

April 15, 2024

Project Number: 1474

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K2S 1E9

**Attention: Marc Pichette, P.Eng**

**Subject: Barrhaven Conservancy East Phase 3 & 4 FSR – Proposed Draft  
Plan Revisions – Preliminary HGL Analysis**

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## Introduction

Barrhaven Conservancy East Development is located in Barrhaven, Ontario, west of the Fraser-Clarke watercourse, east of Borrisokane Road, south of Strandherd Drive and north of the Jock River. The focus of this memo is to provide technical support for a proposed draft plan revision for a portion of the Conservancy East (Phase 3 & 4) land area (~**14.83 ha**).

The revisions to the draft plan affect a drainage area of approximately **14.52 ha** and are contained to the western extent of the greater Barrhaven Conservancy East development, within Phase 3 & 4. The incorporation of a residential site plan block replacing some of the previous single lots in the area and the increase in overall imperviousness from **57%** to **65%** for the drainage area above (~**14.52 ha**) triggered the FSR update. Note that the eastern portion of the development remains unaltered from the approved June 2022 SWM Report for this subdivision and the March 2024 SWM Report for Phase 2B of the Barrhaven Conservancy.

The following outlines the preliminary hydraulic grade line (HGL) assessment for the site, to ensure that the proposed minor system within the development is adequately sized to safely convey flows to the Jock River under various conditions. As such the following memo outlines the approach taken in assessing the development's HGL and summarises the findings of this analysis.

## Analysis Approach

Preliminary hydraulic grade line calculations for the proposed Barrhaven Conservancy West development were completed using PCSWMM modelling software. Pipe data, storm sewer layout, drainage areas and imperviousness are as provided by DSEL. The March 2024 detailed PCSWMM model (**BCDC-P2B\_v04.1**) submitted as part of the SWM Report for Phase 2B of the Barrhaven Conservancy, was the base used to build the latest PCSWMM model used in this analysis. Since Phase 3 & 4 are part of a complex storm sewer network connected to the other detailed phases, instead of only using the storm trunks for the preliminary HGL analysis as presented in the previous FSR submission, this analysis uses a detailed PCSWMM model that includes detailed drainage areas and the representation of the major and minor systems according to the preliminary servicing and grading design provided by DSEL.

The updates to the PCSWMM model within Phases 3 & 4 include changes to the subcatchment areas and imperviousness, as well as pipe data as per DSEL’s latest preliminary servicing design. The minor system release rates for the residential site plan area incorporated into Phase 3 & 4 is limited to the 2-year Rational Method flow, with excess flows to the major system/street. For modelling purposes, the minor system capture rate for the residential site plan area was limited to 114% of the 2-year Rational Method flow in order to account for additional flows captured during the 100-year storm.

As with all other works completed for the Barrhaven Conservancy development phases, the preliminary HGL analysis was completed under two conditions:

- 100-year rainfall event on the development and a 5-year spring water level on the Jock River.
- 5-year rainfall event on the development and a 100-year spring water level on the Jock River.

Note that the water level along the Jock River through the length of this development varies, and as such the nearest corresponding upstream water surface elevation calculated by RVCA’s HEC-RAS floodplain mapping model of the Jock River was applied at each of the respective storm sewer outlets. Also, note that assuming a 5-year spring water level on the Jock River for a 100-year rainfall event on the development is an inherently conservative assumption, as the critical storm for the proposed development is a summer (intense rainfall) event while the critical storm for the Jock River is a spring (snowmelt + rainfall) event. A preliminary Single Station Flood Frequency analysis was completed by JFSA using only summer flows (from May 15 to October 31) based on historical flow data recorded at the Moodie Drive Water Survey Canada gauge. This analysis found that the 100-year summer flow on the Jock River is around **99 m<sup>3</sup>/s**, while the 5-year spring flow is around **123 m<sup>3</sup>/s**, therefore the downstream boundary condition applied is conservative.

Within the proposed development, Oil and Grit Separators (OGS) units in conjunction with LID measures will be implemented to ensure the site meets quality control requirements. Preliminary OGS units and associated by-pass weir elevations have also been included in the model, as specified by Echelon Environmental. **Table 1** below outlines the OGS sizes and configurations as specified. Note that the trunk sewers for this development have been numbered 1-13, however only storm sewer systems **5, 6, 7 & 13** have been updated as part of this study.

**Table 1: OGS Units Details**

Trunk System	Area (ha)	Runoff Coefficient	CDS Model	CDS	Weir
				Inlet/Outlet Pipe (mm)	Elev. (m)
Trunk 5	8.38	0.67	PMSU 5640 -10	750	90.053
Trunk 6	5.20	0.50	PMSU 4040 - 8	525	90.261
Trunk 7	5.58	0.74	PMSU 4040 - 8	600	90.310
Trunk 13	1.23	0.68	PMSU 2025 - 5	375	90.913
<b>Total</b>	<b>20.39</b>	<b>0.65</b>			

## Results

The maximum HGL obtained at each MH has been extracted from the 5-year event / 100-year Jock River water level scenario and the 100-year event / 5-year Jock River water level scenario, with the results from this analysis provided in **Tables 2 & 3**, respectively. As all proposed units within this development will have sump pumps, the simulated HGL was compared against the top of MH elevation to ensure that all storm sewers infrastructure is sufficiently sized and is not surcharging to the major system during the assessed events.

From this analysis, it was found that the critical scenario for HGL within the development was the 5-year event and 100-year water level on the Jock River scenario. Based on this scenario, no MHs will have an HGL elevation above the top of MH (minimum freeboard of **0.04 m** at **MH-529\_2**), with an average freeboard of **0.75 m** from the top of MH throughout the proposed development.

For the 100-year event and 5-year water level on the Jock River, no MHs will have HGL elevations above the top of MH (minimum freeboard of **0.07 m** at **MH-504**), with an average freeboard of **0.69 m** from the top of MH throughout the proposed development. As such it can be concluded that the proposed storm sewer infrastructure is sufficiently sized, to safely convey minor system flows from the development under various extreme conditions.

## Conclusion

A preliminary HGL analysis for Phase 3 & 4 of the Barrhaven Conservancy East Development was completed using PCSWMM based on the updates and details provided by DSEL. From this analysis, it was found that the proposed storm sewer infrastructure is sufficiently sized to convey all minor system flows to the Jock River and will not result in any MHs surcharging to the street under extreme events such as 100-year rainfall event on the development and a 5-year spring water level on the Jock River and a 5-year event on the development and a 100-year spring water level on the Jock River, with the latter being the more critical scenario for the HGL within the development.

Yours truly,  
**JFSA Canada Inc.**



Paulo Pickart, B.Eng, P.Eng  
Water Resources Project Engineer

cc: J.F Sabourin, M.Eng, P.Eng  
Director of Water Resources Projects



### Figures

- Figure 1: Minor System Overview
- Figure 2: Major System Overview
- Figure 3: Subcatchments Overview

### Tables

- Table 1: OGS Units Details
- Table 2: HGL Result Tables - 5-Year BCDC Development & 100-Year Jock River
- Table 3: HGL Result Tables - 100-Year BCDC Development & 5-Year Jock River

### Attachments

- Attachment A: DSEL Rational Method Calculations

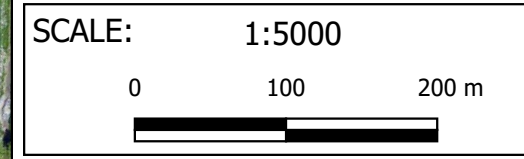
### Modelling Files – Provided Electronically

- PCSWMM BCDC-P3&4\_v05.3-005YrJock.inp
- BCDC-P3&4\_v05.3-100YrJock.inp



**Legend**

- Phase 3 & 4
- Site Plan
- Conduits**
- STM
- Overland
- CB
- CBMH
- CICB
- EX\_CICB
- MH
- OGS
- RYCB
- ▲ Outfalls



BCDC Phase 3 & 4  
(FSR)

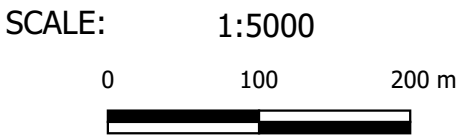
Figure 1: Minor System Overview

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DRAWN	PP
DATE	APRIL 2024



**Legend**

- Phase 3 & 4
- Site Plan
- Conduits
- Major System
- Overland
- Junctions
- Maj
- ▲ Outfalls



BCDC Phase 3 & 4 (FSR)

Figure 2: Major System Overview

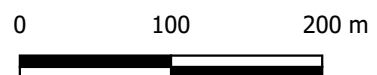
PROJECT	1474 (03)
DRAWN	PP
DATE	APRIL 2024



**Legend**

- Phase 3 & 4
- Site Plan
- Subcatchments
- <ID>
- <AREA>
- <IMP>

SCALE: 1:5000



BCDC Phase 3 & 4  
(FSR)

Figure 3: Subcatchments Overview

PROJECT	1474 (03)
DRAWN	PP
DATE	APRIL 2024

**Table 2: Freeboard Results - 5-Year Chicago 3 Hour Event & 100 Year Jock River  
Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
OGS_1	89.56	92.54	91.73	0.81
MH-100	90.96	93.27	92.56	0.71
MH-101	90.85	93.28	92.55	0.73
MH-102	90.65	93.06	92.48	0.58
MH-104	90.99	93.17	92.53	0.64
MH-105	90.53	93.17	92.46	0.71
MH-108	90.80	93.07	92.46	0.61
MH-109	90.36	92.98	92.40	0.58
MH-110	90.82	93.05	92.33	0.72
MH-111	90.70	93.12	92.49	0.63
MH-112	90.24	92.98	92.34	0.64
MH-114	90.64	92.97	92.32	0.65
MH-115	90.44	92.89	92.31	0.58
MH-116	90.12	92.79	92.26	0.53
MH-117	90.25	92.85	92.08	0.77
MH-1170	90.33	93.85	92.08	1.77
MH-118	90.70	93.05	92.17	0.88
MH-1180	90.62	92.84	92.16	0.68
MH-119	90.27	92.80	92.07	0.73
MH-120	89.93	92.76	92.01	0.75
MH-1201	90.67	93.02	91.80	1.22
MH-1202	90.45	92.90	91.71	1.19
MH-1203	90.33	92.86	91.71	1.15
MH-1206	90.62	93.06	91.93	1.13
MH-1207	90.51	92.99	91.85	1.14
MH-1209	90.22	92.86	91.76	1.10
MH-1209_2	90.22	92.80	91.96	0.84
MH-121	89.86	92.73	91.93	0.80
MH-122	89.82	92.68	91.89	0.80
MH-124	90.35	92.91	92.01	0.91
MH-1240	90.68	92.90	91.91	0.99
MH-125	90.36	92.87	91.84	1.03
MH-1250	90.63	92.86	92.09	0.77
MH-126	90.11	92.49	91.71	0.78
MH-128	89.56	92.54	91.77	0.77
MH-128_2	89.56	92.54	92.22	0.33
MH-1301	90.98	93.11	92.10	1.01
MH-1302	90.91	93.04	92.08	0.96
MH-1304	90.81	93.09	92.04	1.05
MH-1306	90.71	93.14	91.81	1.33
MH-1309	90.61	93.14	91.83	1.31
MH-1309_2	90.61	93.06	91.86	1.21
MH-200	90.51	93.18	92.27	0.91
MH-201	90.19	92.82	92.08	0.74
MH-203	90.00	92.89	91.99	0.91
MH-204	90.68	93.08	92.29	0.79
MH-205	90.28	92.83	92.14	0.69
MH-207	89.86	92.76	91.93	0.83

**Table 2: Freeboard Results - 5-Year Chicago 3 Hour Event & 100 Year Jock River  
Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
MH-208	90.38	92.88	92.02	0.86
MH-209	89.78	92.73	91.89	0.84
MH-211	90.25	92.61	91.83	0.78
MH-212	89.91	92.55	91.80	0.75
MH-213	90.38	92.71	91.86	0.85
MH-214	90.15	92.67	91.81	0.86
MH-215	90.07	92.69	91.80	0.89
MH-216	89.63	92.61	91.78	0.83
MH-2160	89.67	92.61	91.80	0.81
MH-217	89.52	92.61	91.85	0.76
MH-217_2	89.52	92.61	92.13	0.48
MH-300	90.76	93.47	92.53	0.95
MH-301	90.81	93.31	92.50	0.81
MH-302	90.65	93.29	92.48	0.82
MH-3020	90.95	93.28	91.89	1.40
MH-303	90.41	93.45	92.54	0.91
MH-305	90.91	93.36	92.44	0.92
MH-306	90.53	93.19	92.43	0.76
MH-307	90.38	93.31	92.48	0.83
MH-308	90.69	93.07	92.40	0.67
MH-309	90.22	93.06	92.33	0.73
MH-3090	90.38	92.99	92.34	0.65
MH-310	90.45	93.11	92.45	0.66
MH-313	90.04	92.95	92.27	0.68
MH-315	90.67	93.26	92.33	0.93
MH-316	90.74	93.18	92.32	0.86
MH-317	90.44	93.14	92.22	0.92
MH-319	90.12	93.05	92.17	0.88
MH-320	89.90	92.93	92.16	0.77
MH-3200	90.66	92.89	92.03	0.86
MH-321	90.28	92.75	91.95	0.80
MH-322	90.07	92.74	91.88	0.86
MH-323	89.96	92.69	91.84	0.85
MH-325	89.79	92.80	92.01	0.79
MH-326	89.70	92.57	91.93	0.64
MH-327	89.60	92.45	91.77	0.68
MH-330	89.51	92.45	91.87	0.58
MH-330_2	89.51	92.45	92.25	0.21
MH-500	90.81	93.51	92.79	0.72
MH-501	90.55	93.25	92.66	0.59
MH-503	90.29	93.13	92.53	0.60
MH-504	90.58	93.05	92.68	0.37
MH-505	90.30	92.92	92.51	0.41
MH-507	90.19	93.10	92.50	0.60
MH-508	90.45	93.12	92.51	0.62
MH-510	89.91	92.99	92.42	0.57
MH-511	90.52	93.03	92.37	0.66
MH-513	89.80	92.84	92.36	0.48



**Table 2: Freeboard Results - 5-Year Chicago 3 Hour Event & 100 Year Jock River  
Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
MH-515	90.45	92.99	92.35	0.64
MH-516	90.28	92.91	92.35	0.56
MH-519	89.98	92.91	92.32	0.59
MH-5190	90.47	93.05	92.35	0.70
MH-520	89.72	92.77	92.28	0.49
MH-521	89.64	92.77	92.14	0.63
MH-522	89.54	92.49	91.95	0.54
MH-523	90.60	93.03	92.32	0.71
MH-524	90.22	92.83	92.31	0.52
MH-5240	90.34	92.68	92.04	0.64
MH-526	89.97	92.60	91.90	0.70
MH-527	89.80	92.61	91.88	0.73
MH-528	89.50	92.43	91.87	0.56
MH-529	89.42	92.43	91.96	0.47
MH-529_2	89.42	92.43	92.39	0.04
MH-601	90.91	93.36	92.48	0.88
MH-602	90.67	93.05	92.41	0.64
MH-604	90.47	93.02	92.24	0.78
MH-606	90.23	92.94	92.14	0.80
MH-6060	90.40	92.94	92.17	0.78
MH-607	90.11	92.86	92.00	0.86
MH-6080	90.61	92.82	92.00	0.82
MH-609	90.31	92.82	91.90	0.92
MH-610	90.23	92.79	91.90	0.89
MH-611	89.82	92.50	91.82	0.68
MH-6110	90.13	92.50	91.86	0.64
MH-612	89.73	92.50	91.91	0.59
MH-612_2	89.72	92.50	92.11	0.39
MH-6120	90.06	92.50	91.86	0.64
MH-701	90.96	93.22	92.74	0.48
MH-702	90.83	93.29	92.71	0.58
MH-704	90.49	93.16	92.66	0.50
MH-705	90.35	93.17	92.50	0.67
MH-706	90.80	93.31	92.39	0.92
MH-707	90.19	92.97	92.35	0.62
MH-708	90.00	92.70	92.19	0.51
MH-709	89.81	92.58	91.98	0.60
MH-710	91.12	93.36	92.17	1.19
MH-711	90.91	92.67	92.17	0.50
MH-712	90.68	93.06	92.16	0.90
MH-713	90.41	93.03	92.16	0.88
MH-714	90.17	92.70	92.07	0.63
MH-715	90.12	92.63	92.06	0.57
MH-716	90.85	93.30	92.33	0.97
MH-717	90.48	92.90	92.34	0.57
MH-718	90.19	92.75	92.27	0.48
MH-719	90.00	92.70	92.07	0.63
MH-720	89.82	92.75	92.02	0.73

**Table 2: Freeboard Results - 5-Year Chicago 3 Hour Event & 100 Year Jock River  
Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
MH-723	89.71	92.58	92.07	0.52
MH-723_2	89.71	92.60	92.18	0.42
MH-800	90.16	92.98	92.21	0.77
MH-801	90.07	92.81	92.18	0.63
MH-802	89.98	92.84	92.08	0.76
MH-803	89.89	92.82	91.93	0.89
MH-804	89.80	92.53	91.88	0.65
MH-805	89.67	92.51	91.88	0.63
MH-806	89.56	92.51	91.96	0.55
MH-806_2	89.56	92.51	92.41	0.10
MH-900	91.63	93.30	91.82	1.48
MH-901	91.47	93.52	91.79	1.73
OGS_12	90.22	92.80	91.73	1.07
OGS_13	90.61	93.06	91.79	1.27
OGS_2	89.52	92.61	91.81	0.80
OGS_3	89.51	92.45	91.90	0.55
OGS_5	89.42	92.43	92.39	0.04
OGS_6	89.73	92.50	91.93	0.57
OGS_7	89.72	92.60	92.01	0.59
OGS_8	89.56	92.51	91.99	0.52
			<b>Min</b>	0.04
			<b>Max HGL (m)</b>	1.77
			<b>Average</b>	0.75

**Table 3: USF Freeboard Results - 100-Year Chicago 3 Hour Event & 5 Year Jock  
River Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
OGS_1	89.56	92.54	91.14	1.40
MH-100	90.96	93.27	92.95	0.32
MH-101	90.85	93.28	92.92	0.36
MH-102	90.65	93.06	92.83	0.23
MH-104	90.99	93.17	92.94	0.23
MH-105	90.53	93.17	92.80	0.37
MH-108	90.80	93.07	92.82	0.25
MH-109	90.36	92.98	92.72	0.26
MH-110	90.82	93.05	92.64	0.41
MH-111	90.70	93.12	92.79	0.33
MH-112	90.24	92.98	92.63	0.35
MH-114	90.64	92.97	92.63	0.34
MH-115	90.44	92.89	92.62	0.27
MH-116	90.12	92.79	92.51	0.28
MH-117	90.25	92.85	92.53	0.32
MH-1170	90.33	93.85	92.53	1.32
MH-118	90.70	93.05	92.67	0.38
MH-1180	90.62	92.84	92.55	0.29
MH-119	90.27	92.80	92.25	0.55
MH-120	89.93	92.76	92.07	0.69
MH-1201	90.67	93.02	91.75	1.27
MH-1202	90.45	92.90	91.40	1.50
MH-1203	90.33	92.86	91.31	1.55
MH-1206	90.62	93.06	91.81	1.25
MH-1207	90.51	92.99	91.65	1.34
MH-1209	90.22	92.86	91.17	1.69
MH-1209_2	90.22	92.80	91.15	1.65
MH-121	89.86	92.73	91.92	0.81
MH-122	89.82	92.68	91.83	0.86
MH-124	90.35	92.91	92.53	0.38
MH-1240	90.68	92.90	92.32	0.58
MH-125	90.36	92.87	92.09	0.78
MH-1250	90.63	92.86	92.39	0.47
MH-126	90.11	92.49	91.58	0.91
MH-128	89.56	92.54	91.23	1.31
MH-128_2	89.56	92.54	91.26	1.29
MH-1301	90.98	93.11	92.33	0.78
MH-1302	90.91	93.04	92.28	0.76
MH-1304	90.81	93.09	92.11	0.98
MH-1306	90.71	93.14	91.39	1.75
MH-1309	90.61	93.14	91.25	1.89
MH-1309_2	90.61	93.06	91.16	1.91
MH-200	90.51	93.18	92.73	0.45
MH-201	90.19	92.82	92.44	0.38
MH-203	90.00	92.89	92.28	0.61
MH-204	90.68	93.08	92.65	0.43
MH-205	90.28	92.83	92.55	0.28
MH-207	89.86	92.76	92.13	0.63

**Table 3: USF Freeboard Results - 100-Year Chicago 3 Hour Event & 5 Year Jock  
River Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
MH-208	90.38	92.88	92.31	0.57
MH-209	89.78	92.73	91.96	0.77
MH-211	90.25	92.61	92.02	0.59
MH-212	89.91	92.55	91.84	0.71
MH-213	90.38	92.71	92.20	0.51
MH-214	90.15	92.67	92.00	0.67
MH-215	90.07	92.69	91.94	0.75
MH-216	89.63	92.61	91.56	1.05
MH-2160	89.67	92.61	91.65	0.96
MH-217	89.52	92.61	91.43	1.18
MH-217_2	89.52	92.61	91.32	1.29
MH-300	90.76	93.47	92.91	0.56
MH-301	90.81	93.31	92.90	0.41
MH-302	90.65	93.29	92.87	0.43
MH-3020	90.95	93.28	92.11	1.18
MH-303	90.41	93.45	92.91	0.54
MH-305	90.91	93.36	92.82	0.54
MH-306	90.53	93.19	92.82	0.37
MH-307	90.38	93.31	92.93	0.38
MH-308	90.69	93.07	92.86	0.21
MH-309	90.22	93.06	92.72	0.34
MH-3090	90.38	92.99	92.73	0.26
MH-310	90.45	93.11	92.71	0.40
MH-313	90.04	92.95	92.62	0.33
MH-315	90.67	93.26	92.63	0.63
MH-316	90.74	93.18	92.63	0.55
MH-317	90.44	93.14	92.53	0.61
MH-319	90.12	93.05	92.45	0.60
MH-320	89.90	92.93	92.42	0.51
MH-3200	90.66	92.89	92.45	0.44
MH-321	90.28	92.75	92.16	0.59
MH-322	90.07	92.74	91.95	0.79
MH-323	89.96	92.69	91.81	0.88
MH-325	89.79	92.80	92.13	0.67
MH-326	89.70	92.57	91.91	0.66
MH-327	89.60	92.45	91.59	0.86
MH-330	89.51	92.45	91.57	0.88
MH-330_2	89.51	92.45	91.49	0.97
MH-500	90.81	93.51	93.06	0.45
MH-501	90.55	93.25	92.96	0.29
MH-503	90.29	93.13	92.88	0.25
MH-504	90.58	93.05	92.98	0.07
MH-505	90.30	92.92	92.85	0.07
MH-507	90.19	93.10	92.83	0.27
MH-508	90.45	93.12	92.81	0.32
MH-510	89.91	92.99	92.72	0.27
MH-511	90.52	93.03	92.67	0.36
MH-513	89.80	92.84	92.61	0.23

**Table 3: USF Freeboard Results - 100-Year Chicago 3 Hour Event & 5 Year Jock  
River Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
MH-515	90.45	92.99	92.63	0.36
MH-516	90.28	92.91	92.63	0.28
MH-519	89.98	92.91	92.56	0.35
MH-5190	90.47	93.05	92.68	0.38
MH-520	89.72	92.77	92.48	0.29
MH-521	89.64	92.77	92.17	0.60
MH-522	89.54	92.49	91.76	0.73
MH-523	90.60	93.03	92.64	0.39
MH-524	90.22	92.83	92.59	0.24
MH-5240	90.34	92.68	92.23	0.45
MH-526	89.97	92.60	91.77	0.83
MH-527	89.80	92.61	91.67	0.94
MH-528	89.50	92.43	91.58	0.85
MH-529	89.42	92.43	91.45	0.98
MH-529_2	89.42	92.43	92.10	0.33
MH-601	90.91	93.36	92.81	0.55
MH-602	90.67	93.05	92.73	0.32
MH-604	90.47	93.02	92.48	0.54
MH-606	90.23	92.94	92.32	0.62
MH-6060	90.40	92.94	92.35	0.59
MH-607	90.11	92.86	92.05	0.81
MH-6080	90.61	92.82	92.27	0.55
MH-609	90.31	92.82	91.85	0.97
MH-610	90.23	92.79	91.82	0.97
MH-611	89.82	92.50	91.69	0.81
MH-6110	90.13	92.50	91.74	0.76
MH-612	89.73	92.50	91.73	0.77
MH-612_2	89.72	92.50	91.63	0.87
MH-6120	90.06	92.50	91.70	0.80
MH-701	90.96	93.22	93.04	0.19
MH-702	90.83	93.29	93.03	0.26
MH-704	90.49	93.16	92.96	0.20
MH-705	90.35	93.17	92.82	0.35
MH-706	90.80	93.31	92.74	0.57
MH-707	90.19	92.97	92.57	0.40
MH-708	90.00	92.70	92.22	0.48
MH-709	89.81	92.58	91.78	0.80
MH-710	91.12	93.36	92.55	0.81
MH-711	90.91	92.67	92.56	0.11
MH-712	90.68	93.06	92.52	0.54
MH-713	90.41	93.03	92.38	0.66
MH-714	90.17	92.70	92.09	0.61
MH-715	90.12	92.63	92.04	0.59
MH-716	90.85	93.30	92.57	0.73
MH-717	90.48	92.90	92.57	0.34
MH-718	90.19	92.75	92.43	0.32
MH-719	90.00	92.70	91.98	0.72
MH-720	89.82	92.75	91.90	0.86

**Table 3: USF Freeboard Results - 100-Year Chicago 3 Hour Event & 5 Year Jock  
River Water Level**

<b>MH-ID</b>	<b>Invert Elevation (m)</b>	<b>Top of MH (m)</b>	<b>Max HGL (m)</b>	<b>Freeboard (m)</b>
MH-723	89.71	92.58	91.73	0.86
MH-723_2	89.71	92.60	91.63	0.97
MH-800	90.16	92.98	92.13	0.85
MH-801	90.07	92.81	92.09	0.72
MH-802	89.98	92.84	92.01	0.83
MH-803	89.89	92.82	91.95	0.87
MH-804	89.80	92.53	91.84	0.69
MH-805	89.67	92.51	91.65	0.86
MH-806	89.56	92.51	91.63	0.88
MH-806_2	89.56	92.51	91.65	0.86
MH-900	91.63	93.30	92.26	1.04
MH-901	91.47	93.52	92.09	1.43
OGS_12	90.22	92.80	91.16	1.64
OGS_13	90.61	93.06	91.18	1.88
OGS_2	89.52	92.61	91.33	1.28
OGS_3	89.51	92.45	91.48	0.97
OGS_5	89.42	92.43	91.37	1.06
OGS_6	89.73	92.50	91.63	0.87
OGS_7	89.72	92.60	91.64	0.96
OGS_8	89.56	92.51	91.61	0.90
			<b>Min</b>	0.07
			<b>Max</b>	1.91
			<b>Average</b>	0.69



# Attachment A

DSEL Rational Method Calculations





# STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years  
 Collector Roads Return Frequency = 5 years  
 Arterial Roads Return Frequency = 10 years

Manning 0.013

LOCATION			AREA (Ha)																FLOW							SEWER DATA										
			2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO			
Location	From Node	To Node	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full				
Contribution From Peninsula Road, Pipe 511 - 604					0.35				0.00					0.00				0.00	11.40																	
	604	606	0.12	0.58	0.19	0.87			0.00	0.58			0.00	0.00			0.00	0.00	12.62	68.03	92.14	107.95	157.71	113	600	600	CONC	0.15	60.0	237.8056	0.8411	1.1890	0.475			
Contribution From Sapling Grove, Pipe 523 - 606					0.56				0.00					0.00				0.00	11.39																	
Contribution From Sapling Grove, Pipe 6060 - 606					0.00				1.47					0.00				0.00	10.22																	
	606	607	0.15	0.67	0.27	1.70			0.00	2.05			0.00	0.00			0.00	0.00	13.81	64.74	87.62	102.63	149.91	290	750	750	CONC	0.15	59.5	431.1703	0.9760	1.0161	0.672			
	607	611	0.37	0.65	0.67	2.37			0.00	2.05			0.00	0.00			0.00	0.00	14.82	62.19	84.14	98.53	143.89	320	750	750	CONC	0.15	80.5	431.1703	0.9760	1.3747	0.742			
To Storm Outlet 6, Pipe 611 - 612					2.37				2.05					0.00				0.00	16.20																	
<b>Storm Outlet 6</b>																																				
Contribution From Pollination Place, Pipe 610 - 6110					1.21				0.00					0.00				0.00	13.63																	
	6110	611			0.00	1.21			0.00	0.00			0.00	0.00			0.00	0.00	13.63	65.20	88.26	103.38	151.01	79	600	600	CONC	0.15	8.0	237.8056	0.8411	0.1585	0.332			
Contribution From Pollination Place, Pipe 607 - 611					2.37				2.05					0.00				0.00	16.20																	
Contribution From Block 773 (Park), Pipe 6120 - 611					0.00				1.97					0.00				0.00	10.18																	
	611	612			0.00	3.58			0.00	4.02			0.00	0.00			0.00	0.00	16.20	59.08	79.88	93.53	136.55	533	900	900	CONC	0.15	8.0	701.1305	1.1021	0.1210	0.760			
	612	HW6			0.00	3.58			0.00	4.02			0.00	0.00			0.00	0.00	16.32	58.82	79.53	93.11	135.94	530	900	900	CONC	0.15	24.5	701.1305	1.1021	0.3705	0.756			
<b>Crowfoot Lane (LANE 1)</b>																																				
Contribution From Ambit Lane (LANE 2), Pipe 710 - 712					0.43				0.00					0.00				0.00	11.67																	
Contribution From Conservancy Drive, Pipe 711 - 712					0.00				0.24					0.00				0.00	10.44																	
	712	713	0.13	0.84	0.30	0.73			0.00	0.24			0.00	0.00			0.00	0.00	11.67	70.94	96.14	112.65	164.62	74	450	450	CONC	0.20	102.0	127.5033	0.8017	2.1205	0.584			
			0.09	0.86	0.22	0.94			0.00	0.24			0.00	0.00			0.00	0.00																		
	713	714	0.17	0.86	0.41	1.35			0.00	0.24			0.00	0.00			0.00	0.00	13.79	64.78	87.69	102.71	150.02	108	525	525	CONC	0.20	102.0	192.3297	0.8885	1.9134	0.563			
	714	715			0.00	1.35			0.00	0.24			0.00	0.00			0.00	0.00	15.70	60.16	81.36	95.26	139.10	101	525	525	CONC	0.20	9.5	192.3297	0.8885	0.1782	0.523			
	715	720	0.14	0.71	0.28	1.63			0.00	0.24			0.00	0.00			0.00	0.00	15.88	59.77	80.82	94.63	138.17	116	525	525	CONC	0.20	30.0	192.3297	0.8885	0.5628	0.605			
To Ephemeral Crescent, Pipe 720 - 723					1.63				0.24					0.00				0.00	16.44																	
<b>Ephemeral Crescent</b>																																				
Contribution From Deciduous Crescent, Pipe 716 - 717					0.50				0.14					0.00				0.00	11.33																	
					0.00	0.50	0.03	0.68	0.06	0.20			0.00	0.00			0.00	0.00																		
					0.00	0.50	0.03	0.81	0.07	0.27			0.00	0.00			0.00	0.00																		
			0.14	0.78	0.30	0.80			0.00	0.27			0.00	0.00			0.00	0.00																		
	717	718	0.31	0.60	0.52	1.32			0.00	0.27			0.00	0.00			0.00	0.00	11.33	72.04	97.64	114.42	167.22	121	525	525	CONC	0.20	105.0	192.3297	0.8885	1.9697	0.630			
	718	719	0.62	0.78	1.34	2.66			0.00	0.27			0.00	0.00			0.00	0.00	13.30	66.09	89.47	104.81	153.11	200	600	600	CONC	0.15	107.0	237.8056	0.8411	2.1203	0.841			
	719	720			0.00	2.66			0.00	0.27			0.00	0.00			0.00	0.00	15.42	60.79	82.21	96.27	140.57	184	600	600	CONC	0.35	9.0	363.2541	1.2847	0.1168	0.506			
Contribution From Crowfoot Lane (LANE 1), Pipe 715 - 720					1.63				0.24					0.00				0.00	16.44																	
	720	723	0.16	0.60	0.27	4.56			0.00	0.50			0.00	0.00			0.00	0.00	16.44	58.56	79.17	92.69	135.32	307	900	900	CONC	0.20	55.0	809.5958	1.2726	0.7203	0.379			
To Storm Outlet 7, Pipe 723 - HW7					4.56				0.50					0.00				0.00	17.16																	
Contribution From Conservancy Drive, Pipe 705 - 707					3.44				0.44					0.00				0.00	14.59																	
Contribution From Deciduous Crescent, Pipe 706 - 707					0.66				0.08					0.00				0.00	11.26																	
Contribution From Conservancy Drive, Pipe 7170 - 707					0.00				0.00					0.00				0.00	11.20																	
					0.00	4.10	0.04	0.73	0.08	0.60			0.00	0.00			0.00	0.00																		
	707	708	0.12	0.78	0.26	4.36			0.00	0.60			0.00	0.00			0.00	0.00	14.59	62.74	84.89	99.42	145.20	324	750	750	CONC	0.15	115.5	431.1703	0.9760	1.9724	0.752			
			0.14	0.80	0.23	4.59			0.00	0.60			0.00	0.00			0.00	0.00																		
	708	709	0.25	0.78	0.54	5.13			0.00	0.60			0.00	0.00			0.00	0.00	16.57	58.30	78.81	92.28	134.71	347	750	750	CONC	0.15	107.5	431.1703	0.9760	1.8358	0.804			
To Storm Outlet 7, Pipe 709 - 723					5.13				0.60					0.00				0.00	18.40																	
<b>Storm Outlet 7</b>																																				
Contribution From Ephemeral Crescent, Pipe 708 - 709					5.13				0.60					0.00				0.00	18.40																	
	709	723	0.43	0.78	0.93	6.07			0.00	0.60			0.00	0.00			0.00	0.00	18.40	54.75	73.96	86.57	126.35	377	750	750	CONC	0.20	13.0	497.8726	1.1270	0.1923	0.756			
Contribution From Ephemeral Crescent, Pipe 720 - 723					4.56				0.50					0.00				0.00	17.16																	
	723	HW7			0.00	10.62			0.00	1.10			0.00	0.00			0.00	0.00	18.59	54.41	73.49	86.02	125.54	659	900	900	CONC	0.40	12.0	1144.9413	1.7997	0.1111	0.576			
<b>Peninsula Road</b>																																				
	204	313	0.04	0.67	0.07	0.07			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	5	300	300	PVC	0.35	41.5	57.2089	0.8093	0.8546	0.088			
To Anemone Mews, Pipe 313 - 320					0.07				0.00					0.00				0.00	10.85																	

Definitions:  
 Q = 2.78 AIR, where  
 Q = Peak Flow in Litres per second (L/s)  
 A = Areas in hectares (ha)  
 I = Rainfall Intensity (mm/h)  
 R = Runoff Coefficient

Notes:  
 1) Ottawa Rainfall-Intensity Curve  
 2) Min. Velocity = 0.80 m/s

Designed:	R.B.	PROJECT:	BARRHAVEN CONSERVANCY EAST, PHASE 3 AND 4		
Checked:	W.L.	LOCATION:	City of Ottawa		
Dwg. Reference:	File Ref:	Date:	20-118		

**STORM SEWER CALCULATION SHEET (RATIONAL METHOD)**

Local Roads Return Frequency = 2 years  
 Collector Roads Return Frequency = 5 years  
 Arterial Roads Return Frequency = 10 years

Manning 0.013

LOCATION			AREA (Ha)																FLOW						SEWER DATA											
			2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO			
Location	From Node	To Node	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full				
	310	313	0.26	0.70	0.51	0.51													10.00	76.81	104.19	122.14	178.56	39	375	375	PVC	0.30	82.5	96.0323	0.8695	1.5814	0.409			
To Anemone Mews, Pipe 313 - 320						0.51				0.00	0.00							0.00	0.00	11.58																
	310	513	0.29	0.70	0.56	0.56													10.00	76.81	104.19	122.14	178.56	43	375	375	PVC	0.30	89.5	96.0323	0.8695	1.7156	0.450			
To Mineral Street, Pipe 513 - 520						0.56				0.00	0.00							0.00	0.00	11.72																
	511	604	0.16	0.78	0.35	0.35													10.00	76.81	104.19	122.14	178.56	27	450	450	CONC	0.20	67.5	127.5033	0.8017	1.4033	0.209			
To Pollination Place, Pipe 604 - 606						0.35				0.00	0.00							0.00	0.00	11.40																
	511	513	0.42	0.78	0.91	0.91													10.00	76.81	104.19	122.14	178.56	70	375	375	PVC	0.30	112.5	96.0323	0.8695	2.1564	0.728			
To Mineral Street, Pipe 513 - 520						0.91				0.00	0.00							0.00	0.00	12.16																
<b>Conservancy Drive</b>																																				
	200	309			0.00	0.00	0.11	0.70	0.22	0.22									10.00	76.81	104.19	122.14	178.56	23	300	300	PVC	0.35	36.0	57.2089	0.8093	0.7413	0.400			
To Anemone Mews, Pipe 309 - 313						0.00				0.22	0.22							0.00	0.00	10.74																
	307	510	0.21	0.54	0.00	0.00	0.18	0.75	0.38	0.38									10.00	76.81	104.19	122.14	178.56	64	450	450	CONC	0.25	51.0	142.5531	0.8963	0.9483	0.450			
To Mineral Street, Pipe 510 - 513						0.32				0.38	0.38							0.00	0.00	10.95																
	508	602			0.00	0.00	0.07	0.78	0.15	0.15									10.00	76.81	104.19	122.14	178.56	16	300	300	PVC	0.35	64.0	57.2089	0.8093	1.3179	0.276			
To Pollination Place, Pipe 602 - 604						0.00				0.15	0.15							0.00	0.00	11.32																
	508	510			0.00	0.00	0.12	0.78	0.26	0.26									10.00	76.81	104.19	122.14	178.56	124	525	525	CONC	0.20	117.0	192.3297	0.8885	2.1948	0.642			
To Mineral Street, Pipe 510 - 513						0.00				0.26	0.26							0.00	0.00	12.19																
	601	602			0.00	0.00	0.16	0.80	0.36	0.36									10.00	76.81	104.19	122.14	178.56	37	300	300	PVC	1.00	32.5	96.7008	1.3680	0.3959	0.383			
To Pollination Place, Pipe 602 - 604						0.00				0.36	0.36							0.00	0.00	10.40																
	711	712			0.00	0.00	0.12	0.71	0.24	0.24									10.00	76.81	104.19	122.14	178.56	25	300	300	PVC	0.35	21.5	57.2089	0.8093	0.4427	0.431			
To Crowfoot Lane (LANE 1), Pipe 712 - 713						0.00				0.24	0.24							0.00	0.00	10.44																
	7170	707			0.00	0.00			0.00	0.00									10.00	76.81	104.19	122.14	178.56	0	375	375	PVC	0.30	62.5	96.0323	0.8695	1.1980	0.000			
To Ephemeral Crescent, Pipe 707 - 708						0.00				0.00	0.00							0.00	0.00	11.20																
	307	308			0.00	0.00	0.35	0.73	0.71	0.71									10.00	76.81	104.19	122.14	178.56	74	375	375	PVC	0.50	53.5	123.9771	1.1225	0.7944	0.597			
	308	309	0.13	0.54	0.20	0.20													10.79	73.88	100.18	117.41	171.61	110	525	525	CONC	0.25	67.0	215.0311	0.9933	1.1242	0.510			
To Anemone Mews, Pipe 309 - 313						0.20				0.95	0.95							0.00	0.00	11.92																
	601	705			0.00	0.00			0.00	0.00									10.00	76.81	104.19	122.14	178.56	0	300	300	PVC	0.45	40.0	64.8688	0.9177	0.7264	0.000			
Contribution From Les Emmerson Drive (N), Pipe 704 - 705						3.44				0.10	0.10							0.00	0.00	13.52																
	705	707			0.00	3.44	0.18	0.67	0.34	0.44									13.52	65.49	88.66	103.85	151.70	264	675	675	CONC	0.15	58.5	325.5584	0.9098	1.0717	0.812			
To Ephemeral Crescent, Pipe 707 - 708						3.44				0.44	0.44							0.00	0.00	14.59																
<b>Les Emmerson Drive (S)</b>																																				
	303	507	0.22	0.54	0.33	0.33													10.00	76.81	104.19	122.14	178.56	60	600	600	CONC	0.15	69.0	237.8056	0.8411	1.3673	0.253			
To Mineral Street, Pipe 507 - 510						0.33				0.00	0.00							0.00	0.00	11.37																
	303	306	0.33	0.70	0.64	0.81													10.00	76.81	104.19	122.14	178.56	62	375	375	PVC	0.35	102.0	103.7267	0.9392	1.8101	0.602			
To Anemone Mews, Pipe 306 - 3090						0.81				0.00	0.00							0.00	0.00	11.81																

Definitions:  
 Q = 2.78 AIR, where  
 Q = Peak Flow in Litres per second (L/s)  
 A = Areas in hectares (ha)  
 I = Rainfall Intensity (mm/h)  
 R = Runoff Coefficient

Notes:  
 1) Ottawa Rainfall-Intensity Curve  
 2) Min. Velocity = 0.80 m/s

Designed:	R.B.	PROJECT:	BARRHAVEN CONSERVANCY EAST, PHASE 3 AND 4
Checked:	W.L.	LOCATION:	City of Ottawa
Dwg. Reference:	File Ref:	Date:	Sheet No.
Stm Tributary Area Plan Dwg No. 4	20-1180	05 Apr 2024	SHEET 3 OF 6

# STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years  
 Collector Roads Return Frequency = 5 years  
 Arterial Roads Return Frequency = 10 years

Manning 0.013

LOCATION			AREA (Ha)																FLOW							SEWER DATA										
			2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO			
Location	From Node	To Node	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full				
	305	306	0.29	0.72	0.58	0.58			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	45	450	450	CONC	0.25	91.5	142.5531	0.8963	1.7014	0.313			
To Anemone Mews, Pipe 306 - 3090					0.58				0.00				0.00				0.00		11.70																	
	504	704	0.70	0.80	1.56	1.56			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	120	675	675	CONC	0.15	20.0	325.5584	0.9098	0.3664	0.367			
To Les Emmerson Drive (N), Pipe 704 - 705					1.56				0.00				0.00				0.00		10.37																	
	505	507	0.49	0.80	1.09	1.09			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	84	675	675	PVC	0.15	20.0	325.5584	0.9098	0.3664	0.257			
To Mineral Street, Pipe 507 - 510					1.09				0.00				0.00				0.00		10.37																	
<b>Les Emmerson Drive (N)</b>																																				
	300	503			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	0	450	450	CONC	0.25	37.0	142.5531	0.8963	0.6880	0.000			
To Mineral Street, Pipe 503 - 507					0.00				0.00				0.00				0.00		10.69																	
	300	301	0.28	0.73	0.57	0.57			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	44	450	450	CONC	0.20	67.0	127.5033	0.8017	1.3929	0.342			
	301	302	0.20	0.73	0.41	0.97			0.00	0.00			0.00	0.00			0.00	0.00	11.39	71.84	97.37	114.11	166.76	70	600	600	CONC	0.15	66.5	237.8056	0.8411	1.3178	0.294			
To Anemone Mews, Pipe 302 - 306					0.97				0.00				0.00				0.00		12.71																	
	500	501	0.41	0.78	0.89	0.89			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	68	450	450	CONC	0.20	109.0	127.5033	0.8017	2.2660	0.536			
			0.09	0.71	0.18	1.07			0.00	0.00			0.00	0.00			0.00	0.00																		
	501	503	0.17	0.78	0.37	1.44			0.00	0.00			0.00	0.00			0.00	0.00	12.27	69.08	93.57	109.64	160.20	99	525	525	CONC	0.17	65.0	177.3192	0.8191	1.3226	0.559			
To Mineral Street, Pipe 503 - 507					1.44				0.00				0.00				0.00		13.59																	
	500	701	0.30	0.78	0.65	0.65			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	50	375	375	PVC	0.30	52.5	96.0323	0.8695	1.0063	0.520			
	701	702			0.00	0.65			0.00	0.00			0.00	0.00			0.00	0.00	11.01	73.15	99.16	116.22	169.85	48	375	375	PVC	0.30	17.5	96.0323	0.8695	0.3354	0.495			
			0.15	0.60	0.25	0.90			0.00	0.00			0.00	0.00			0.00	0.00																		
	702	704	0.16	0.80	0.36	1.26			0.00	0.00			0.00	0.00			0.00	0.00	11.34	72.01	97.60	114.38	167.16	90	450	450	CONC	0.20	52.0	127.5033	0.8017	1.0810	0.710			
Contribution From Les Emmerson Drive (S), Pipe 504 - 704					1.56				0.00				0.00				0.00		10.37																	
					0.00	2.81	0.05	0.75	0.10	0.10			0.00	0.00			0.00	0.00																		
	704	705	0.29	0.78	0.63	3.44			0.00	0.10			0.00	0.00			0.00	0.00	12.42	68.61	92.93	108.88	159.08	246	675	675	CONC	0.14	58.0	314.5193	0.8789	1.0998	0.782			
To Conservancy Drive, Pipe 705 - 707					3.44				0.00	0.10			0.00				0.00		13.52																	
<b>Anemone Mews</b>																																				
Contribution From Les Emmerson Drive (N), Pipe 301 - 302					0.97				0.00	0.00			0.00	0.00			0.00	0.00	12.71																	
	302	306	0.17	0.75	0.35	1.33			0.00	0.00			0.00	0.00			0.00	0.00	12.71	67.76	91.77	107.51	157.07	90	600	600	CONC	0.15	62.5	237.8056	0.8411	1.2385	0.379			
Contribution From Les Emmerson Drive (S), Pipe 303 - 306					0.81				0.00	0.00			0.00	0.00			0.00	0.00	11.81																	
Contribution From Les Emmerson Drive (S), Pipe 305 - 306					0.58				0.00	0.00			0.00	0.00			0.00	0.00	11.70																	
	306	3090	0.16	0.74	0.33	3.05			0.00	0.00			0.00	0.00			0.00	0.00	13.95	64.36	87.11	102.03	149.03	196	600	600	CONC	0.15	48.0	237.8056	0.8411	0.9512	0.826			
	3090	309			0.00	3.05			0.00	0.00			0.00	0.00			0.00	0.00	14.90	62.00	83.88	98.23	143.45	189	675	675	CONC	0.15	10.0	325.5584	0.9098	0.1832	0.581			
Contribution From Conservancy Drive, Pipe 200 - 309					0.00				0.00	0.22			0.00	0.00			0.00	0.00	10.74																	
Contribution From Conservancy Drive, Pipe 308 - 309					0.20				0.00	0.95			0.00	0.00			0.00	0.00	11.92																	
			0.11	0.54	0.16	3.41			0.00	1.17			0.00	0.00			0.00	0.00																		
			0.11	0.56	0.18	3.58			0.00	1.17			0.00	0.00			0.00	0.00																		
	309	313	0.13	0.56	0.20	3.78			0.00	1.17			0.00	0.00			0.00	0.00	15.08	61.57	83.29	97.54	142.43	331	825	825	CONC	0.10	62.5	453.9246	0.8492	1.2267	0.728			
Contribution From Peninsula Road, Pipe 204 - 313					0.07				0.00	0.00			0.00	0.00			0.00	0.00	10.85																	
Contribution From Peninsula Road, Pipe 310 - 313					0.51				0.00	0.00			0.00	0.00			0.00	0.00	11.58																	
			0.04	0.77	0.08	4.44			0.00	1.17			0.00	0.00			0.00	0.00																		
			0.12	0.54	0.19	4.62			0.00	1.17			0.00	0.00			0.00	0.00																		
	313	320	0.22	0.54	0.32	4.95			0.00	1.17			0.00	0.00			0.00	0.00	16.31	58.84	79.55	93.14	135.98	384	900	900	CONC	0.11	60.0	600.4123	0.9438	1.0596	0.640			
To Sapling Grove, Pipe 320 - 325					4.95				0.00	1.17			0.00				0.00		17.37																	
<b>Syringa Court</b>																																				
	315	515			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	0	375	375	PVC	0.30	48							



**STORM SEWER CALCULATION SHEET (RATIONAL METHOD)**

Local Roads Return Frequency = 2 years  
 Collector Roads Return Frequency = 5 years  
 Arterial Roads Return Frequency = 10 years

Manning 0.013

LOCATION			AREA (Ha)																FLOW							SEWER DATA									
			2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCITY	TIME OF	RATIO		
Location	From Node	To Node	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full			
	710	1301			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	0	300	300	PVC	0.35	44.5	57.2089	0.8093	0.9164	0.000		
	1301	1302			0.00	0.00			0.00	0.00			0.00	0.00			0.00	0.00	10.92	73.46	99.59	116.72	170.59	0	300	300	PVC	0.35	9.5	57.2089	0.8093	0.1956	0.000		
	1302	1304	0.16	0.71	0.32	0.32			0.00	0.00			0.00	0.00			0.00	0.00	11.11	72.78	98.66	115.63	168.99	23	375	375	CONC	0.27	23.0	91.1042	0.8249	0.4647	0.252		
To Deciduous Crescent, Pipe 1304 - 1309					0.32					0.00				0.00				0.00	11.58																
<b>Deciduous Crescent</b>																																			
			0.08	0.60	0.13	0.13			0.00	0.00			0.00	0.00			0.00	0.00																	
			0.22	0.78	0.48	0.61			0.00	0.00			0.00	0.00			0.00	0.00																	
	706	1306	0.24	0.60	0.40	1.01			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	78	375	375	PVC	0.30	69.0	96.0323	0.8695	1.3226	0.808		
To Storm Outlet 13, Pipe 1306 - 1309					1.01					0.00				0.00				0.00	11.32																
					0.00	0.00	0.04	0.72	0.08	0.08			0.00	0.00			0.00	0.00																	
			0.12	0.60	0.20	0.20			0.00	0.08			0.00	0.00			0.00	0.00																	
	706	707	0.21	0.78	0.46	0.66			0.00	0.08			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	59	375	375	PVC	0.30	65.5	96.0323	0.8695	1.2555	0.611		
To Ephemeral Crescent, Pipe 707 - 708					0.66					0.08				0.00				0.00	11.26																
					0.00	0.00	0.03	0.79	0.07	0.07			0.00	0.00			0.00	0.00																	
			0.00	0.00	0.00	0.00	0.05	0.55	0.08	0.14			0.00	0.00			0.00	0.00																	
	716	717	0.23	0.78	0.50	0.50			0.00	0.14			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	53	375	375	PVC	0.27	66.0	91.1042	0.8249	1.3335	0.583		
To Ephemeral Crescent, Pipe 717 - 718					0.50					0.14				0.00				0.00	11.33																
	716	1304	0.26	0.78	0.56	0.56			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	43	300	300	PVC	0.35	69.5	57.2089	0.8093	1.4312	0.757		
Contribution From Ambit Lane (LANE 2), Pipe 1302 - 1304					0.32					0.00				0.00				0.00	11.58																
	1304	1309	0.16	0.60	0.27	1.15			0.00	0.00			0.00	0.00			0.00	0.00	11.58	71.24	96.54	113.13	165.33	82	450	450	CONC	0.20	59.0	127.5033	0.8017	1.2266	0.641		
To Storm Outlet 13, Pipe 1309 - HW13					1.15					0.00				0.00				0.00	12.80																
<b>Storm Outlet 13</b>																																			
Contribution From Deciduous Crescent, Pipe 706 - 1306					1.01					0.00				0.00				0.00	11.32																
	1306	1309	0.12	0.78	0.26	1.27			0.00	0.00			0.00	0.00			0.00	0.00	11.32	72.07	97.69	114.48	167.31	92	450	450	CONC	0.20	11.5	127.5033	0.8017	0.2391	0.718		
Contribution From Deciduous Crescent, Pipe 1304 - 1309					1.15					0.00				0.00				0.00	12.80																
	1309	HW13			0.00	2.42			0.00	0.00			0.00	0.00			0.00	0.00	12.80	67.49	91.40	107.08	156.44	163	600	600	CONC	0.15	5.5	237.8056	0.8411	0.1090	0.686		
<b>Borrisokane Road</b>																																			
	800	801			0.00	0.00			0.00	0.00	2.71	0.80	6.03	6.03			0.00	0.00	10.00	76.81	104.19	122.14	178.56	736	1200	1200	CONC	0.10	68.5	1232.8868	1.0901	1.0473	0.597		
	801	802			0.00	0.00			0.00	0.00	0.54	0.77	1.16	7.18			0.00	0.00	11.05	73.00	98.97	115.99	169.52	833	1200	1200	CONC	0.10	70.0	1232.8868	1.0901	1.0702	0.676		
	802	803			0.00	0.00			0.00	0.00	0.40	0.80	0.89	8.07			0.00	0.00	12.12	69.53	94.20	110.37	161.27	891	1200	1200	CONC	0.10	70.0	1232.8868	1.0901	1.0702	0.723		
	803	804			0.00	0.00			0.00	0.00	0.62	0.80	1.38	9.45			0.00	0.00	13.19	66.40	89.91	105.32	153.86	995	1200	1200	CONC	0.10	70.0	1232.8868	1.0901	1.0702	0.807		
	804	805			0.00	0.00			0.00	0.00			0.00	9.45			0.00	0.00	14.26	63.58	86.03	100.76	147.17	952	1200	1200	CONC	0.10	70.0	1232.8868	1.0901	1.0702	0.772		
To Storm Outlet 8, Pipe 805 - 806					0.00					0.00				9.45				0.00	15.33																
<b>Storm Outlet 8</b>																																			
Contribution From Borrisokane Road, Pipe 804 - 805					0.00					0.00				9.45				0.00	15.33																
	805	806			0.00	0.00			0.00	0.00	0.21	0.80	0.47	9.92			0.00	0.00	15.33	61.00	82.51	96.62	141.09	958	1200	1200	CONC	0.10	27.5	1232.8868	1.0901	0.4204	0.777		
	806	HW8			0.00	0.00			0.00	0.00			0.00	9.92			0.00	0.00	15.75	60.06	81.21	95.10	138.85	943	1200	1200	CONC	0.10	8.5	1232.8868	1.0901	0.1300	0.765		

Definitions:  
 Q = 2.78 AIR, where  
 Q = Peak Flow in Litres per second (L/s)  
 A = Areas in hectares (ha)  
 I = Rainfall Intensity (mm/h)  
 R = Runoff Coefficient

Notes:  
 1) Ottawa Rainfall-Intensity Curve  
 2) Min. Velocity = 0.80 m/s

Designed:	R.B.	PROJECT:	BARRHAVEN CONSERVANCY EAST, PHASE 3 AND 4
Checked:	W.L.	LOCATION:	City of Ottawa
Dwg. Reference:	File Ref:	Date:	Sheet No.
Stm Tributary Area Plan Dwg No. 4	20-1180	05 Apr 2024	SHEET 6 OF 6