656 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

<table>
<tr><th>Step</th><th>Task</th><th colspan="10">Notes</th><th>Value Used</th><th>Req'd Fire Flow (L/min)</th></tr>
<tr><td>1</td><td>Determine Type of Construction</td><td colspan="10">Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction</td><td>0.8</td><td>-</td></tr>
<tr><td rowspan="2">2</td><td rowspan="2">Determine Effective Floor Area</td><td colspan="5">Sum of Two Largest Floors + 50% of Eight Additional Floors</td><td colspan="5">Vertical Openings Protected?</td><td>NO</td><td>-</td></tr>
<tr><td>2656</td><td>2656</td><td>2656</td><td>2656</td><td>2656</td><td>2656</td><td>2656</td><td>2656</td><td>2656</td><td></td><td>14608</td><td>-</td></tr>
<tr><td>3</td><td>Determine Required Fire Flow</td><td colspan="10">(F = 220 x C x A$^{1/2}$). Round to nearest 1000 L/min</td><td>-</td><td>21000</td></tr>
<tr><td>4</td><td>Determine Occupancy Charge</td><td colspan="10">Limited Combustible</td><td>-15%</td><td>17850</td></tr>
<tr><td rowspan="4">5</td><td rowspan="4">Determine Sprinkler Reduction</td><td colspan="10">Conforms to NFPA 13</td><td>-30%</td><td rowspan="4">-8925</td></tr>
<tr><td colspan="10">Standard Water Supply</td><td>-10%</td></tr>
<tr><td colspan="10">Fully Supervised</td><td>-10%</td></tr>
<tr><td colspan="10">% Coverage of Sprinkler System</td><td>100%</td></tr>
<tr><td rowspan="5">6</td><td rowspan="5">Determine Increase for Exposures (Max. 75%)</td><td>Direction</td><td>Exposure Distance (m)</td><td>Exposed Length (m)</td><td>Exposed Height (Stories)</td><td>Length-Height Factor (m x stories)</td><td colspan="2">Construction of Adjacent Wall</td><td colspan="3">Firewall / Sprinklered ?</td><td>-</td><td>-</td></tr>
<tr><td>North</td><td>20.1 to 30</td><td>61.1</td><td>9</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td><td rowspan="4">0</td></tr>
<tr><td>East</td><td>20.1 to 30</td><td>66.4</td><td>9</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td>South</td><td>20.1 to 30</td><td>25</td><td>9</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td>West</td><td>20.1 to 30</td><td>25</td><td>6</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td rowspan="4">7</td><td rowspan="4">Determine Final Required Fire Flow</td><td colspan="11">Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min</td><td>9000</td></tr>
<tr><td colspan="11">Total Required Fire Flow in L/s</td><td>150.0</td></tr>
<tr><td colspan="11">Required Duration of Fire Flow (hrs)</td><td>2.00</td></tr>
<tr><td colspan="11">Required Volume of Fire Flow (m^3)</td><td>1080</td></tr>
</table>

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 7
Description: Plot 'B3', 34-Storey High-Rise Tower with 6-Storey Podium
Podium Area: 1302.3 m^2; Tower Area: 763.5 m^2
Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction										0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?					NO	-
		1302.3	1302.3	1302.3	1302.3	1302.3	1302.3	763.5	763.5	763.5	763.5	6736.2	-
3	Determine Required Fire Flow	(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min										-	14000
4	Determine Occupancy Charge	Limited Combustible										-15%	11900
5	Determine Sprinkler Reduction	Conforms to NFPA 13										-30%	-5950
		Standard Water Supply										-10%	
		Fully Supervised										-10%	
		% Coverage of Sprinkler System										100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall		Firewall / Sprinklered ?			-	-
		North	20.1 to 30	25	9	> 100	Type I-II - Unprotected Openings		YES			0%	0
		East	20.1 to 30	24	24	> 100	Type I-II - Unprotected Openings		YES			0%	
		South	20.1 to 30	7.6	24	> 100	Type I-II - Unprotected Openings		YES			0%	
		West	> 30	0	0	0-20	Type V		NO			0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min											6000
		Total Required Fire Flow in L/s											100.0
		Required Duration of Fire Flow (hrs)											2.00
		Required Volume of Fire Flow (m^3)											720

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 8
Description: Plot 'C1', 9-Storey Medium-Rise Building
Building Area: 1020 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

<table>
<tr><th>Step</th><th>Task</th><th colspan="10">Notes</th><th>Value Used</th><th>Req'd Fire Flow (L/min)</th></tr>
<tr><td>1</td><td>Determine Type of Construction</td><td colspan="10">Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction</td><td>0.8</td><td>-</td></tr>
<tr><td rowspan="2">2</td><td rowspan="2">Determine Effective Floor Area</td><td colspan="5">Sum of Two Largest Floors + 50% of Eight Additional Floors</td><td colspan="5">Vertical Openings Protected?</td><td>NO</td><td>-</td></tr>
<tr><td>1020</td><td>1020</td><td>1020</td><td>1020</td><td>1020</td><td>1020</td><td>1020</td><td>1020</td><td>1020</td><td></td><td>5610</td><td>-</td></tr>
<tr><td>3</td><td>Determine Required Fire Flow</td><td colspan="10">(F = 220 x C x A^{1/2}). Round to nearest 1000 L/min</td><td>-</td><td>13000</td></tr>
<tr><td>4</td><td>Determine Occupancy Charge</td><td colspan="10">Limited Combustible</td><td>-15%</td><td>11050</td></tr>
<tr><td rowspan="4">5</td><td rowspan="4">Determine Sprinkler Reduction</td><td colspan="10">Conforms to NFPA 13</td><td>-30%</td><td rowspan="4">-5525</td></tr>
<tr><td colspan="10">Standard Water Supply</td><td>-10%</td></tr>
<tr><td colspan="10">Fully Supervised</td><td>-10%</td></tr>
<tr><td colspan="10">% Coverage of Sprinkler System</td><td>100%</td></tr>
<tr><td rowspan="5">6</td><td rowspan="5">Determine Increase for Exposures (Max. 75%)</td><td>Direction</td><td>Exposure Distance (m)</td><td>Exposed Length (m)</td><td>Exposed Height (Stories)</td><td>Length-Height Factor (m x stories)</td><td colspan="2">Construction of Adjacent Wall</td><td colspan="3">Firewall / Sprinklered ?</td><td>-</td><td>-</td></tr>
<tr><td>North</td><td>> 30</td><td>0</td><td>0</td><td>0-20</td><td colspan="2">Type V</td><td colspan="3">NO</td><td>0%</td><td rowspan="4">0</td></tr>
<tr><td>East</td><td>> 30</td><td>0</td><td>0</td><td>0-20</td><td colspan="2">Type V</td><td colspan="3">NO</td><td>0%</td></tr>
<tr><td>South</td><td>10.1 to 20</td><td>25</td><td>24</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td>West</td><td>20.1 to 30</td><td>34.4</td><td>9</td><td>> 100</td><td colspan="2">Type I-II - Unprotected Openings</td><td colspan="3">YES</td><td>0%</td></tr>
<tr><td rowspan="4">7</td><td rowspan="4">Determine Final Required Fire Flow</td><td colspan="11">Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min</td><td>6000</td></tr>
<tr><td colspan="11">Total Required Fire Flow in L/s</td><td>100.0</td></tr>
<tr><td colspan="11">Required Duration of Fire Flow (hrs)</td><td>2.00</td></tr>
<tr><td colspan="11">Required Volume of Fire Flow (m³)</td><td>720</td></tr>
</table>

Stantec

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 9
Description: Plot 'C2', 24-Storey High-Rise Tower with 6-Storey Podium
Podium Area: 1660 m^2; Tower Area: 782 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction										0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?					NO	-
		1660	1660	1660	1660	1660	1660	782	782	782	782	8204	-
3	Determine Required Fire Flow	(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min										-	16000
4	Determine Occupancy Charge	Limited Combustible										-15%	13600
5	Determine Sprinkler Reduction	Conforms to NFPA 13										-30%	-6800
		Standard Water Supply										-10%	
		Fully Supervised										-10%	
		% Coverage of Sprinkler System										100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall		Firewall / Sprinklered ?			-	-
		North	10.1 to 20	25	9	> 100	Type I-II - Unprotected Openings		YES			0%	0
		East	> 30	0	0	0-20	Type V		NO			0%	
		South	> 30	0	0	0-20	Type V		NO			0%	
		West	20.1 to 30	66.4	9	> 100	Type I-II - Unprotected Openings		YES			0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min											7000
		Total Required Fire Flow in L/s											116.7
		Required Duration of Fire Flow (hrs)											2.00
		Required Volume of Fire Flow (m^3)											840

Stantec

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 10
Description: Plot 'D1', 24-Storey High-Rise Tower with 6-Storey Podium
Podium Area: 3503 m^2; Tower Area: 1564 m^2
Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction										0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?					NO	-
		3503	3503	3503	3503	3503	3503	1564	1564	1564	1564	17140	-
3	Determine Required Fire Flow	(F = 220 x C x $A^{1/2}$). Round to nearest 1000 L/min										-	23000
4	Determine Occupancy Charge	Limited Combustible										-15%	19550
5	Determine Sprinkler Reduction	Conforms to NFPA 13										-30%	-9775
		Standard Water Supply										-10%	
		Fully Supervised										-10%	
		% Coverage of Sprinkler System										100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall		Firewall / Sprinklered ?			-	-
		North	> 30	0	0	0-20	Type V		NO			0%	0
		East	> 30	0	0	0-20	Type V		NO			0%	
		South	> 30	0	0	0-20	Type V		NO			0%	
		West	10.1 to 20	25	24	> 100	Type I-II - Unprotected Openings		YES			0%	
7	Determine Final Required Fire Flow	**Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min**											**10000**
		Total Required Fire Flow in L/s											**166.7**
		Required Duration of Fire Flow (hrs)											**2.00**
		Required Volume of Fire Flow (m^3)											**1200**

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401751
Project Name: 8600 Jeanne D'Arc Boulevard (Petries Landing III)
Date: 2023-06-19
Fire Flow Calculation #: 11
Description: Plot 'D2', 24-Storey High-Rise Tower with 6-Storey Podium
Podium Area: 1658.2 m^2; Tower Area: 782 m^2

Notes: Footprint areas as per BDP Quadrangle Architects Site Plan (April 25th, 2023)

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction										0.8	-
2	Determine Effective Floor Area	Sum of Two Largest Floors + 50% of Eight Additional Floors					Vertical Openings Protected?					NO	-
		1658.2	1658.2	1658.2	1658.2	1658.2	1658.2	782	782	782	782	8196.8	-
3	Determine Required Fire Flow	(F = 220 x C x A$^{1/2}$). Round to nearest 1000 L/min										-	16000
4	Determine Occupancy Charge	Limited Combustible										-15%	13600
5	Determine Sprinkler Reduction	Conforms to NFPA 13										-30%	-6800
		Standard Water Supply										-10%	
		Fully Supervised										-10%	
		% Coverage of Sprinkler System										100%	
6	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall		Firewall / Sprinklered ?			-	-
		North	20.1 to 30	7.64	24	> 100	Type I-II - Unprotected Openings		YES			0%	0
		East	10.1 to 20	25	24	> 100	Type I-II - Unprotected Openings		YES			0%	
		South	> 30	0	0	0-20	Type V		NO			0%	
		West	> 30	0	0	0-20	Type V		NO			0%	
7	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min											7000
		Total Required Fire Flow in L/s											116.7
		Required Duration of Fire Flow (hrs)											2.00
		Required Volume of Fire Flow (m^3)											840

W:\active\160401751\design\drawing\160401751 FUS.dwg
2023/06/22 9:00 AM By: Wu, Michael

Stantec
Stantec Consulting Ltd.
300 - 1331 Clyde Avenue
Ottawa ON
Tel. 613.722.4420
www.stantec.com

Legend

Notes

Client/Project
BRIGIL
PETRIES LANDING III
8600 Jeanne D'Arc Boulevard

Figure No.
1.0

Title
FUS Exposures Sketch

C.3 Boundary Conditions (City of Ottawa)

Boundary Conditions
8600 Jeanne D'Arc Boulevard

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	1,050	17.50
Maximum Daily Demand	2,634	43.90
Peak Hour	5,790	96.50
Fire Flow Demand #1	10,002	166.70

Location

Results

Connection 1 - Jeanne D'Arc

Demand Scenario	Head (m)	Pressure[1] (psi)
Maximum HGL	113.7	88.6
Peak Hour	106.1	77.8
Max Day plus Fire Flow	102.1	72.2

[1] Ground Elevation = 51.3 m

Notes

1. System level peaking factors and domestic demands should be used for applications exceeding 500 persons.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

C.4 Preliminary H2OMAP Results Summary

Petries III H2OMap

Prepared By:

Date: 2024-03-06

Junction Results - Basic Day

ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)	Pressure (kPa)
10	2.30	51.25	113.69	88.76	611.98
12	6.00	52.51	113.69	86.97	599.64
14	0.00	53.39	113.68	85.71	590.95
16	6.50	53.83	113.68	85.08	586.61
18	2.80	52.96	113.69	86.34	595.29
20	0.00	50.30	113.70	90.13	621.42

Link Results - Basic Day

ID	FROM	TO	Length (m)	Diameter (mm)	Roughness	Flow (L/s)	Velocity (m/s)
15	JDA1	10	84.19	204	110	3.75	0.11
17	10	12	114.40	204	110	1.45	0.04
21	12	14	151.67	250	110	2.49	0.05
23	14	16	44.03	250	110	2.49	0.05
25	16	18	171.30	250	110	-4.01	0.08
27	18	JDA3	58.50	250	110	-6.81	0.14
29	20	12	83.96	250	110	7.04	0.14
31	JDA1	20	123.70	393	120	3.37	0.03
33	20	JDA3	105.81	393	120	-3.67	0.03

Junction Results - Peak Hour

ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)	Pressure (kPa)
10	12.60	51.25	105.84	77.61	535.10
12	33.00	52.51	105.78	75.73	522.14
14	0.00	53.39	105.70	74.37	512.76
16	35.60	53.83	105.68	73.71	508.21
18	15.30	52.96	105.90	75.26	518.90
20	0.00	50.30	106.09	79.31	546.82

Link Results - Peak Hour

ID	FROM	TO	Length (m)	Diameter (mm)	Roughness	Flow (L/s)	Velocity (m/s)
15	JDA1	10	84.19	204	110	20.56	0.63
17	10	12	114.40	204	110	7.96	0.24
21	12	14	151.67	250	110	13.59	0.28
23	14	16	44.03	250	110	13.59	0.28
25	16	18	171.30	250	110	-22.01	0.45
27	18	JDA3	58.50	250	110	-37.31	0.76
29	20	12	83.96	250	110	38.64	0.79
31	JDA1	20	123.70	393	120	18.51	0.15
33	20	JDA3	105.81	393	120	-20.13	0.17

Fire Flow Results - Max Day + 10,000L/min

ID	Static Demand (L/s)	Static Pressure (kPa)	Static Pressure (psi)	Static Head (m)	Fire Flow Demand (L/s)	Residual Pressure (psi)	Available Flow (L/s)	Available Pressure (psi)
10	5.70	497.80	72.20	102.04	166.67	65.61	531.74	20.01
12	15.00	485.32	70.39	102.03	166.67	67.45	874.72	20.01
16	16.20	472.15	68.48	102.00	166.67	62.21	557.21	20.01
18	6.90	481.19	69.79	102.05	166.67	66.63	805.07	20.01

Appendix D Sanitary

D.1 Sanitary Sewer Flow

Stantec

SUBDIVISION:

PETRIES III

DATE: 8/4/2023
REVISION: 1
DESIGN BY: MJS
CHECKED BY: RB

SANITARY SEWER DESIGN SHEET
(City of Ottawa)

FILE NUMBER: 160401751

DESIGN PARAMETERS

MAX PEAK FACTOR (RES.)=	4.0	AVG. DAILY FLOW / PERSON	280 l/p/day	MINIMUM VELOCITY	0.60 m/s
MIN PEAK FACTOR (RES.)=	2.0	COMMERCIAL	28,000 l/ha/day	MAXIMUM VELOCITY	3.00 m/s
PEAKING FACTOR (INDUSTRIAL):	2.4	INDUSTRIAL (HEAVY)	55,000 l/ha/day	MANNINGS n	0.013
PEAKING FACTOR (ICI >20%):	1.5	INDUSTRIAL (LIGHT)	35,000 l/ha/day	BEDDING CLASS	B
PERSONS / SINGLE	3.4	INSTITUTIONAL	28,000 l/ha/day	MINIMUM COVER	2.50 m
PERSONS / TOWNHOME	2.7	INFILTRATION	0.33 l/s/Ha	HARMON CORRECTION FACTOR	0.8
PERSONS / APARTMENT	1.8				

LOCATION			RESIDENTIAL AREA AND POPULATION									COMMERCIAL		INDUSTRIAL (L)		INDUSTRIAL (H)		INSTITUTIONAL		GREEN / UNUSED		C+I+I	INFILTRATION			TOTAL	PIPE								
AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (ha)	UNITS SINGLE	UNITS TOWN	UNITS APT	POP.	CUMULATIVE AREA (ha)	CUMULATIVE POP.	PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	FLOW (l/s)	LENGTH (m)	DIA (mm)	MATERIAL	CLASS	SLOPE (%)	CAP. (FULL) (l/s)	CAP. V PEAK FLOW (%)	VEL. (FULL) (m/s)	VEL. (ACT.) (m/s)
G3A	3	2	0.00	0	0	0	0	0.00	0	3.80	0.0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.54	0.54	0.0	0.54	0.54	0.2	0.2	135.3	250	PVC	SDR 35	0.40	38.3	**0.46%**	0.77	0.17
R4A, G4A	4	2	0.70	0	0	0	705	0.70	705	3.31	7.6	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.82	0.82	0.0	1.52	1.52	0.5	8.1	114.4	250	PVC	SDR 35	0.40	38.3	**21.05%**	0.77	0.51
R2A, G2A	2	1	0.75	0	0	0	1852	1.45	2557	3.00	24.9	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.60	1.96	0.0	1.35	3.41	1.1	26.0	104.5	250	PVC	SDR 35	0.40	38.3	**67.77%**	0.77	0.72
																												250							
R6A, R6B, G6A, G6B, G6C	6	5	0.79	0	0	0	2857	0.79	2857	2.97	27.5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.31	1.31	0.0	2.10	2.10	0.7	28.2	234.7	250	PVC	SDR 35	0.40	38.3	**73.47%**	0.77	0.74
																												250							

D.2 Sanitary Sewer Capacity

Brandrick, Robert

From:	Polyak, Alex <alex.polyak@ottawa.ca>
Sent:	Thursday, August 31, 2023 9:09 AM
To:	Wu, Michael
Cc:	Kilborn, Kris; Brandrick, Robert; Murshid, Shoma; Martinov, Amya
Subject:	RE: 8600 Jeanne D'Arc Boulevard (Petries Landing III) Sanitary Sewer Capacity

Hello Michael,

Our apologies, I am not sure why it is taking us this long to get back to you with the sanitary sewer capacity. I'll follow up with our group.

Regards,

Oleksandr (Alex) Polyak, B.Eng., P.Eng
Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.
Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement économique

City of Ottawa | Ville d'Ottawa
110 Laurier Ave., 4th Fl East, Ottawa ON K1P 1J1
Email: alex.polyak@ottawa.ca
Cell : 613-857-4380
www.Ottawa.ca

From: Wu, Michael <Michael.Wu@stantec.com>
Sent: August 18, 2023 10:30 AM
To: Polyak, Alex <alex.polyak@ottawa.ca>
Cc: Kilborn, Kris <kris.kilborn@stantec.com>; Brandrick, Robert <Robert.Brandrick@stantec.com>; Murshid, Shoma <Shoma.Murshid@ottawa.ca>; Martinov, Amya <amya.martinov@ottawa.ca>
Subject: RE: 8600 Jeanne D'Arc Boulevard (Petries Landing III) Sanitary Sewer Capacity

Good morning, Alex, anything new on on when we can receive the confirmation on the sanitary sewer capacity?

Thanks,

Michael Wu EIT
Civil Engineering Intern, Community Development

Direct: 1 (613) 738-6033
Michael.Wu@stantec.com

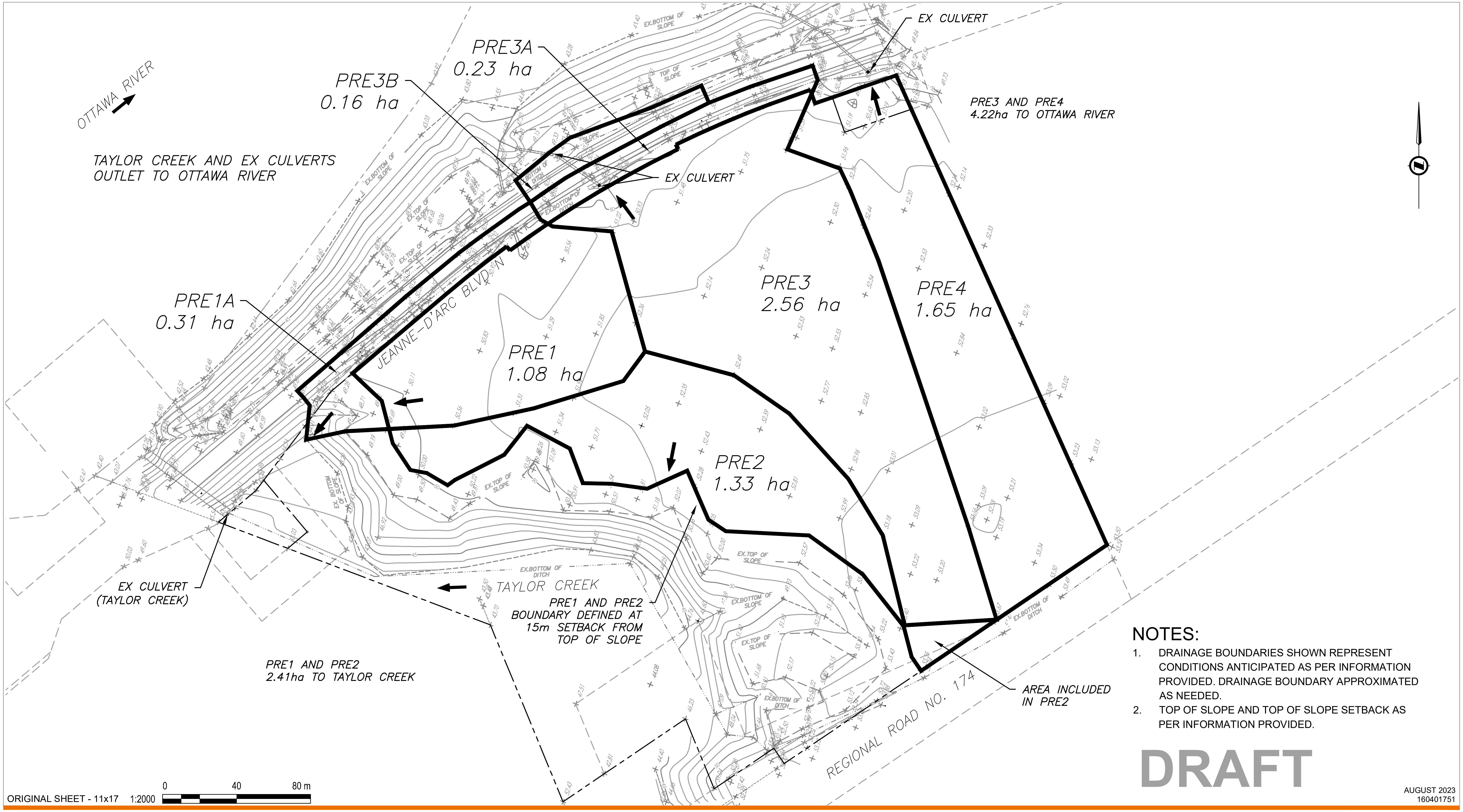
Stantec
300-1331 Clyde Avenue
Ottawa ON K2C 3G4

'

'

Appendix E Storm

E.1 Pre-Development



NOTES:

1. DRAINAGE BOUNDARIES SHOWN REPRESENT CONDITIONS ANTICIPATED AS PER INFORMATION PROVIDED. DRAINAGE BOUNDARY APPROXIMATED AS NEEDED.
2. TOP OF SLOPE AND TOP OF SLOPE SETBACK AS PER INFORMATION PROVIDED.

DRAFT

AUGUST 2023
160401751

\\ca0218-ppfss01\work_group\01-604\active\160401751\design\analysis\swm\petrie-dr.dwg
12:29 PM 4 August 2023 - Brandrick, Robert

Stantec

300 – 1331 Clyde Avenue
Ottawa, ON K2C 3G4
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Client/Project
BRIGIL
PETRIES LANDING III
FUNCTIONAL SERVICING

Figure No.
E1.1

Title
PRE-DEVELOPMENT
STORM DRAINAGE AREA

Stormwater Management Calculations

File No: **160401751**
Project: **Petries III**
Date: **03-Aug-23**

SWM Approach:
Post-development to Pre-development flows (Taylor Creek)

Pre-Development Site Conditions:

Overall Runoff Coefficient for Site and Sub-Catchment Areas

Runoff Coefficient Table

Sub-catchment Area			**Area (ha) "A"**		**Runoff Coefficient "C"**	**"A x C"**		**Overall Runoff Coefficient**
Catchment Type	**ID / Description**							
Uncontrolled								
Pre-Development 1	PRE1	Hard	0.00		0.9	0.00		
Taylor Creek		Soft	1.08		0.3	0.32		
		Subtotal		1.08			0.32	0.30
Pre-Development 1A	PRE1A	Hard	0.10		0.9	0.09		
Taylor Creek		Soft	0.21		0.2	0.04		
		Subtotal		0.31			0.13	0.43
Pre-Development 2	PRE2	Hard	0.00		0.9	0.00		
Taylor Creek		Soft	1.33		0.3	0.40		
		Subtotal		1.33			0.40	0.30
Pre-Development 3	PRE3	Hard	0.00		0.9	0.00		
Ottawa River		Soft	2.56		0.3	0.77		
		Subtotal		2.56			0.77	0.30
Pre-Development 3A	PRE3A	Hard	0.11		0.9	0.10		
Ottawa River		Soft	0.12		0.2	0.02		
		Subtotal		0.23			0.12	0.53
Pre-Development 3B	PRE3B	Hard	0.05		0.9	0.05		
Ottawa River		Soft	0.11		0.2	0.02		
		Subtotal		0.16			0.07	0.42
Pre-Development 4	PRE4	Hard	0.00		0.9	0.00		
Ottawa River		Soft	1.65		0.3	0.50		
		Subtotal		1.65			0.50	0.30
Total				**7.32**			**2.31**	
Overall Runoff Coefficient= C:								**0.32**

Total Tributary Surface Areas #1 (Taylor Creek)	**2.72**
Total Tributary Surface Areas #2 (Ottawa River)	**2.95**
Total Tributary Surface Areas #3 (Ottawa River)	**1.65 ha**
Total Tributary Surface Areas (Controlled and Uncontrolled)	**7.32 ha**

Project # 160401751, Petries III
Target Release Rates

5-yr Intensity
City of Ottawa

$I = a/(T+b)^c$		
	a =	998.071
	b =	6.053
	c =	0.814

5-Year Pre-Development Target Release to Taylor Creek

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
PRE1	1.08	0.30	25	60.90	**54.9**
PRE1A	0.31	0.43	10	104.19	**38.2**
PRE2	1.33	0.30	26	59.35	**65.8**
				Total	**158.9**

Tc from calculated values using topographic survey data
Tc for PRE1A set to 10 based on existing roadway condition

5-Year Pre-Development Target Release to Ottawa River (West Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
PRE3	2.56	0.30	41	43.42	**92.7**
PRE3A	0.23	0.53	10	104.19	**35.6**
PRE3B	0.16	0.42	10	104.19	**19.4**
				Total	**147.7**

Tc from calculated values using topographic survey data
Tc for PRE3A and PRE3B set to 10 based on existing roadway condition

5-Year Pre-Development Target Release to Ottawa River (East Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
PRE4	1.65	0.30	45	40.63	**55.9**

Tc from calculated values using topographic survey data

Project # 160401751, Petries III
Target Release Rates

100-yr Intensity
City of Ottawa

$I = a/(T+b)^c$		
	a =	1735.688
	b =	6.014
	c =	0.820

100-Year Pre-Development Target Release to Taylor Creek

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
PRE1	1.08	0.30	25	103.85	**93.5**
PRE1A	0.31	0.43	10	178.56	**65.5**
PRE2	1.33	0.30	26	101.18	**112.2**
				Total	**271.3**

Tc from calculated values using topographic survey data
Tc for PRE1A set to 10 based on existing roadway condition

100-Year Pre-Development Target Release to Ottawa River (West Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
PRE3	2.56	0.30	41	73.83	**157.6**
PRE3A	0.23	0.53	10	178.56	**61.1**
PRE3B	0.16	0.42	10	178.56	**33.3**
				Total	**251.9**

Tc from calculated values using topographic survey data
Tc for PRE3A and PRE3B set to 10 based on existing roadway condition

100-Year Pre-Development Target Release to Ottawa River (East Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
PRE4	1.65	0.30	45	69.05	**95.0**

Tc from calculated values using topographic survey data

Project	**Petrie III**	No.	**160401751**
PRE-DEVELOPMENT CONDITIONS **Calculation of Time of Concentration**			
Revision:	0	Prepared By:	RB
Revision Date	August 2023	Checked By:	

OVERLAND SHEET FLOW TIME

Runoff Coeffient = 0.30

Length = 150 m (longest overland flow path)
DEM Slope = 2.13% (along overland flow path)

C > 0.4 **Bransby Williams Method**
tc = 0.057 x L /($S_w^{0.2}$ x $A^{0.1}$)

L	0	m (longest flow path)
S_w	2.13%	
A	1.08	ha
t_c	0.00	min

C ≤ 0.4 **Airport Method**
tc = [3.26 x (1.1-C) x $L^{0.5}$] / $S_w^{0.33}$

L	150	m
S_w	2.13%	
C	0.30	
t_c	24.9	min

SHALLOW CONCENTRATED FLOW TIME

Uplands Method

1. Channel Segment 1 - Overland

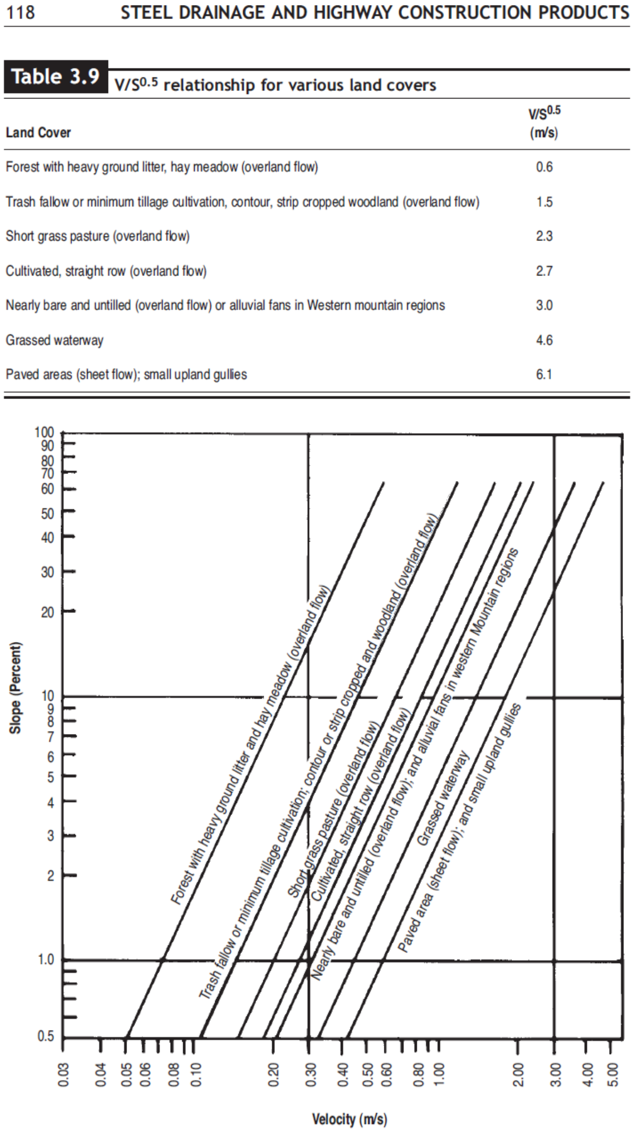
Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Short grass pasture (overland flow)
k = 2.3
V = k * S^(1/2)
Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

2. Channel Segment 2 - Overland within creek channel

Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Forest with heavy ground liter, hay meadow (overland flow)
k = 0.6
V = k * S^(1/2)
Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

Therefore, Total T_c = **24.9** min
0.4 hrs

Uplands Method Chart



118 STEEL DRAINAGE AND HIGHWAY CONSTRUCTION PRODUCTS

Table 3.9 $V/S^{0.5}$ relationship for various land covers

Land Cover	$V/S^{0.5}$ (m/s)
Forest with heavy ground litter, hay meadow (overland flow)	0.6
Trash fallow or minimum tillage cultivation, contour, strip cropped woodland (overland flow)	1.5
Short grass pasture (overland flow)	2.3
Cultivated, straight row (overland flow)	2.7
Nearly bare and untilled (overland flow) or alluvial fans in Western mountain regions	3.0
Grassed waterway	4.6
Paved areas (sheet flow); small upland gullies	6.1

Stantec	Project	**Petrie III**	No.	**160401751**
	PRE-DEVELOPMENT CONDITIONS Calculation of Time of Concentration			
	Revision:	0	Prepared By:	RB
	Revision Date	August 2023	Checked By:	

OVERLAND SHEET FLOW TIME

Runoff Coeffient = 0.30

Length = 100 m (longest overland flow path)
DEM Slope = 1.00% (along overland flow path)

C > 0.4 **Bransby Williams Method**
tc = 0.057 x L /($S_w^{0.2}$ x $A^{0.1}$)

L	0	m (longest flow path)
S_w	1.00%	
A	1.33	ha
t_c	0.00	min

C ≤ 0.4 **Airport Method**
tc = [3.26 x (1.1-C) x $L^{0.5}$] / $S_w^{0.33}$

L	100	m
S_w	1.00%	
C	0.30	
t_c	26.1	min

SHALLOW CONCENTRATED FLOW TIME

Uplands Method

1. Channel Segment 1 - Overland

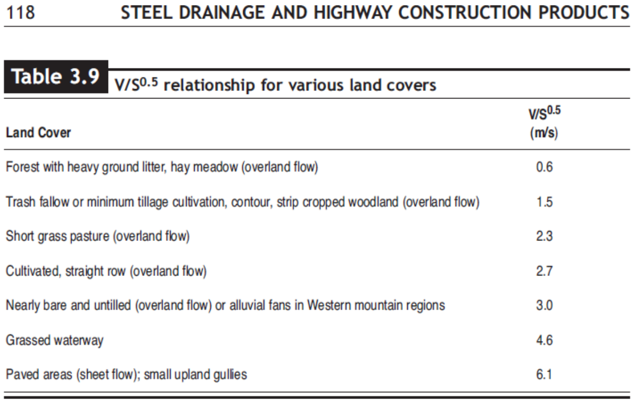
Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Short grass pasture (overland flow)
k = 2.3
V = k * S^(1/2)
Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

2. Channel Segment 2 - Overland within creek channel

Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Forest with heavy ground liter, hay meadow (overland flow)
k = 0.6
V = k * S^(1/2)
Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

Therefore, Total T_c = **26.1** min
0.4 hrs

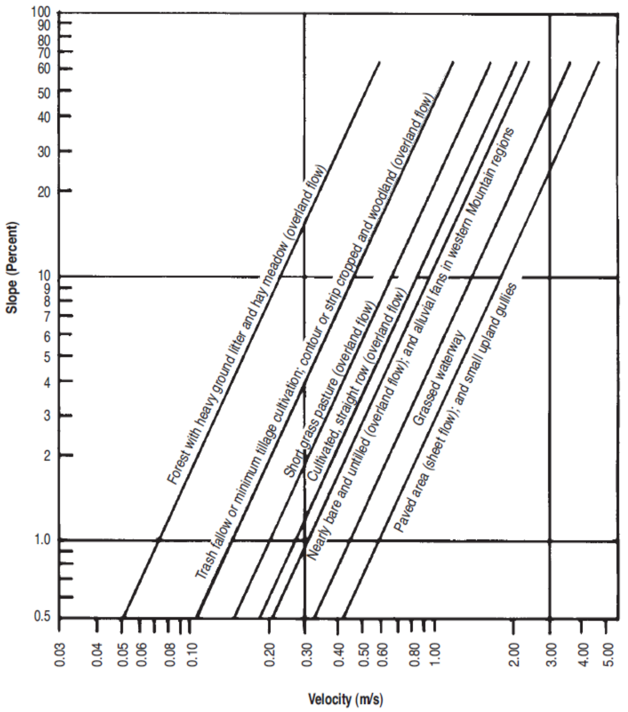
Uplands Method Chart



118 STEEL DRAINAGE AND HIGHWAY CONSTRUCTION PRODUCTS

Table 3.9 $V/S^{0.5}$ relationship for various land covers

Land Cover	$V/S^{0.5}$ (m/s)
Forest with heavy ground litter, hay meadow (overland flow)	0.6
Trash fallow or minimum tillage cultivation, contour, strip cropped woodland (overland flow)	1.5
Short grass pasture (overland flow)	2.3
Cultivated, straight row (overland flow)	2.7
Nearly bare and untilled (overland flow) or alluvial fans in Western mountain regions	3.0
Grassed waterway	4.6
Paved areas (sheet flow); small upland gullies	6.1



Project	**Petrie III**	No.	**160401751**
PRE-DEVELOPMENT CONDITIONS **Calculation of Time of Concentration**			
Revision:	0	Prepared By:	RB
Revision Date	August 2023	Checked By:	

OVERLAND SHEET FLOW TIME

Runoff Coeffient = 0.30

Length = 316 m (longest overland flow path)
DEM Slope = 1.52% (along overland flow path)

C > 0.4 **Bransby Williams Method**

$tc = 0.057 \times L /(S_w^{0.2} \times A^{0.1})$

L 0 m (longest flow path)
S_w 1.52%
A 2.56 ha
t_c 0.00 min

C ≤ 0.4 **Airport Method**

$tc = [3.26 \times (1.1\text{-}C) \times L^{0.5}] / S_w^{0.33}$

L 316 m
S_w 1.52%
C 0.30
t_c 40.4 min

SHALLOW CONCENTRATED FLOW TIME

Uplands Method

1. Channel Segment 1 - Overland

Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Short grass pasture (overland flow)
k = 2.3

V = k * S^(1/2)

Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

2. Channel Segment 2 - Overland within creek channel

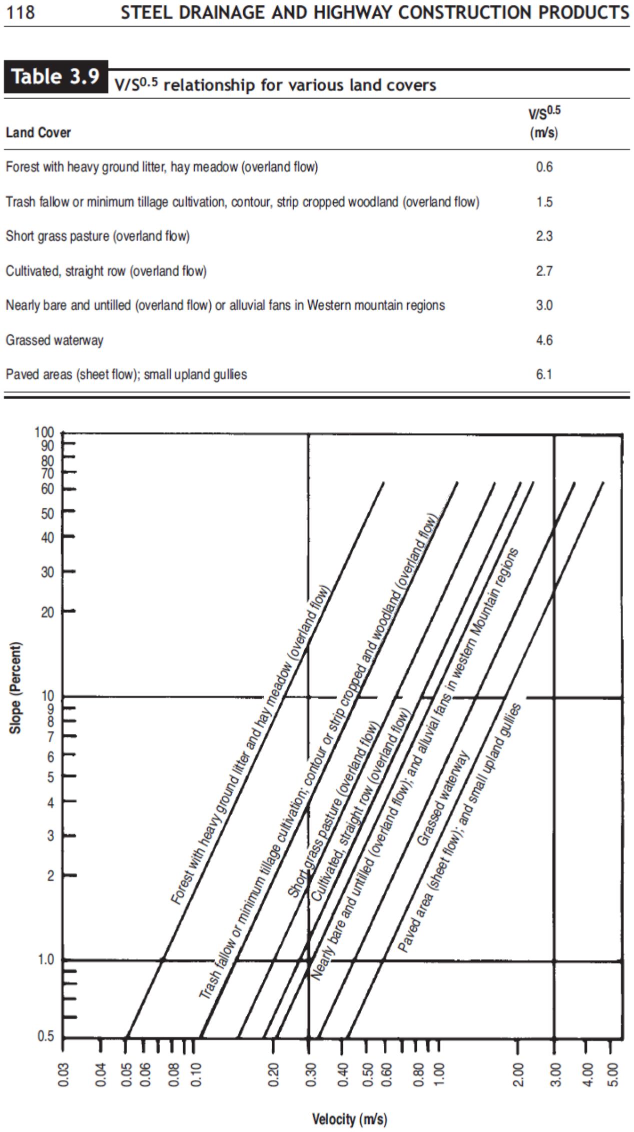
Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Forest with heavy ground liter, hay meadow (overland flow)
k = 0.6

V = k * S^(1/2)

Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

Therefore, Total T_c = **40.4** min
0.7 hrs

Uplands Method Chart



118 STEEL DRAINAGE AND HIGHWAY CONSTRUCTION PRODUCTS

Table 3.9 $V/S^{0.5}$ relationship for various land covers

Land Cover	$V/S^{0.5}$ (m/s)
Forest with heavy ground litter, hay meadow (overland flow)	0.6
Trash fallow or minimum tillage cultivation, contour, strip cropped woodland (overland flow)	1.5
Short grass pasture (overland flow)	2.3
Cultivated, straight row (overland flow)	2.7
Nearly bare and untilled (overland flow) or alluvial fans in Western mountain regions	3.0
Grassed waterway	4.6
Paved areas (sheet flow); small upland gullies	6.1

Project	**Petrie III**	No.	**160401751**
PRE-DEVELOPMENT CONDITIONS **Calculation of Time of Concentration**			
Revision:	0	Prepared By:	RB
Revision Date	August 2023	Checked By:	

OVERLAND SHEET FLOW TIME

Runoff Coeffient = 0.30

Length = 300 m (longest overland flow path)
DEM Slope = 1.07% (along overland flow path)

C > 0.4 **Bransby Williams Method**
$tc = 0.057 \times L /(S_w^{0.2} \times A^{0.1})$

L 0 m (longest flow path)
S_w 1.07%
A 1.65 ha
t_c 0.00 min

C ≤ 0.4 **Airport Method**
$tc = [3.26 \times (1.1\text{-}C) \times L^{0.5}] / S_w^{0.33}$

L 300 m
S_w 1.07%
C 0.30
t_c 44.2 min

SHALLOW CONCENTRATED FLOW TIME

Uplands Method

1. Channel Segment 1 - Overland

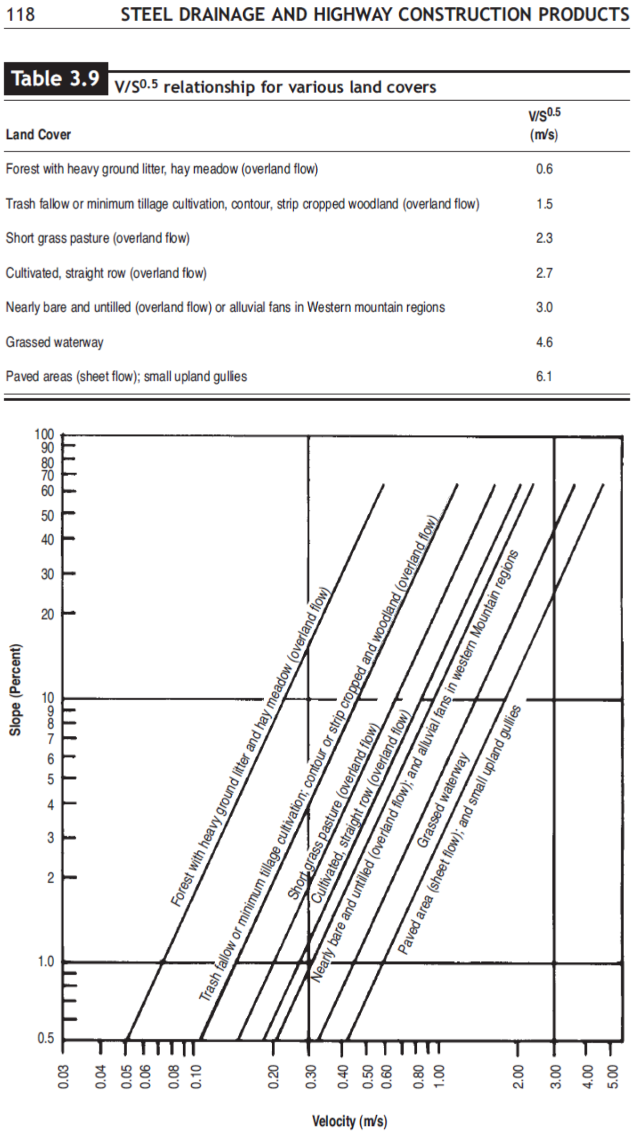
Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Short grass pasture (overland flow)
k = 2.3
V = k * S^(1/2)
Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

2. Channel Segment 2 - Overland within creek channel

Length = m (longest flow path)
DEM Slope (along overland flow path)
Channel Type = Forest with heavy ground liter, hay meadow (overland flow)
k = 0.6
V = k * S^(1/2)
Velocity = 0.00 m/s
Channel Length = 0 m
Travel time = 0.0 min

Therefore, Total T_c = 44.2 min
0.7 hrs

Uplands Method Chart



118 STEEL DRAINAGE AND HIGHWAY CONSTRUCTION PRODUCTS

Table 3.9 $V/S^{0.5}$ relationship for various land covers

Land Cover	$V/S^{0.5}$ (m/s)
Forest with heavy ground litter, hay meadow (overland flow)	0.6
Trash fallow or minimum tillage cultivation, contour, strip cropped woodland (overland flow)	1.5
Short grass pasture (overland flow)	2.3
Cultivated, straight row (overland flow)	2.7
Nearly bare and untilled (overland flow) or alluvial fans in Western mountain regions	3.0
Grassed waterway	4.6
Paved areas (sheet flow); small upland gullies	6.1

E.2 Post-Development

Stormwater Management Calculations

File No: **160401751**
Project: **Petries III**
Date: **03-Aug-23**

SWM Approach:
Post-development to Pre-development flows (Taylor Creek)

Post-Development Site Conditions:

Overall Runoff Coefficient for Site and Sub-Catchment Areas

Runoff Coefficient Table

Sub-catchment Area			Area (ha) "A"		Runoff Coefficient "C"	"A x C"		Overall Runoff Coefficient
Catchment Type	**ID / Description**							
Controlled - Ottawa River (West Culvert)								
Block A - Private	C102A	Building	0.82		0.90	0.74		
		Site	0.70		0.75	0.53		
		Subtotal		1.52			1.26	0.83
Block B - Private	C101A	Building	0.75		0.90	0.68		
		Site	0.61		0.75	0.46		
		Subtotal		1.36			1.13	0.83
Road - Public	C101B	Road	0.54		0.80	0.43		
Ottawa River		Soft	0.00		0.20	0.00		
		Subtotal		0.54			0.43	0.80
		Subtotal		3.42			2.83	0.83
Controlled - Ottawa River (East Culvert)								
Block C - Private	C201B	Building	0.27		0.90	0.24		
Ottawa River		Site	0.34		0.75	0.26		
		Subtotal		0.61			0.50	0.82
Block D - Private	C201C	Building	0.52		0.90	0.47		
Ottawa River		Site	0.43		0.75	0.32		
		Subtotal		0.95			0.79	0.83
Road - Public	C201A	Road	0.54		0.80	0.43		
Ottawa River		Soft	0.00		0.20	0.00		
		Subtotal		0.54			0.43	0.80
		Subtotal		2.10			1.72	0.82

Stormwater Management Calculations

File No: **160401751**
Project: **Petries III**
Date: **03-Aug-23**

SWM Approach:
Post-development to Pre-development flows (Taylor Creek)

Post-Development Site Conditions:

Overall Runoff Coefficient for Site and Sub-Catchment Areas

Runoff Coefficient Table

Sub-catchment Area			Area (ha)	Runoff Coefficient		Overall Runoff
Catchment Type	**ID / Description**		**"A"**	**"C"**	**"A x C"**	**Coefficient**
Uncontrolled - Taylor Creek						
Pre-Development 1A	Pre1A	Hard	0.10	0.90	0.09	
Taylor Creek		Soft	0.21	0.20	0.04	
		Subtotal	0.31		0.13	0.43
Block A Buffer	UNC-1	Hard	0.00	0.90	0.00	
Taylor Creek		Soft	0.22	0.35	0.08	
		Subtotal	0.22		0.08	0.35
Park - Public	UNC-2	Hard	0.00	0.90	0.00	
Taylor Creek		Soft	0.66	0.35	0.23	
		Subtotal	0.66		0.23	0.35
Block D Buffer	UNC-3	Hard	0.00	0.90	0.00	
Taylor Creek		Soft	0.22	0.35	0.08	
		Subtotal	0.22		0.08	0.35
Uncontrolled - Ottawa River						
Pre-Development 3A	PRE3A	Hard	0.11	0.9	0.10	
Ottawa River		Soft	0.12	0.2	0.02	
		Subtotal	0.23		0.12	0.53
Pre-Development 3B	PRE3B	Hard	0.05	0.9	0.05	
Ottawa River		Soft	0.11	0.2	0.02	
		Subtotal	0.16		0.07	0.42

Total Controlled Areas - Ottawa River (West Culvert)	**3.42 ha**
Total Controlled Areas - Ottawa River (East Culvert)	**2.10 ha**
Total Uncontrolled Areas - Taylor Creek	**1.41 ha**
Total Uncontrolled Areas - Ottawa River	**0.39 ha**
Total Tributary Area to Outlet	**7.32 ha**

Project # 160401751, Petries III
Design Discharge Rates

5-yr Intensity
City of Ottawa

$I = a/(T+b)^c$		
	a =	998.071
	b =	6.053
	c =	0.814

5-Year Post-Development Discharge to Taylor Creek

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
Pre1A	0.31	0.43	10	104.19	**38.2**
UNC-1	0.22	0.35	10	104.19	**22.3**
UNC-2	0.66	0.35	10	104.19	**66.9**
UNC-3	0.22	0.35	10	104.19	**22.3**
				Total	**149.8**

5-Year Post-Development Discharge to Ottawa River (West Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
C102A	1.52	0.83	10	104.19	**365.8**
C101A	1.36	0.83	10	104.19	**328.0**
C101B	0.54	0.80	10	104.19	**125.1**
				Total	**819.0**

5-Year Post-Development Discharge to Ottawa River (East Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
C201B	0.61	0.82	10	104.19	**144.2**
C201C	0.95	0.83	10	104.19	**229.0**
C201A	0.54	0.80	10	104.19	**125.1**
				Total	**498.4**

Project # 160401751, Petries III
Design Discharge Rates

100-yr Intensity
City of Ottawa

$I = a/(T+b)^c$		
	a =	1735.688
	b =	6.014
	c =	0.820

100-Year Post-Development Discharge to Taylor Creek

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
Pre1A	0.31	0.43	10	178.56	**65.5**
UNC-1	0.22	0.35	10	178.56	**38.2**
UNC-2	0.66	0.35	10	178.56	**114.7**
UNC-3	0.22	0.35	10	178.56	**38.2**

Total **256.6**

100-Year Post-Development Discharge to Ottawa River (West Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
C102A	1.52	0.83	10	178.56	**626.9**
C101A	1.36	0.83	10	178.56	**562.2**
C101B	0.54	0.80	10	178.56	**214.4**

Total **1403.6**

100-Year Post-Development Discharge to Ottawa River (East Culvert)

Area Label	Area (ha)	Coefficient (C)	Tc (min)	Intensity (mm/hr)	Qtarget (L/s)
C201B	0.61	0.82	10	178.56	**247.2**
C201C	0.95	0.83	10	178.56	**392.4**
C201A	0.54	0.80	10	178.56	**214.4**

Total **854.0**

PETRIES III

DATE: 2023-08-04
REVISION: 1
DESIGNED BY: MJS
CHECKED BY: RB

STORM SEWER DESIGN SHEET
(City of Ottawa)

FILE NUMBER: **160401751**

DESIGN PARAMETERS

$I = a / (t+b)^c$ (As per City of Ottawa Guidelines, 2012)

	1:2 yr	1:5 yr	1:10 yr	1:100 yr
a =	732.951	998.071	1174.184	1735.688
b =	6.199	6.053	6.014	6.014
c =	0.810	0.814	0.816	0.820

MANNING'S n = 0.013
BEDDING CLASS = B
MINIMUM COVER: 2.00 m
TIME OF ENTRY 10 min

LOCATION			DRAINAGE AREA																									PIPE SELECTION											
AREA ID	FROM	TO	AREA	AREA	AREA	AREA	AREA	C	C	C	C	A x C	ACCUM	A x C	ACCUM.	A x C	ACCUM.	A x C	ACCUM.	T of C	$I_{2\text{-YEAR}}$	$I_{5\text{-YEAR}}$	$I_{10\text{-YEAR}}$	$I_{100\text{-YEAR}}$	$Q_{CONTROL}$	ACCUM.	Q_{ACT}	LENGTH	PIPE WIDTH	PIPE	PIPE	MATERIAL	CLASS	SLOPE	Q_{CAP}	% FULL	VEL.	VEL.	TIME OF
NUMBER	M.H.	M.H.	(2-YEAR)	(5-YEAR)	(10-YEAR)	(100-YEAR)	(ROOF)	(2-YEAR)	(5-YEAR)	(10-YEAR)	(100-YEAR)	(2-YEAR)	AxC (2YR)	(5-YEAR)	AxC (5YR)	(10-YEAR)	AxC (10YR)	(100-YEAR)	AxC (100YR)							$Q_{CONTROL}$	(CIA/360)		OR DIAMETE	HEIGHT	SHAPE				(FULL)		(FULL)	(ACT)	FLOW
			(ha)	(ha)	(ha)	(ha)	(ha)	(-)	(-)	(-)	(-)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(min)	(mm/h)	(mm/h)	(mm/h)	(mm/h)	(L/s)	(L/s)	(L/s)	(m)	(mm)	(mm)	(-)	(-)	(-)	%	(L/s)	(-)	(m/s)	(m/s)	(min)
C201B, C201A, C201C	201	200	0.00	2.10	0.00	0.00	0.00	0.00	0.82	0.00	0.00	0.000	0.000	1.721	1.721	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	498.2	202.3	600	600	CIRCULAR	CONCRETE	-	0.70	535.9	**92.96%**	1.84	1.89	1.78
	200	HDWL 2	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.000	1.721	0.000	0.000	0.000	0.000	11.78	70.57	95.63	112.06	163.75	0.0	0.0	457.3	18.9	675	675	CIRCULAR	CONCRETE	-	0.50	620.1	**73.74%**	1.68	1.61	0.19
																				11.98									675	675									
C101A, C101B	101	100	0.00	1.90	0.00	0.00	0.00	0.00	0.82	0.00	0.00	0.000	0.000	1.562	1.562	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	452.2	214.0	600	600	CIRCULAR	CONCRETE	-	0.70	535.9	**84.37%**	1.84	1.84	1.94
																				11.94																			
	104	103	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	0.0	90.5	450	450	CIRCULAR	CONCRETE	-	0.50	210.3	**0.00%**	1.28	0.00	0.00
	103	102	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	0.0	68.9	450	450	CIRCULAR	CONCRETE	-	0.50	210.3	**0.00%**	1.28	0.00	0.00
C102A	102	100	0.00	1.52	0.00	0.00	0.00	0.00	0.82	0.00	0.00	0.000	0.000	1.246	1.246	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	360.7	123.4	600	600	CIRCULAR	CONCRETE	-	0.45	429.7	**83.94%**	1.47	1.47	1.39
																				11.39																			
	100	HDWL 1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.000	2.809	0.000	0.000	0.000	0.000	11.94	70.08	94.95	111.26	162.58	0.0	0.0	740.8	60.2	900	900	CIRCULAR	CONCRETE	-	0.20	844.6	**87.71%**	1.29	1.30	0.77
																				12.71									900	900									

E.3 PCSWMM Methodology

PCSWMM Methodology

The use of PCSWMM for modeling of the site hydrology and hydraulics allows for an analysis of the systems response during various design storm events. It also allows for the analysis to use a dual conduit system to represent the minor and major drainage systems, with 1) closed circular and rectangular conduits representing the sewers; 2) irregular conduits using street-shaped cross-sections (as transects) to represent the saw-toothed overland road network from high-point to low-point; 3) storage or junction nodes representing maintenance holes (MH), catch basins (CB), connections between the road conduits, and internal storage conditions within site blocks where separate runoff storage control is required..

The dual conduit systems are connected via orifice or outlet objects (which represent Inlet control devices) from CB to MH. Subcatchments, defining the contributing surface runoff to the drainage system, are linked to the storage node representing the CB to direct runoff hydrographs to the minor system. The following figure offers a schematic representation of a typical dual drainage analysis configuration in PCSWMM.

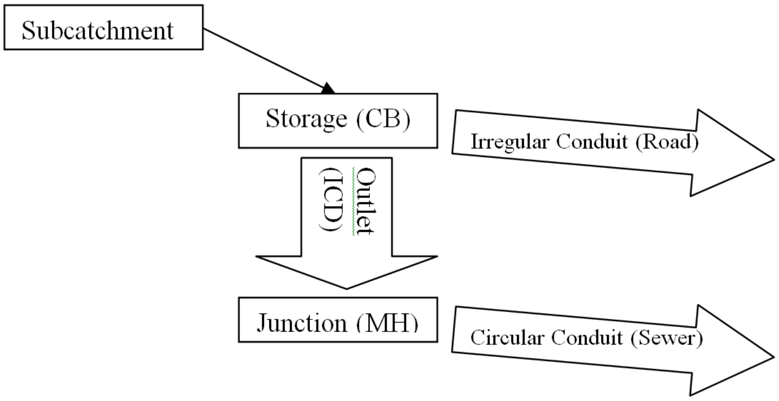


Figure: Schematic Representing PCSWMM Object Roles

The following describes the general conditions typically applied for each of the primary model inputs. Where non-typical conditions are needed, additional detail is provided with the project specific PCSWMM input information provided.

Design Storms

The typical storm distributions, as described by the City of Ottawa Sewer Design Guidelines (OSDG), are assessed: 3-hour Chicago Storm distribution for the 2, 5, and 100-year return periods, the 24-hour SCS Storm distribution for the 100-year return period.

To ‘stress test’ the system a ‘climate change’ scenario is created by adding 20% of the individual intensity values of the 100-year Chicago design storm event at each specified time step.

Subcatchments

General parameters are applied to each subcatchment based on the OSDG. These include parameters for the infiltration method and associated values, Manning's 'n' for pervious and impervious surfaces, and depression storage values for pervious and impervious surface.

The following summarizes the general subcatchment parameters applied to PCSWM analyses.

Parameter	Value
Infiltration Method	Horton
Max. Infiltration Rate (mm/hr)	76.2
Min. Infiltration Rate (mm/hr)	13.2
Decay Constant (1/hr)	4.14
Drying Time (d)	7
N Impervious	0.013
N Pervious	0.25
Dstore Imperv. (mm)	1.57
Dstore Perv. (mm)	4.67
Zero Imperv. (%)	0

Subcatchment areas are defined from high-point to high-point where sags occur. Subcatchment width is determined by calculating 2.0 x primary flow segment length (length of overland flow path measured from high-point to high-point) for street (double-sided) catchments, 1.5 x primary flow segment length for single-loaded roads, 1.0 x primary flow segment length for single-sided catchments, or by multiplying the subcatchment area by 225 m where a street segment flow path has not otherwise been defined.

Subcatchment imperviousness is calculated based on the project conditions related to grading and anticipated finished surface treatments. Where applicable imperviousness is converted to or from the equivalent Rational Method runoff coefficient, the relationship C = (Imp. x 0.7) + 0.2 is used.

Note that recent changes in interpretation of the OSDG introduced the requirement to determine the proposed subcatchment imperviousness based on maximum zoning constraints rather than those of the builder anticipated maximum building size or based on other prevailing criteria such as minimum tree setbacks.

Subcatchment slope is applied based on the project grading condition applicable to each area defined.

Subcatchment routing is generally applied at 100% routed to the outlet assigned.

Junctions

Junctions are used to join conduits where the details of the hydraulic analysis by the model are less sensitive to potential irregularity. The use of storage nodes rather than junctions for conduit connections is generally considered to allow for a more stable hydraulic modeling condition, even with no specific storage condition applied to a storage node.

Storage Nodes – Catch Basins (Sags)

For storage nodes representing CBs, the invert elevation of the storage node represents the invert of the CB. The rim elevation used on the CB storage nodes is not directly representative of a design value. The rim elevation of the storage node is set at an elevation to allow for representation of depth of surface water over the adjacent road high point storage node to allow routing from one surface storage to the next.

The functional storage curve values are set to zero for each CB. This allows the storage at each road sag supported by the CB to be represented by the transect defined for the roadway cross-section.

Where additional detail is needed to define a storage condition at a road sag or other low point, a tabular storage condition with a defined storage curve may be used. A tabular storage curve is defined by a depth and corresponding surface area that is derived from the associated design condition.

Storage Nodes – Road High Points

The invert elevation of the storage node used to represent a road high point is set based on the design spill elevation at the curb line. The rim elevation is set at a fixed distance above the invert elevation (typically 0.5 m) to allow for representation of depth of surface water over the storage node to allow routing from one surface storage to the next. The volume stored within the road sags then includes the total static volume and the ponded depth above the node representing the dynamic flow depth.

The functional storage curve values are set to zero for each high point nodes to disable storage being considered at the high points. In this manner, storage accumulates according to the actual ponding depths, from sag elevation to high point elevation. Runoff exceeding the sag storage available in the roadway (transect) will spill at the associated high/spill point into the next sag and continue routing through the system until ultimately flows either re-enter the minor system or reach the outfall of the major system.

Storage Nodes – Maintenance Holes

The invert and rim elevations for the MH storage nodes represent the design elevations. The functional storage curve values for the 'coefficient' and 'exponent' are set to zero. The functional storage curve value for the 'constant' is set to 1.13 m^2 to represent the volume available within a typical MH.

Where 'Fixed' outfall conditions are applied based on anticipated downstream water levels in a storm sewer system, 'Initial Depth' values may be applied. The 'Initial Depth' value is set to match the static downstream water level elevation at the applicable outfall.

Storage Nodes – Site Storage and Stormwater Management Facilities

Where additional detail is needed to define a storage condition within an existing or proposed adjacent development, site plan area, open space, stormwater management facility, or any other relevant storage feature, a tabular storage condition with a defined storage curve is used. A tabular storage curve is defined by a depth and corresponding surface area that is derived from the associated design condition or set based on a generic condition.

The use of tabular storage curves generally allows storage volumes to be defined for the applicable subcatchment area draining to the associated storage feature.

Conduit – Storm Sewer (Minor System)

Conduit parameters for the storm sewer system are set from the design conditions. The roughness is set based on the Manning's n conditions defined by the OSDG. Hydraulic loss through the minor system under surcharge conditions is represented by assigning an 'Exit Loss Coefficient' value to each applicable conduit. The loss coefficients applied are assigned based on the deflection angle between the upstream and downstream segments. Based on Appendix 6-B of the OSDG, assuming no flow deflector in the MH, the following typical values are used.

Deflection Angle	Value
0	0.022
15	0.094
45	0.384
90	1.344

Conduit – Roads (Major System)

Conduit parameters for the road segments are set from the design conditions. The roughness is set based on the Manning's n conditions defined by the OSDG. There are no hydraulic losses considered.

The conduit cross-section is defined with an irregular condition. The transect applied is based on the cross-section of the associated road type and width (i.e., residential, collector, etc.).

Orifice and Outlet

To maintain target inflow rates to the storm sewer, CB inflow is restricted with inlet-control devices (ICDs). ICDs as represented by orifice and/or outlet links with a user-specified diameter and discharge coefficient or functional head relationship taken from manufacturer's specifications for the chosen ICD model. Orifice sizes are chosen as needed at each CB to achieve the desired design objectives.

All orifices are generally assigned as Type = 'Side', Cross-Section = 'Circular', and with a discharge coefficient of 0.572 to correspond to manufacturer supplied discharge curves for IPEX Tempest HF/MHF models. The value for a flap gate is set to 'No'. Invert elevations are set to correspond to the invert of the associated CB elevation.

The height of the orifice is also set to correspond to the IPEX Tempest HF/MHF models. The following orifice size/heights are the most common.

Typical Orifice Height (m)
0.083
0.095
0.102
0.108
0.127
0.152
0.178

Where additional detail is needed to define a controlled flow condition (e.g., to define an outflow condition from an existing or proposed adjacent development, site plan area, open space, stormwater management facility, or any other relevant feature), an outlet with an associated rating curve may be used. An outlet rating curve is defined by a head and corresponding outflow that is derived from the associated design condition or set based on a desired condition.

The use of outlets with rating curves is generally paired with tabular storage curves and allows for defined flow limits to be applied either as inputs to the minor system or as outflow from a storm pond.

Outfall

Outfalls are used to define the end of a series of conduit segments representing either the storm sewer (minor) or roadway (major) systems. Invert and rim elevations are generally set to represent the design condition. The type of outfall is generally set to ‘Free’ for major system outlets.

For minor system outlets the type of outfall is set to correspond with the anticipated condition at the system end point considered. A ‘Fixed’ outfall type is often used where a downstream water level will influence the hydraulic grade line (HGL) within the storm sewer system. The applicable downstream water elevation is assigned with the ‘Fixed’ outfall condition. A ‘Fixed’ outfall condition maintains a static water level through the dynamic model analysis.

E.4 Project Specific PCSWMM Data

he following tables summarize the input parameters for subcatchments, storage nodes, orifice, and outlets applied to the PCSWMM analysis.

Subcatchment Parameters

Name	Outlet	Area (ha)	Width (m)	Slope (%)	Imperviousness (%)
C101A	SU-B	1.36	306	1.5	90
C101B	STM101	0.54	520	1.5	85
C102A	SU-A	1.52	342	1.5	90
C201A	STM201	0.54	460	1.5	85
C201B	SU-C	0.61	137	1.5	90
C201C	SU-D	0.95	214	1.5	90
PRE3A	STM100	0.23	170	1.5	50
PRE3B	STM100	0.16	110	1.5	35

Storage Node Parameters

Name	Invert (m)	Rim (m)	Depth (m)	Storage Curve	Curve Name
EC-in	48.36	51.00	2.64	FUNCTIONAL	*
HW2	48.85	51.00	2.15	FUNCTIONAL	*
STM100	47.12	50.53	3.41	FUNCTIONAL	*
STM101	48.92	53.38	4.46	FUNCTIONAL	*
STM200	48.94	50.91	1.97	FUNCTIONAL	*
STM201	50.43	53.60	3.17	FUNCTIONAL	*
SU-A	49.30	51.90	2.60	TABULAR	SiteStorage
SU-B	49.60	52.20	2.60	TABULAR	SiteStorage
SU-C	51.20	53.80	2.60	TABULAR	SiteStorage
SU-D	51.20	53.80	2.60	TABULAR	SiteStorage

Stroage Curves

Name	Head	Outflow (m^3/s)
SiteStorage	0.0	1
	1.29	1
	1.6	1000
	2.6	1000

Orifice Parameters **(None Used)**

Name	Inlet	Outlet	Inlet Elev. (m)	Type	Diameter (m)

Outlet Parameters

Name	Inlet	Outlet	Inlet Elev. (m)	Curve Type	Curve Name
SU-A_STM100	SU-A	STM100	49.3	TABULAR/DEPTH	BLKA-out
SU-B_STM101	SU-B	STM101	49.6	TABULAR/DEPTH	BLKB-out
SU-C_STM201	SU-C	STM201	51.2	TABULAR/DEPTH	BLKC-out
SU-D_STM201	SU-D	STM201	51.2	TABULAR/DEPTH	BLKD-out

Rating Curves

Name	Head	Outflow (m^3/s)
BLKA-out	0.0	0.00
	1.3	0.36
	1.6	0.36
	2.6	0.36
BLKB-out	0.0	0.00
	1.3	0.32
	1.6	0.32
	2.6	0.32
BLKC-out	0.0	0.00
	1.3	0.14
	1.6	0.14
	2.6	0.14
BLKD-out	0.0	0.00
	1.3	0.22
	1.6	0.22
	2.6	0.22

```
[TITLE]
;;Project Title/Notes
Pertries III
Post-Development
Functional Servicing
August 2023 - Version 0-2
All flow to Ottawa River, west culvert replacement
************************************************
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[OPTIONS]
;;Option             Value
FLOW_UNITS           CMS
INFILTRATION         HORTON
FLOW_ROUTING         DYNWAVE
LINK_OFFSETS         ELEVATION
MIN_SLOPE            0
ALLOW_PONDING        NO
SKIP_STEADY_STATE    NO

START_DATE           6/30/2023
START_TIME           00:00:00
REPORT_START_DATE    6/30/2023
REPORT_START_TIME    00:00:00
END_DATE             7/1/2023
END_TIME             00:00:00
SWEEP_START          1/1
SWEEP_END            12/31
DRY_DAYS             0
REPORT_STEP          00:01:00
WET_STEP             00:05:00
DRY_STEP             00:05:00
ROUTING_STEP         5
RULE_STEP            00:00:00
```

Petries III – PCSWMM Input Data

Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

```
INERTIAL_DAMPING     PARTIAL
NORMAL_FLOW_LIMITED  BOTH
FORCE_MAIN_EQUATION  H-W
VARIABLE_STEP        0.75
LENGTHENING_STEP     0
MIN_SURFAREA         0
MAX_TRIALS           8
HEAD_TOLERANCE       0
SYS_FLOW_TOL         5
LAT_FLOW_TOL         5
MINIMUM_STEP         0.5
THREADS              4

[EVAPORATION]
;;Data Source    Parameters
;;-------------- ----------------
CONSTANT         0.0
DRY_ONLY         NO

[RAINGAGES]
;;Name           Format    Interval SCF      Source
;;-------------- --------- -------- -------- ----------
August4_1988     INTENSITY 0:05     1.0      TIMESERIES August4_1988
August8_1996     INTENSITY 0:05     1.0      TIMESERIES August8_1996
Chicago_100yr_3h INTENSITY 0:10     1.0      TIMESERIES Chicago_100yr_3h
Chicago_100yr_6h INTENSITY 0:10     1.0      TIMESERIES Chicago_100yr_6h
Chicago_100yr+20%_3h INTENSITY 0:10     1.0      TIMESERIES Chicago_100yr+20%_3h
Chicago_2yr_3h   INTENSITY 0:10     1.0      TIMESERIES Chicago_2yr_3h
Chicago_5yr_3h   INTENSITY 0:10     1.0      TIMESERIES Chicago_5yr_3h
July1_1979       INTENSITY 0:05     1.0      TIMESERIES July1_1979
SCS_Type_II_100yr-24hr_103.2mm INTENSITY 1:00     1.0      TIMESERIES SCS_Type_II_100yr-24hr_103.2mm

[SUBCATCHMENTS]
;;Name           Rain Gage        Outlet           Area     %Imperv  Width    %Slope   CurbLen  SnowPack
;;-------------- ---------------- ---------------- -------- -------- -------- -------- -------- ----------------
C101A            Chicago_100yr_3h SU-B             1.36     90       306      1.5      0
C101B            Chicago_100yr_3h STM101           0.54     85       520      1.5      0
C102A            Chicago_100yr_3h SU-A             1.52     90       342      1.5      0
C201A            Chicago_100yr_3h STM201           0.54     85       460      1.5      0
C201B            Chicago_100yr_3h SU-C             0.61     90       137      1.5      0
C201C            Chicago_100yr_3h SU-D             0.95     90       214      1.5      0
Pre3A            Chicago_100yr_3h STM100           0.23     50       170      1.5      0
Pre3B            Chicago_100yr_3h STM100           0.16     35       110      1.5      0

[SUBAREAS]
```

Petries III – PCSWMM Input Data

Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

```
;;Subcatchment   N-Imperv   N-Perv     S-Imperv   S-Perv     PctZero    RouteTo    PctRouted
;;-------------- ---------- ---------- ---------- ---------- ---------- ---------- ----------
C101A            0.013      0.25       1.57       4.67       0          OUTLET
C101B            0.013      0.25       1.57       4.67       0          OUTLET
C102A            0.013      0.25       1.57       4.67       0          OUTLET
C201A            0.013      0.25       1.57       4.67       0          OUTLET
C201B            0.013      0.25       1.57       4.67       0          OUTLET
C201C            0.013      0.25       1.57       4.67       0          OUTLET
Pre3A            0.013      0.25       1.57       4.67       0          OUTLET
Pre3B            0.013      0.25       1.57       4.67       0          OUTLET

[INFILTRATION]
;;Subcatchment   Param1     Param2     Param3     Param4     Param5
;;-------------- ---------- ---------- ---------- ---------- ----------
C101A            76.2       13.2       4.14       7          0
C101B            76.2       13.2       4.14       7          0
C102A            76.2       13.2       4.14       7          0
C201A            76.2       13.2       4.14       7          0
C201B            76.2       13.2       4.14       7          0
C201C            76.2       13.2       4.14       7          0
Pre3A            76.2       13.2       4.14       7          0
Pre3B            76.2       13.2       4.14       7          0

[OUTFALLS]
;;Name           Elevation  Type       Stage Data       Gated    Route To
;;-------------- ---------- ---------- ---------------- -------- ----------------
EC-out           47.33      FREE                        NO
HW1              47         FREE                        NO

[STORAGE]
;;Name           Elev.    MaxDepth   InitDepth  Shape      Curve Name/Params            N/A      Fevap    Psi      Ksat
IMD
;;-------------- -------- ---------- ---------- ---------- ---------------------------- -------- --------          -------- -
-------
EC-in            48.36    2.64       0          FUNCTIONAL 0          0         1         0        0
HW2              48.85    2.15       0          FUNCTIONAL 0          0         1         0        0
STM100           47.12    3.41       0          FUNCTIONAL 0          0         1.13      0        0
STM101           48.92    4.46       0          FUNCTIONAL 0          0         1.13      0        0
STM200           48.94    1.97       0          FUNCTIONAL 0          0         1.13      0        0
STM201           50.43    3.17       0          FUNCTIONAL 0          0         1.13      0        0
SU-A             49.3     2.6        0          TABULAR    SiteStorage                  0        0
SU-B             49.6     2.6        0          TABULAR    SiteStorage                  0        0
SU-C             51.2     2.6        0          TABULAR    SiteStorage                  0        0
SU-D             51.2     2.6        0          TABULAR    SiteStorage                  0        0
```

Petries III – PCSWMM Input Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

```
[CONDUITS]
;;Name           From Node        To Node          Length     Roughness  InOffset   OutOffset  InitFlow   MaxFlow
;;-------------- ---------------- ---------------- ---------- ---------- ---------- ---------- ---------- ----------
100-HW1(C-STRM)  STM100           HW1              60.192     0.013      47.12      47         0          0
101-100_(C-STRM) STM101           STM100           214.02     0.013      48.92      47.42      0          0
200-HW2_(C-STRM) STM200           HW2              18.885     0.013      48.94      48.85      0          0
201-200_(C-STRM) STM201           STM200           202.271    0.013      50.43      49.02      0          0
EastCulvert      EC-in            EC-out           29.172     0.024      48.36      47.33      0          0
EastDitch        HW2              EC-in            20.427     0.035      48.85      48.36      0          0

[OUTLETS]
;;Name           From Node        To Node          Offset     Type             QTable/Qcoeff    Qexpon     Gated
;;-------------- ---------------- ---------------- ---------- ---------------- ---------------- ---------- --------
SU-A_STM100      SU-A             STM100           49.3       TABULAR/DEPTH    BLKA-out                    NO
SU-B_STM101      SU-B             STM101           49.6       TABULAR/DEPTH    BLKB-out                    NO
SU-C_STM201      SU-C             STM201           51.2       TABULAR/DEPTH    BLKC-out                    NO
SU-D_STM201      SU-D             STM201           51.2       TABULAR/DEPTH    BLKD-out                    NO

[XSECTIONS]
;;Link           Shape        Geom1            Geom2      Geom3      Geom4      Barrels    Culvert
;;-------------- ------------ ---------------- ---------- ---------- ---------- ---------- ----------
100-HW1(C-STRM)  CIRCULAR     0.9              0          0          0          1
101-100_(C-STRM) CIRCULAR     0.6              0          0          0          1
200-HW2_(C-STRM) CIRCULAR     0.675            0          0          0          1
201-200_(C-STRM) CIRCULAR     0.6              0          0          0          1
EastCulvert      CIRCULAR     0.6              0          0          0          1          6
EastDitch        TRAPEZOIDAL  2.7              2          4          4          1

[TRANSECTS]
;;Transect Data in HEC-2 format
;
NC 0.025    0.025    0.013
X1 18mROW            7        4.75     13.25    0.0      0.0      0.0      0.0      0.0
GR 0.3      0        0.15     4.75     0        4.75     0.13     9        0        13.25
GR 0.15     13.25    0.3      18
;
NC 0.025    0.025    0.013
X1 24mROW            7        6.75     17.5     0.0      0.0      0.0      0.0      0.0
GR 0.38     0        0.19     6.75     0        6.75     0.17     12       0        17.5
GR 0.19     17.5     0.38     24

[LOSSES]
;;Link           Kentry     Kexit      Kavg       Flap Gate  Seepage
;;-------------- ---------- ---------- ---------- ---------- ----------
101-100_(C-STRM) 0          0.384      0          NO         0
```

```
201-200_(C-STRM) 0          1.344      0          NO         0

[CURVES]
;;Name           Type       X-Value    Y-Value   
;;-------------- ---------- ---------- ----------
;Allowable flow out of Block A based on 5-yr flow rate
BLKA-out         Rating     0          0         
BLKA-out                    1.3        0.36      
BLKA-out                    1.6        0.36      
BLKA-out                    2.6        0.36      

;Allowable flow out of Block B based on 5-yr flow rate
BLKB-out         Rating     0          0         
BLKB-out                    1.3        0.32      
BLKB-out                    1.6        0.32      
BLKB-out                    2.6        0.32      

;Allowable flow out of Block C based on 5-yr flow rate
BLKC-out         Rating     0          0         
BLKC-out                    1.3        0.14      
BLKC-out                    1.6        0.14      
BLKC-out                    2.6        0.14      

;Allowable flow out of Block D based on 5-yr flow rate
BLKD-out         Rating     0          0         
BLKD-out                    1.3        0.22      
BLKD-out                    1.6        0.22      
BLKD-out                    2.6        0.22      

;Generic Site Storage
SiteStorage      Storage    0          1         
SiteStorage                 1.29       1         
SiteStorage                 1.3        1000      
SiteStorage                 2.6        1000      

[TIMESERIES]
;;Name           Date       Time       Value     
;;-------------- ---------- ---------- ----------
August4_1988                0:00       0         
August4_1988                0:05       0.1       
August4_1988                0:10       0.1       
August4_1988                0:15       0         
August4_1988                0:20       3.7       
August4_1988                0:25       6.2       
August4_1988                0:30       101.5     
```

Petries III – PCSWMM Input Data

Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

```
August4_1988                0:35       15.5
August4_1988                0:40       29.3
August4_1988                0:45       19.8
August4_1988                0:50       1.5
August4_1988                0:55       1.7
August4_1988                1:00       5.4
August4_1988                1:05       24.6
August4_1988                1:10       26.5
August4_1988                1:15       34.9
August4_1988                1:20       10.2
August4_1988                1:25       27.1
August4_1988                1:30       104.4
August4_1988                1:35       27.5
August4_1988                1:40       62.5
August4_1988                1:45       31.8
August4_1988                1:50       79.8
August4_1988                1:55       67.5
August4_1988                2:00       156.2
August4_1988                2:05       5.1
August4_1988                2:10       0.2
August4_1988                2:15       0.2
August4_1988                2:20       0.2
August4_1988                2:25       0.2
August4_1988                2:30       0.2
August4_1988                2:35       0.2
August4_1988                2:40       0.2
August4_1988                2:45       0.2
August4_1988                2:50       0.2
August4_1988                2:55       0.2
August4_1988                3:00       12.8
August4_1988                3:05       14
August4_1988                3:10       22.2
August4_1988                3:15       21.8
August4_1988                3:20       1.4
August4_1988                3:25       0.2
August4_1988                3:30       0.2
August4_1988                3:35       0.2
August4_1988                3:40       0.2
August4_1988                3:45       0.2
August4_1988                3:50       0.2
August4_1988                3:55       0.2
August4_1988                4:00       0.2
August4_1988                4:05       0.2
August4_1988                4:10       0.2
August4_1988                4:15       0.2
```

Petries III – PCSWMM Input Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

August4_1988	4:20	0.2
August4_1988	4:25	0.2
August4_1988	4:30	0.2
August4_1988	4:35	0.2
August4_1988	4:40	0.2
August4_1988	4:45	0.2
August4_1988	4:50	0.2
August4_1988	4:55	0.2
August4_1988	5:00	2.9
August4_1988	5:05	7.8
August4_1988	5:10	10
August4_1988	5:15	6.3
August4_1988	5:20	5.1
August4_1988	5:25	9.8
August4_1988	5:30	2.6
August4_1988	5:35	1.7

August8_1996	0:00	4
August8_1996	0:05	11.9
August8_1996	0:10	26.5
August8_1996	0:15	13.3
August8_1996	0:20	0
August8_1996	0:25	2.7
August8_1996	0:30	0
August8_1996	0:35	8
August8_1996	0:40	18.6
August8_1996	0:45	10.6
August8_1996	0:50	21.2
August8_1996	0:55	2.7
August8_1996	1:00	2.7
August8_1996	1:05	15.9
August8_1996	1:10	66.3
August8_1996	1:15	55.7
August8_1996	1:20	122
August8_1996	1:25	88.9
August8_1996	1:30	9.63
August8_1996	1:35	8
August8_1996	1:40	4
August8_1996	1:45	0
August8_1996	1:50	2.7
August8_1996	1:55	0
August8_1996	2:00	0
August8_1996	2:05	0
August8_1996	2:10	5.3
August8_1996	2:15	0

Petries III – PCSWMM Input Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

```
August8_1996                2:20       0
August8_1996                2:25       0
August8_1996                2:30       0
August8_1996                2:35       0
August8_1996                2:40       0
August8_1996                2:45       4
August8_1996                2:50       53.1
August8_1996                2:55       69
August8_1996                3:00       63.7
August8_1996                3:05       58.4
August8_1996                3:10       47.8
August8_1996                3:15       15.9
August8_1996                3:20       13.3
August8_1996                3:25       8
August8_1996                3:30       5.3
August8_1996                3:35       6.6
August8_1996                3:40       2.7
August8_1996                3:45       4
August8_1996                3:50       2.7
August8_1996                3:55       4
August8_1996                4:00       2.7
August8_1996                4:05       5.3
August8_1996                4:10       4
August8_1996                4:15       2.7
August8_1996                4:20       4
August8_1996                4:25       2.7
August8_1996                4:30       1.3
August8_1996                4:35       1.3
August8_1996                4:40       0
August8_1996                4:45       0
August8_1996                4:50       0
August8_1996                4:55       0
August8_1996                5:00       2.7
August8_1996                5:05       0
August8_1996                5:10       0
August8_1996                5:15       0
August8_1996                5:20       0
August8_1996                5:25       0
August8_1996                5:30       0
August8_1996                5:35       0
August8_1996                5:40       1.3

Chicago_100yr_3h            0:00       0
Chicago_100yr_3h            0:10       6.05
Chicago_100yr_3h            0:20       7.54
```

Petries III – PCSWMM Input Data

Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

Chicago_100yr_3h	0:30	10.16
Chicago_100yr_3h	0:40	15.97
Chicago_100yr_3h	0:50	40.65
Chicago_100yr_3h	1:00	178.56
Chicago_100yr_3h	1:10	54.05
Chicago_100yr_3h	1:20	27.32
Chicago_100yr_3h	1:30	18.24
Chicago_100yr_3h	1:40	13.74
Chicago_100yr_3h	1:50	11.06
Chicago_100yr_3h	2:00	9.29
Chicago_100yr_3h	2:10	8.02
Chicago_100yr_3h	2:20	7.08
Chicago_100yr_3h	2:30	6.35
Chicago_100yr_3h	2:40	5.76
Chicago_100yr_3h	2:50	5.28
Chicago_100yr_3h	3:00	4.88

Chicago_100yr_6h	0:10	2.91
Chicago_100yr_6h	0:20	3.17
Chicago_100yr_6h	0:30	3.48
Chicago_100yr_6h	0:40	3.88
Chicago_100yr_6h	0:50	4.39
Chicago_100yr_6h	1:00	5.08
Chicago_100yr_6h	1:10	6.05
Chicago_100yr_6h	1:20	7.55
Chicago_100yr_6h	1:30	10.17
Chicago_100yr_6h	1:40	15.98
Chicago_100yr_6h	1:50	40.67
Chicago_100yr_6h	2:00	178.56
Chicago_100yr_6h	2:10	54.04
Chicago_100yr_6h	2:20	27.31
Chicago_100yr_6h	2:30	18.23
Chicago_100yr_6h	2:40	13.73
Chicago_100yr_6h	2:50	11.05
Chicago_100yr_6h	3:00	9.28
Chicago_100yr_6h	3:10	8.02
Chicago_100yr_6h	3:20	7.08
Chicago_100yr_6h	3:30	6.34
Chicago_100yr_6h	3:40	5.76
Chicago_100yr_6h	3:50	5.28
Chicago_100yr_6h	4:00	4.88
Chicago_100yr_6h	4:10	4.54
Chicago_100yr_6h	4:20	4.25
Chicago_100yr_6h	4:30	3.99
Chicago_100yr_6h	4:40	3.77

Petries III – PCSWMM Input Data

Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

```
Chicago_100yr_6h            4:50       3.57
Chicago_100yr_6h            5:00       3.4
Chicago_100yr_6h            5:10       3.24
Chicago_100yr_6h            5:20       3.1
Chicago_100yr_6h            5:30       2.97
Chicago_100yr_6h            5:40       2.85
Chicago_100yr_6h            5:50       2.74
Chicago_100yr_6h            6:00       2.64

Chicago_100yr+20%_3h            0:00       0
Chicago_100yr+20%_3h            0:10       7.26
Chicago_100yr+20%_3h            0:20       9.048
Chicago_100yr+20%_3h            0:30       12.192
Chicago_100yr+20%_3h            0:40       19.164
Chicago_100yr+20%_3h            0:50       48.78
Chicago_100yr+20%_3h            1:00       214.272
Chicago_100yr+20%_3h            1:10       64.86
Chicago_100yr+20%_3h            1:20       32.784
Chicago_100yr+20%_3h            1:30       21.888
Chicago_100yr+20%_3h            1:40       16.488
Chicago_100yr+20%_3h            1:50       13.272
Chicago_100yr+20%_3h            2:00       11.148
Chicago_100yr+20%_3h            2:10       9.624
Chicago_100yr+20%_3h            2:20       8.496
Chicago_100yr+20%_3h            2:30       7.62
Chicago_100yr+20%_3h            2:40       6.912
Chicago_100yr+20%_3h            2:50       6.336
Chicago_100yr+20%_3h            3:00       5.856

Chicago_2yr_3h              0:00       0
Chicago_2yr_3h              0:10       2.81
Chicago_2yr_3h              0:20       3.5
Chicago_2yr_3h              0:30       4.69
Chicago_2yr_3h              0:40       7.3
Chicago_2yr_3h              0:50       18.21
Chicago_2yr_3h              1:00       76.81
Chicago_2yr_3h              1:10       24.08
Chicago_2yr_3h              1:20       12.36
Chicago_2yr_3h              1:30       8.32
Chicago_2yr_3h              1:40       6.3
Chicago_2yr_3h              1:50       5.09
Chicago_2yr_3h              2:00       4.29
Chicago_2yr_3h              2:10       3.72
Chicago_2yr_3h              2:20       3.29
Chicago_2yr_3h              2:30       2.95
```

Design Storm: 100 Year 3-hour Chicago Storm

```
Chicago_2yr_3h              2:40       2.68
Chicago_2yr_3h              2:50       2.46
Chicago_2yr_3h              3:00       2.28

Chicago_5yr_3h              0:00       0
Chicago_5yr_3h              0:10       3.68
Chicago_5yr_3h              0:20       4.58
Chicago_5yr_3h              0:30       6.15
Chicago_5yr_3h              0:40       9.61
Chicago_5yr_3h              0:50       24.17
Chicago_5yr_3h              1:00       104.19
Chicago_5yr_3h              1:10       32.04
Chicago_5yr_3h              1:20       16.34
Chicago_5yr_3h              1:30       10.96
Chicago_5yr_3h              1:40       8.29
Chicago_5yr_3h              1:50       6.69
Chicago_5yr_3h              2:00       5.63
Chicago_5yr_3h              2:10       4.87
Chicago_5yr_3h              2:20       4.3
Chicago_5yr_3h              2:30       3.86
Chicago_5yr_3h              2:40       3.51
Chicago_5yr_3h              2:50       3.22
Chicago_5yr_3h              3:00       2.98

July1_1979                  0:00       0
July1_1979                  0:05       2.3
July1_1979                  0:10       2.3
July1_1979                  0:15       8.89
July1_1979                  0:20       8.89
July1_1979                  0:25       8.89
July1_1979                  0:30       8.89
July1_1979                  0:35       38.1
July1_1979                  0:40       38.1
July1_1979                  0:45       38.1
July1_1979                  0:50       38.1
July1_1979                  0:55       38.1
July1_1979                  1:00       38.1
July1_1979                  1:05       38.1
July1_1979                  1:10       50.8
July1_1979                  1:15       50.8
July1_1979                  1:20       76.2
July1_1979                  1:25       106.7
July1_1979                  1:30       106.7
July1_1979                  1:35       71.1
July1_1979                  1:40       71.1
```

Design Storm: 100 Year 3-hour Chicago Storm

```
July1_1979                  1:45       30.5
July1_1979                  1:50       30.5
July1_1979                  1:55       30.5
July1_1979                  2:00       30.5
July1_1979                  2:05       3.8
July1_1979                  2:10       3.8
July1_1979                  2:15       3.8
July1_1979                  2:20       3.8
July1_1979                  2:25       3.8
July1_1979                  2:30       3.8
July1_1979                  2:35       3.8
July1_1979                  2:40       3.8
July1_1979                  2:45       3.8
July1_1979                  2:50       3.8
July1_1979                  2:55       3.8
July1_1979                  3:00       3.8

SCS_Type_II_100yr-24hr_103.2mm            0:00       0
SCS_Type_II_100yr-24hr_103.2mm            1:00       3.10
SCS_Type_II_100yr-24hr_103.2mm            2:00       1.44
SCS_Type_II_100yr-24hr_103.2mm            3:00       2.68
SCS_Type_II_100yr-24hr_103.2mm            4:00       2.68
SCS_Type_II_100yr-24hr_103.2mm            5:00       3.51
SCS_Type_II_100yr-24hr_103.2mm            6:00       3.10
SCS_Type_II_100yr-24hr_103.2mm            7:00       4.13
SCS_Type_II_100yr-24hr_103.2mm            8:00       4.13
SCS_Type_II_100yr-24hr_103.2mm            9:00       5.57
SCS_Type_II_100yr-24hr_103.2mm            10:00      7.02
SCS_Type_II_100yr-24hr_103.2mm            11:00      11.15
SCS_Type_II_100yr-24hr_103.2mm            12:00      88.34
SCS_Type_II_100yr-24hr_103.2mm            13:00      22.50
SCS_Type_II_100yr-24hr_103.2mm            14:00      9.91
SCS_Type_II_100yr-24hr_103.2mm            15:00      6.60
SCS_Type_II_100yr-24hr_103.2mm            16:00      5.78
SCS_Type_II_100yr-24hr_103.2mm            17:00      4.54
SCS_Type_II_100yr-24hr_103.2mm            18:00      4.75
SCS_Type_II_100yr-24hr_103.2mm            19:00      3.10
SCS_Type_II_100yr-24hr_103.2mm            20:00      2.48
SCS_Type_II_100yr-24hr_103.2mm            21:00      3.51
SCS_Type_II_100yr-24hr_103.2mm            22:00      2.27
SCS_Type_II_100yr-24hr_103.2mm            23:00      2.06
SCS_Type_II_100yr-24hr_103.2mm            24:00      2.06

[REPORT]
;;Reporting Options
```

Petries III – PCSWMM Input Data

Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

```
INPUT      YES
CONTROLS   NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]
Subcatch   C101A            Block
Subcatch   C101B            Road
Subcatch   C102A            Block
Subcatch   C201A            Road
Subcatch   C201B            Block
Subcatch   C201C            Block
Subcatch   Pre3A            JDB
Subcatch   Pre3B            JDB
Node       EC-in            Culvert-In
Node       HW2              HW
Node       STM100           MH
Node       STM101           MH
Node       STM200           MH
Node       STM201           MH
Node       SU-A             Onsite
Node       SU-B             Onsite
Node       SU-C             Onsite
Node       SU-D             Onsite
Link       100-HW1(C-STRM)  MinorSystem
Link       101-100_(C-STRM) MinorSystem
Link       200-HW2_(C-STRM) MinorSystem
Link       201-200_(C-STRM) MinorSystem
Link       EastCulvert      Culvert
Link       EastDitch        Ditch

[MAP]
DIMENSIONS       383597.2846      5039718.6057     384026.5574      5040079.5883
UNITS            Meters

[COORDINATES]
;;Node           X-Coord            Y-Coord
;;-------------- ------------------ ------------------
EC-out           383855.668         5040063.18
HW1              383689.493         5040001.255
EC-in            383877.306         5040043.615
HW2              383858.016         5040036.897
STM100           383747.401         5039984.837
STM101           383867.743         5039807.863
```

```
STM200           383841.722         5040027.35
STM201           383924.547         5039842.819
SU-A             383726.589         5039910.78
SU-B             383813.207         5039955.248
SU-C             383884.726         5040017.619
SU-D             383939.727         5039805.127

[VERTICES]
;;Link           X-Coord            Y-Coord
;;-------------- ------------------ ------------------

[POLYGONS]
;;Subcatchment   X-Coord            Y-Coord
;;-------------- ------------------ ------------------
C101A            383775.18          5040001.271
C101A            383773.978         5040003.744
C101A            383776.25          5040004.842
C101A            383778.527         5040005.93
C101A            383780.808         5040007.009
C101A            383783.093         5040008.078
C101A            383785.384         5040009.137
C101A            383787.678         5040010.186
C101A            383789.977         5040011.226
C101A            383792.281         5040012.256
C101A            383794.589         5040013.276
C101A            383796.901         5040014.286
C101A            383799.217         5040015.287
C101A            383801.538         5040016.277
C101A            383803.863         5040017.258
C101A            383806.192         5040018.229
C101A            383808.525         5040019.19
C101A            383810.862         5040020.141
C101A            383813.203         5040021.082
C101A            383815.548         5040022.014
C101A            383817.897         5040022.935
C101A            383820.25          5040023.846
C101A            383822.607         5040024.748
C101A            383824.968         5040025.639
C101A            383824.968         5040025.639
C101A            383833.053         5040022.245
C101A            383854.934         5039973.494
C101A            383865.99          5039948.862
C101A            383914.145         5039841.573
C101A            383914.199         5039841.441
C101A            383914.243         5039841.305
```

C101A	383914.278	5039841.166
C101A	383914.302	5039841.026
C101A	383914.317	5039840.884
C101A	383914.321	5039840.741
C101A	383914.315	5039840.598
C101A	383914.298	5039840.457
C101A	383914.272	5039840.316
C101A	383914.236	5039840.178
C101A	383914.19	5039840.043
C101A	383914.135	5039839.911
C101A	383914.07	5039839.784
C101A	383913.996	5039839.662
C101A	383913.914	5039839.545
C101A	383913.824	5039839.435
C101A	383913.726	5039839.331
C101A	383913.621	5039839.234
C101A	383913.509	5039839.145
C101A	383913.391	5039839.065
C101A	383913.268	5039838.993
C101A	383913.14	5039838.929
C101A	383913.14	5039838.929
C101A	383872.968	5039820.899
C101A	383870.496	5039821.599
C101A	383870.496	5039821.599
C101A	383787.774	5039943.249
C101A	383758.603	5039986.146
C101A	383760.538	5039993.905
C101A	383761.199	5039994.249
C101A	383761.86	5039994.592
C101A	383762.522	5039994.933
C101A	383763.185	5039995.275
C101A	383763.848	5039995.615
C101A	383764.511	5039995.954
C101A	383765.174	5039996.293
C101A	383765.839	5039996.631
C101A	383766.503	5039996.967
C101A	383767.168	5039997.304
C101A	383767.833	5039997.639
C101A	383768.499	5039997.973
C101A	383769.165	5039998.307
C101A	383769.832	5039998.639
C101A	383770.499	5039998.971
C101A	383771.167	5039999.302
C101A	383771.834	5039999.633
C101A	383772.503	5039999.962

Design Storm: 100 Year 3-hour Chicago Storm

C101A	383773.171	5040000.29
C101A	383773.841	5040000.618
C101A	383774.51	5040000.945
C101A	383775.18	5040001.271
C101A	383775.18	5040001.271
C101A	383775.18	5040001.271
C101B	383741.048	5039976.396
C101B	383732.494	5039978.329
C101B	383733.751	5039979.07
C101B	383735.009	5039979.808
C101B	383736.268	5039980.542
C101B	383737.53	5039981.274
C101B	383738.793	5039982.002
C101B	383740.058	5039982.727
C101B	383741.325	5039983.45
C101B	383742.594	5039984.169
C101B	383743.864	5039984.885
C101B	383745.137	5039985.597
C101B	383746.411	5039986.307
C101B	383747.686	5039987.014
C101B	383748.964	5039987.717
C101B	383750.243	5039988.417
C101B	383751.524	5039989.114
C101B	383752.807	5039989.808
C101B	383754.091	5039990.499
C101B	383755.377	5039991.187
C101B	383756.665	5039991.871
C101B	383757.954	5039992.552
C101B	383759.245	5039993.23
C101B	383760.538	5039993.905
C101B	383760.538	5039993.905
C101B	383758.603	5039986.146
C101B	383787.774	5039943.249
C101B	383870.496	5039821.599
C101B	383872.968	5039820.899
C101B	383872.968	5039820.899
C101B	383913.14	5039838.929
C101B	383917.235	5039829.806
C101B	383921.329	5039820.683
C101B	383864.329	5039795.099
C101B	383800.951	5039888.304
C101B	383777.896	5039922.208
C101B	383772.273	5039930.477
C101B	383741.048	5039976.396
C101B	383741.048	5039976.396

C102A	383657.209	5039835.787
C102A	383623.47	5039885.402
C102A	383616.797	5039894.999
C102A	383636.087	5039911.308
C102A	383649.818	5039922.918
C102A	383651.264	5039924.137
C102A	383652.714	5039925.351
C102A	383654.167	5039926.56
C102A	383655.625	5039927.765
C102A	383657.086	5039928.965
C102A	383658.551	5039930.161
C102A	383660.02	5039931.352
C102A	383661.493	5039932.538
C102A	383662.969	5039933.719
C102A	383664.449	5039934.896
C102A	383665.933	5039936.068
C102A	383667.421	5039937.235
C102A	383668.912	5039938.398
C102A	383670.407	5039939.556
C102A	383671.906	5039940.709
C102A	383673.408	5039941.857
C102A	383674.914	5039943.001
C102A	383676.424	5039944.139
C102A	383677.937	5039945.273
C102A	383679.454	5039946.402
C102A	383680.974	5039947.527
C102A	383682.498	5039948.646
C102A	383682.498	5039948.646
C102A	383684.278	5039946.541
C102A	383686.401	5039948.087
C102A	383688.531	5039949.624
C102A	383690.667	5039951.151
C102A	383692.81	5039952.668
C102A	383694.96	5039954.176
C102A	383697.117	5039955.675
C102A	383699.28	5039957.164
C102A	383701.45	5039958.643
C102A	383703.626	5039960.113
C102A	383705.809	5039961.573
C102A	383707.999	5039963.023
C102A	383710.194	5039964.464
C102A	383712.396	5039965.895
C102A	383714.605	5039967.316
C102A	383716.819	5039968.727
C102A	383719.04	5039970.128

Petries III – PCSWMM Input Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

C102A	383721.268	5039971.52
C102A	383723.501	5039972.902
C102A	383725.74	5039974.273
C102A	383727.986	5039975.635
C102A	383730.237	5039976.987
C102A	383732.494	5039978.329
C102A	383732.494	5039978.329
C102A	383741.048	5039976.396
C102A	383772.273	5039930.477
C102A	383777.896	5039922.208
C102A	383800.951	5039888.304
C102A	383719.134	5039832.668
C102A	383716.643	5039839.323
C102A	383693.455	5039851.614
C102A	383677.882	5039849.845
C102A	383657.209	5039835.787
C102A	383657.209	5039835.787
C201A	383951.51	5039910.957
C201A	383958.19	5039896.074
C201A	383959.7	5039892.711
C201A	383920.926	5039875.308
C201A	383941.4	5039829.692
C201A	383921.329	5039820.683
C201A	383917.235	5039829.806
C201A	383913.14	5039838.929
C201A	383913.268	5039838.993
C201A	383913.391	5039839.065
C201A	383913.509	5039839.145
C201A	383913.621	5039839.234
C201A	383913.726	5039839.331
C201A	383913.824	5039839.435
C201A	383913.914	5039839.545
C201A	383913.996	5039839.662
C201A	383914.07	5039839.784
C201A	383914.135	5039839.911
C201A	383914.19	5039840.043
C201A	383914.236	5039840.178
C201A	383914.272	5039840.316
C201A	383914.298	5039840.457
C201A	383914.315	5039840.598
C201A	383914.321	5039840.741
C201A	383914.317	5039840.884
C201A	383914.302	5039841.026
C201A	383914.278	5039841.166
C201A	383914.243	5039841.305

Petries III – PCSWMM Input Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

C201A	383914.199	5039841.441
C201A	383914.145	5039841.573
C201A	383914.145	5039841.573
C201A	383865.99	5039948.862
C201A	383854.934	5039973.494
C201A	383833.053	5040022.245
C201A	383824.968	5040025.639
C201A	383825.643	5040025.892
C201A	383826.319	5040026.144
C201A	383826.996	5040026.396
C201A	383827.672	5040026.647
C201A	383828.349	5040026.896
C201A	383829.027	5040027.146
C201A	383829.704	5040027.394
C201A	383830.382	5040027.641
C201A	383831.06	5040027.888
C201A	383831.739	5040028.133
C201A	383832.418	5040028.378
C201A	383833.097	5040028.622
C201A	383833.776	5040028.866
C201A	383834.456	5040029.108
C201A	383835.136	5040029.35
C201A	383835.816	5040029.59
C201A	383836.497	5040029.83
C201A	383837.177	5040030.069
C201A	383837.859	5040030.308
C201A	383838.54	5040030.545
C201A	383839.222	5040030.782
C201A	383839.904	5040031.017
C201A	383839.904	5040031.017
C201A	383846.655	5040033.347
C201A	383849.181	5040026.027
C201A	383858.4	5040029.208
C201A	383855.185	5040021.777
C201A	383912.736	5039893.554
C201A	383951.51	5039910.957
C201A	383951.51	5039910.957
C201B	383858.4	5040029.208
C201B	383893.065	5040041.17
C201B	383951.51	5039910.957
C201B	383912.736	5039893.554
C201B	383855.185	5040021.777
C201B	383858.4	5040029.208
C201B	383858.4	5040029.208
C201C	383959.7	5039892.711

Petries III – PCSWMM Input Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

C201C	384007.045	5039787.227
C201C	383989.813	5039792.65
C201C	383938.957	5039758.068
C201C	383931.312	5039769.348
C201C	383918.068	5039760.371
C201C	383925.748	5039749.039
C201C	383905.054	5039735.014
C201C	383866.608	5039791.737
C201C	383864.329	5039795.099
C201C	383921.329	5039820.683
C201C	383941.4	5039829.692
C201C	383920.926	5039875.308
C201C	383959.7	5039892.711
C201C	383959.7	5039892.711
Pre3A	383775.18	5040001.271
Pre3A	383773.978	5040003.744
Pre3A	383776.25	5040004.842
Pre3A	383778.527	5040005.93
Pre3A	383780.808	5040007.009
Pre3A	383783.093	5040008.078
Pre3A	383785.384	5040009.137
Pre3A	383787.678	5040010.186
Pre3A	383789.977	5040011.226
Pre3A	383792.281	5040012.256
Pre3A	383794.589	5040013.276
Pre3A	383796.901	5040014.286
Pre3A	383799.217	5040015.287
Pre3A	383801.538	5040016.277
Pre3A	383803.862	5040017.258
Pre3A	383806.191	5040018.229
Pre3A	383808.524	5040019.19
Pre3A	383810.862	5040020.141
Pre3A	383813.203	5040021.082
Pre3A	383815.548	5040022.014
Pre3A	383817.897	5040022.935
Pre3A	383820.25	5040023.846
Pre3A	383822.606	5040024.748
Pre3A	383824.967	5040025.639
Pre3A	383824.967	5040025.639
Pre3A	383824.968	5040025.639
Pre3A	383825.948	5040026.004
Pre3A	383826.929	5040026.367
Pre3A	383827.911	5040026.729
Pre3A	383828.893	5040027.09
Pre3A	383829.876	5040027.449

Petries III – PCSWMM Input Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

Pre3A	383830.859	5040027.807
Pre3A	383831.842	5040028.164
Pre3A	383832.827	5040028.519
Pre3A	383833.811	5040028.873
Pre3A	383834.796	5040029.225
Pre3A	383835.782	5040029.576
Pre3A	383836.768	5040029.926
Pre3A	383837.755	5040030.274
Pre3A	383838.742	5040030.621
Pre3A	383839.729	5040030.967
Pre3A	383840.717	5040031.311
Pre3A	383841.706	5040031.654
Pre3A	383842.695	5040031.995
Pre3A	383843.684	5040032.335
Pre3A	383844.674	5040032.674
Pre3A	383845.664	5040033.011
Pre3A	383846.655	5040033.347
Pre3A	383846.655	5040033.347
Pre3A	383847.386	5040031.23
Pre3A	383850.622	5040038.645
Pre3A	383847.912	5040046.468
Pre3A	383814.403	5040035.112
Pre3A	383791.865	5040026.341
Pre3A	383766.371	5040013.657
Pre3A	383725.26	5039991.905
Pre3A	383714.177	5039985.595
Pre3A	383702.288	5039977.539
Pre3A	383695.248	5039972.565
Pre3A	383700.851	5039963.12
Pre3A	383704.795	5039960.896
Pre3A	383706.033	5039961.722
Pre3A	383707.273	5039962.544
Pre3A	383708.515	5039963.363
Pre3A	383709.759	5039964.179
Pre3A	383711.005	5039964.992
Pre3A	383712.253	5039965.802
Pre3A	383713.504	5039966.609
Pre3A	383714.756	5039967.412
Pre3A	383716.01	5039968.213
Pre3A	383717.266	5039969.01
Pre3A	383718.524	5039969.804
Pre3A	383719.785	5039970.595
Pre3A	383721.047	5039971.383
Pre3A	383722.311	5039972.167
Pre3A	383723.577	5039972.949

Petries III – PCSWMM Input Data

Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

Pre3A	383724.845	5039973.727
Pre3A	383726.115	5039974.502
Pre3A	383727.387	5039975.274
Pre3A	383728.661	5039976.042
Pre3A	383729.937	5039976.808
Pre3A	383731.215	5039977.57
Pre3A	383732.494	5039978.329
Pre3A	383732.494	5039978.329
Pre3A	383733.751	5039979.07
Pre3A	383735.009	5039979.808
Pre3A	383736.268	5039980.542
Pre3A	383737.53	5039981.274
Pre3A	383738.793	5039982.002
Pre3A	383740.058	5039982.727
Pre3A	383741.325	5039983.45
Pre3A	383742.594	5039984.169
Pre3A	383743.864	5039984.885
Pre3A	383745.137	5039985.597
Pre3A	383746.411	5039986.307
Pre3A	383747.686	5039987.014
Pre3A	383748.964	5039987.717
Pre3A	383750.243	5039988.417
Pre3A	383751.524	5039989.114
Pre3A	383752.807	5039989.808
Pre3A	383754.091	5039990.499
Pre3A	383755.377	5039991.187
Pre3A	383756.665	5039991.871
Pre3A	383757.954	5039992.552
Pre3A	383759.245	5039993.23
Pre3A	383760.538	5039993.905
Pre3A	383760.538	5039993.905
Pre3A	383761.199	5039994.249
Pre3A	383761.86	5039994.592
Pre3A	383762.522	5039994.933
Pre3A	383763.185	5039995.275
Pre3A	383763.848	5039995.615
Pre3A	383764.511	5039995.954
Pre3A	383765.174	5039996.293
Pre3A	383765.839	5039996.631
Pre3A	383766.503	5039996.967
Pre3A	383767.168	5039997.304
Pre3A	383767.833	5039997.639
Pre3A	383768.499	5039997.973
Pre3A	383769.165	5039998.307
Pre3A	383769.832	5039998.639

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 Pre3A            383770.499         5039998.971
 Pre3A            383771.167         5039999.302
 Pre3A            383771.834         5039999.633
 Pre3A            383772.503         5039999.962
 Pre3A            383773.171         5040000.29
 Pre3A            383773.841         5040000.618
 Pre3A            383774.51          5040000.945
 Pre3A            383775.18          5040001.271
 Pre3A            383775.18          5040001.271
 Pre3B            383766.371         5040013.657
 Pre3B            383791.865         5040026.341
 Pre3B            383787.75          5040035.636
 Pre3B            383760.967         5040023.855
 Pre3B            383716.801         5040007.43
 Pre3B            383705.808         5039999.579
 Pre3B            383687.259         5039984.34
 Pre3B            383695.248         5039972.565
 Pre3B            383702.288         5039977.539
 Pre3B            383714.177         5039985.595
 Pre3B            383725.26          5039991.905
 Pre3B            383766.371         5040013.657

 [SYMBOLS]
 ;;Gage           X-Coord            Y-Coord
;;-------------- ------------------ ------------------
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Petries III – PCSWMM Output Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

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  EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)
  --------------------------------------------------------------

  Pertries III
  Post-Development
  Functional Servicing


  *************
  Element Count
  *************
  Number of rain gages ...... 9
  Number of subcatchments ... 8
  Number of nodes ........... 12
  Number of links ........... 10
  Number of pollutants ...... 0
  Number of land uses ....... 0


  ****************
  Raingage Summary
  ****************
                                                       Data       Recording
  Name                 Data Source                     Type       Interval 
  ------------------------------------------------------------------------
  August4_1988         August4_1988                    INTENSITY    5 min.
  August8_1996         August8_1996                    INTENSITY    5 min.
  Chicago_100yr_3h     Chicago_100yr_3h                INTENSITY   10 min.
  Chicago_100yr_6h     Chicago_100yr_6h                INTENSITY   10 min.
  Chicago_100yr+20%_3h Chicago_100yr+20%_3h            INTENSITY   10 min.
  Chicago_2yr_3h       Chicago_2yr_3h                  INTENSITY   10 min.
  Chicago_5yr_3h       Chicago_5yr_3h                  INTENSITY   10 min.
  July1_1979           July1_1979                      INTENSITY    5 min.
  SCS_Type_II_100yr-24hr_103.2mm SCS_Type_II_100yr-24hr_103.2mm INTENSITY   60 min.


  ********************
  Subcatchment Summary
  ********************
  Name                       Area     Width   %Imperv    %Slope Rain Gage            Outlet              
  -----------------------------------------------------------------------------------------------------------
  C101A                      1.36    306.00     90.00    1.5000 Chicago_100yr_3h     SU-B                
  C101B                      0.54    520.00     85.00    1.5000 Chicago_100yr_3h     STM101              
  C102A                      1.52    342.00     90.00    1.5000 Chicago_100yr_3h     SU-A                
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  C201A                      0.54    460.00     85.00    1.5000 Chicago_100yr_3h     STM201
  C201B                      0.61    137.00     90.00    1.5000 Chicago_100yr_3h     SU-C
  C201C                      0.95    214.00     90.00    1.5000 Chicago_100yr_3h     SU-D
  Pre3A                      0.23    170.00     50.00    1.5000 Chicago_100yr_3h     STM100
  Pre3B                      0.16    110.00     35.00    1.5000 Chicago_100yr_3h     STM100


  ************
  Node Summary
  ************
                                           Invert      Max.    Ponded    External
  Name                 Type                 Elev.     Depth      Area    Inflow
  --------------------------------------------------------------------------------
  EC-out               OUTFALL              47.33      0.60       0.0
  HW1                  OUTFALL              47.00      0.90       0.0
  EC-in                STORAGE              48.36      2.64       0.0
  HW2                  STORAGE              48.85      2.15       0.0
  STM100               STORAGE              47.12      3.41       0.0
  STM101               STORAGE              48.92      4.46       0.0
  STM200               STORAGE              48.94      1.97       0.0
  STM201               STORAGE              50.43      3.17       0.0
  SU-A                 STORAGE              49.30      2.60       0.0
  SU-B                 STORAGE              49.60      2.60       0.0
  SU-C                 STORAGE              51.20      2.60       0.0
  SU-D                 STORAGE              51.20      2.60       0.0


  ************
  Link Summary
  ************
  Name             From Node        To Node          Type            Length    %Slope Roughness
  ----------------------------------------------------------------------------------------------
  100-HW1(C-STRM)  STM100           HW1              CONDUIT           60.2    0.1994    0.0130
  101-100_(C-STRM) STM101           STM100           CONDUIT          214.0    0.7009    0.0130
  200-HW2_(C-STRM) STM200           HW2              CONDUIT           18.9    0.4766    0.0130
  201-200_(C-STRM) STM201           STM200           CONDUIT          202.3    0.6971    0.0130
  EastCulvert      EC-in            EC-out           CONDUIT           29.2    3.5330    0.0240
  EastDitch        HW2              EC-in            CONDUIT           20.4    2.3995    0.0350
  SU-A_STM100      SU-A             STM100           OUTLET
  SU-B_STM101      SU-B             STM101           OUTLET
  SU-C_STM201      SU-C             STM201           OUTLET
  SU-D_STM201      SU-D             STM201           OUTLET


  *********************
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Petries III – PCSWMM Output Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

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Cross Section Summary
*********************
                                        Full     Full     Hyd.     Max.   No. of     Full
Conduit          Shape                 Depth     Area     Rad.    Width  Barrels     Flow
---------------------------------------------------------------------------------------
100-HW1(C-STRM)  CIRCULAR               0.90     0.64     0.23     0.90        1     0.81
101-100_(C-STRM) CIRCULAR               0.60     0.28     0.15     0.60        1     0.51
200-HW2_(C-STRM) CIRCULAR               0.68     0.36     0.17     0.68        1     0.58
201-200_(C-STRM) CIRCULAR               0.60     0.28     0.15     0.60        1     0.51
EastCulvert      CIRCULAR               0.60     0.28     0.15     0.60        1     0.63
EastDitch        TRAPEZOIDAL            2.70    34.56     1.42    23.60        1   193.66



****************
Transect Summary
****************

Transect 18mROW
Area:
            0.0004     0.0017     0.0039     0.0069     0.0109
            0.0156     0.0213     0.0278     0.0352     0.0434
            0.0525     0.0625     0.0734     0.0851     0.0977
            0.1112     0.1255     0.1407     0.1568     0.1737
            0.1915     0.2101     0.2290     0.2478     0.2666
            0.2858     0.3059     0.3268     0.3486     0.3712
            0.3947     0.4190     0.4441     0.4701     0.4969
            0.5245     0.5530     0.5823     0.6125     0.6435
            0.6754     0.7081     0.7416     0.7760     0.8113
            0.8473     0.8842     0.9220     0.9606     1.0000
Hrad:
            0.0178     0.0357     0.0535     0.0714     0.0892
            0.1070     0.1249     0.1427     0.1606     0.1784
            0.1963     0.2141     0.2319     0.2498     0.2676
            0.2855     0.3033     0.3211     0.3390     0.3568
            0.3747     0.3983     0.4333     0.4683     0.5032
            0.5376     0.5699     0.6003     0.6290     0.6560
            0.6815     0.7057     0.7287     0.7505     0.7712
            0.7910     0.8099     0.8280     0.8453     0.8619
            0.8779     0.8933     0.9082     0.9225     0.9364
            0.9499     0.9629     0.9756     0.9880     1.0000
Width:
            0.0218     0.0436     0.0654     0.0872     0.1090
            0.1308     0.1526     0.1744     0.1962     0.2179
            0.2397     0.2615     0.2833     0.3051     0.3269
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              0.3487     0.3705     0.3923     0.4141     0.4359
              0.4577     0.4722     0.4722     0.4722     0.4722
              0.4933     0.5144     0.5356     0.5567     0.5778
              0.5989     0.6200     0.6411     0.6622     0.6833
              0.7044     0.7256     0.7467     0.7678     0.7889
              0.8100     0.8311     0.8522     0.8733     0.8944
              0.9156     0.9367     0.9578     0.9789     1.0000

  Transect 24mROW
  Area:
              0.0004     0.0016     0.0037     0.0066     0.0103
              0.0148     0.0202     0.0264     0.0334     0.0412
              0.0499     0.0594     0.0697     0.0808     0.0928
              0.1055     0.1191     0.1336     0.1488     0.1649
              0.1818     0.1995     0.2179     0.2364     0.2548
              0.2737     0.2935     0.3142     0.3358     0.3584
              0.3818     0.4062     0.4314     0.4576     0.4847
              0.5127     0.5416     0.5714     0.6021     0.6337
              0.6663     0.6997     0.7341     0.7693     0.8055
              0.8426     0.8806     0.9195     0.9593     1.0000
  Hrad:
              0.0185     0.0370     0.0556     0.0741     0.0926
              0.1111     0.1296     0.1482     0.1667     0.1852
              0.2037     0.2222     0.2408     0.2593     0.2778
              0.2963     0.3148     0.3334     0.3519     0.3704
              0.3889     0.4074     0.4373     0.4736     0.5099
              0.5455     0.5788     0.6098     0.6389     0.6662
              0.6918     0.7159     0.7386     0.7601     0.7805
              0.7998     0.8182     0.8357     0.8524     0.8684
              0.8837     0.8985     0.9127     0.9264     0.9396
              0.9524     0.9648     0.9769     0.9886     1.0000
  Width:
              0.0200     0.0400     0.0601     0.0801     0.1001
              0.1201     0.1402     0.1602     0.1802     0.2002
              0.2203     0.2403     0.2603     0.2803     0.3004
              0.3204     0.3404     0.3604     0.3805     0.4005
              0.4205     0.4405     0.4479     0.4479     0.4479
              0.4700     0.4921     0.5142     0.5363     0.5583
              0.5804     0.6025     0.6246     0.6467     0.6687
              0.6908     0.7129     0.7350     0.7571     0.7792
              0.8013     0.8233     0.8454     0.8675     0.8896
              0.9117     0.9338     0.9558     0.9779     1.0000


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Petries III – PCSWMM Output Data
Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

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NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
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****************
Analysis Options
****************
Flow Units ............... CMS
Process Models:
  Rainfall/Runoff ........ YES
  RDII ................... NO
  Snowmelt ............... NO
  Groundwater ............ NO
  Flow Routing ........... YES
  Ponding Allowed ........ NO
  Water Quality .......... NO
Infiltration Method ...... HORTON
Flow Routing Method ...... DYNWAVE
Surcharge Method ......... EXTRAN
Starting Date ............ 06/30/2023 00:00:00
Ending Date .............. 07/01/2023 00:00:00
Antecedent Dry Days ...... 0.0
Report Time Step ......... 00:01:00
Wet Time Step ............ 00:05:00
Dry Time Step ............ 00:05:00
Routing Time Step ........ 5.00 sec
Variable Time Step ....... YES
Maximum Trials ........... 8
Number of Threads ........ 1
Head Tolerance ........... 0.001524 m


**************************        Volume         Depth
Runoff Quantity Continuity     hectare-m            mm
**************************     ---------       -------
Total Precipitation ......         0.424        71.667
Evaporation Loss .........         0.000         0.000
Infiltration Loss ........         0.037         6.178
Surface Runoff ...........         0.383        64.826
Final Storage ............         0.008         1.351
Continuity Error (%) .....        -0.961


**************************        Volume        Volume
```

Design Storm: 100 Year 3-hour Chicago Storm

```
  Flow Routing Continuity        hectare-m      10^6 ltr
  **************************     ---------     ---------
  Dry Weather Inflow .......         0.000         0.000
  Wet Weather Inflow .......         0.383         3.831
  Groundwater Inflow .......         0.000         0.000
  RDII Inflow ..............         0.000         0.000
  External Inflow ..........         0.000         0.000
  External Outflow .........         0.383         3.832
  Flooding Loss ............         0.000         0.000
  Evaporation Loss .........         0.000         0.000
  Exfiltration Loss ........         0.000         0.000
  Initial Stored Volume ....         0.000         0.000
  Final Stored Volume ......         0.000         0.000
  Continuity Error (%) .....        -0.034


  ***************************
  Time-Step Critical Elements
  ***************************
  Link 200-HW2_(C-STRM) (4.52%)


  ********************************
  Highest Flow Instability Indexes
  ********************************
  Link EastCulvert (3)
  Link 200-HW2_(C-STRM) (2)


  *************************
  Routing Time Step Summary
  *************************
  Minimum Time Step           :     3.10 sec
  Average Time Step           :     4.95 sec
  Maximum Time Step           :     5.00 sec
  Percent in Steady State     :     0.00
  Average Iterations per Step :     2.02
  Percent Not Converging      :     0.05
  Time Step Frequencies       :
      5.000 -  3.155 sec      :    99.99 %
      3.155 -  1.991 sec      :     0.01 %
      1.991 -  1.256 sec      :     0.00 %
      1.256 -  0.792 sec      :     0.00 %
      0.792 -  0.500 sec      :     0.00 %
```

Petries III – PCSWMM Output Data
Design Storm
Design Storm: 100 Year 3-hour Chicago Storm

```
  ***************************
  Subcatchment Runoff Summary
  ***************************

  ------------------------------------------------------------------------------------------------------------------------------
--
                            Total      Total      Total      Total     Imperv       Perv      Total       Total     Peak
Runoff
                           Precip      Runon       Evap      Infil     Runoff     Runoff     Runoff      Runoff   Runoff
Coeff
  Subcatchment                 mm         mm         mm         mm         mm         mm         mm    10^6 ltr      CMS
  ------------------------------------------------------------------------------------------------------------------------------
--
  C101A                     71.67       0.00       0.00       4.42      63.51       2.99      66.49        0.90     0.66
0.928
  C101B                     71.67       0.00       0.00       6.58      59.67       4.78      64.45        0.35     0.26
0.899
  C102A                     71.67       0.00       0.00       4.42      63.51       2.99      66.49        1.01     0.74
0.928
  C201A                     71.67       0.00       0.00       6.59      59.68       4.75      64.43        0.35     0.26
0.899
  C201B                     71.67       0.00       0.00       4.42      63.51       2.99      66.49        0.41     0.30
0.928
  C201C                     71.67       0.00       0.00       4.42      63.51       2.99      66.49        0.63     0.46
0.928
  Pre3A                     71.67       0.00       0.00      22.24      35.09      14.52      49.61        0.11     0.10
0.692
  Pre3B                     71.67       0.00       0.00      29.10      24.56      18.44      42.99        0.07     0.06
0.600


  ******************
  Node Depth Summary
  ******************

  ----------------------------------------------------------------------------------
                                 Average  Maximum  Maximum  Time of Max    Reported
                                   Depth    Depth      HGL   Occurrence   Max Depth
  Node                 Type       Meters   Meters   Meters  days hr:min      Meters
  ----------------------------------------------------------------------------------
  EC-out               OUTFALL      0.02     0.43    47.76     0  01:15        0.42
  HW1                  OUTFALL      0.03     0.61    47.61     0  01:10        0.61
  EC-in                STORAGE      0.05     0.96    49.32     0  01:15        0.95
  HW2                  STORAGE      0.02     0.47    49.32     0  01:15        0.47
```

```
  STM100               STORAGE       0.04      0.75     47.87      0  01:10          0.75
  STM101               STORAGE       0.03      0.72     49.64      0  01:10          0.72
  STM200               STORAGE       0.03      0.52     49.46      0  01:12          0.51
  STM201               STORAGE       0.04      1.09     51.52      0  01:11          1.08
  SU-A                 STORAGE       0.06      1.48     50.78      0  01:13          1.48
  SU-B                 STORAGE       0.06      1.46     51.06      0  01:13          1.46
  SU-C                 STORAGE       0.06      1.37     52.57      0  01:14          1.37
  SU-D                 STORAGE       0.06      1.41     52.61      0  01:14          1.41


  *******************
  Node Inflow Summary
  *******************

  -------------------------------------------------------------------------------------------------
                                  Maximum  Maximum                  Lateral       Total        Flow
                                  Lateral    Total  Time of Max      Inflow      Inflow     Balance
                                   Inflow   Inflow   Occurrence      Volume      Volume       Error
  Node                 Type           CMS      CMS  days hr:min    10^6 ltr    10^6 ltr     Percent
  -------------------------------------------------------------------------------------------------
  EC-out               OUTFALL      0.000    0.533     0  01:15           0        1.38       0.000
  HW1                  OUTFALL      0.000    1.064     0  01:10           0        2.45       0.000
  EC-in                STORAGE      0.000    0.666     0  01:13           0        1.38       0.094
  HW2                  STORAGE      0.000    0.606     0  01:10           0        1.38      -0.025
  STM100               STORAGE      0.162    1.065     0  01:10       0.183        2.45      -0.001
  STM101               STORAGE      0.262    0.582     0  01:10       0.348        1.26       0.009
  STM200               STORAGE      0.000    0.568     0  01:11           0        1.38       0.001
  STM201               STORAGE      0.262    0.622     0  01:10       0.348        1.38       0.017
  SU-A                 STORAGE      0.739    0.739     0  01:10        1.01        1.01      -0.010
  SU-B                 STORAGE      0.661    0.661     0  01:10       0.904       0.904      -0.346
  SU-C                 STORAGE      0.297    0.297     0  01:10       0.406       0.406       0.130
  SU-D                 STORAGE      0.462    0.462     0  01:10       0.632       0.632       0.017


  **********************
  Node Surcharge Summary
  **********************

  No nodes were surcharged.


  *********************
  Node Flooding Summary
  *********************
```

```
No nodes were flooded.


**********************
Storage Volume Summary
**********************

--------------------------------------------------------------------------------------------------
                       Average     Avg  Evap Exfil       Maximum     Max    Time of Max    Maximum
                        Volume    Pcnt  Pcnt  Pcnt        Volume    Pcnt     Occurrence    Outflow
Storage Unit           1000 m3    Full  Loss  Loss       1000 m3    Full    days hr:min        CMS
--------------------------------------------------------------------------------------------------
EC-in                    0.000       2     0     0         0.001      36       0  01:15      0.533
HW2                      0.000       1     0     0         0.000      22       0  01:15      0.666
STM100                   0.000       1     0     0         0.001      22       0  01:10      1.064
STM101                   0.000       1     0     0         0.001      16       0  01:10      0.547
STM200                   0.000       2     0     0         0.001      26       0  01:12      0.606
STM201                   0.000       1     0     0         0.001      34       0  01:11      0.568
SU-A                     0.003       0     0     0         0.184      14       0  01:13      0.360
SU-B                     0.003       0     0     0         0.170      13       0  01:13      0.320
SU-C                     0.001       0     0     0         0.078       6       0  01:14      0.140
SU-D                     0.002       0     0     0         0.119       9       0  01:14      0.220


***********************
Outfall Loading Summary
***********************

-----------------------------------------------------------
                       Flow       Avg       Max       Total
                       Freq      Flow      Flow      Volume
Outfall Node           Pcnt       CMS       CMS    10^6 ltr
-----------------------------------------------------------
EC-out                27.11     0.075     0.533       1.383
HW1                   28.67     0.127     1.064       2.449
-----------------------------------------------------------
System                27.89     0.201     1.565       3.832


********************
Link Flow Summary
********************

-----------------------------------------------------------------------------
                               Maximum  Time of Max   Maximum    Max/    Max/
```

Petries III – PCSWMM Output Data
Design Storm

Design Storm: 100 Year 3-hour Chicago Storm

```
                                |Flow|   Occurrence   |Veloc|     Full    Full
  Link                 Type        CMS  days hr:min     m/sec     Flow   Depth
  -----------------------------------------------------------------------------
  100-HW1(C-STRM)      CONDUIT    1.064     0  01:10     2.06     1.32    0.76
  101-100_(C-STRM)     CONDUIT    0.547     0  01:11     2.10     1.06    0.90
  200-HW2_(C-STRM)     CONDUIT    0.606     0  01:10     2.93     1.04    0.71
  201-200_(C-STRM)     CONDUIT    0.568     0  01:11     2.10     1.11    0.91
  EastCulvert          CONDUIT    0.533     0  01:15     2.07     0.85    0.86
  EastDitch            CONDUIT    0.666     0  01:13     0.34     0.00    0.26
  SU-A_STM100          DUMMY      0.360     0  01:03
  SU-B_STM101          DUMMY      0.320     0  01:02
  SU-C_STM201          DUMMY      0.140     0  01:03
  SU-D_STM201          DUMMY      0.220     0  01:03


  ***************************
  Flow Classification Summary
  ***************************

  -------------------------------------------------------------------------------------
                      Adjusted    ---------- Fraction of Time in Flow Class ----------
                       /Actual         Up    Down  Sub   Sup   Up    Down  Norm  Inlet
  Conduit               Length    Dry  Dry   Dry   Crit  Crit  Crit  Crit  Ltd   Ctrl
  -------------------------------------------------------------------------------------
  100-HW1(C-STRM)         1.00   0.01  0.00  0.00  0.98  0.01  0.00  0.00  0.00  0.00
  101-100_(C-STRM)        1.00   0.01  0.00  0.00  0.00  0.00  0.00  0.99  0.00  0.00
  200-HW2_(C-STRM)        1.00   0.01  0.00  0.00  0.76  0.23  0.00  0.00  0.08  0.00
  201-200_(C-STRM)        1.00   0.01  0.00  0.00  0.00  0.00  0.00  0.99  0.00  0.00
  EastCulvert             1.00   0.02  0.00  0.00  0.76  0.23  0.00  0.00  0.00  0.44
  EastDitch               1.00   0.02  0.01  0.00  0.98  0.00  0.00  0.00  0.96  0.00


  *************************
  Conduit Surcharge Summary
  *************************

  ----------------------------------------------------------------------------
                                                             Hours        Hours
                         --------- Hours Full --------   Above Full   Capacity
  Conduit                Both Ends  Upstream  Dnstream   Normal Flow   Limited
  ----------------------------------------------------------------------------
  100-HW1(C-STRM)             0.01      0.01      0.01        0.29        0.01
  101-100_(C-STRM)            0.01      0.11      0.01        0.16        0.01
  200-HW2_(C-STRM)            0.01      0.01      0.01        0.02        0.01
  201-200_(C-STRM)            0.01      0.23      0.01        0.18        0.01
```

```
  EastCulvert                 0.01      0.52      0.01      0.01         0.01


  Analysis begun on:  Sun Aug  6 14:06:53 2023
  Analysis ended on:  Sun Aug  6 14:06:54 2023
  Total elapsed time: 00:00:01
```

E.5 Preliminary Stormceptor Sizing Reports

Stormceptor EF Sizing Report

Imbrium® Systems
ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

03/04/2024

Province:	Ontario
City:	Ottawa
Nearest Rainfall Station:	OTTAWA CDA RCS
Climate Station Id:	6105978
Years of Rainfall Data:	20

Site Name:	West Outlet

Drainage Area (ha):	3.42
Runoff Coefficient 'c':	0.83

Project Name:	Petries 3
Project Number:	160401751
Designer Name:	Michael Wu
Designer Company:	Stantec
Designer Email:	Michael.Wu@stantec.com
Designer Phone:	613-738-6033
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0
Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	91.62
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	3453
Estimated Average Annual Sediment Volume (L/yr):	2807

Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EFO4	51
EFO6	67
EFO8	77
EFO10	84
EFO12	88

Recommended Stormceptor EFO Model:	**EFO10**
Estimated Net Annual Sediment (TSS) Load Reduction (%):	**84**
Water Quality Runoff Volume Capture (%):	**> 90**

THIRD-PARTY TESTING AND VERIFICATION

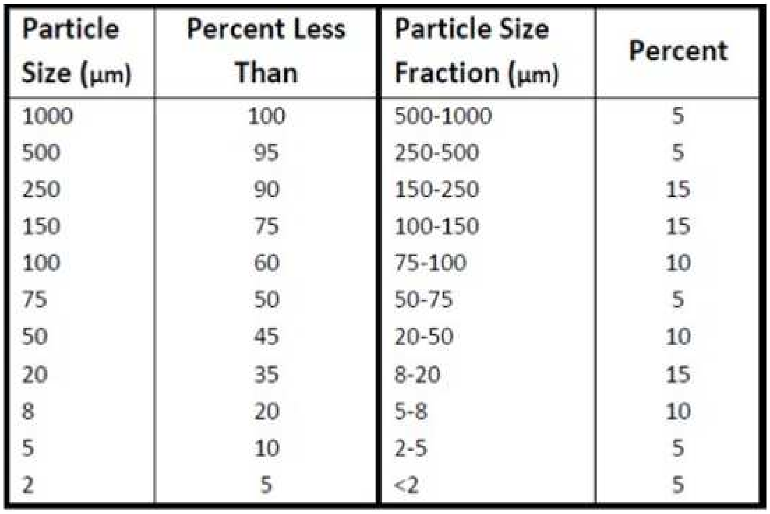
►**Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

►**Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

►The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV ***Procedure for Laboratory Testing of Oil-Grit Separators*** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

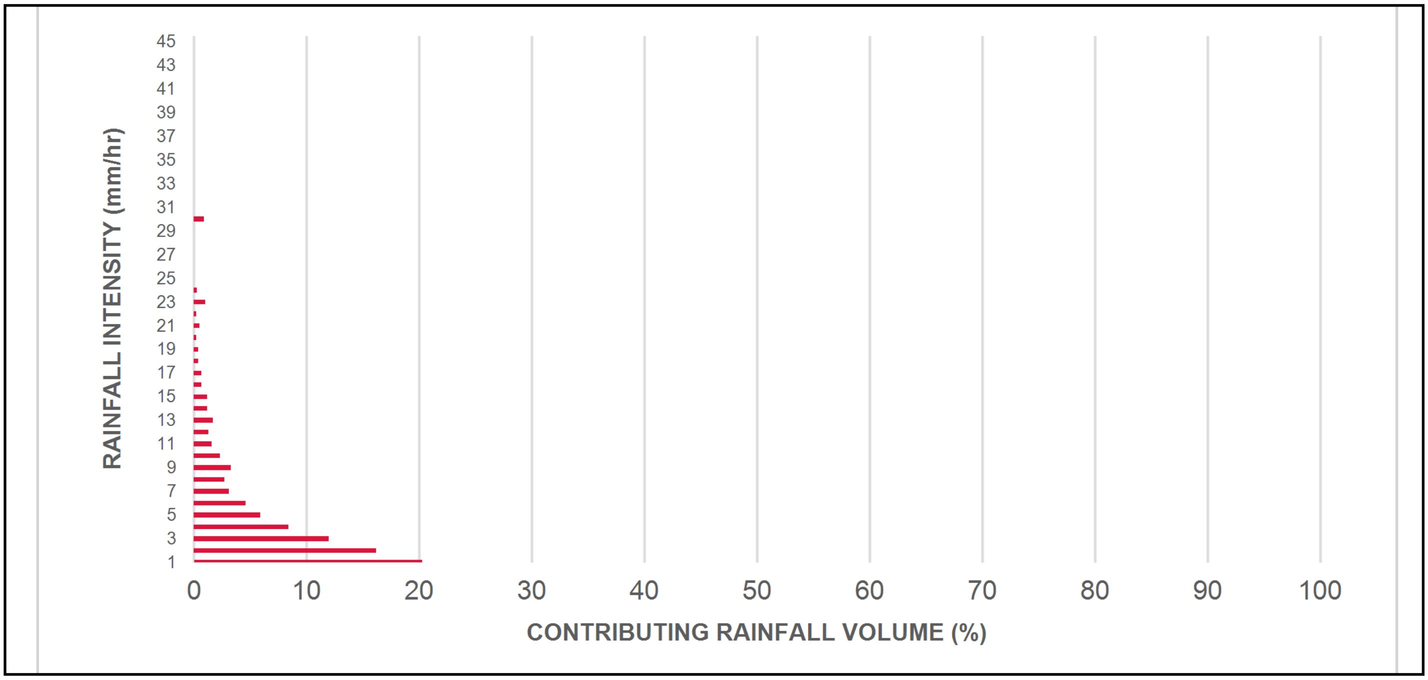


Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

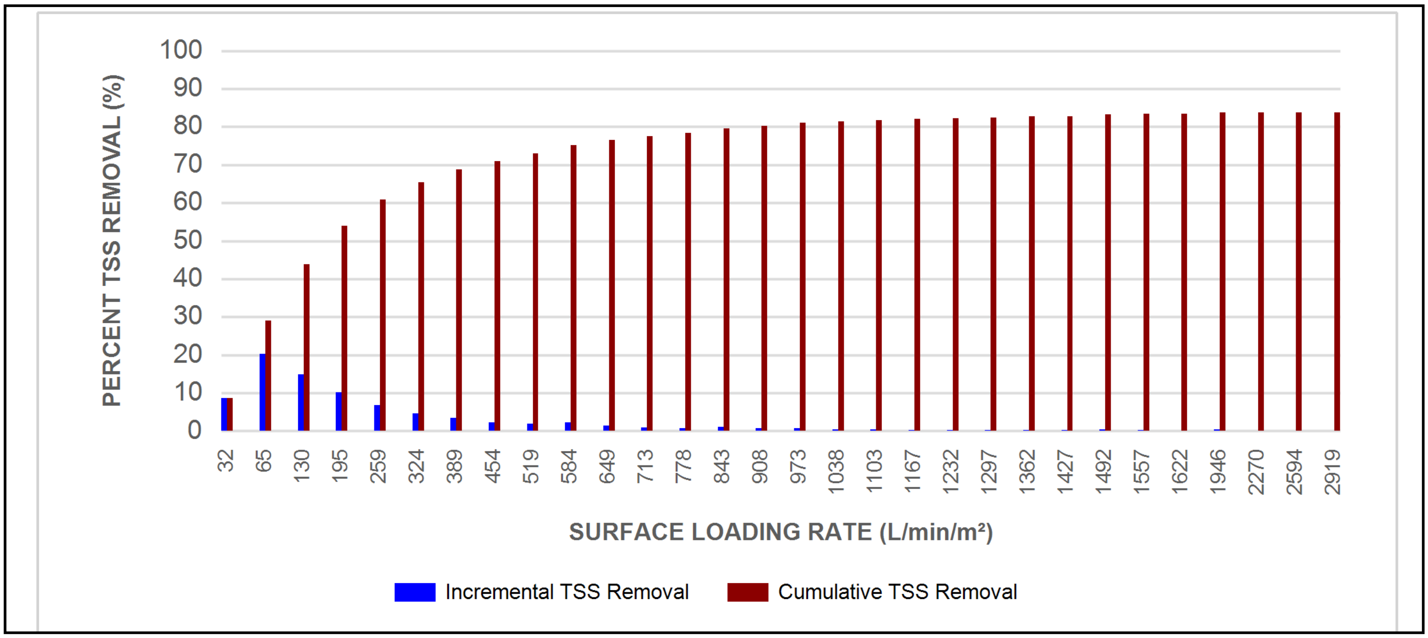
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.6	8.6	3.95	237.0	32.0	100	8.6	8.6
1.00	20.3	29.0	7.89	473.0	65.0	100	20.3	29.0
2.00	16.2	45.2	15.78	947.0	130.0	92	14.9	43.9
3.00	12.0	57.2	23.67	1420.0	195.0	84	10.1	54.0
4.00	8.4	65.6	31.57	1894.0	259.0	80	6.8	60.8
5.00	5.9	71.6	39.46	2367.0	324.0	78	4.6	65.4
6.00	4.6	76.2	47.35	2841.0	389.0	74	3.4	68.8
7.00	3.1	79.3	55.24	3314.0	454.0	72	2.2	71.0
8.00	2.7	82.0	63.13	3788.0	519.0	69	1.9	72.9
9.00	3.3	85.3	71.02	4261.0	584.0	66	2.2	75.1
10.00	2.3	87.6	78.91	4735.0	649.0	64	1.5	76.6
11.00	1.6	89.2	86.80	5208.0	713.0	64	1.0	77.6
12.00	1.3	90.5	94.70	5682.0	778.0	63	0.8	78.4
13.00	1.7	92.2	102.59	6155.0	843.0	63	1.1	79.5
14.00	1.2	93.5	110.48	6629.0	908.0	62	0.8	80.3
15.00	1.2	94.6	118.37	7102.0	973.0	62	0.7	81.0
16.00	0.7	95.3	126.26	7576.0	1038.0	61	0.4	81.4
17.00	0.7	96.1	134.15	8049.0	1103.0	59	0.4	81.8
18.00	0.4	96.5	142.04	8523.0	1167.0	58	0.2	82.1
19.00	0.4	96.9	149.93	8996.0	1232.0	56	0.2	82.3
20.00	0.2	97.1	157.83	9470.0	1297.0	55	0.1	82.4
21.00	0.5	97.5	165.72	9943.0	1362.0	53	0.2	82.7
22.00	0.2	97.8	173.61	10417.0	1427.0	52	0.1	82.8
23.00	1.0	98.8	181.50	10890.0	1492.0	49	0.5	83.3
24.00	0.3	99.1	189.39	11363.0	1557.0	47	0.1	83.4
25.00	0.0	99.1	197.28	11837.0	1622.0	45	0.0	83.4
30.00	0.9	100.0	236.74	14204.0	1946.0	38	0.4	83.8
35.00	0.0	100.0	276.20	16572.0	2270.0	32	0.0	83.8
40.00	0.0	100.0	315.65	18939.0	2594.0	28	0.0	83.8
45.00	0.0	100.0	355.11	21307.0	2919.0	25	0.0	83.8
Estimated Net Annual Sediment (TSS) Load Reduction =								**84 %**

Climate Station ID: 6105978 *Years of Rainfall Data*: 20

RAINFALL DATA FROM OTTAWA CDA RCS RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

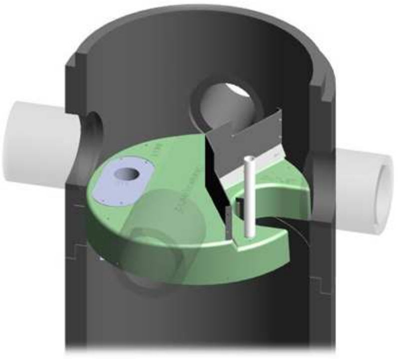
►**Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

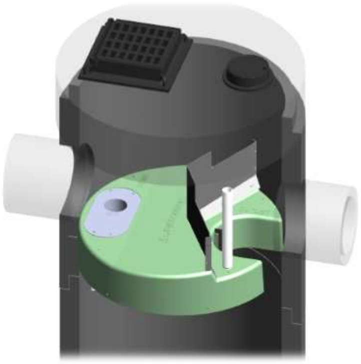
DESIGN FLEXIBILITY

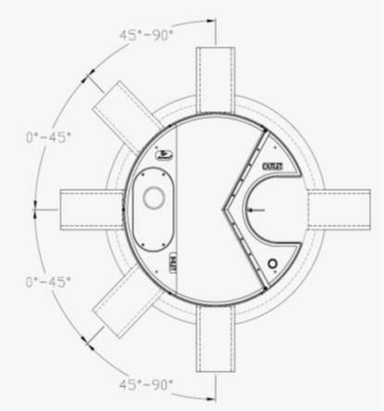
►**Stormceptor® EF** and **EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

►While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.







INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity
** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef

STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m^3 sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m^3 sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m^3 sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m^3 sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m^3 sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m^2 to 1400 L/min/m^2, and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m^2 and 1400 L/min/m^2 shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m^2 shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m^2. No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m^2.

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m^2 shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m^2, and shall be calculated using a simple proportioning formula, with 1400 L/min/m^2 in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m^2.

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators.**

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m^2.

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators,** with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m^2 to 2600 L/min/m^2) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators.** However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.

Stormceptor EF Sizing Report

Imbrium® Systems
ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

03/04/2024

Province:	Ontario
City:	Ottawa
Nearest Rainfall Station:	OTTAWA CDA RCS
Climate Station Id:	6105978
Years of Rainfall Data:	20

Site Name:	East Outlet

Drainage Area (ha):	2.1
Runoff Coefficient 'c':	0.82

Project Name:	Petries 3
Project Number:	160401751
Designer Name:	Michael Wu
Designer Company:	Stantec
Designer Email:	Michael.Wu@stantec.com
Designer Phone:	613-738-6033
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0
Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	55.58
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	2105
Estimated Average Annual Sediment Volume (L/yr):	1712

Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EFO4	61
EFO6	76
EFO8	85
EFO10	90
EFO12	93

Recommended Stormceptor EFO Model: **EFO8**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **85**

Water Quality Runoff Volume Capture (%): **> 90**

THIRD-PARTY TESTING AND VERIFICATION

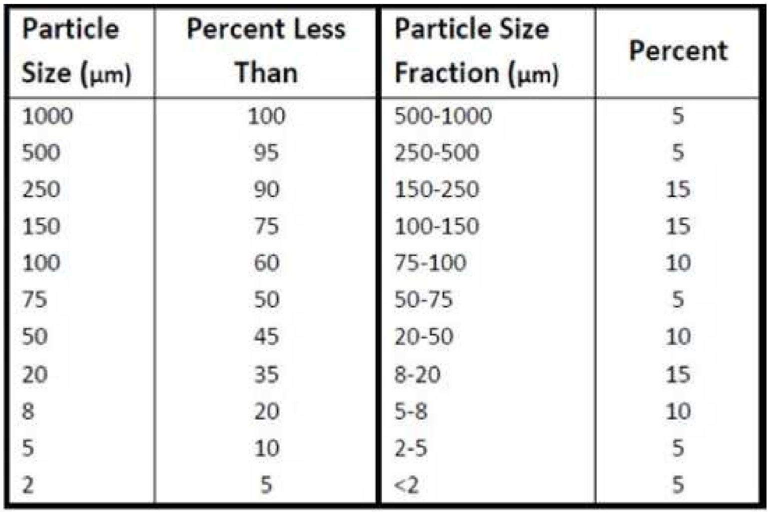
►**Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

►**Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

►The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV ***Procedure for Laboratory Testing of Oil-Grit Separators*** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

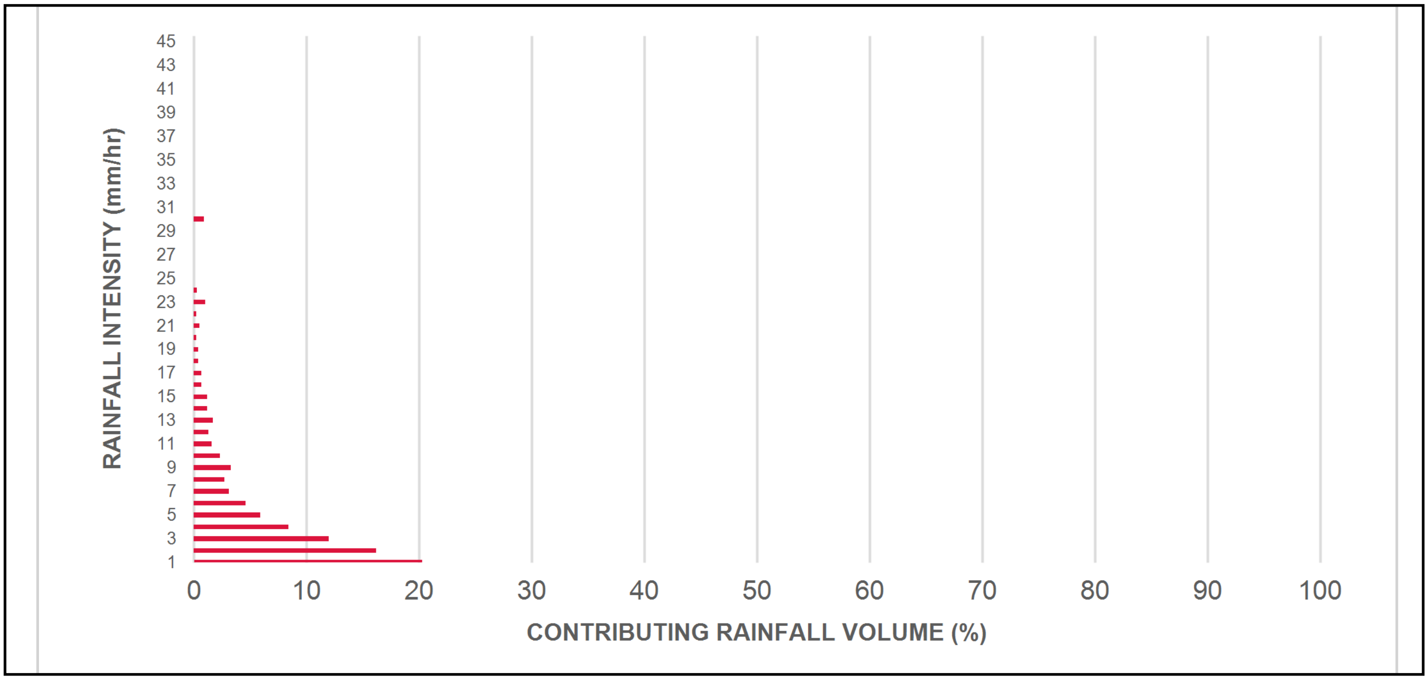


Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

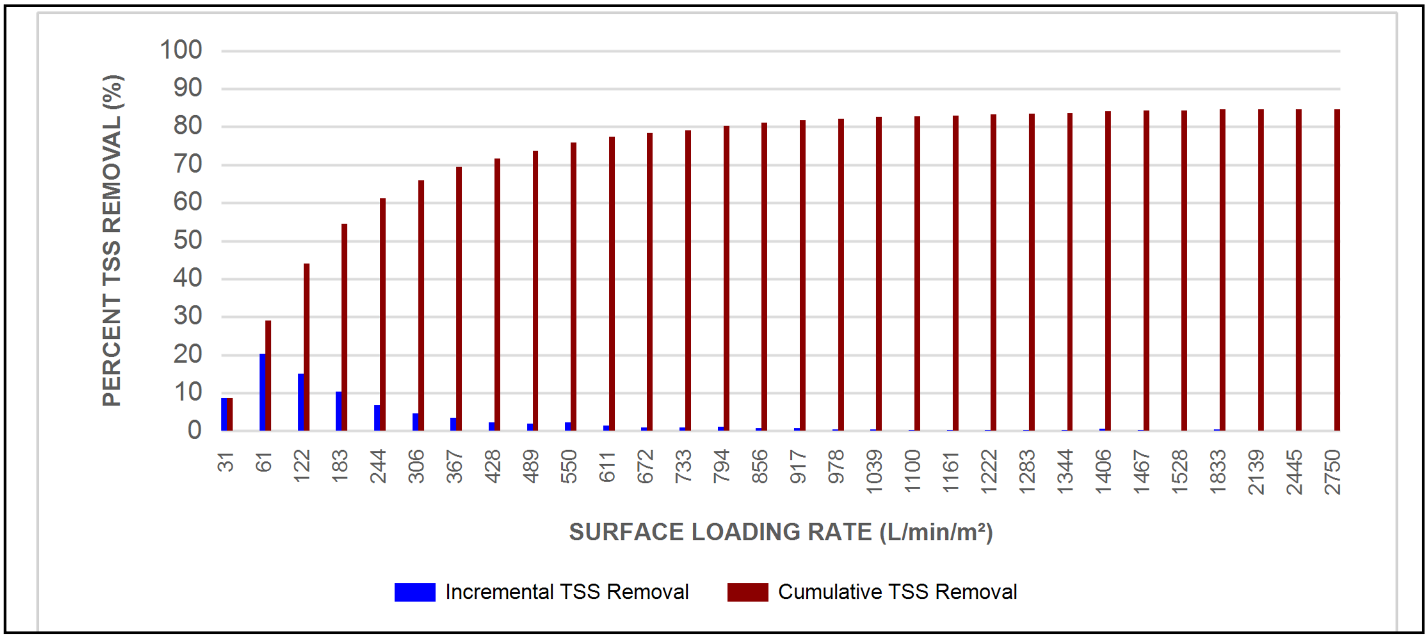
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.6	8.6	2.39	144.0	31.0	100	8.6	8.6
1.00	20.3	29.0	4.79	287.0	61.0	100	20.3	29.0
2.00	16.2	45.2	9.57	574.0	122.0	93	15.1	44.1
3.00	12.0	57.2	14.36	862.0	183.0	86	10.3	54.4
4.00	8.4	65.6	19.15	1149.0	244.0	81	6.9	61.2
5.00	5.9	71.6	23.94	1436.0	306.0	78	4.7	65.9
6.00	4.6	76.2	28.72	1723.0	367.0	76	3.5	69.4
7.00	3.1	79.3	33.51	2011.0	428.0	73	2.2	71.6
8.00	2.7	82.0	38.30	2298.0	489.0	70	1.9	73.6
9.00	3.3	85.3	43.08	2585.0	550.0	67	2.2	75.8
10.00	2.3	87.6	47.87	2872.0	611.0	65	1.5	77.3
11.00	1.6	89.2	52.66	3160.0	672.0	64	1.0	78.3
12.00	1.3	90.5	57.45	3447.0	733.0	64	0.8	79.1
13.00	1.7	92.2	62.23	3734.0	794.0	63	1.1	80.2
14.00	1.2	93.5	67.02	4021.0	856.0	63	0.8	81.0
15.00	1.2	94.6	71.81	4308.0	917.0	62	0.7	81.7
16.00	0.7	95.3	76.59	4596.0	978.0	62	0.4	82.1
17.00	0.7	96.1	81.38	4883.0	1039.0	61	0.5	82.6
18.00	0.4	96.5	86.17	5170.0	1100.0	59	0.2	82.8
19.00	0.4	96.9	90.96	5457.0	1161.0	58	0.2	83.0
20.00	0.2	97.1	95.74	5745.0	1222.0	56	0.1	83.2
21.00	0.5	97.5	100.53	6032.0	1283.0	55	0.3	83.4
22.00	0.2	97.8	105.32	6319.0	1344.0	54	0.1	83.6
23.00	1.0	98.8	110.10	6606.0	1406.0	52	0.5	84.1
24.00	0.3	99.1	114.89	6894.0	1467.0	50	0.1	84.2
25.00	0.0	99.1	119.68	7181.0	1528.0	48	0.0	84.2
30.00	0.9	100.0	143.61	8617.0	1833.0	40	0.4	84.6
35.00	0.0	100.0	167.55	10053.0	2139.0	34	0.0	84.6
40.00	0.0	100.0	191.49	11489.0	2445.0	30	0.0	84.6
45.00	0.0	100.0	215.42	12925.0	2750.0	27	0.0	84.6
Estimated Net Annual Sediment (TSS) Load Reduction =								**85 %**

Climate Station ID: 6105978 *Years of Rainfall Data*: 20

RAINFALL DATA FROM OTTAWA CDA RCS RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter (m)	Model Diameter (ft)	Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter (mm)	Max Inlet Pipe Diameter (in)	Max Outlet Pipe Diameter (mm)	Max Outlet Pipe Diameter (in)	Peak Conveyance Flow Rate (L/s)	Peak Conveyance Flow Rate (cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

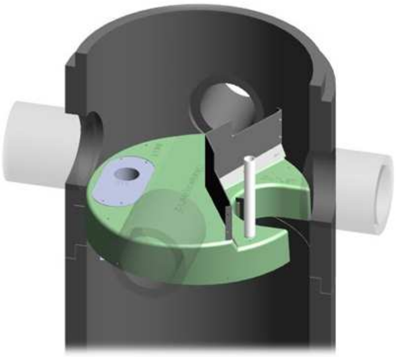
►**Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

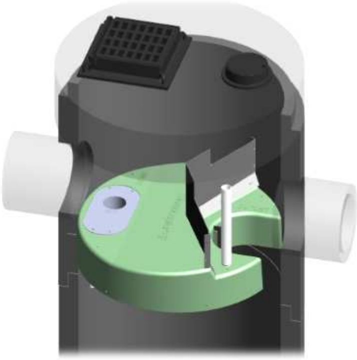
DESIGN FLEXIBILITY

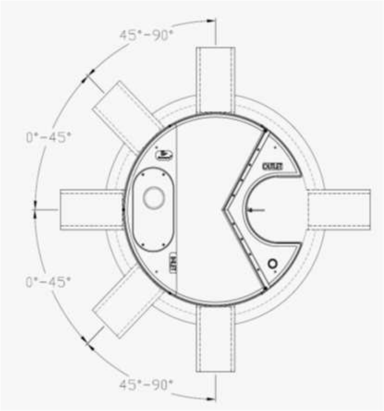
►**Stormceptor® EF** and **EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

►While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.







INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft^3)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft^3)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

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STANDARD STORMCEPTOR EF/EFO SPECIFICATION

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STANDARD PERFORMANCE SPECIFICATION FOR "OIL GRIT SEPARATOR" (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators**

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1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

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2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m^3 sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m^3 sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m^3 sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m^3 sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m^3 sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m^2 to 1400 L/min/m^2, and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m^2 and 1400 L/min/m^2 shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m^2 shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m^2. No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m^2.

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m^2 shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m^2, and shall be calculated using a simple proportioning formula, with 1400 L/min/m^2 in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m^2.

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

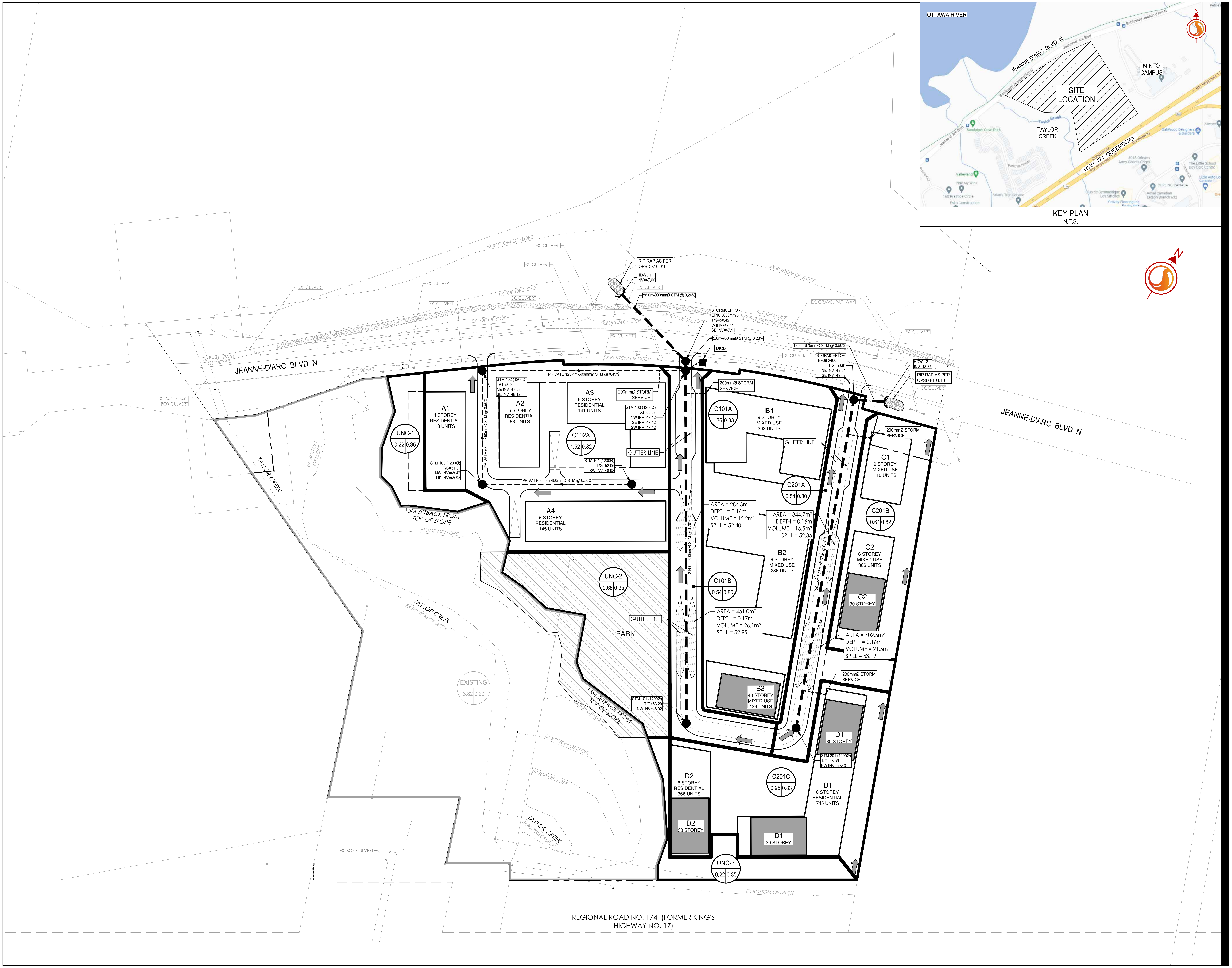
The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators.**

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m^2.

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators,** with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m^2 to 2600 L/min/m^2) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators.** However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.



Stantec

Stantec Consulting Ltd.
400 - 1331 Clyde Avenue
Ottawa ON
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Legend

▬ ➤ ● ➤ ▬	TRUNK STORM SEWER
▬ ➤ ● ➤ ▬	EXISTING TRUNK STORM SEWER
- - ➤ ● ➤ - -	PRIVATE STORM SEWER
⇦	MAJOR OVERLAND FLOW ROUTE
ST101	DRAINAGE AREA No.
0.80	RUNOFF COEFFICIENT
1.00	STORM DRAINAGE AREA (ha)
▬	STORM DRAINAGE BOUNDARY
▨	MAXIMUM PONDING LIMITS

Notes

1	REVISED AS PER CITY COMMENTS	MJS	RB	24.03.06
0	ISSUED FOR DRAFT PLAN APPROVAL	MJS	RB	23.08.08
Revision		By	Appd.	YY.MM.DD

File Name: 160401751 DRAFT LEVEL.dwg	MJS	RB	MJS	23.06.01
	Dwn.	Chkd.	Dsgn.	YY.MM.DD

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LICENSED PROFESSIONAL ENGINEER
R. J. B. BRANDRICK
100570025
March 6 2024
PROVINCE OF ONTARIO

Client/Project

BRIGIL
98 LOIS STREET
GATINEAU, QC J8T 3R7

PETRIES LANDING III
OTTAWA, ON

Title

OVERALL STORM SEWER SYSTEM

Project No. 160401751
Scale 1:1000 (0 10 30 50m)
Drawing No. STM-1
Sheet 1 of 4
Revision 1
PLAN #
D07-16-23-0021

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2024/03/06 10:08 AM By: Sharp, Mike

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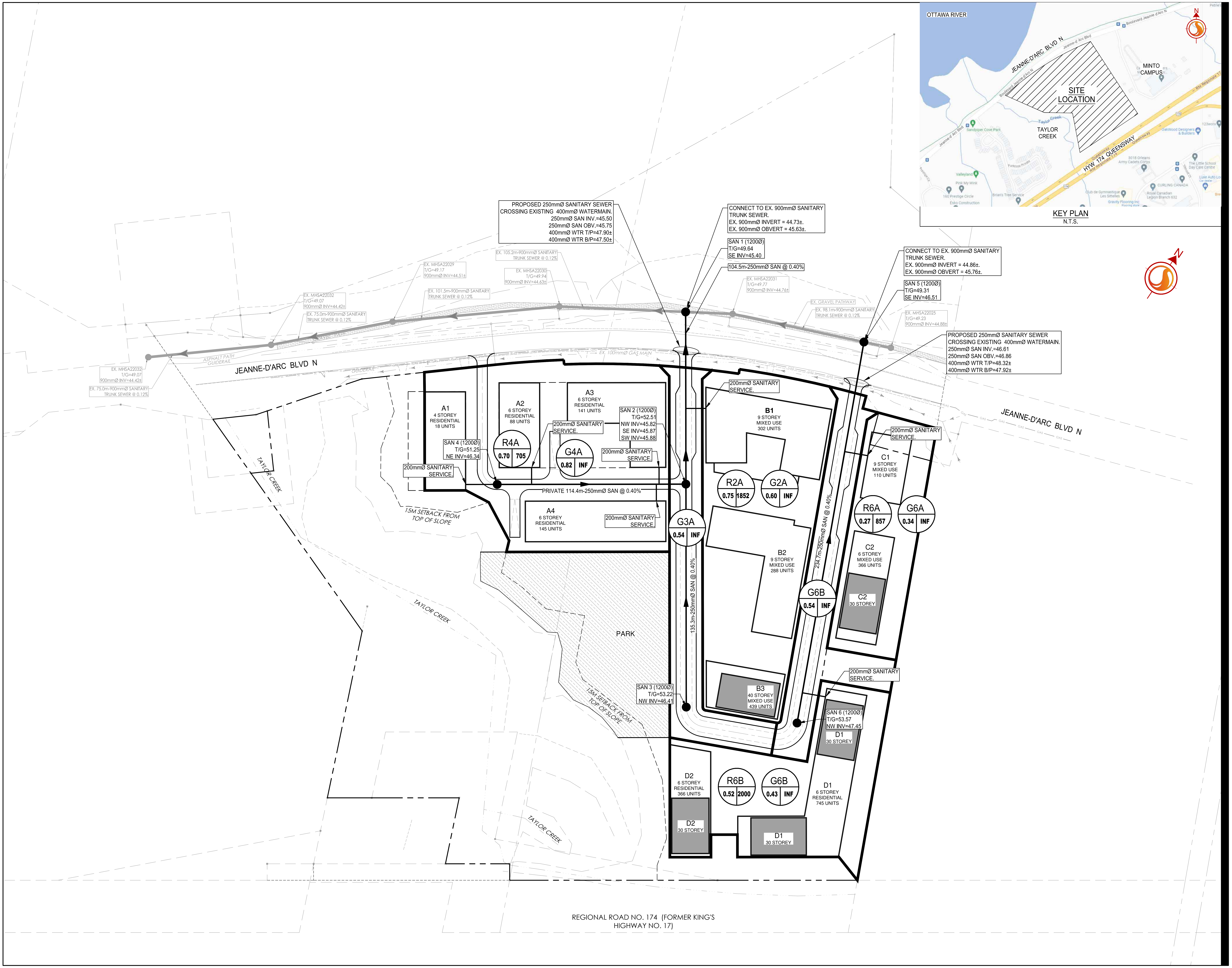

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PROPOSED SANITARY SEWER
EXISTING TRUNK SANITARY SEWER
SANITARY DRAINAGE BOUNDARY
EXISTING SANITARY DRAINAGE BOUNDARY

R1A 10.04 100
SANITARY DRAINAGE AREA ID#
POPULATION COUNTS
SANITARY DRAINAGE AREA ha.

G1A 10.04 INF
SANITARY DRAINAGE AREA ID#
INFILTRATION RATES
SANITARY DRAINAGE AREA ha.

Notes

BUILDING STATS

BUILDING A1 - 18 - UNITS @ 1.8PPU = 32 PEOPLE
BUILDING A2 - 88 - UNITS @ 1.8PPU = 158 PEOPLE
BUILDING A3 - 141 - UNITS @ 1.8PPU = 254 PEOPLE
BUILDING A4 - 145 - UNITS @ 1.8PPU = 261 PEOPLE
BUILDING B1 - 302 - UNITS @ 1.8PPU = 544 PEOPLE
BUILDING B2 - 288 - UNITS @ 1.8PPU = 518 PEOPLE
BUILDING B3 - 439 - UNITS @ 1.8PPU = 790 PEOPLE
BUILDING C1 - 110 - UNITS @ 1.8PPU = 198 PEOPLE
BUILDING C2 - 366 - UNITS @ 1.8PPU = 659 PEOPLE
BUILDING D1 - 745 - UNITS @ 1.8PPU = 1341 PEOPLE
BUILDING D2 - 366 - UNITS @ 1.8PPU = 659 PEOPLE
TOTAL POPULATION = 5414

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R. J. B. BRANDRICK
100570025
March 6 2024

Client/Project

BRIGIL
98 LOIS STREET
GATINEAU, QC J8T 3R7

PETRIES LANDING III
OTTAWA, ON

Title

OVERALL SANITARY SEWER SYSTEM

Project No.
160401751

Scale
1:1000 0 10 30 50m

Drawing No.
SAN-1

Sheet
2 of 4

Revision
1

PLAN #
D07-16-23-0021

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Legend

- 400mmØ WATERMAIN
- 300mmØ WATERMAIN
- 250mmØ WATERMAIN
- 200mmØ WATERMAIN
- 150mmØ WATERMAIN
- EX. 400mmØ WATERMAIN
- EX. 300mmØ WATERMAIN
- EX. 250mmØ WATERMAIN
- EX. 200mmØ WATERMAIN
- EX. 150mmØ WATERMAIN

Notes

		By	Appd.	YY.MM.DD
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Revision

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100570025
March 6 2024

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BRIGIL
98 LOIS STREET
GATINEAU, QC J8T 3R7

PETRIES LANDING III
OTTAWA, ON

Title

OVERALL WATERMAIN SYSTEM

Project No.
160401751

Scale
1:1000
0 10 30 50m

Drawing No.
WTR-1

Sheet
3 of 4

Revision
1

PLAN #
D07-16-23-0021

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KEY PLAN
N.T.S.

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Legend

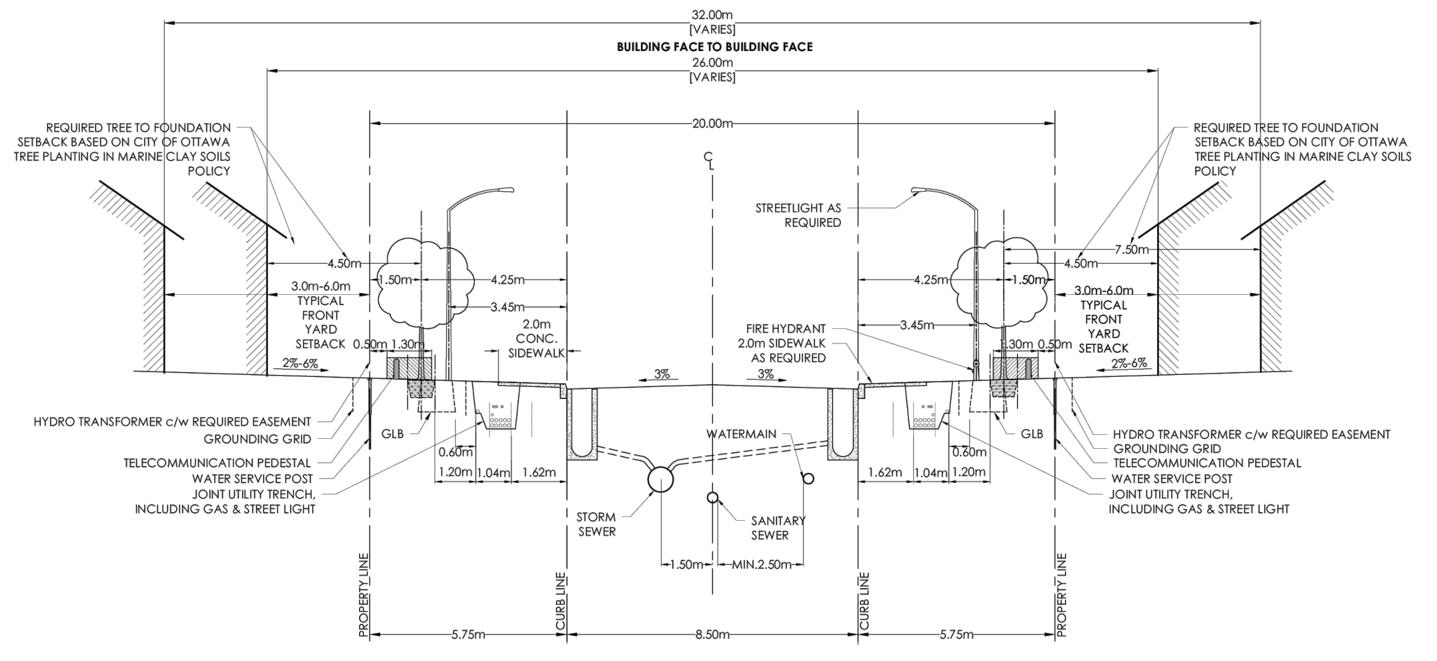
× 87.65	EXISTING ELEVATION
89.40 ×	PROPOSED ELEVATION
⬅	MAJOR OVERLAND FLOW ROUTE
■■■■■■■■■■	MAJOR SYSTEM DIVIDE

PROPOSED GRADE RAISE PROVIDED BY PATERSON GROUP, REPORT TITLED GEOTECHNICAL INVESTIGATION - PROPOSED RESIDENTIAL DEVELOPMENT - EDEN PARK - EAST PORTION - RENAUD ROAD, REPORT DATED DECEMBER 29, 2008, REPORT No. PG0861-3.

⬡	AREAS WHERE GRADE RAISES UP TO 0.0m ARE PERMITTED.
▨	AREAS WHERE GRADE RAISES UP TO 1.0m ARE PERMITTED.
⊠	AREAS WHERE GRADE RAISES UP TO 1.5m ARE PERMITTED.

Notes

1. STANDARD CROSS-SECTIONS TO BE READ IN CONJUNCTION WITH THE GENERAL STANDARD CROSS-SECTION NOTES AND OTHER APPLICABLE CITY AND UTILITY PLANS AND DETAILS.
2. CONCRETE CURBS TO BE CONSTRUCTED AS PER CITY OF OTTAWA STANDARD DETAILS.
3. TYPICAL FRONT YARD SETBACK IS TO BE CLEAR AND UNENCUMBERED OF ANY SUBSURFACE BUILDING ENCROACHMENTS.
4. FIRE HYDRANTS TO BE LOCATED ON THE WATERMAIN SIDE OF THE STREET.
5. CATCH BASINS TO BE PER CITY OF OTTAWA DETAIL S2.
6. GAS MAIN SHALL HAVE A MINIMUM OF 0.6M CLEARANCE FROM STRUCTURES E.G.CATCH BASINS AND HYDRANTS) AND 1.2 M FROM TREE ROOT BALL.
7. STREETLIGHTS CAN BE LOCATED ON EITHER SIDE OF THE RIGHT-OF-WAY.
8. GAS MAIN SHALL HAVE A MINIMUM OF 0.6 M CLEARANCE FROM STRUCTURES E.G. CATCH BASINS AND HYDRANTS) AND 1.2 M FROM TREE ROOT BALL.
9. JOINT-USE UTILITY TRENCH (JUT) UNDER SIDEWALK AS PER DETAIL UDS0049 (REV 22) HELD BY OTTAWA HYDRO.
10. GRADE LEVEL BOX (GLB) AS DRAWN SHOWS GLB3660. EXACT LOCATION TO BE CONFIRMED.
    THIS CROSS SECTION TO BE USED IF CONCRETE ENCASED HYDRO DUCT OR ANOTHER SEPARATE UTILITY DUCT IS REQUIRED.
    IF CONCRETE ENCASED HYDRO DUCT IS UTILIZED, INSTALATION AS PER DETAIL UDS0051.
11. WHEN CONCRETE DUCT BANKS ARE REQUIRED, ADDITIONAL CLEARANCE IS REQUIRED FOR THE INSTALLATION OF A 2.2M X 4.0M MAINTENANCE HOLE PER OTTAWA HYDRO DETAIL UCS0014.
    LOCATIONS TO BE DETERMINED DURING DESIGN PHASE.
12. TREE CLEARANCES TO HYDRO OTTAWA PLANT SHALL FOLLOW GCS0038.
13. CLEARANCES SHOWN ARE MINIMUMS.



Ottawa

20.0m ROW CROSS SECTION

REV.DATE: AUG. 2022
DWG. No. ROW-20.0

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PROVINCE OF ONTARIO

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BRIGIL
98 LOIS STREET
GATINEAU, QC J8T 3R7

PETRIES LANDING III
OTTAWA, ON

Title

OVERALL GRADE CONTROL PLAN

Project No. 160401751
Scale 1:1000 0 10 30 50m
Drawing No. GP-1
Sheet 4 of 4
Revision 1
PLAN #

D07-16-23-0021

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